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# Hydrology Technical Report

## San Diego State University Mission Valley Campus Project

*Prepared for*

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## ACRONYMS AND ABBREVIATIONS

BMP	Best Management Practice
FEMA	Flood Emergency Management Agency
FIRM	Flood Insurance Rate Map
IDF	Intensity Duration Frequency
LiDAR	Light detection and ranging
NFIP	National Flood Insurance Program
REC	Rick Engineering Company
SDCHM	San Diego County Hydrology Manual
SDDDM	City of San Diego Drainage Design Manual
SDSU	San Diego State University

## 1. PURPOSE

This technical report presents an analysis of anticipated changes to the hydrologic and hydraulic conditions associated with development of the proposed San Diego State University (SDSU) Mission Valley Campus Master Plan Project (Project). The Project proposes the redevelopment of the approximately 170-acre existing San Diego County Credit Union (SDCCU) stadium site and associated parking lot. Geosyntec prepared this hydrology technical report to support the preparation of a draft Environmental Impact Report (DEIR) and the environmental review process under the California Environmental Quality Act (CEQA).

This technical report evaluates the existing (pre-project) and proposed (post-project) hydrologic and hydraulic conditions of the site to assess if the proposed Project conditions would result in increased runoff. The existing site conditions include the SDCCU stadium and associated parking lot. The Project conditions were based on January 2019 design drawings provided by Rick Engineering Company (REC).

The remainder of this report is structured as follows:

- Section 2: Discusses the approach to evaluate existing and Project peak discharges from the site.
- Section 3: Discusses the existing storm drain infrastructure capacity and peak discharge.
- Section 4: Discusses the proposed storm drain infrastructure capacity and peak discharge.
- Section 5: Discusses the impacts to offsite drainage areas as a result of project activities.
- Section 5: Discusses the known and expected run-on and floodplain conditions.

The work described in this technical report was conducted by Geosyntec Consultants (Geosyntec) on behalf of Gatzke Dillon & Ballance LLP. The primary author of the report was Rachel Hill, PE. Senior review was conducted by Trevor Alsop and Courtney Wilson, PE in accordance with Geosyntec's quality assurance protocols.

## 2. APPROACH

Peak flows were estimated for onsite runoff associated with 50- and 100-year frequency storm events for the existing and proposed conditions to assess changes in peak runoff as a result of the Project.

### 2.1 Hydrology

The Project occupies less than one square mile of area, so in accordance with the City of San Diego Drainage Design Manual (SDDDM), the Rational Method was used to determine the peak runoff from the site. The rational method, Equation 1, relates site runoff to size of drainage area, rainfall intensity, and land cover characteristics.

$$Q = C * i * A \quad \text{Equation 1}$$

Where Q = runoff (cubic feet per second [cfs])  
 C = runoff coefficient (unitless)  
 i = rainfall intensity (inches/hour)  
 A = area (acres)

#### 2.1.1 Drainage Area Delineation

Onsite drainage areas (i.e., subcatchments) were delineated based on existing and proposed catch basin locations. Catch basins were identified on as-built plans and the January 2019 design drawings, for the existing and proposed conditions, respectively. Existing and offsite topography for the site was obtained from SanGIS data downloads, which provide two-foot contours developed from LiDAR data collected in 2014. Proposed grading was indicated in the design drawings provided by REC.

#### 2.1.2 Runoff Coefficient

A runoff coefficient (i.e., C-Factor) is a factor within the Rational Method equation to account for rainfall losses that occur based on the soil type and land cover. In general, pervious areas such as parks create less runoff than impervious areas such as parking lots. Values applicable to the project site were determined based on Hydrologic Soil Group (HSG) Type D soils. The SDDDM provides runoff coefficients for urban land uses in Table A-1 and recommends that for “parks, golf courses, or other types of nonurban land use that are expected to be permanent, the appropriate value should be selected based upon the soil and cover and approved by the City.” Table 1 provides a summary of the C-factors used based on percent of impervious and pervious areas. The percent of impervious area was determined from existing conditions and planning documents provided by Carrier Johnson (Carrier Johnson, 2019).

**Table 1: Runoff Coefficients (C-Factors)**

% Impervious	% Pervious	C
90	10	0.95
80	20	0.85
70	30	0.70
60	40	0.64
40	60	0.57

% Impervious	% Pervious	C
30	70	0.52
20	80	0.46
10	90	0.41
0	100	0.35

### 2.1.3 Time of Concentration

For the existing condition subcatchments, the time of concentration (T) was determined by the longest flow distance and Figure A-4 of the SDDDM, which indicates that Equation 2 shall be used for lengths greater than 100 feet.

$$T = \frac{1.8 * (1.1 - C) * \sqrt{D}}{\sqrt[3]{s}} \quad \text{Equation 2}$$

Where T = time of concentration (minutes)  
 D = flow distance (feet)  
 s = slope (feet/feet)  
 C = runoff coefficient (unitless)

## 2.2 Hydraulics

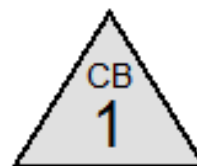
HydroCAD—a software program that allows the designer to input hydrologic conditions including area, runoff coefficient, and time of concentration—was used to complete the Rational Method runoff analysis and drainage system routing evaluations.

### 2.2.1 IDF Curves

The SDDDM provides intensity duration frequency (IDF) curves that relate the duration of a storm event, which is equivalent to the time of concentration as determined by the Rational Method, to the rainfall intensity in a topographic region. The SDDDM IDF curves were inputs to the HydroCAD model using an elevation factor of 1.00 as appropriate for sites below an elevation of 1,500 feet. The 50- and 100-year storm event intensity was determined using a storm duration equal to the longest time of concentration for the drainage system. For rainfall intensities used for each storm event, see existing and proposed HydroCAD reports in Appendix A.4 and B.5, respectively.

### 2.2.2 Catch Basins

In HydroCAD, catch basins were modeled as a Catch Basin “pond” node with insignificant storage, shown as a grey node in Figure 1. Catch basin inlet and outlet structures were modeled based on information (e.g., orifice size and pipe slope) gleaned from the available as-built plans, field data, and proposed grading plans as appropriate. A standard catch basin inlet included a horizontal orifice to model the grate inlet and a vertical orifice to model the storm drain pipe outlet from the catch basin. Three inches of localized ponding were allowed before the catch basin overflows to the next downstream catch basin.

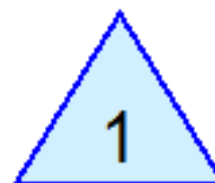


**Figure 1: HydroCAD Catch Basin “pond” node**

Catch basin inlets were assumed to be San Diego Regional Standard Drawing “Catch Basin – Type G” with a 2-foot by 3-foot grate inlet. See Storm Drain Network Exhibits, Standard Drawings, and Grading Plans in Appendix B.1. A sensitivity analysis was conducted to determine whether grate inlets or pipe sizes would control. At no point were the grate inlets controlling compared to pipe flow. Therefore, to optimize model convergence, the grate inlets were not included in the full pipe conveyance models.

### 2.2.3 Detention Pond

In the existing condition, localized ponding is known to occur along the southern boundary prior to discharge for large storm events. Based on grading plans provided, ponding will also be designed to occur in the proposed condition. To model the ponded area, the surface area at incremental elevations was measured in GIS and input as a Detention Pond “pond” node in HydroCAD, shown as a blue node in Figure 2. The pond was modeled using the Storage-Indication routing method and a 0.01-hour time step (HydroCAD User Manual). The maximum ponding elevation was used to generate figures illustrating the existing and proposed estimated extents of ponding located in Appendix A.5 and B.6 respectively.



**Figure 2: HydroCAD Detention Pond “pond” node**



### 3. EXISTING DRAINAGE SYSTEM ANALYSIS

#### 3.1 Existing Conditions

The Project site currently consists of a large multi-purpose stadium and associated parking lot. The parking lot covers most of the site (Figure 3). The Project site is bordered by the San Diego River on the south and Murphy Canyon Creek on the east. There are currently eight major outfalls from the project, six that discharge south into the San Diego River and two that discharge east into the Murphy Canyon Channel. Four outfalls are impacted by the project and included in this evaluation—Drainage Systems A, B, C and D (Figure 3)—that discharge runoff from the existing site to the San Diego River. There is no project impact to the two outfalls into Murphy Canyon Channel. The existing site is mostly impervious and includes the stadium, buildings, and surrounding parking lot. The current stadium was constructed on a raised earthen mound above the San Diego River 100-year floodplain. Much of the parking lot is within the 100-year floodplain. More detailed information about the floodplain is presented in Section 5.

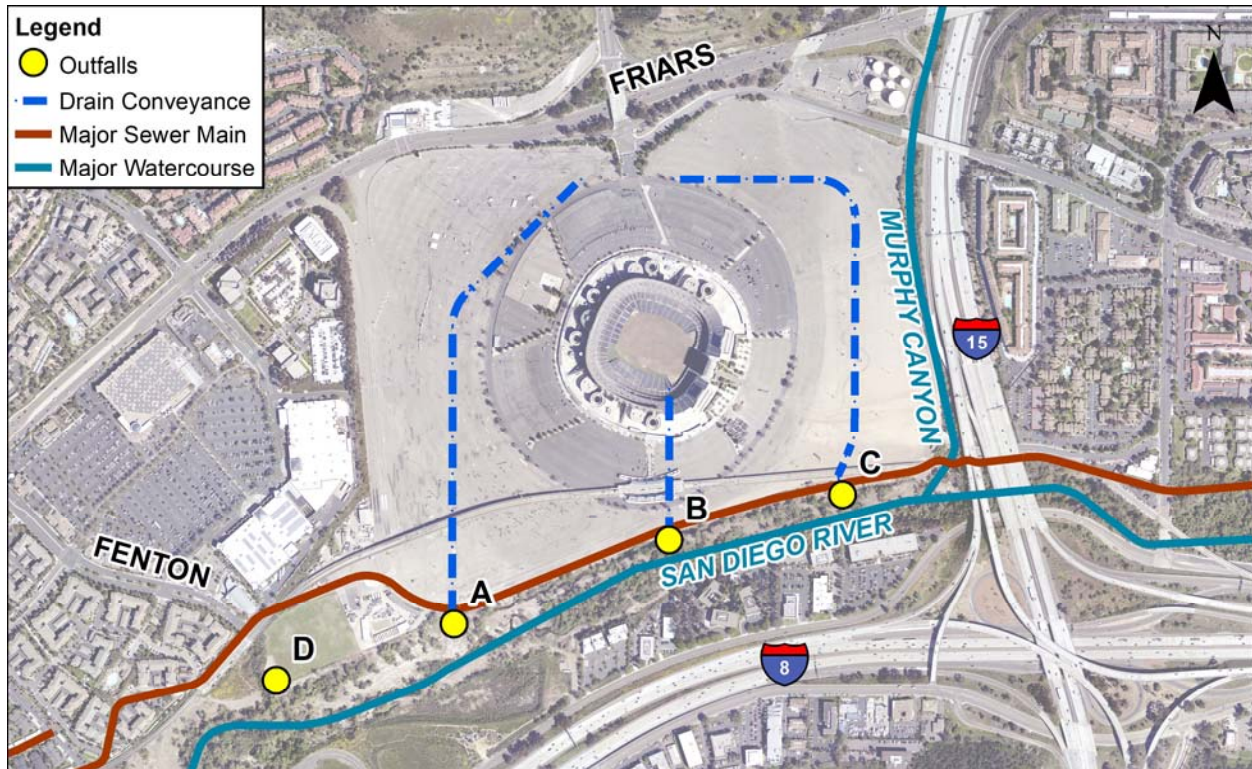


Figure 3: Existing Conditions Site Map

#### 3.2 Existing Conditions Hydrology

##### 3.2.1 Drainage Area

Drainage systems A and C collect runoff from and drain the parking lot area, while drainage system B drains the stadium. Drainage system D drains the practice fields and building area in the south west corner of the site. Minor areas of offsite run-on from the adjacent road and hillside discharge



onto the parking lot on the north and west sides. As-built plans for the existing drainage systems are presented in Appendix A.1. A drainage area exhibit is provided in Appendix A.2.

### 3.2.2 Runoff Coefficient

The area surrounding the stadium is predominantly asphalt parking lot. Inside the stadium the turf was assumed to be lined and therefore all precipitation would be collected in drainage System B rather than infiltrating into the ground. As such, a runoff coefficient (i.e., C-factor) of 0.95 was used for most areas in the existing condition. See runoff coefficient references in Appendix A.3.

### 3.2.3 Time of Concentration

For each subcatchment the time of concentration was determined by the longest flow distance and Equation 2. The longest time of concentration for the drainage system was used to determine the design storm rainfall intensity.

### 3.2.4 Results

A summary of the existing conditions is provided in Table 2.

**Table 2: Summary of Existing Hydrology**

Drainage system	Drainage Area (acres)	C (unitless)	Max. Time of Concentration (min)	Rainfall Intensity (in/hr)		Peak Runoff (cfs)	
				50-yr	100-yr	50-yr	100-yr
A	95	0.95	7	3.80	4.00	291	309
B	10	0.95	5	4.20	4.50	37	39
C	64	0.95	7	3.80	4.00	200	212
D	3.0	0.52	20	3.25	3.45	3.6	3.8

## 3.3 Existing Conditions Hydraulics

### 3.3.1 Data Sources

As-built plans were used in conjunction with field-collected data to determine the reach lengths of the storm drain network and dimensions of the existing catch basins. As-builts for the original storm drain construction were dated 1966. The survey datum for the United States was updated in the early 1980s, resulting in inconsistent elevation data provided for a particular point before and after this adjustment, (Caltrans Survey Manual Chapter 4: Survey Datums). As-built data from 1966 plans were used to determine storm drain slopes and catch basin depths, which are independent of elevation datum discrepancies. Catch basin depths were field-verified to account for any potential discrepancies due to updated elevation datums. LiDAR data from 2014 as provided by SanGIS was used to determine catch basin grate inlet elevations.

Prior to discharging, the existing storm drains penetrate through an 84- to 96-inch diameter sanitary sewer main paralleling the north bank of the San Diego River. Drainage systems A, B, and C discharge into the San Diego River via 36-inch reinforced concrete pipes (RCP). The storm drain lines are reduced to 34-inch steel pipes to pass through the sewer main and are cased in

polyethylene to prevent comingling of sewer and storm water flows. Because of this design, the outfalls cannot be modified. Drainage System D discharges into an earthen channel which discharges into the San Diego River. See Storm Drain Network Exhibits and As-Builts in Appendix A.2. Model inputs and results are presented in Appendix A.4: HydroCAD Reports.

**3.3.2 Results**

The diameter of the three major storm drain outfalls to the San Diego River is the limiting factor for the drainage systems discharge capacity. For large storm events (i.e., 50- and 100-year events), excess runoff ponds above ground until it can be conveyed through the pipe drainage system to the river outfalls.

Hydraulic modeling results are presented in Table 3. Estimated extents of localized ponding are shown in Appendix A.5.

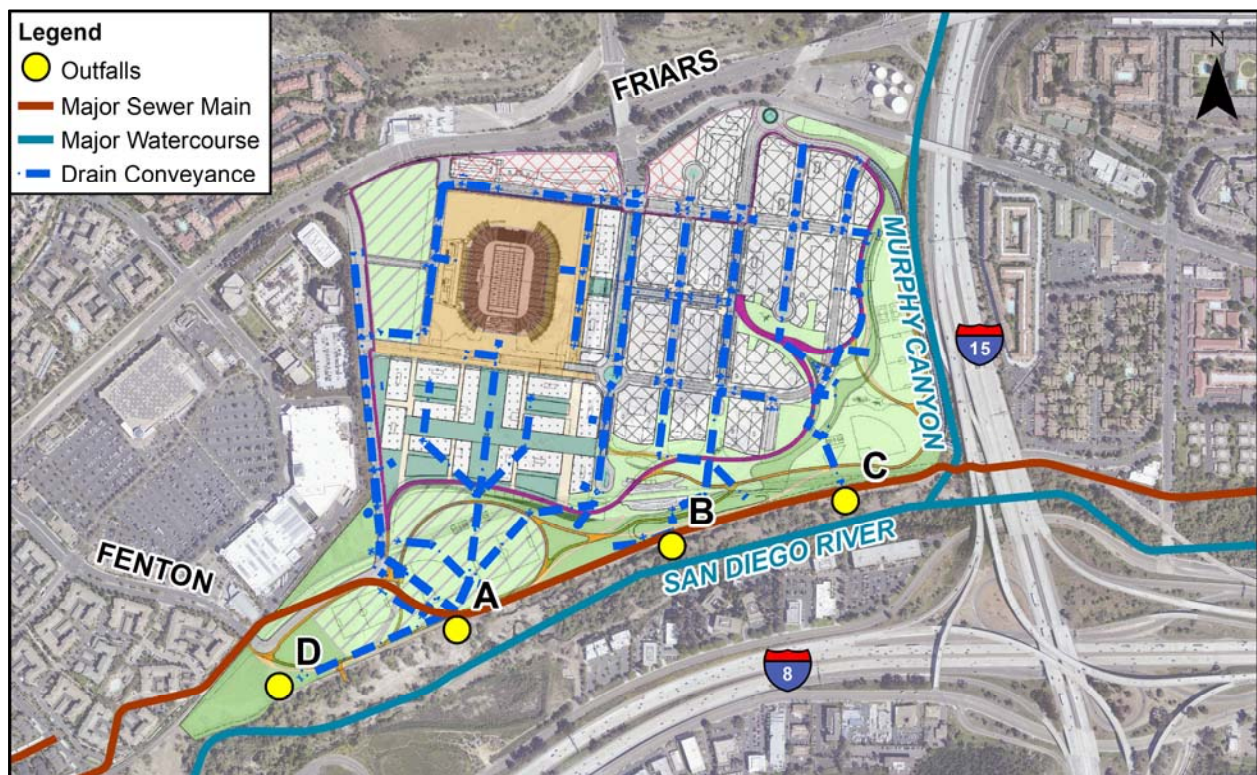
**Table 3: Summary of Existing Hydraulics**

Drainage system	Peak Discharge (cfs)		Max Ponding Depth (ft)		Velocity (fps)	
	50-yr	100-yr	50-yr	100-yr	50-yr	100-yr
A	62	69	3.3	3.4	9.8	9.9
B	13	13	n/a	n/a	4.0	4.0
C	56	56	0.89	0.90	8.9	9.0
D	3.6	3.8	n/a	n/a	n/a	n/a

## 4. PROPOSED ONSITE DRAINAGE SYSTEM ANALYSIS

### 4.1 Proposed Conditions

The Project development includes new campus-related residential buildings, commercial/retail, stadium, roads, and creation of a River Park along the San Diego River. The River Park will serve as a floodplain buffer between the San Diego River and the developed portions of the Project, which will be constructed on pads elevated above the floodplain depths. The drainage design for the Project includes routing onsite runoff through permanent storm water quality basins (addressed under a separate technical report effort). After passing through the water quality basins, storm water would be conveyed through proposed pipe drainage systems and discharge at the existing storm drain outfalls. Water quality basins are designed to treat a “low-flow” storm event to address pollutant loads. Flows in excess of the “low-flow” bypasses the basin and is conveyed directly to the storm drain outlets. Therefore, for the purpose of flood condition modeling, the water quality basins were assumed to be full/clogged and the storage capacity of the basins was excluded from the model. As discussed in Section 3.3, the existing outfalls for drainage systems A, B, and C penetrate through an 84- to 96-inch diameter sanitary sewer main paralleling the north bank of the San Diego River and the outfalls cannot be modified. The proposed drainage system would tie into the existing outfalls for the three drainage systems indicated in Figure 4. Flow in excess of the capacity of Outfalls B and C are designed to pond above ground before discharge, similar to the existing condition. Flow in excess of the capacity of Outfall A is conveyed in a constructed channel to Outfall D.



**Figure 4: Proposed Conditions Site Map**

## **4.2 Proposed Conditions Hydrology**

### **4.2.1 Drainage Area**

Drainage area delineations were made using SanGIS 2014 2-foot contours and grading plans in the 75% Design Drawings provided by REC. The site was generally assumed to drain southerly toward the San Diego River. Buildings were assumed to have flat roofs with symmetrical downspouts discharging to the adjacent streets and alleyways. Areas of offsite run-on that were included in the existing delineations were assumed to be maintained in the proposed conditions. Drainage areas were delineated to each catch basin inlet as called out in the plans; see Storm Drain Network Exhibits and Grading Plans in Appendix B.1 and Drainage Area Exhibit provided in Appendix B.2.

### **4.2.2 Runoff Coefficient**

The Project land cover will be a campus with a mix of park areas, sport fields, buildings, and parking lots. Per the SDDDM, all soils were assumed to be HSG Type D. Drainage System A will drain the stadium, soccer fields, mall, the western portion of the proposed housing, and a large portion of the River Park. Drainage System B will drain the entrance, the central portion of housing, trolley stop, and the eastern portions of the stadium and mall. Drainage System C will drain the eastern portion of the housing and a large portion of the River Park. Drainage System D will drain the ball field and excess runoff from System A. Runoff coefficients were adapted from the Landcover Plan in the January 2019 Consultant Package provided by Carrier Johnson (Carrier Johnson, 2019), and references as discussed in Section 2.1.2, Appendix B.3 Proposed Condition: Runoff Coefficient.

### **4.2.3 Time of Concentration**

A time of concentration for the proposed condition for small areas (i.e., less than 3 acres) was assumed to be five minutes. Five minutes is a typical conservative estimate of overland flow for sub-catchments of small size and is the smallest value reported on SDDDM intensity-duration-frequency (IDF) curves. For larger drainage areas with primarily overland flow the time of concentration was determined by the longest flow distance and Equation 2. The longest time of concentration for the drainage system was used to determine the design storm rainfall intensity. The longest flow path and time of concentration for each system was calculated as provided in Appendix B.4.

### **4.2.4 Results**

A summary of the runoff for the 24-hour, 50- and 100-years storms in the proposed conditions is provided in Table 4.

**Table 4: Summary of Proposed Hydrology**

Drainage system	Drainage Area (acres)	C (unitless)	Max. Time of Concentration (min)	Rainfall Intensity (in/hr)		Peak Runoff (cfs)	
				50-yr	100-yr	50-yr	100-yr
A	87.2	0.66	30	1.90	2.00	109	116
B	43.7	0.70	24	2.15	2.26	66	69
C	37.8	0.64	29	1.90	2.00	46	49
D	2.9	0.52	20	*	*	2.9	3.1

\*System D analyzed as a part of System A in the proposed models.

### 4.3 Proposed Conditions Hydraulics

#### 4.3.1 Data Sources

Storm drain infrastructure and grading plans were developed and provided by REC (January 2019 design drawings). If not otherwise provided in the grading plans, pipes were assumed to be 18-inch diameter pipes at a 1% slope.

For the purposes of flood modeling, the proposed water quality basins were assumed to be full and their storage capacity was excluded from the model. Water quality basin surface areas were incorporated in their appropriate subcatchment as grassed park land use.

#### 4.3.2 Results

Similar to the existing condition, the diameter of the three major storm drain outfalls to the San Diego River will be the limiting factor of the drainage systems discharge capacity in the proposed condition. For large storm events (i.e., 50- and 100-year events), the Drainage Systems B and C will be designed to allow excess runoff to pond above ground in the River Park buffer areas until it can be conveyed through the pipe drainage system to the three storm drain outfalls. Drainage System A is designed to convey excess runoff through a spillway to Outfall D.

Model inputs and results at each node can be found in Appendix B.5: HydroCAD Reports. A summary of hydraulic modeling results for the 50- and 100-year events are presented in Table 5. Estimated extents of localized ponding at outfalls B and C are shown in Appendix B.6.

**Table 5: Summary of Proposed Hydraulics**

Drainage system	Peak Discharge (cfs)		Max Ponding Depth (ft)		Velocity (fps)	
	50-yr	100-yr	50-yr	100-yr	50-yr	100-yr
A	70	70	n/a	n/a	11.0	11.1
B	63	64	0.05	0.14	10.0	10.2
C	46	49	0	0	8.7	8.7
D	34	42	n/a	n/a	1.3	1.4



## 5. OFFSITE

The Project involves some minor improvements to offsite areas including road widening along Friars Road and extending the existing 96-inch storm drain outfall under the new road extension of Fenton Parkway. These activities are addressed in the following section.

### 5.1 Road Widening

The Project includes minor impacts to Friars Road north of the site, as seen in Appendix C.1. These impacts are associated with lane widening for the on/off ramps from Friars Road to Mission Village Drive and the intersection at Friars Road and Northside Drive. Water quality from the road widening will be addressed by green street design (Rick 2019a). The result of these impacts is a slight increase in impervious area. The change in impervious area creates a negligible impact on the runoff from the affected areas. See Table 6 below for a summary of existing and proposed offsite runoff peak flow rates for the 50- and 100-year storms.

**Table 6: Summary of Offsite Runoff for Road Widening**

Condition	Drainage Area (ac)	% Impervious	Runoff Coefficient (C)	Peak Runoff (cfs)	
				50-yr	100-yr
Existing	27.2	83	0.88	81	86
Proposed	27.2	84	0.89	83	88
% Change	0	1%	1%	2%	2%

### 5.2 Storm Drain Extension

The Project includes extending Fenton Parkway into the proposed development from the intersection with the trolley line at the southwest corner of the site. In the existing conditions, there is a 96-inch diameter storm drain outfall that discharges stormwater from a large upland drainage area (offsite) into the storm drain running under Fenton Parkway. Runoff to this storm drain is from the canyons and urban areas north of the site as seen in Appendix C.4. Outfall D discharges just below this outfall and is not anticipated to have any impact on the peak discharge to this point. Because of the relative size of the Project site compared to the size of the drainage area to the 96-inch outfall, the peak in storm events will happen at significantly different times and is therefore not anticipated to affect either peak flows. The 96-inch outfall has a Tc of over an hour while the drainage from the Project area is a tenth of that size and has a time of concentration half that size. This means that the smaller peaks of the two events will not coincide and the discharge to outfall D will have minimal impact on the discharge of the 96-inch outfall to the San Diego River.

The extension of the outfall will require removal of vegetation and restoration of the existing earthen channel. The design for this channel will need to incorporate energy dissipation features to convey flow in a non-erosive manner all the way to the San Diego River.

### 5.3 Flood Plain

The site includes Murphy Canyon Channel within the eastern project boundary. The San Diego River is to the south of the project site. Portions of the site are located within the 100-year

floodplain for both the San Diego River and Murphy Canyon Channel, as shown on the Federal Emergency Management Agency (FEMA) floodplain maps included in Appendix D. Therefore, the Project would be subject to floodplain requirements in accordance with the FEMA National Flood Insurance Program (NFIP). The Project development areas would be setback from the channels, allowing for active and passive park areas to be incorporated along the easterly and southerly edge of the development. Park areas would provide a more natural floodplain during larger storms events and reduce or eliminate the commingling of flood waters with developed areas and associated pollutants.

## 6. FINDINGS

As discussed in Sections 3 and 4, the Project will maintain the same outfalls and localized ponding condition in the proposed condition as the existing condition. The Project will include significant area that will be developed as active and passive park areas to further isolate flooding from developed areas. This feature of the project results in a significant reduction in the overall impervious area, as can be seen in the corresponding reductions in the C-factors, relative to the existing condition. This has the effect of reducing the overall peak runoff from the site in the proposed condition as indicated in Table 7.

**Table 7: Comparison of Existing and Proposed Site Conditions for 100-year Rational Method Event**

Outfall	Existing Condition				Proposed Condition			
	Area (acres)	C	Runoff (cfs)	Discharge (cfs)	Area (acres)	C	Runoff (cfs)	Discharge (cfs)
A	95	0.95	309	63	87	0.66	116	70
B	10	0.95	39	13	44	0.7	69	64
C	64	0.95	212	56	38	0.64	49	49
D	3	0.52	3.8	3.8	2.9	0.52	3.1	42



## 7. REFERENCES

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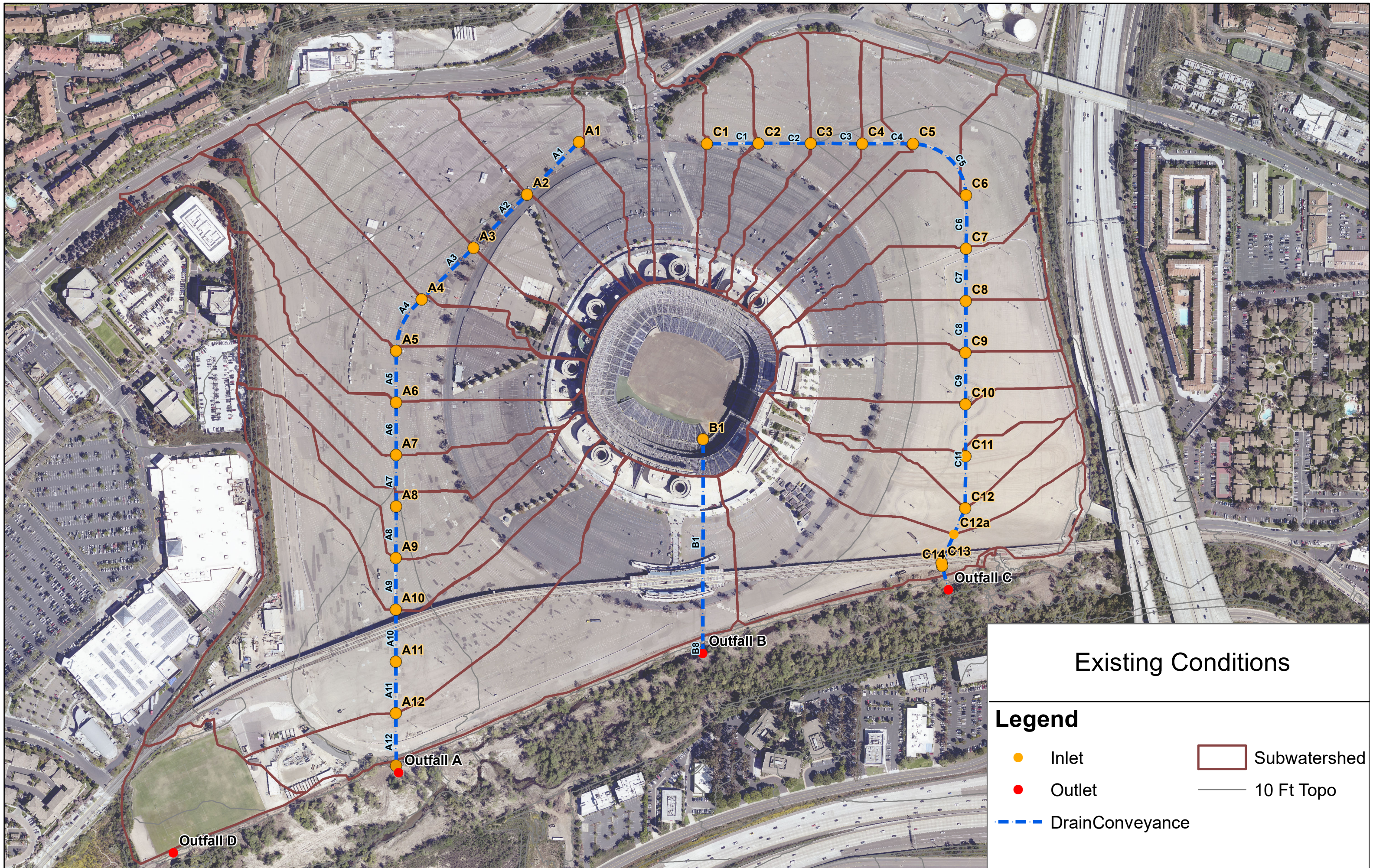
# **APPENDIX A**

## **Existing Conditions Supporting Material**

# **APPENDIX A.1**

## Existing Conditions: Storm Drain Network & As-Builts



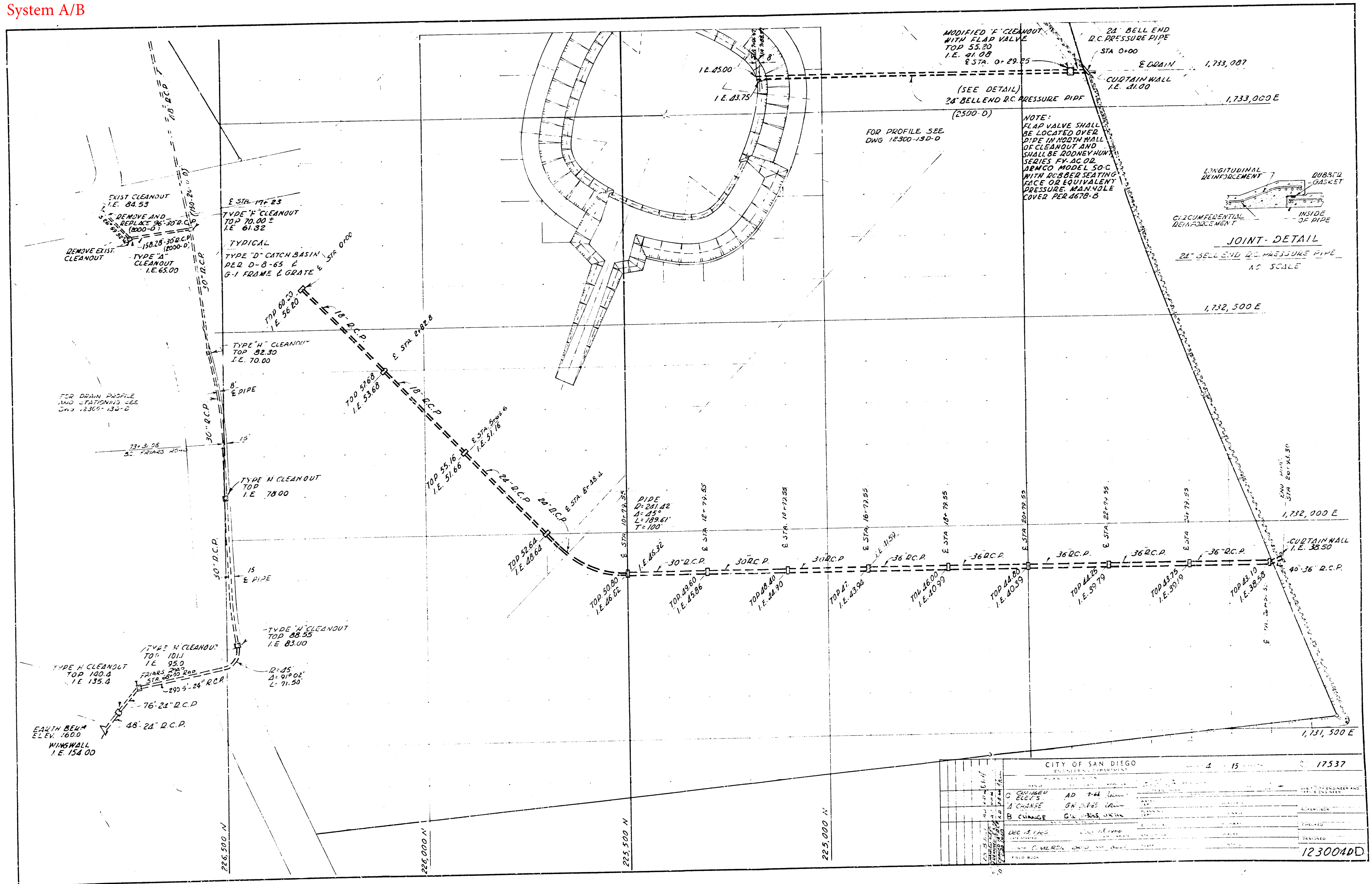


### Existing Conditions

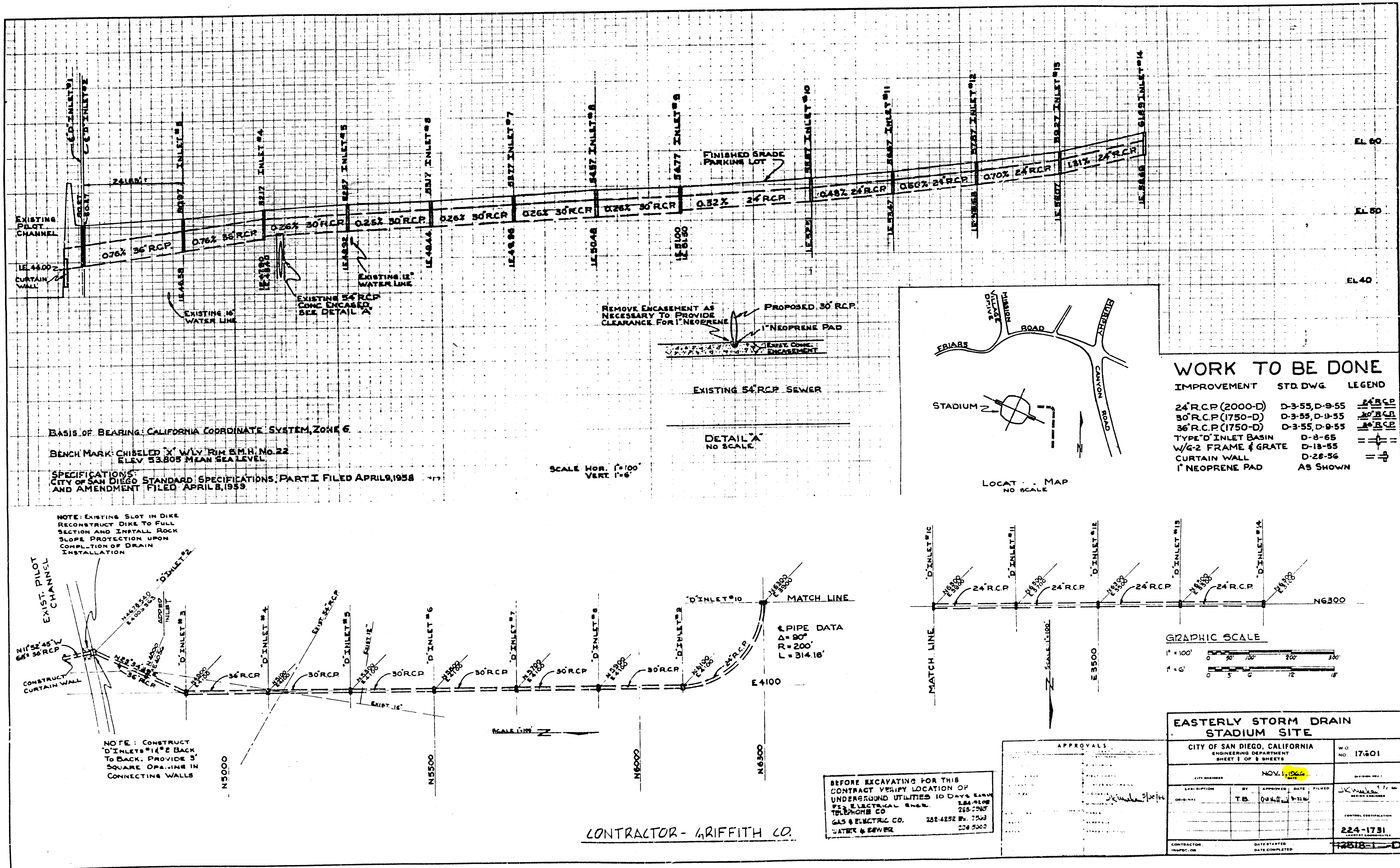
#### Legend

- Inlet
- Outlet
- .- DrainConveyance
- Subwatershed
- 10 Ft Topo







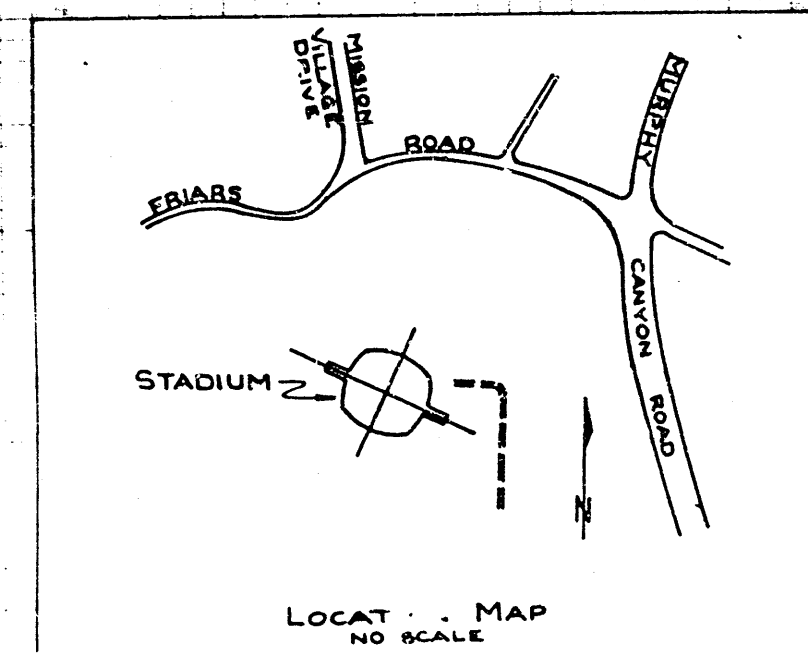


BASIS OF BEARING: CALIFORNIA COORDINATE SYSTEM, ZONE 6

BENCH MARK: CHISELED 'X' W/VY RIM 6M.H. No. 22  
 ELEV. 53.805 MEAN SEA LEVEL

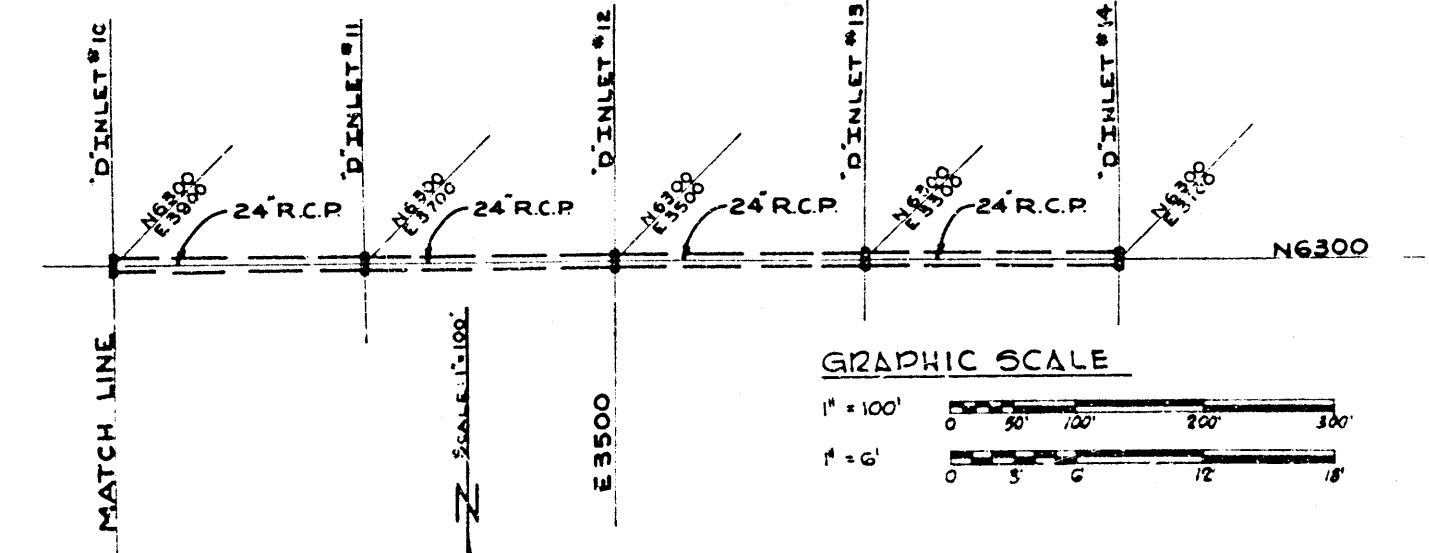
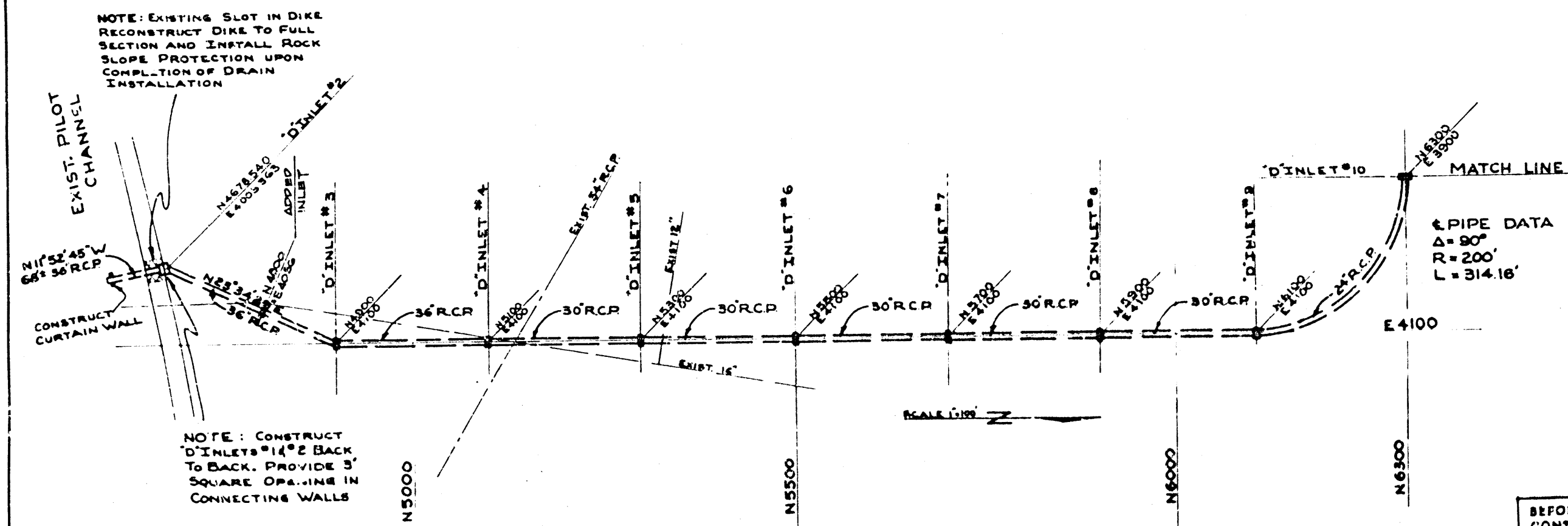
SPECIFICATIONS:  
 CITY OF SAN DIEGO STANDARD SPECIFICATIONS, PART I FILED APRIL 9, 1958  
 AND AMENDMENT FILED APRIL 8, 1959

SCALE HOR. 1"=100'  
 VERT. 1"=6'



**WORK TO BE DONE**

IMPROVEMENT	STD. DWG.	LEGEND
24" R.C.P. (2000-D)	D-3-55, D-9-55	
30" R.C.P. (1750-D)	D-3-55, D-9-55	
36" R.C.P. (1750-D)	D-3-55, D-9-55	
TYPE 'D' INLET BASIN	D-8-65	
W/G-2 FRAME & GRATE	D-13-55	
CURTAIN WALL	D-28-56	
1" NEOPRENE PAD	AS SHOWN	



BEFORE EXCAVATING FOR THIS CONTRACT VERIFY LOCATION OF UNDERGROUND UTILITIES 10 DAYS EARLY  
 P.E. ELECTRICAL ENGR. 284-9108  
 TELEPHONE CO. 288-2587  
 GAS & ELECTRIC CO. 282-4252 Ex. 7548  
 WATER & SEWER 238-5000

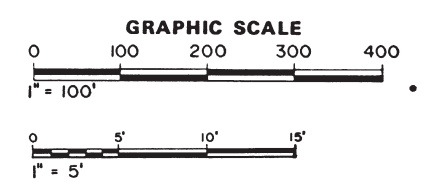
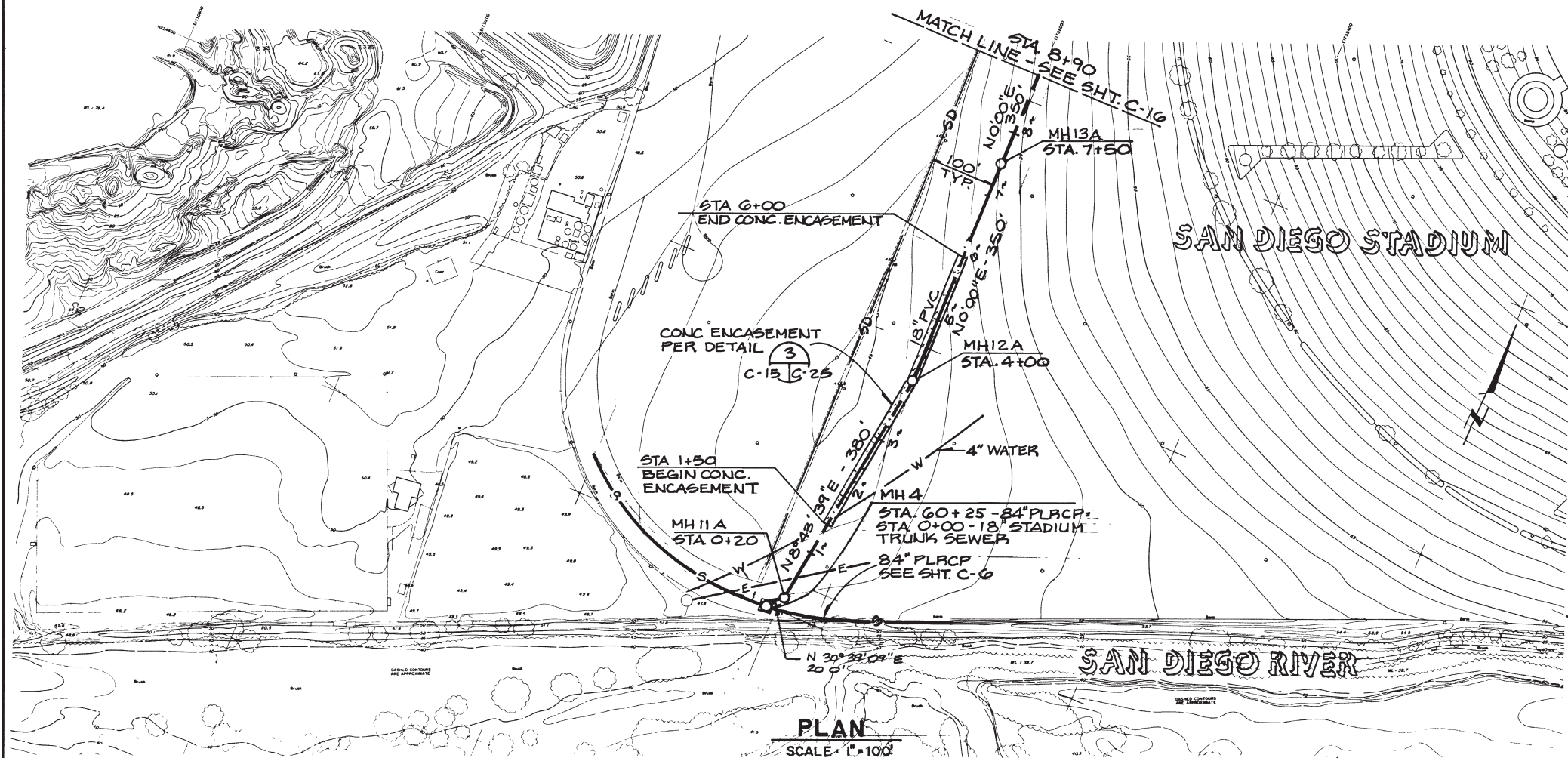
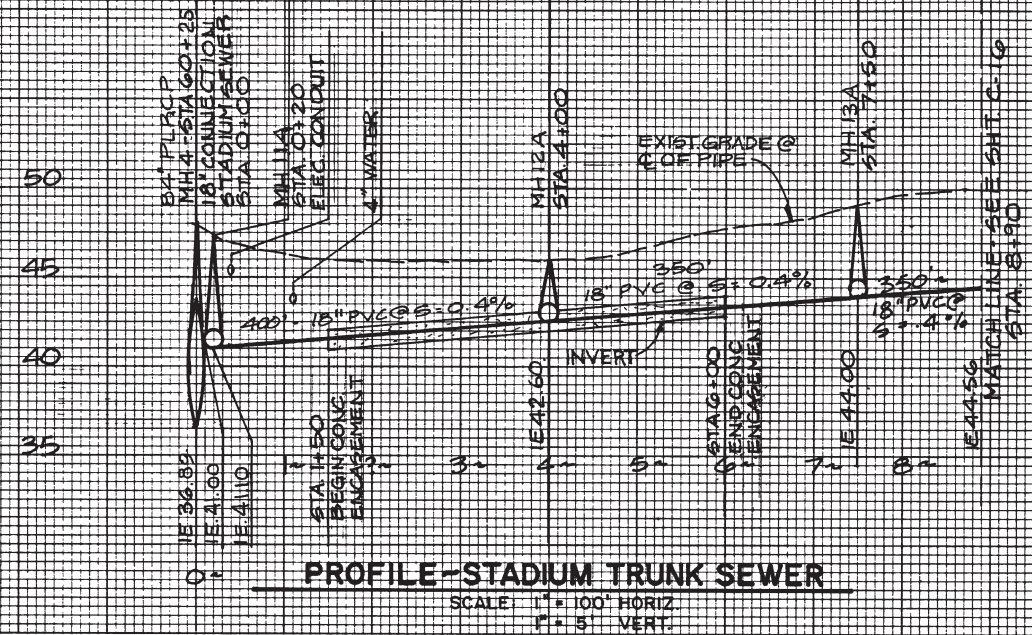
**APPROVALS**

DATE	BY	FILED
NOV. 1, 1966	T.B.	NOV. 1, 1966

**EASTERLY STORM DRAIN STADIUM SITE**

CITY OF SAN DIEGO, CALIFORNIA		W.D. NO. 17601
ENGINEERING DEPARTMENT		
SHEET 1 OF 8 SHEETS		
CITY ENGINEER	NOV. 1, 1966	BY: J. K. ...
DATE	APPROVED	FILED
NOV. 1, 1966	T.B.	NOV. 1, 1966
CONTRACTOR	DATE STARTED	CONTROL CERTIFICATION
GRIFFITH CO.		224-1731
INSPECTOR	DATE COMPLETED	12518-1-D





IF SHEET IS LESS THAN  
 24X36  
 IT IS A REDUCED PRINT -  
 SCALE REDUCED ACCORDINGLY

**HIRSCH & COMPANY**  
 CONSULTING ENGINEERS  
 4420 Ramer Ave. Suite 100  
 San Diego, California 92120

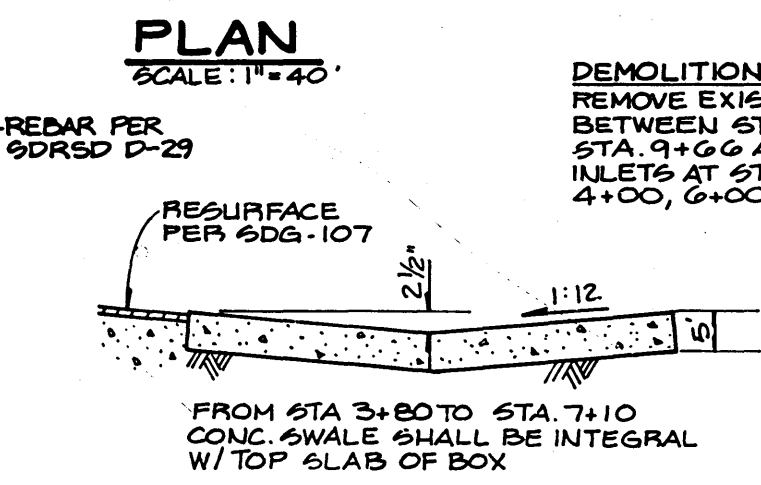
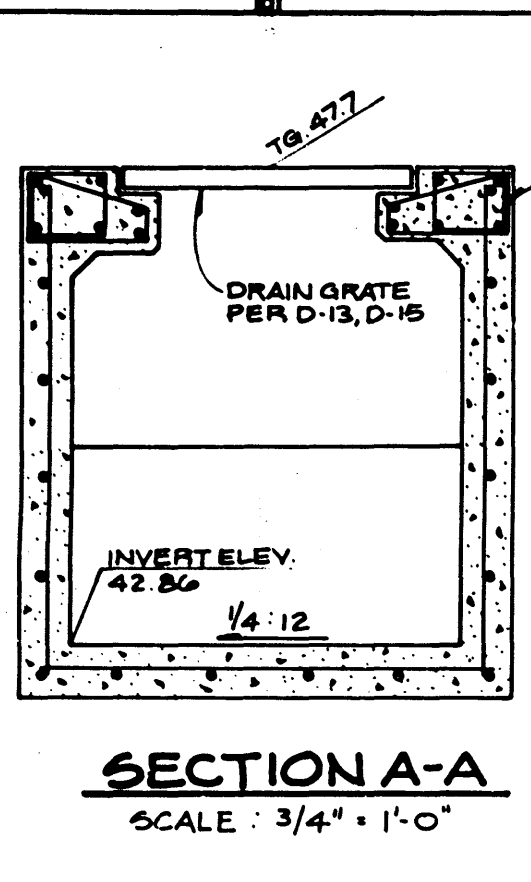
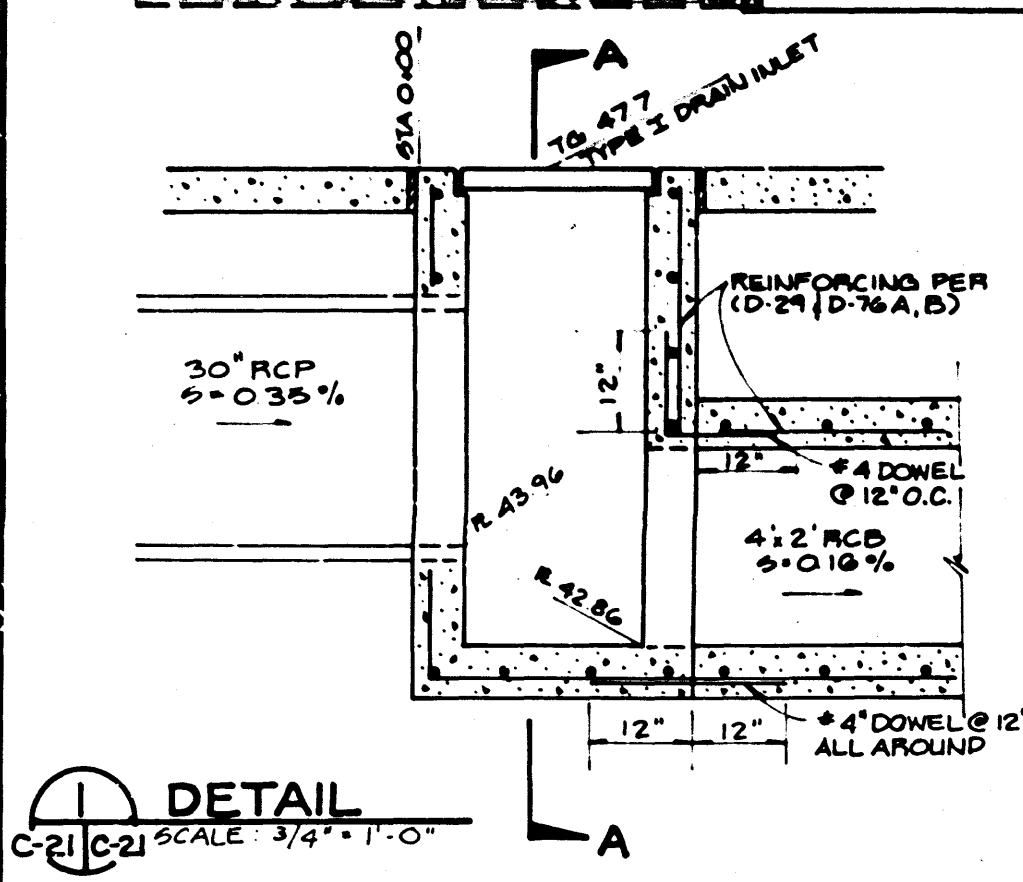
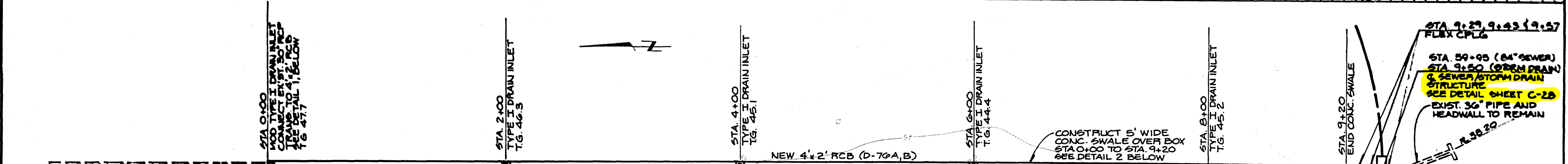
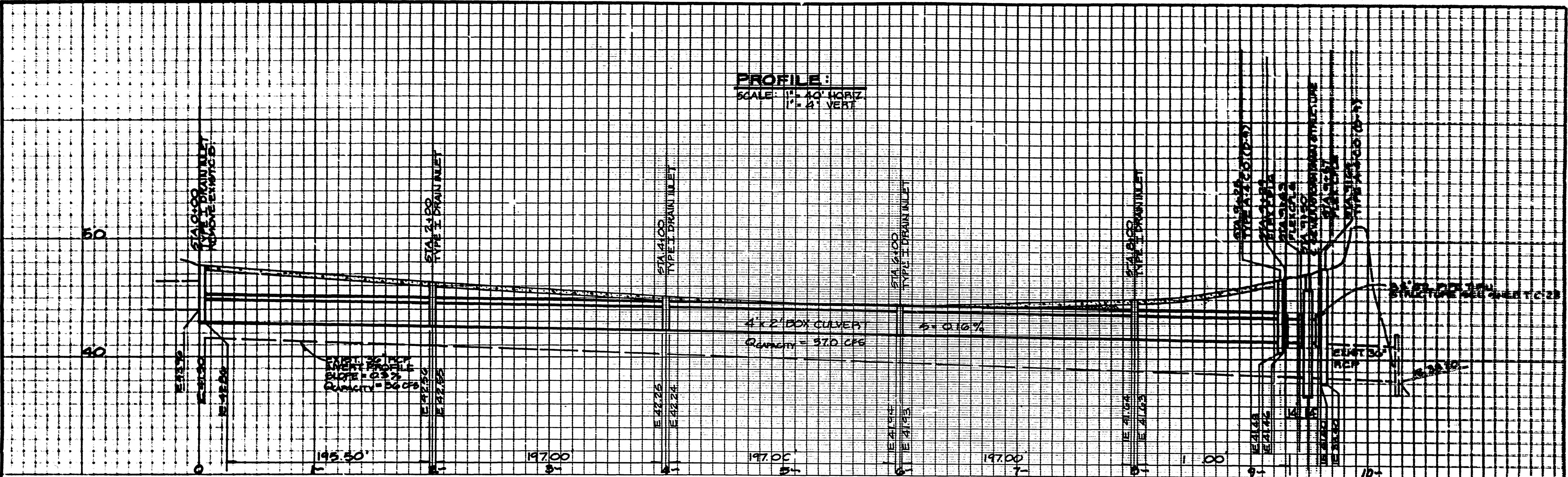
**C-15**

<b>NORTH MISSION VALLEY INTERCEPTOR SEWER REPLACEMENT</b>			
<b>STADIUM TRUNK SEWER STA. 0+00 TO STA. 8+90</b>			
CITY OF SAN DIEGO, CALIFORNIA UTILITIES DEPARTMENT SHEET 18 OF 48 SHEETS		E.W.O. 170401 U.W.O. 170401	
UTILITY DIRECTOR	DATE	DESIGN ENGINEER	
DESCRIPTION	BY	APPROVED	FILMED
ORIGINAL	HIRSCH	[Signature]	[Signature]
CONSTRUCTION RECORD			CONTROL CERTIFICATION
CONTRACTOR			224-1732
INSPECTOR			LAMBERT COORDINATES
CONNECTIONS BY:			25499-18 -D

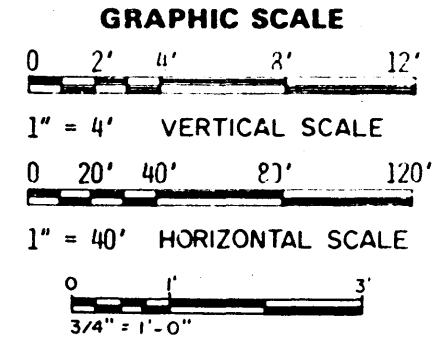




**PROFILE:**  
SCALE: 1" = 40' HORIZ  
1" = 2' VERT

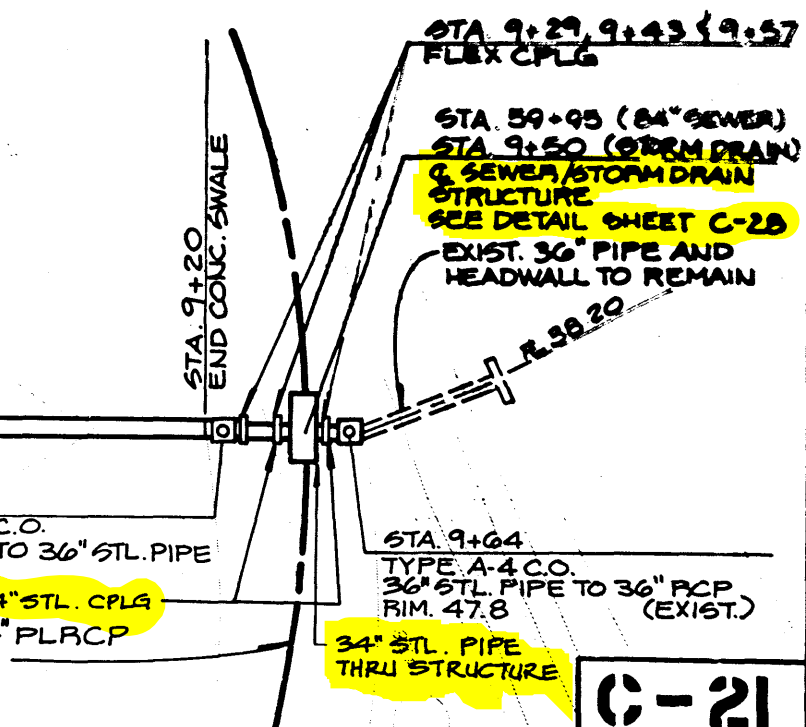


**DEMOLITION NOTE**  
REMOVE EXIST. 36" RCP BETWEEN STA. 0+00 TO STA. 9+66 AND EXIST. DRAIN INLETS AT STA. 0+00, 2+00, 4+00, 6+00 & 8+00



IF SHEET IS LESS THAN 24X36 IT IS A REDUCED PRINT - SCALE REDUCED ACCORDINGLY

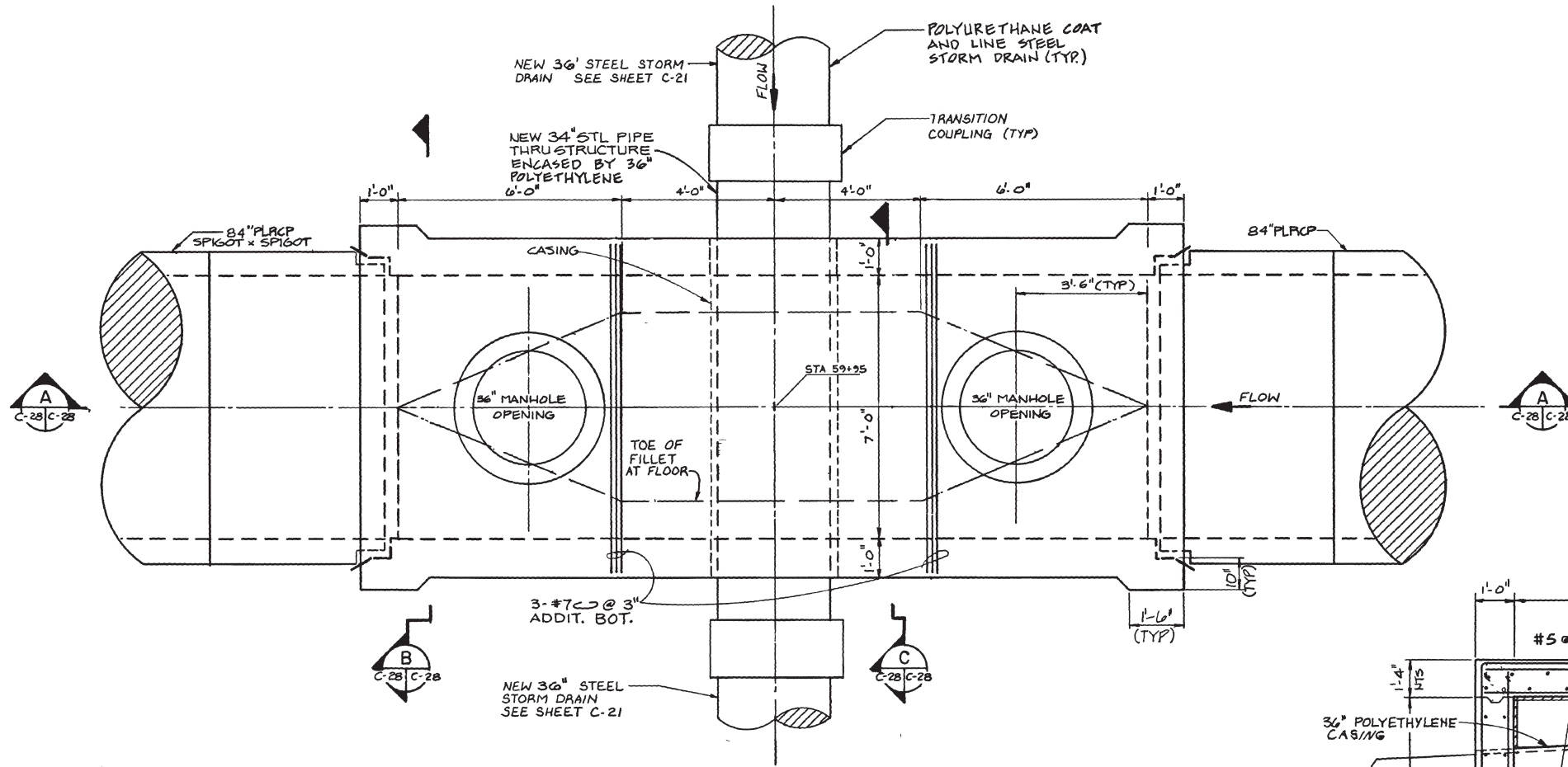
**HIRSCH & COMPANY**  
CONSULTING ENGINEERS  
4420 Ramer Ave. Suite 100  
San Diego, California 92120



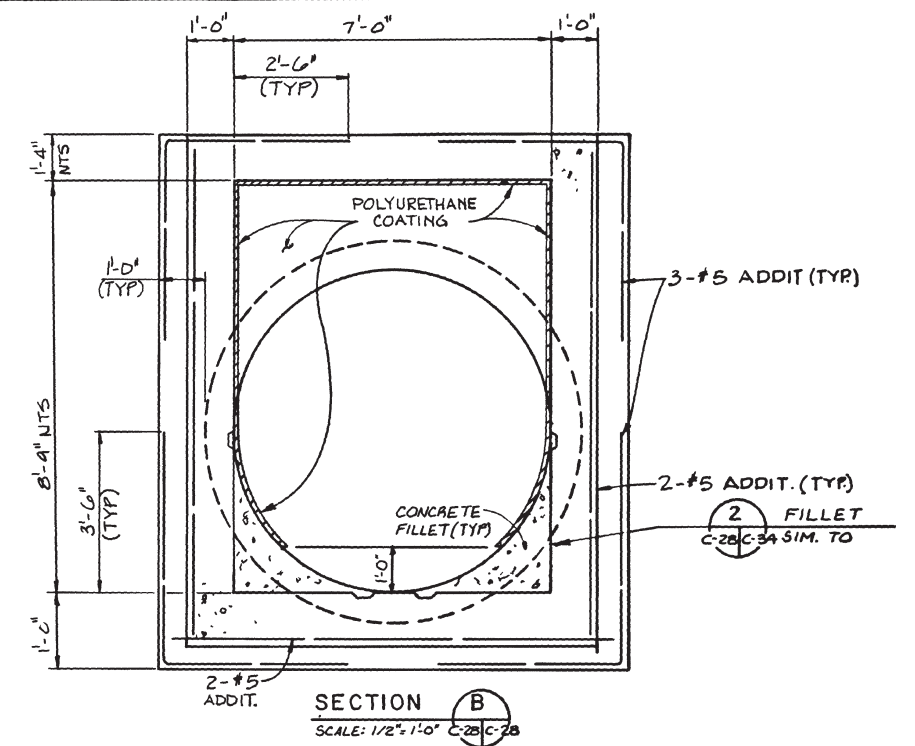
<b>NORTH MISSION VALLEY INTERCEPTOR SEWER REPLACEMENT</b>			
<b>RECONSTRUCT STADIUM STORM DRAIN - 1</b>			
CITY OF SAN DIEGO, CALIFORNIA UTILITIES DEPARTMENT SHEET 24 OF 70 SHEETS		E.W.O. 170401 U.W.O. 170401	
APPROVED BY: <i>[Signature]</i> UTILITIES DIRECTOR	DATE: 5/1/92	DESIGNED BY: <i>[Signature]</i> DESIGN ENGINEER	CONTROL CERTIFICATION
DESCRIPTION: ORIGINAL	BY: HIPOCH	APPROVED: <i>[Signature]</i>	DATE: 2-25-91
CONTRACTOR: <i>[Signature]</i>	DATE STARTED: 4-25-91	DATE COMPLETED: 2-1-93	CONNECTIONS BY:
			224-1728 LAMBERT COORDINATES 25499-24-D



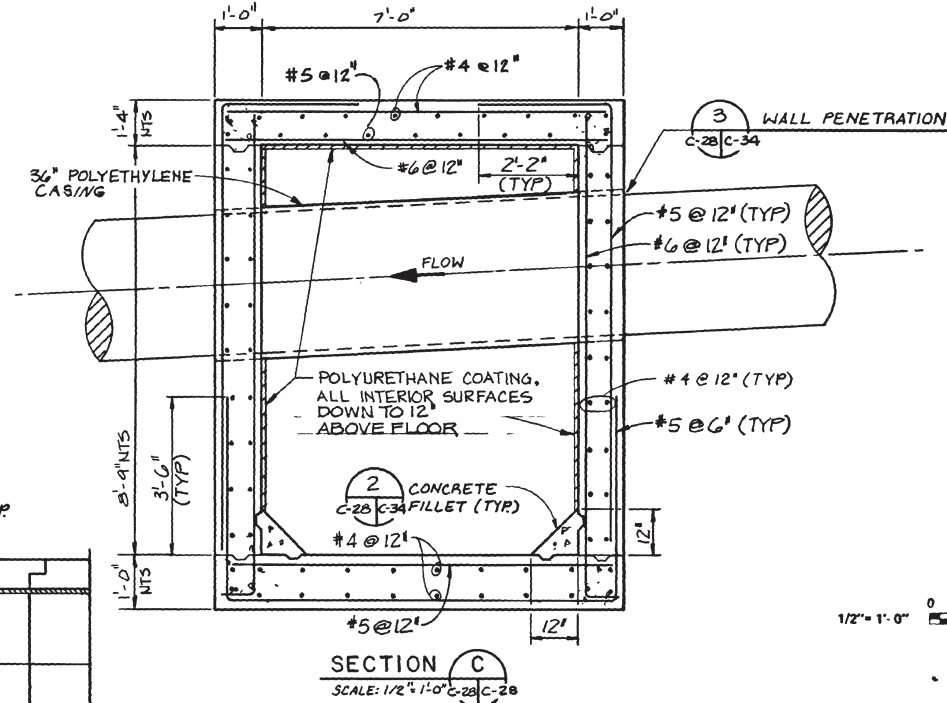
**System A**



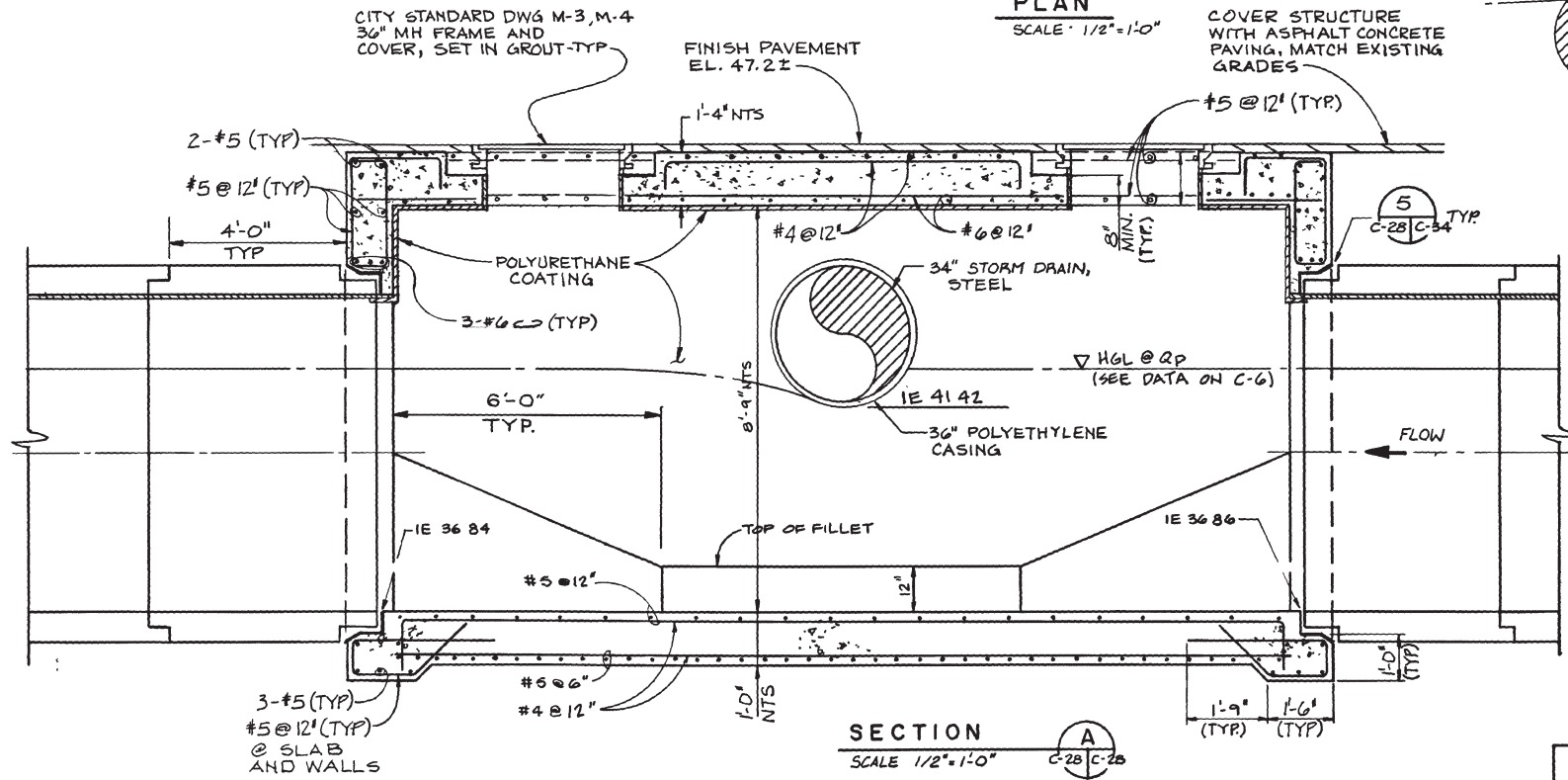
**PLAN**  
SCALE: 1/2" = 1'-0"



**SECTION B**  
SCALE: 1/2" = 1'-0" C-28/C-34



**SECTION C**  
SCALE: 1/2" = 1'-0" C-28/C-34



**SECTION A**  
SCALE: 1/2" = 1'-0" C-28/C-25



**C-28**



NOTES:  
1. FOR GENERAL STRUCTURAL NOTES AND DETAILS, SEE SHEET C-35.

<b>NORTH MISSION VALLEY INTERCEPTOR SEWER REPLACEMENT</b>			
<b>SEWER / STORM DRAIN STRUCTURE 59+95</b>			
CITY OF SAN DIEGO, CALIFORNIA UTILITIES DEPARTMENT SHEET 31 OF 68 SHEETS		E.W.O. 170401 U.W.O. 170401	
ASST. UTILITIES DIRECTOR	DATE	DESIGN ENGINEER	
DESCRIPTION	BY	APPROVED	FILMED
ORIGINAL	KAVC	Jay K. Chilton	
CONSTRUCTION RECORD		CONTROL CERTIFICATION	
CONTRACTOR	DATE STARTED	LAMBERT COORDINATES	
INSPECTOR	DATE COMPLETED	25430 --31 -D	
CONNECTIONS BY:			

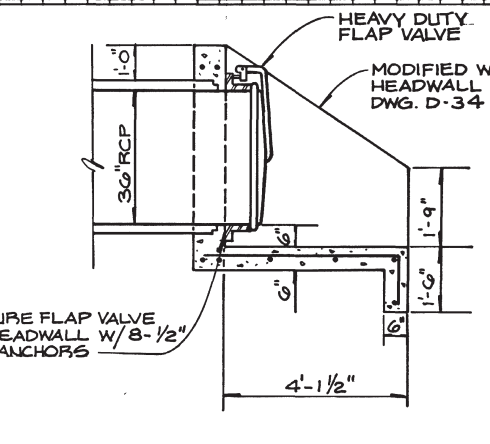
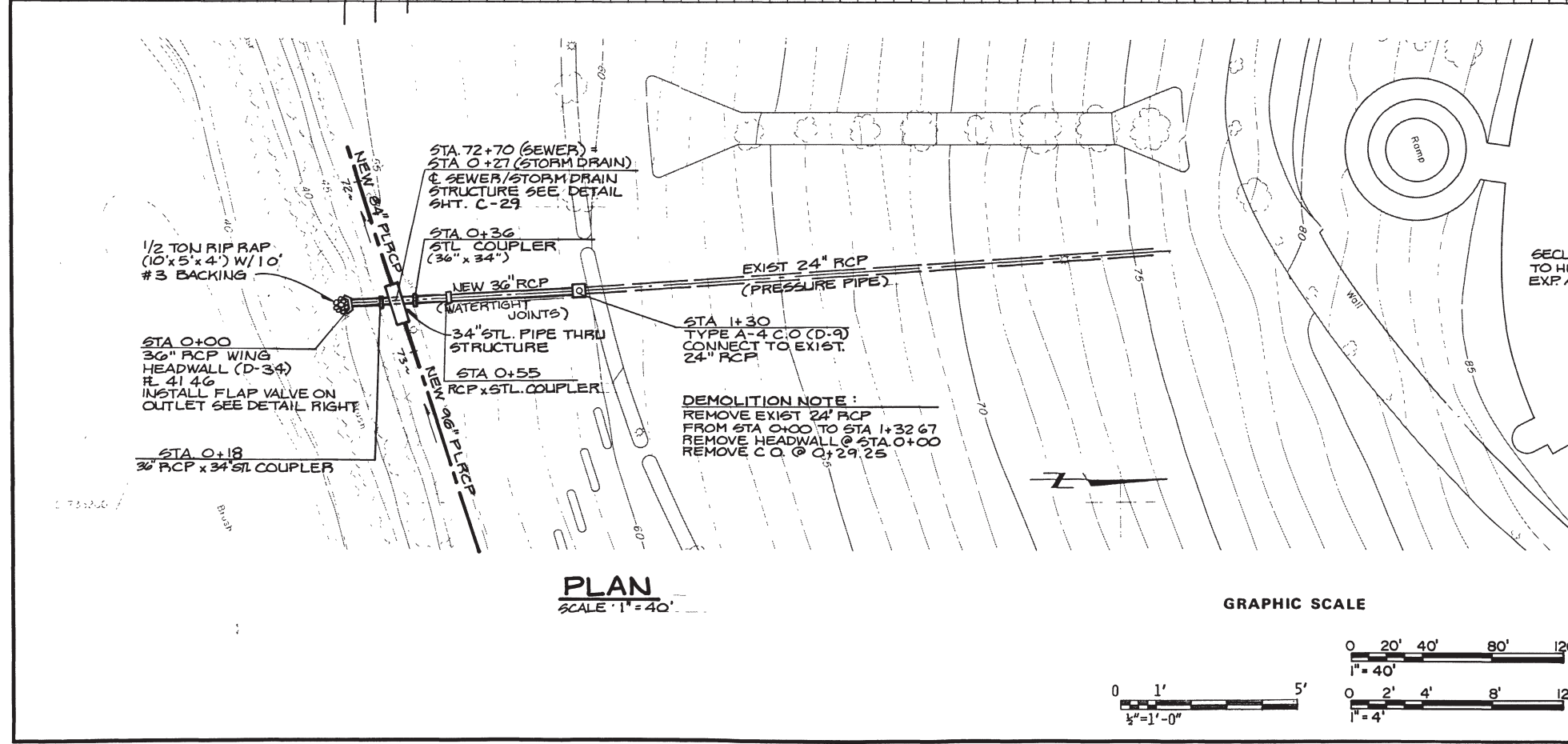
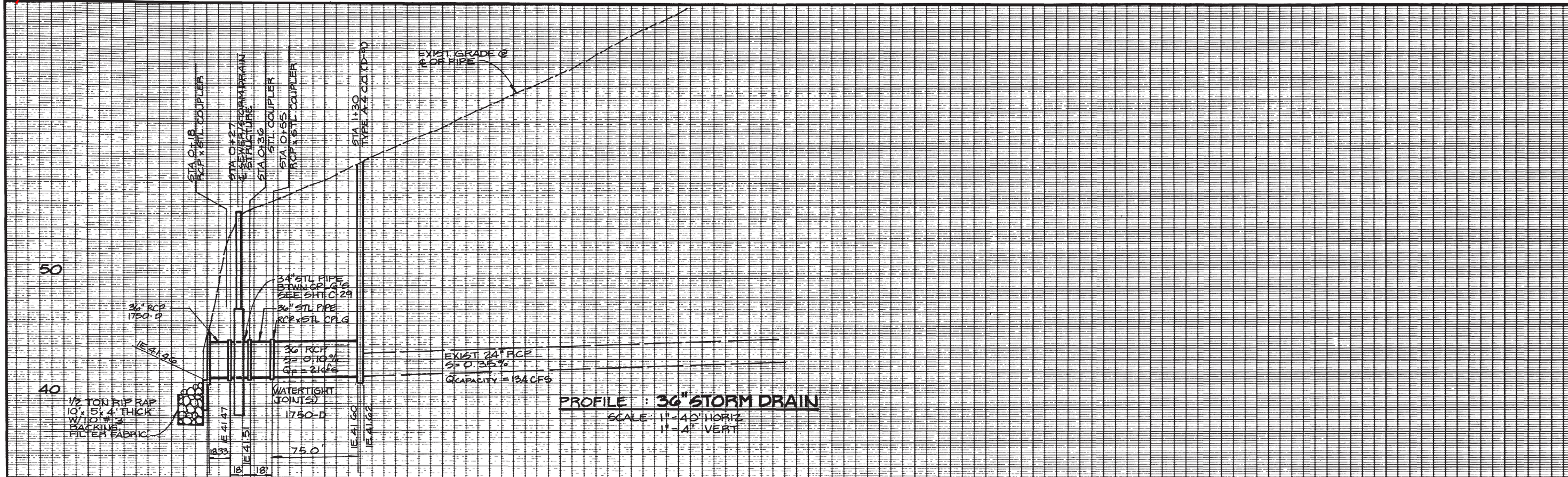
**Kennedy/Jenks/Chilton**  
17310 Red Hill Avenue, Suite 220  
Irvine, California 92714  
714-261-1577

**HIRSCH & COMPANY**  
CONSULTING ENGINEERS  
4420 Ramer Ave Suite 100  
San Diego, California 92120

1/17/90



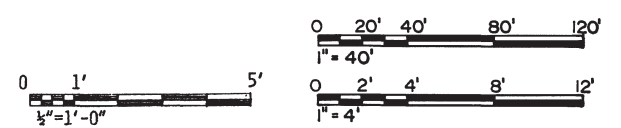




**1 FLAP VALVE DETAIL**  
SCALE: 1/2" = 1'-0"



IF SHEET IS LESS THAN 24X36 IT IS A REDUCED PRINT - SCALE REDUCED ACCORDINGLY



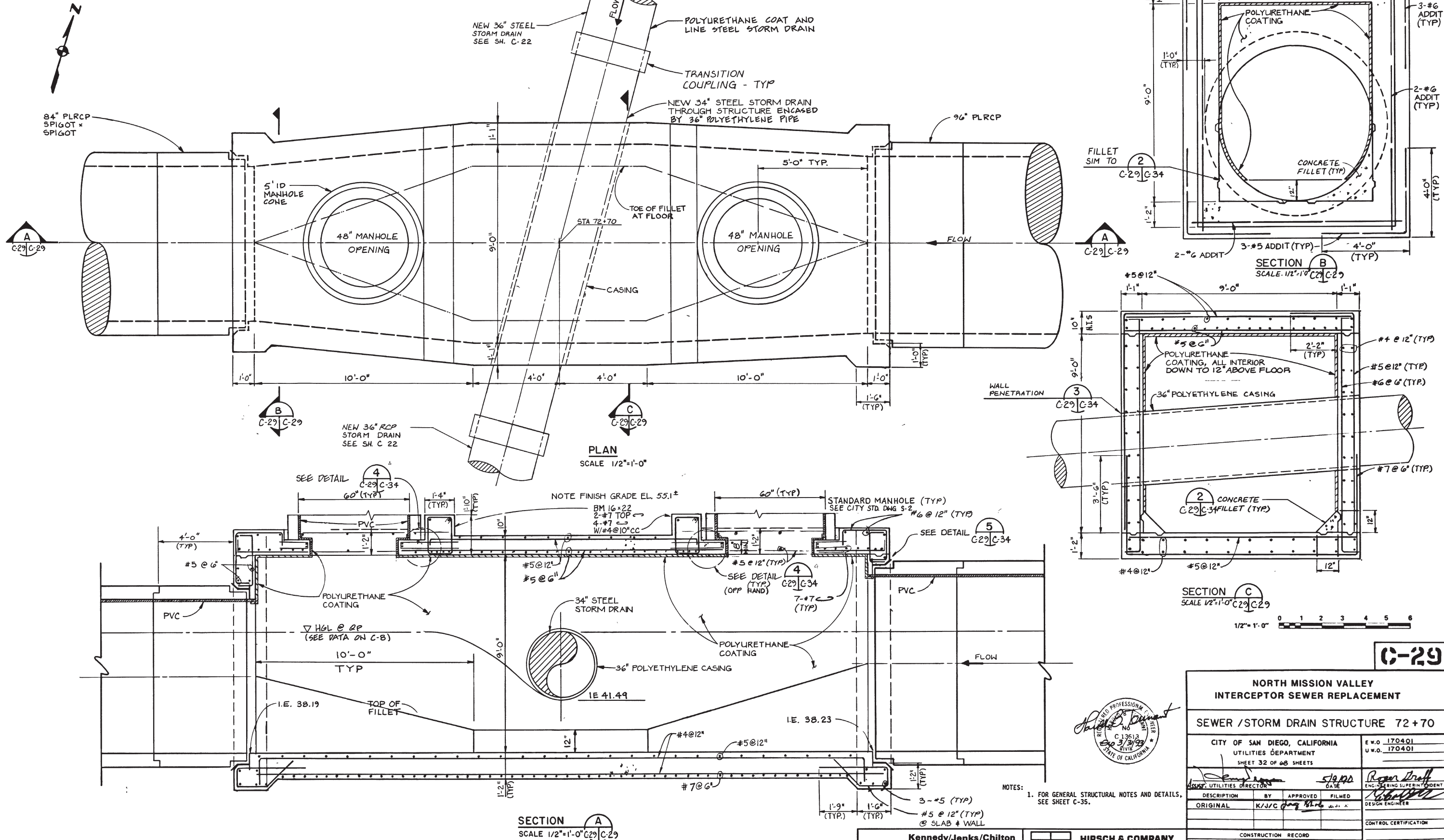
**HIRSCH & COMPANY**  
CONSULTING ENGINEERS  
4420 Ramer Ave. Suite 100  
San Diego, California 92120

<b>NORTH MISSION VALLEY INTERCEPTOR SEWER REPLACEMENT</b>			
<b>RECONSTRUCT STADIUM STORM DRAIN - 2</b>			
CITY OF SAN DIEGO, CALIFORNIA UTILITIES DEPARTMENT SHEET 25 OF 68 SHEETS	E.W.O. 170401 U.W.O. 170401		
UTILITIES DIRECTOR	DATE 5/19/10	DESIGN ENGINEER	REGISTERED PROFESSIONAL ENGINEER
DESCRIPTION ORIGINAL	BY HIRSCH	APPROVED	FILMED
CONSTRUCTION RECORD		CONTROL CERTIFICATION	
CONTRACTOR:		224-1733	
INSPECTOR:		LANSBERT COORDINATES	
CONNECTIONS BY:		25499-25 -D	





# System B



NOTES:  
1. FOR GENERAL STRUCTURAL NOTES AND DETAILS, SEE SHEET C-35.



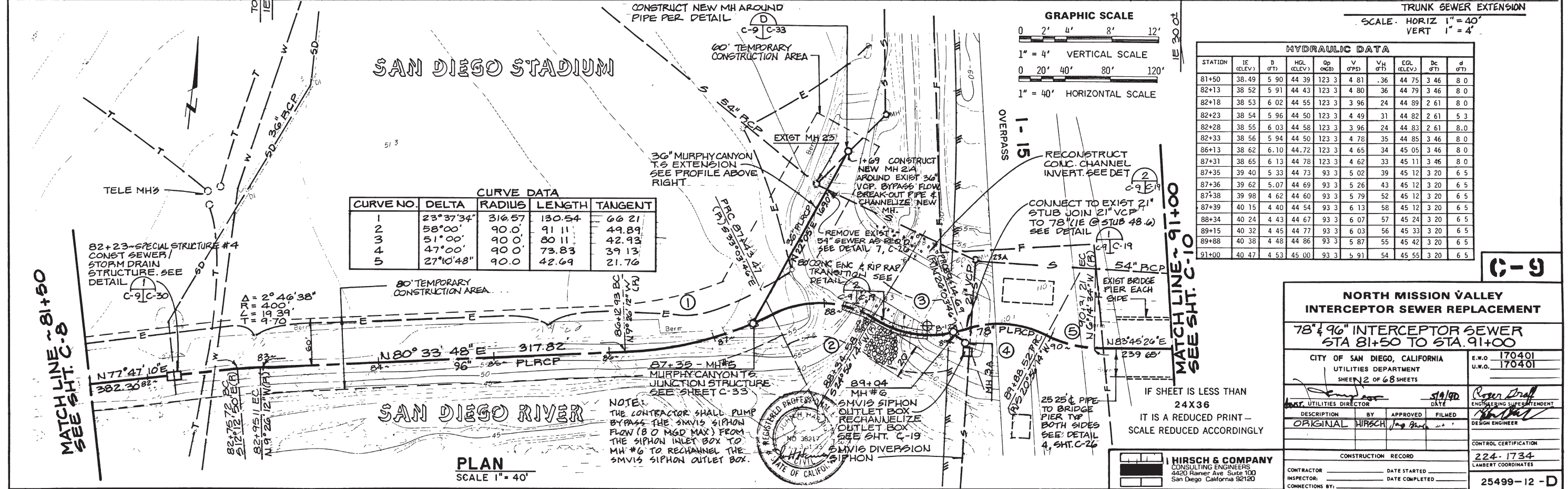
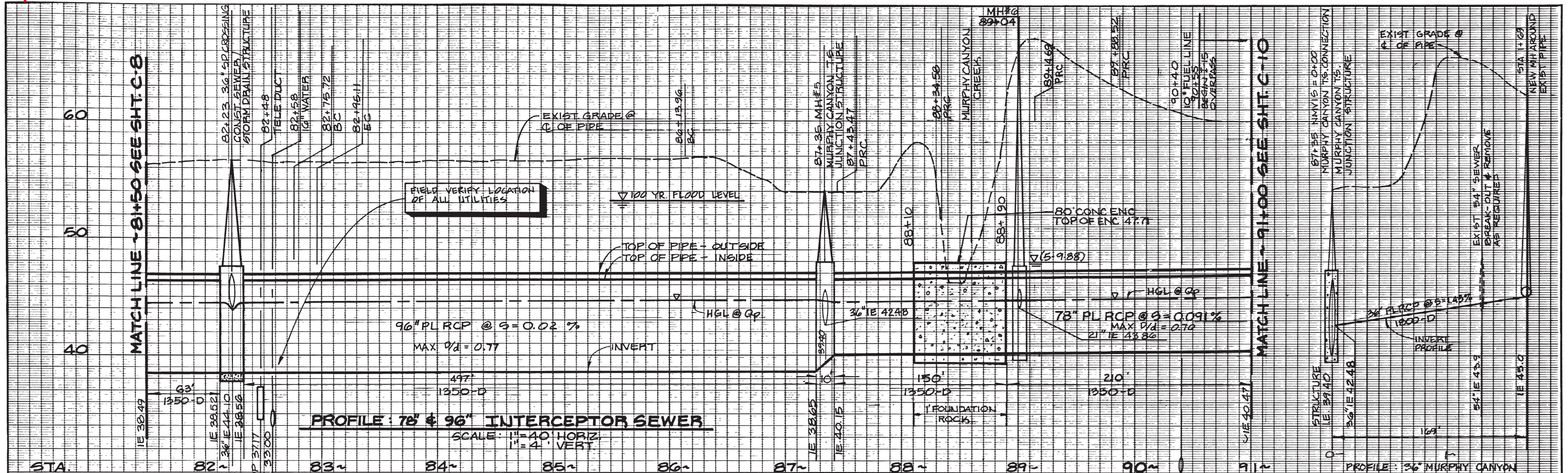
**Kennedy/Jenks/Chilton**  
17310 Red Hill Avenue, Suite 220  
Irvine, California 92714  
714-261-1577

**HIRSCH & COMPANY**  
CONSULTING ENGINEERS  
4420 Rainier Ave Suite 100  
San Diego, California 92120

<b>NORTH MISSION VALLEY INTERCEPTOR SEWER REPLACEMENT</b>			
<b>SEWER /STORM DRAIN STRUCTURE 72+70</b>			
CITY OF SAN DIEGO, CALIFORNIA UTILITIES DEPARTMENT SHEET 32 OF 68 SHEETS		E.W.O. 170401 U.W.O. 170401	
UTILITIES DIRECTOR	DATE	DESIGNER	ENGINEERING SUPERINTENDENT
DESCRIPTION	BY	APPROVED	FILMED
ORIGINAL	K/J/C	JMB	
CONSTRUCTION RECORD		CONTROL CERTIFICATION	
CONTRACTOR	DATE STARTED	LAMBERT COORDINATES	
INSPECTOR	DATE COMPLETED	25499-32-D	
CONNECTIONS BY:			







**HYDRAULIC DATA**

STATION	IE (ELEV)	D (FT)	HGL (ELEV)	Op (CFS)	V (FPS)	Vh (FPS)	EGL (ELEV)	Dc (FT)	d (FT)
81+50	38.49	5.90	44.39	123.3	4.81	.36	44.75	3.46	8.0
82+13	38.52	5.91	44.43	123.3	4.80	.36	44.79	3.46	8.0
82+18	38.53	6.02	44.55	123.3	3.96	.24	44.89	2.61	8.0
82+23	38.54	5.96	44.50	123.3	4.49	.31	44.82	2.61	5.3
82+28	38.55	6.03	44.58	123.3	3.96	.24	44.83	2.61	8.0
82+33	38.56	5.94	44.50	123.3	4.78	.35	44.85	3.46	8.0
86+13	38.62	6.10	44.72	123.3	4.65	.34	45.05	3.46	8.0
87+31	38.65	6.13	44.78	123.3	4.62	.33	45.11	3.46	8.0
87+35	39.40	5.33	44.73	93.3	5.02	.39	45.12	3.20	6.5
87+36	39.62	5.07	44.69	93.3	5.26	.43	45.12	3.20	6.5
87+38	39.98	4.62	44.60	93.3	5.79	.52	45.12	3.20	6.5
87+39	40.15	4.40	44.54	93.3	6.13	.58	45.12	3.20	6.5
88+34	40.24	4.43	44.67	93.3	6.07	.57	45.24	3.20	6.5
89+15	40.32	4.45	44.77	93.3	6.03	.56	45.33	3.20	6.5
89+88	40.38	4.48	44.86	93.3	5.87	.55	45.42	3.20	6.5
91+00	40.47	4.53	45.00	93.3	5.91	.54	45.55	3.20	6.5

**C-9**

**NORTH MISSION VALLEY INTERCEPTOR SEWER REPLACEMENT**  
**78" & 96" INTERCEPTOR SEWER**  
**STA 81+50 TO STA. 91+00**

CITY OF SAN DIEGO, CALIFORNIA  
 UTILITIES DEPARTMENT  
 SHEET 2 OF 68 SHEETS

E.W.O. 170401  
 U.W.O. 170401

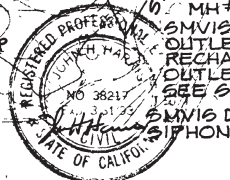
DATE: 5/19/90  
 UTILITIES DIRECTOR: [Signature]  
 ENGINEERING SUPERINTENDENT: [Signature]

DESCRIPTION: ORIGINAL  
 BY: HIRSCH, JAG  
 APPROVED: [Signature]  
 FILMED: [Signature]  
 DESIGN ENGINEER: [Signature]

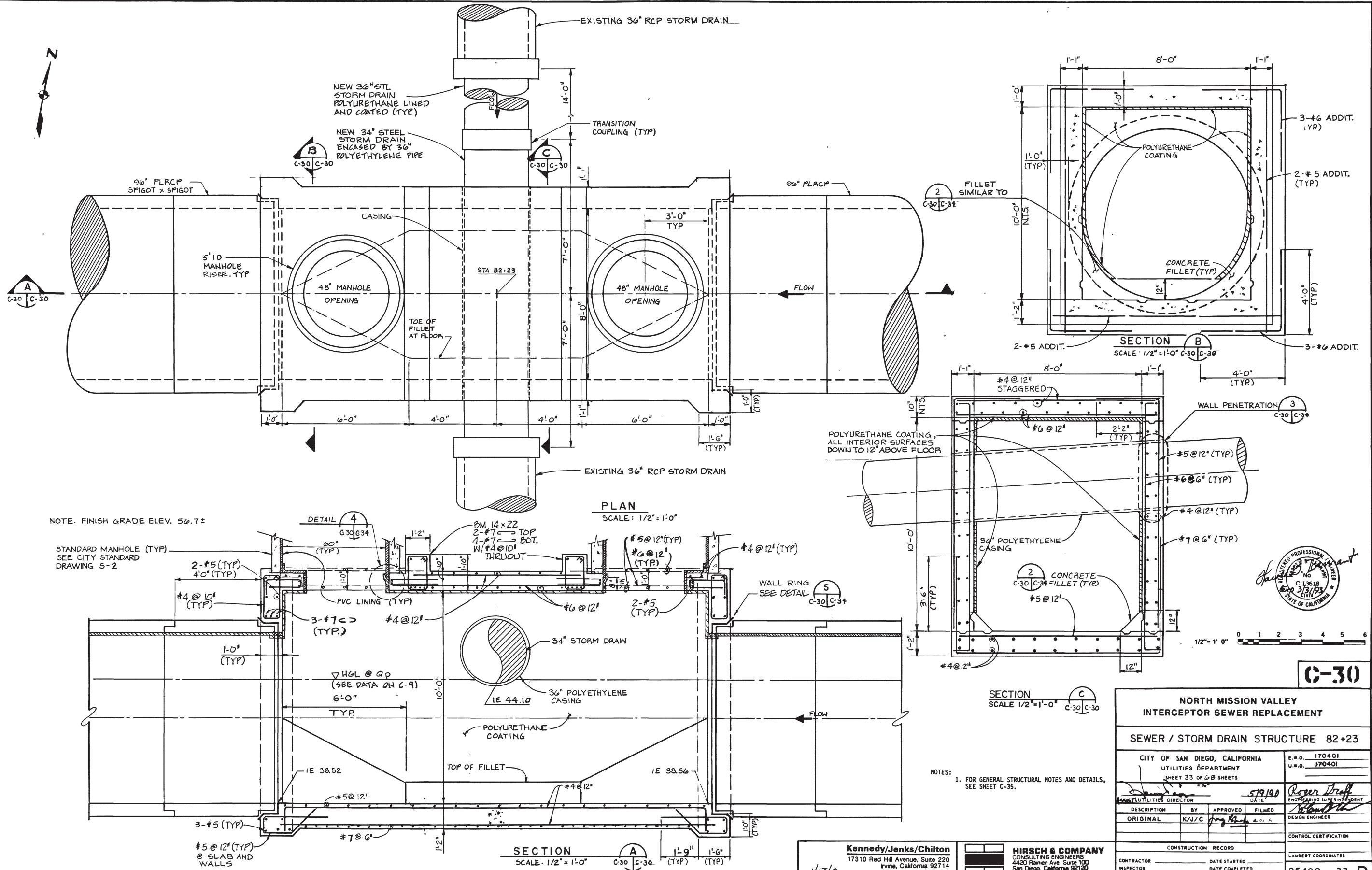
CONTROL CERTIFICATION  
 CONSTRUCTION RECORD  
 224-1734  
 LAMBERT COORDINATES

CONTRACTOR: \_\_\_\_\_ DATE STARTED: \_\_\_\_\_  
 INSPECTOR: \_\_\_\_\_ DATE COMPLETED: \_\_\_\_\_  
 CONNECTIONS BY: \_\_\_\_\_

25499-12-D







NOTE: FINISH GRADE ELEV. 56.7±

STANDARD MANHOLE (TYP)  
SEE CITY STANDARD  
DRAWING S-2

NOTES:  
1. FOR GENERAL STRUCTURAL NOTES AND DETAILS,  
SEE SHEET C-35.



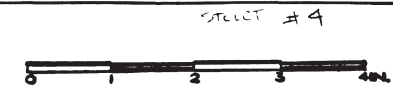
**C-30**

<b>NORTH MISSION VALLEY INTERCEPTOR SEWER REPLACEMENT</b>	
<b>SEWER / STORM DRAIN STRUCTURE 82+23</b>	
CITY OF SAN DIEGO, CALIFORNIA UTILITIES DEPARTMENT SHEET 33 OF 68 SHEETS	E.W.O. 170401 U.W.O. 170401
UTILITIES DIRECTOR	DATE 5/19/90
DESCRIPTION BY APPROVED FILMED	DESIGN ENGINEER
ORIGINAL K/J/C	CONTROL CERTIFICATION
CONSTRUCTION RECORD	LAMBERT COORDINATES
CONTRACTOR DATE STARTED	INSPECTOR DATE COMPLETED
CONNECTIONS BY:	<b>25499 - 33 - D</b>

**Kennedy/Jenks/Chilton**  
17310 Red Hill Avenue, Suite 220  
Irvine, California 92714  
714-261-1577

**HIRSCH & COMPANY**  
CONSULTING ENGINEERS  
4420 Ramier Ave Suite 100  
San Diego, California 92120

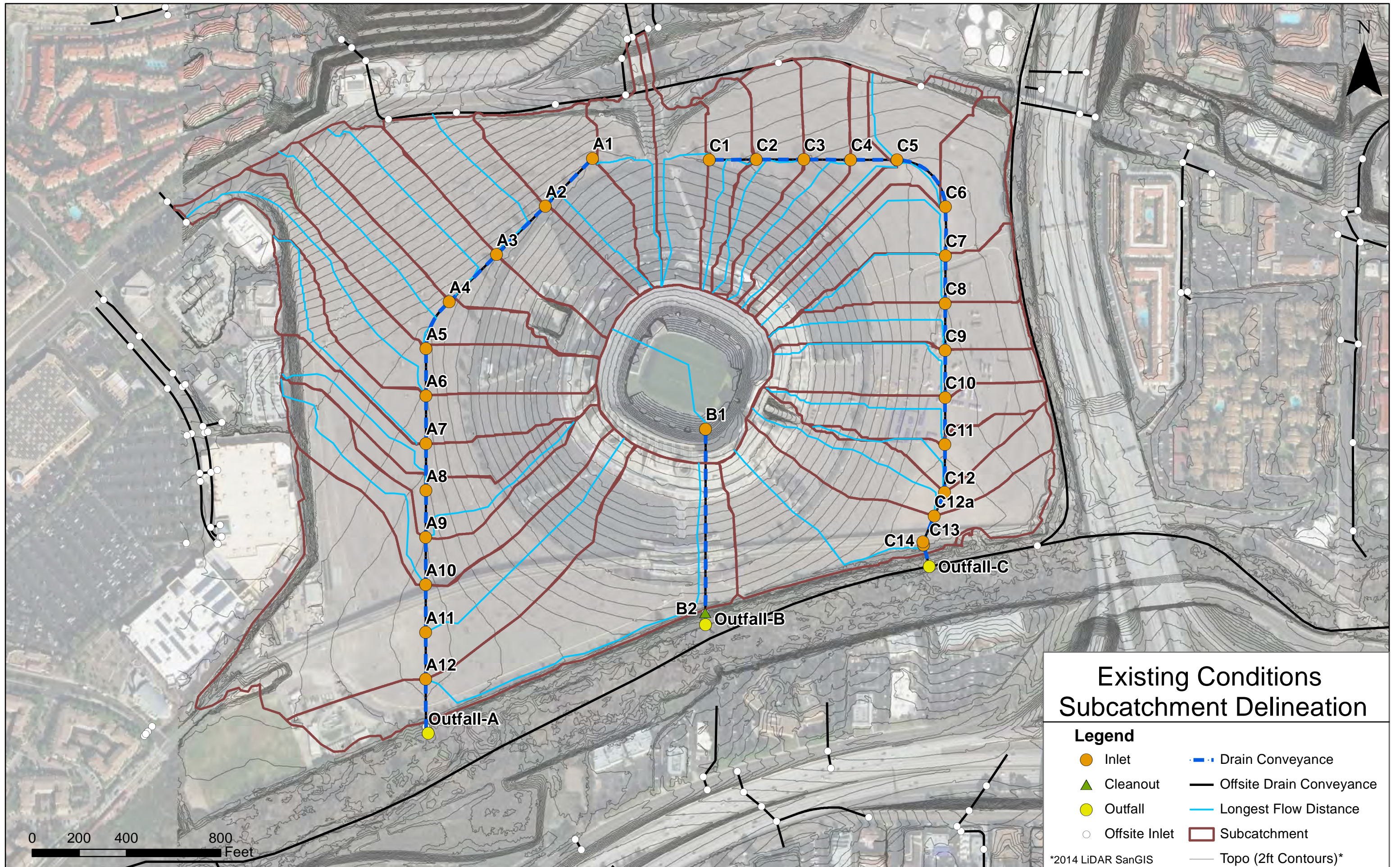
1/17/90



## **APPENDIX A.2**

### Existing Conditions: Drainage Area Exhibit





### Existing Conditions Subcatchment Delineation

- Legend**
- Inlet
  - ▲ Cleanout
  - Outfall
  - Offsite Inlet
  - · — Drain Conveyance
  - Offsite Drain Conveyance
  - Longest Flow Distance
  - Subcatchment
  - Topo (2ft Contours)\*

\*2014 LiDAR SanGIS



## **APPENDIX A.3**

### Existing Conditions: Runoff Coefficients



## APPENDIX A: RATIONAL METHOD AND MODIFIED RATIONAL METHOD

Combining the units for the expression CIA yields:

$$\left( \frac{1 \text{ acre} \times \text{inch}}{\text{hour}} \right) \left( \frac{43,560 \text{ ft}^2}{\text{acre}} \right) \left( \frac{1 \text{ foot}}{12 \text{ inches}} \right) \left( \frac{1 \text{ hour}}{3,600 \text{ seconds}} \right) \Rightarrow 1.008 \text{ cfs}$$

For practical purposes, the unit conversion coefficient difference of 0.8% can be ignored.

The RM formula is based on the assumption that for constant rainfall intensity, the peak discharge rate at a point will occur when the raindrop that falls at the most upstream point in the tributary drainage basin arrives at the point of interest.

Unlike the MRM (discussed in Appendix A.2) or the NRCS hydrologic method (discussed in Appendix B), the RM does not create hydrographs and therefore does not add separate subarea hydrographs at collection points. Instead, the RM develops peak discharges in the main line by increasing the  $T_c$  as flow travels downstream.

Characteristics of, or assumptions inherent to, the RM are listed below:

1. The discharge resulting from any I is maximum when the I lasts as long as or longer than the  $T_c$ .
2. The storm frequency of peak discharges is the same as that of I for the given  $T_c$ .
3. The fraction of rainfall that becomes runoff (or the runoff coefficient, C) is independent of I or precipitation zone number (PZN) condition (PZN Condition is discussed in the NRCS method).
4. The peak rate of runoff is the only information produced by using the RM.

### A.1.2. Runoff Coefficient

The runoff coefficients are based on land use (see Table A-1). Soil type "D" is used throughout the City of San Diego for storm drain conveyance design. An appropriate runoff coefficient (C) for each type of land use in the subarea should be selected from this table and multiplied by the percentage of the total area (A) included in that class. The sum of the products for all land uses is the weighted runoff coefficient ( $\Sigma[CA]$ ). Good engineering judgment should be used when applying the values presented in Table A-1, as adjustments to these values may be appropriate based on site-specific characteristics.

## APPENDIX A: RATIONAL METHOD AND MODIFIED RATIONAL METHOD

**Table A-1. Runoff Coefficients for Rational Method**

Land Use	Runoff Coefficient (C)
	Soil Type <sup>(1)</sup>
<b>Residential:</b>	
Single Family	0.55
Multi-Units	0.70
Mobile Homes	0.65
Rural (lots greater than 1/2 acre)	0.45
<b>Commercial <sup>(2)</sup></b>	
80% Impervious	0.85
<b>Industrial <sup>(2)</sup></b>	
<b>90% Impervious</b>	<b>0.95</b>

**Note:**

<sup>(1)</sup> Type D soil to be used for all areas.

<sup>(2)</sup> Where actual conditions deviate significantly from the tabulated imperviousness values of 80% or 90%, the values given for coefficient C, may be revised by multiplying 80% or 90% by the ratio of actual imperviousness to the tabulated imperviousness. However, in case shall the final coefficient be less than 0.50. For example: Consider commercial property on D soil.

$$\begin{array}{lcl}
 \text{Actual imperviousness} & = & 50\% \\
 \text{Tabulated imperviousness} & = & 80\% \\
 \text{Revised C} & = & (50/80) \times 0.85 = 0.53
 \end{array}$$

The values in Table A-1 are typical for urban areas. However, if the basin contains rural or agricultural land use, parks, golf courses, or other types of nonurban land use that are expected to be permanent, the appropriate value should be selected based upon the soil and cover and approved by the City.

### A.1.3. Rainfall Intensity

The rainfall intensity (I) is the rainfall in inches per hour (in/hr.) for a duration equal to the  $T_c$  for a selected storm frequency. Once a particular storm frequency has been selected for design and a  $T_c$  calculated for the drainage area, the rainfall intensity can be determined from the Intensity-Duration-Frequency Design Chart (Figure A-1).



# **APPENDIX A.4**

## **Existing Conditions: Time of Concentration**

# Existing System A Time of Concentration

The Time of Concentration calculations were performed using the following equation:

$$T = \frac{1.8 * (1.1 - C) * \sqrt{D}}{\sqrt[3]{S}}$$

Data in the table below was taken from the as-builts in Appendix A.3. Inlet IDs coincide with the labels on the Existing Conditions Figure in Appendix A.2-1.

Existing System A Time of Concentration				
INLET ID	C	LFP		T (min)
		Length	Slope (%)	
A1	0.95	792.63	3.02	5
A2	0.95	659.07	4.02	5
A3	0.95	810.57	3.93	5
A4	0.95	1102.57	4.07	6
A5	0.95	1133.47	4.54	5
A6	0.95	1485.84	4.18	6
A7	0.95	1175.67	5.07	5
A8	0.95	892.87	4.06	5
A9	0.95	866.51	3.88	5
A10	0.95	1214.76	3.59	6
A11	0.95	1210.02	3.18	6
A12	0.95	1421.00	2.67	7

# Existing System B Time of Concentration

The Time of Concentration calculations were performed using the following equation:

$$T = \frac{1.8 * (1.1 - C) * \sqrt{D}}{\sqrt[3]{S}}$$

Data in the table below was taken from the as-builts in Appendix A.3. Inlet IDs coincide with the labels on the Existing Conditions Figure in Appendix A.2-1.

Existing System B Time of Concentration				
INLET ID	C	LFP		T (min)
		Length	Slope (%)	
B1 to B2	0.95	590.51	12.08	5

# Existing System C Time of Concentration

The Time of Concentration calculations were performed using the following equation:

$$T = \frac{1.8 * (1.1 - C) * \sqrt{D}}{\sqrt[3]{S}}$$

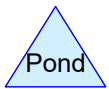
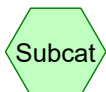
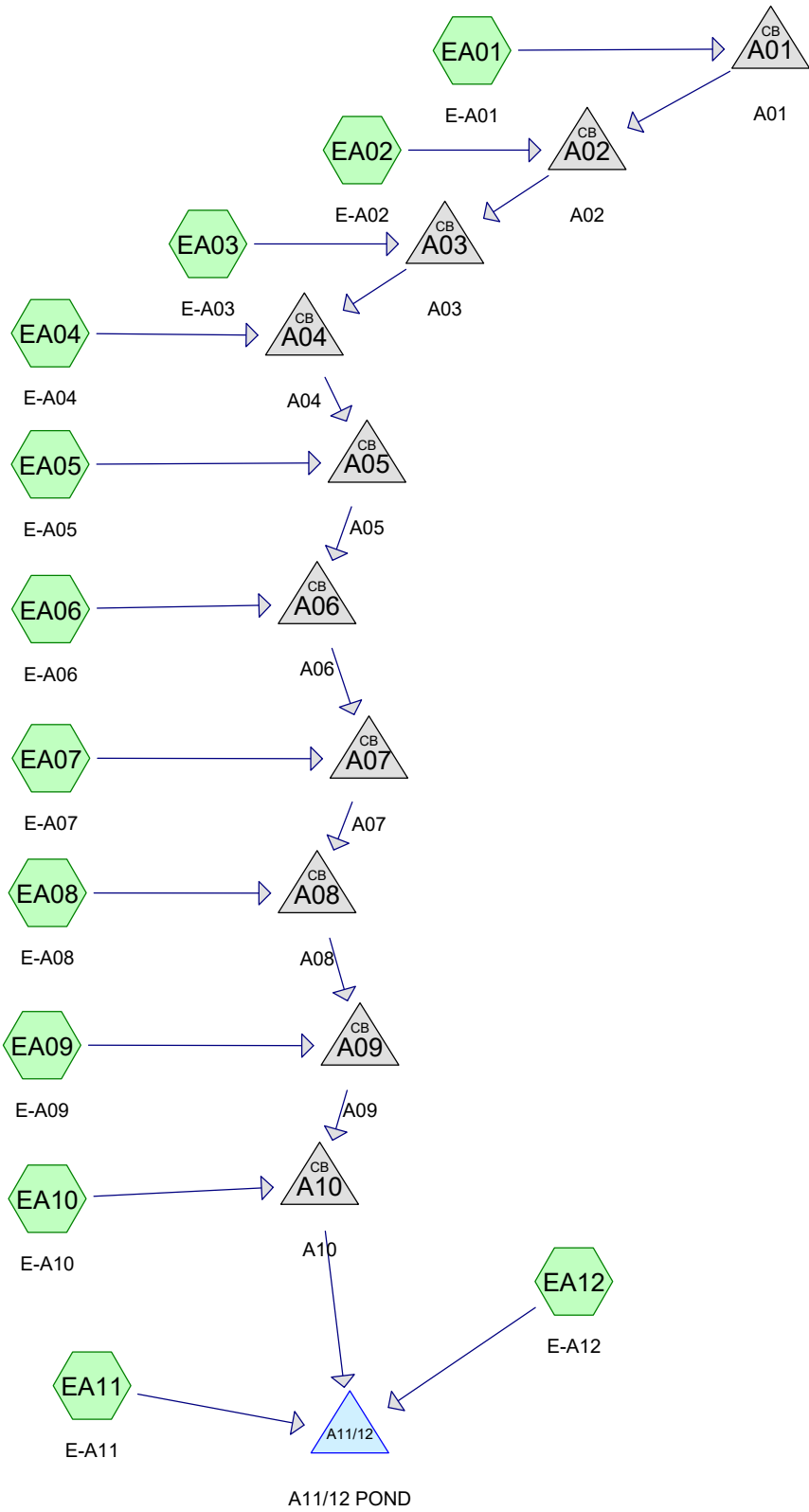
Data in the table below was taken from the as-builts in Appendix A.3. Inlet IDs coincide with the labels on the Existing Conditions Figure in Appendix A.2-1.

Existing System C Time of Concentration				
INLET ID	C	LFP		Tc (min)
		Length	Slope (%)	
C1	0.95	745.33	3.11	5
C2	0.95	703.66	3.54	5
C3	0.95	793.04	3.35	5
C4	0.95	838.76	3.26	5
C5	0.95	922.69	3.04	6
C6	0.95	706.53	1.43	6
C7	0.95	1176.20	2.53	7
C8	0.95	887.74	2.49	6
C9	0.95	895.73	3.49	5
C10	0.95	927.16	3.44	5
C11	0.95	934.95	3.64	5
C12	0.95	733.44	3.12	5
C12a	0.95	976.92	4.13	5
C13	0.95	1295.96	2.64	7
C14	0.95	29.93	6.62	5

# **APPENDIX A.5**

## Existing Conditions: HydroCAD Reports





**Routing Diagram for Existing System A**  
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## Existing System A

Prepared by SCCM

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### Area Listing (all nodes)

Area (acres)	C	Description (subcatchment-numbers)
94.950	0.95	Paved parking, HSG D (EA01, EA02, EA03, EA04, EA05, EA06, EA07, EA08, EA09, EA10, EA11, EA12)
<b>94.950</b>	<b>0.95</b>	<b>TOTAL AREA</b>

## Existing System A

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### Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	A01	59.12	56.16	282.8	0.0105	0.013	18.0	0.0	0.0
2	A02	56.16	54.30	285.8	0.0065	0.013	18.0	0.0	0.0
3	A03	54.30	51.21	279.8	0.0110	0.013	24.0	0.0	0.0
4	A04	51.21	49.52	231.1	0.0073	0.013	24.0	0.0	0.0
5	A05	49.52	48.03	200.0	0.0075	0.013	30.0	0.0	0.0
6	A06	48.03	47.71	200.0	0.0016	0.013	30.0	0.0	0.0
7	A07	47.71	43.96	200.0	0.0187	0.013	30.0	0.0	0.0
8	A08	42.86	42.56	200.0	0.0015	0.013	4.0	2.0	0.0
9	A09	42.55	42.25	200.0	0.0015	0.013	4.0	2.0	0.0
10	A10	42.24	41.94	200.0	0.0015	0.013	4.0	2.0	0.0
11	A11/12	41.93	38.20	376.0	0.0099	0.013	36.0	0.0	0.0

50-year Rainfall Intensity

ELEV.	FACTOR
0-1500	1.00
1500-3000	1.25
3000-4000	1.42
4000-5000	1.60
5000-6000	1.7
DESERT	1.25

TO OBTAIN CORRECT INTENSITY, MULTIPLY INTENSITY ON CHART BY FACTOR FOR DESIGN ELEVATION.

Figure A-1. Intensity-Duration-Frequency Design Chart

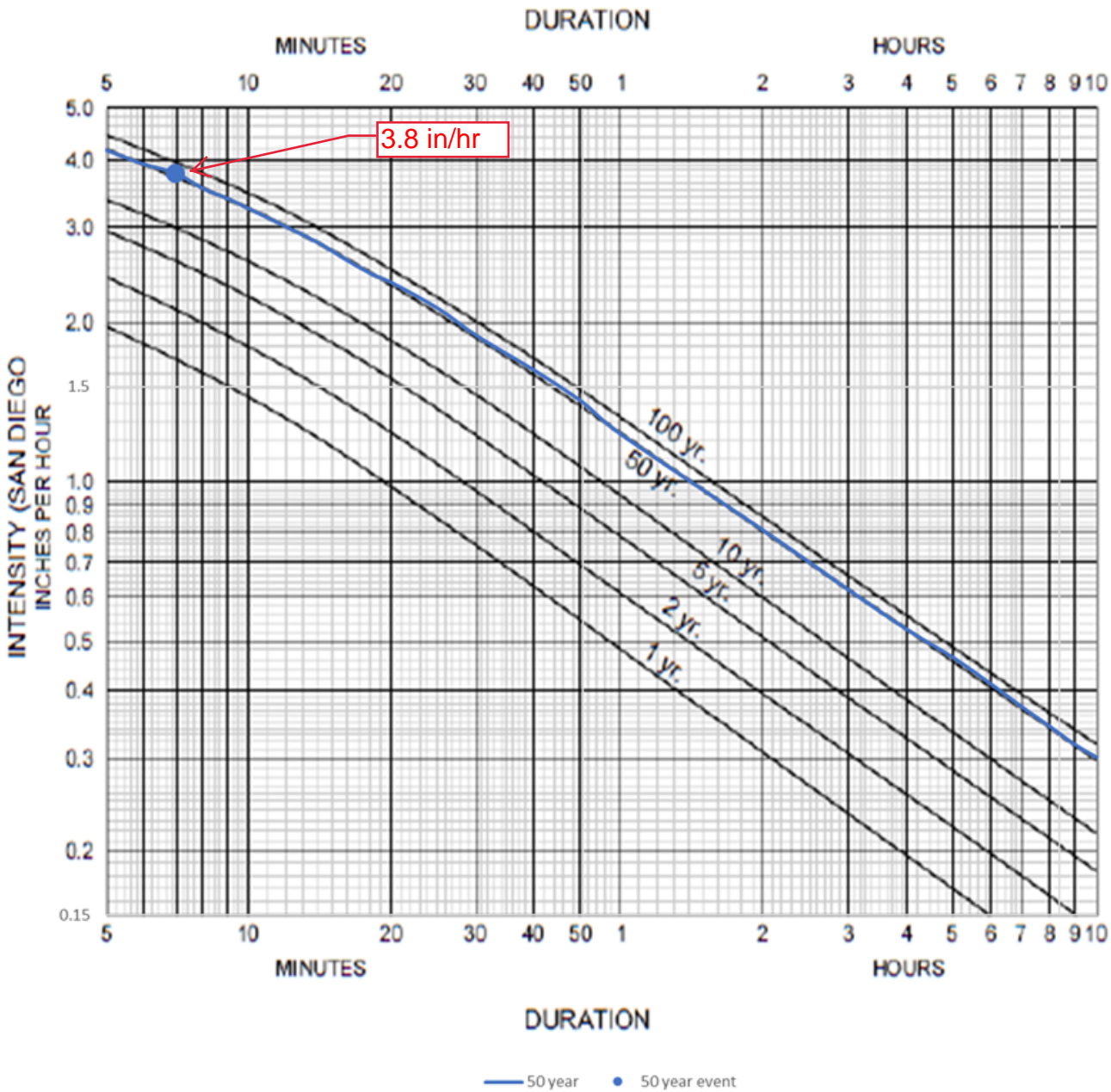
COUNTY OF SAN DIEGO

FOR

CURVES

INTENSITY - DURATION - FREQUENCY

RAINFALL



**Existing System A**

City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

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Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points x 3

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Pond A01: A01</b>	Peak Elev=63.45' Inflow=10.43 cfs 0.144 af Primary=9.18 cfs 0.138 af Secondary=1.25 cfs 0.006 af Outflow=10.43 cfs 0.144 af
<b>Pond A02: A02</b>	Peak Elev=60.66' Inflow=27.65 cfs 0.381 af Primary=8.44 cfs 0.160 af Secondary=20.20 cfs 0.220 af Outflow=27.65 cfs 0.381 af
<b>Pond A03: A03</b>	Peak Elev=58.81' Inflow=51.84 cfs 0.714 af Primary=17.36 cfs 0.353 af Secondary=34.96 cfs 0.361 af Outflow=51.84 cfs 0.714 af
<b>Pond A04: A04</b>	Peak Elev=56.58' Inflow=77.07 cfs 1.058 af Primary=17.99 cfs 0.353 af Secondary=62.08 cfs 0.705 af Outflow=77.07 cfs 1.058 af
<b>Pond A05: A05</b>	Peak Elev=55.02' Inflow=96.86 cfs 1.331 af Primary=24.95 cfs 0.532 af Secondary=74.31 cfs 0.799 af Outflow=96.86 cfs 1.331 af
<b>Pond A06: A06</b>	Peak Elev=53.93' Inflow=120.95 cfs 1.679 af Primary=26.21 cfs 0.477 af Secondary=101.31 cfs 1.206 af Outflow=120.95 cfs 1.679 af
<b>Pond A07: A07</b>	Peak Elev=53.10' Inflow=138.97 cfs 1.931 af Primary=39.35 cfs 0.882 af Secondary=100.36 cfs 1.050 af Outflow=138.97 cfs 1.931 af
<b>Pond A08: A08</b>	Peak Elev=49.88' Inflow=151.33 cfs 2.102 af Primary=26.51 cfs 0.552 af Secondary=127.69 cfs 1.549 af Outflow=151.33 cfs 2.102 af
<b>Pond A09: A09</b>	Peak Elev=48.72' Inflow=160.77 cfs 2.231 af Primary=32.37 cfs 0.600 af Secondary=130.34 cfs 1.632 af Outflow=160.77 cfs 2.231 af
<b>Pond A10: A10</b>	Peak Elev=47.90' Inflow=182.00 cfs 2.524 af Primary=30.81 cfs 0.519 af Secondary=152.43 cfs 2.005 af Outflow=182.00 cfs 2.524 af
<b>Pond A11/12: A11/12 POND</b>	Peak Elev=47.74' Storage=117,702 cf Inflow=290.86 cfs 4.046 af 34.0" Round Culvert n=0.013 L=376.0' S=0.0099 '/' Outflow=62.12 cfs 4.050 af
<b>Subcatchment EA01: E-A01</b>	Runoff Area=3.350 ac 100.00% Impervious Runoff Depth=0.51" Tc=7.6 min C=0.95 Runoff=10.43 cfs 0.144 af
<b>Subcatchment EA02: E-A02</b>	Runoff Area=5.530 ac 100.00% Impervious Runoff Depth=0.51" Tc=6.9 min C=0.95 Runoff=17.22 cfs 0.237 af
<b>Subcatchment EA03: E-A03</b>	Runoff Area=7.770 ac 100.00% Impervious Runoff Depth=0.51" Tc=7.7 min C=0.95 Runoff=24.19 cfs 0.333 af
<b>Subcatchment EA04: E-A04</b>	Runoff Area=8.040 ac 100.00% Impervious Runoff Depth=0.51" Tc=9.0 min C=0.95 Runoff=25.24 cfs 0.345 af
<b>Subcatchment EA05: E-A05</b>	Runoff Area=6.350 ac 100.00% Impervious Runoff Depth=0.51" Tc=9.1 min C=0.95 Runoff=19.78 cfs 0.272 af
<b>Subcatchment EA06: E-A06</b>	Runoff Area=8.460 ac 100.00% Impervious Runoff Depth=0.49" Tc=10.4 min C=0.95 Runoff=24.88 cfs 0.349 af

**Existing System A***City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr*

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<b>Subcatchment EA07: E-A07</b>	Runoff Area=5.790 ac 100.00% Impervious Runoff Depth=0.51" Tc=9.3 min C=0.95 Runoff=18.03 cfs 0.248 af
<b>Subcatchment EA08: E-A08</b>	Runoff Area=3.970 ac 100.00% Impervious Runoff Depth=0.51" Tc=8.1 min C=0.95 Runoff=12.36 cfs 0.170 af
<b>Subcatchment EA09: E-A09</b>	Runoff Area=3.030 ac 100.00% Impervious Runoff Depth=0.51" Tc=7.9 min C=0.95 Runoff=9.43 cfs 0.130 af
<b>Subcatchment EA10: E-A10</b>	Runoff Area=6.820 ac 100.00% Impervious Runoff Depth=0.51" Tc=9.4 min C=0.95 Runoff=21.25 cfs 0.292 af
<b>Subcatchment EA11: E-A11</b>	Runoff Area=19.520 ac 100.00% Impervious Runoff Depth=0.51" Tc=9.4 min C=0.95 Runoff=60.82 cfs 0.837 af
<b>Subcatchment EA12: E-A12</b>	Runoff Area=16.320 ac 100.00% Impervious Runoff Depth=0.50" Tc=10.2 min C=0.95 Runoff=48.94 cfs 0.686 af
<b>Pond OA: OUTFALLA</b>	Inflow=62.12 cfs 4.050 af Primary=62.12 cfs 4.050 af

**Total Runoff Area = 94.950 ac Runoff Volume = 4.042 af Average Runoff Depth = 0.51"**  
**0.00% Pervious = 0.000 ac 100.00% Impervious = 94.950 ac**

**Existing System A**

City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

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**Summary for Pond A01: A01**

Inflow Area = 3.350 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 10.43 cfs @ 0.13 hrs, Volume= 0.144 af  
 Outflow = 10.43 cfs @ 0.16 hrs, Volume= 0.144 af, Atten= 0%, Lag= 1.8 min  
 Primary = 9.18 cfs @ 0.17 hrs, Volume= 0.138 af  
 Secondary = 1.25 cfs @ 0.16 hrs, Volume= 0.006 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 63.45' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	59.12'	<b>18.0" Round CB A1</b> L= 282.8' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 59.12' / 56.16' S= 0.0105 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#2	Secondary	63.37'	<b>20.0' long x 282.8' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=9.18 cfs @ 0.17 hrs HW=63.44' TW=60.65' (Dynamic Tailwater)  
 ↑1=**CB A1** (Outlet Controls 9.18 cfs @ 5.19 fps)

**Secondary OutFlow** Max=1.25 cfs @ 0.16 hrs HW=63.45' TW=60.66' (Dynamic Tailwater)  
 ↑2=**Broad-Crested Rectangular Weir** (Weir Controls 1.25 cfs @ 0.77 fps)

**Summary for Pond A02: A02**

Inflow Area = 8.880 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 27.65 cfs @ 0.14 hrs, Volume= 0.381 af  
 Outflow = 27.65 cfs @ 0.14 hrs, Volume= 0.381 af, Atten= 0%, Lag= 0.0 min  
 Primary = 8.44 cfs @ 0.04 hrs, Volume= 0.160 af  
 Secondary = 20.20 cfs @ 0.16 hrs, Volume= 0.220 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 60.66' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.16'	<b>18.0" Round CB A02</b> L= 285.8' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.16' / 54.30' S= 0.0065 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#2	Secondary	60.14'	<b>20.0' long x 285.8' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=8.16 cfs @ 0.04 hrs HW=60.23' TW=58.00' (Dynamic Tailwater)  
 ↑1=**CB A02** (Outlet Controls 8.16 cfs @ 4.62 fps)

**Secondary OutFlow** Max=20.20 cfs @ 0.16 hrs HW=60.66' TW=58.81' (Dynamic Tailwater)  
 ↑2=**Broad-Crested Rectangular Weir** (Weir Controls 20.20 cfs @ 1.95 fps)



**Existing System A**

City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

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**Summary for Pond A03: A03**

Inflow Area = 16.650 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 51.84 cfs @ 0.14 hrs, Volume= 0.714 af  
 Outflow = 51.84 cfs @ 0.15 hrs, Volume= 0.714 af, Atten= 0%, Lag= 0.6 min  
 Primary = 17.36 cfs @ 0.05 hrs, Volume= 0.353 af  
 Secondary = 34.96 cfs @ 0.16 hrs, Volume= 0.361 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 58.81' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	54.30'	<b>24.0" Round CB A03</b> L= 279.8' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 54.30' / 51.21' S= 0.0110 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Secondary	58.05'	<b>20.0' long x 279.8' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=17.35 cfs @ 0.05 hrs HW=58.22' TW=55.85' (Dynamic Tailwater)  
 ↑**1=CB A03** (Outlet Controls 17.35 cfs @ 5.52 fps)

**Secondary OutFlow** Max=34.96 cfs @ 0.16 hrs HW=58.81' TW=56.57' (Dynamic Tailwater)  
 ↑**2=Broad-Crested Rectangular Weir** (Weir Controls 34.96 cfs @ 2.31 fps)

**Summary for Pond A04: A04**

Inflow Area = 24.690 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 77.07 cfs @ 0.15 hrs, Volume= 1.058 af  
 Outflow = 77.07 cfs @ 0.16 hrs, Volume= 1.058 af, Atten= 0%, Lag= 0.0 min  
 Primary = 17.99 cfs @ 0.03 hrs, Volume= 0.353 af  
 Secondary = 62.08 cfs @ 0.16 hrs, Volume= 0.705 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 56.58' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	51.21'	<b>24.0" Round CB A04</b> L= 231.1' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 51.21' / 49.52' S= 0.0073 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Secondary	55.46'	<b>20.0' long x 231.1' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=16.09 cfs @ 0.03 hrs HW=55.08' TW=53.29' (Dynamic Tailwater)  
 ↑**1=CB A04** (Outlet Controls 16.09 cfs @ 5.12 fps)

**Secondary OutFlow** Max=61.83 cfs @ 0.16 hrs HW=56.57' TW=55.02' (Dynamic Tailwater)  
 ↑**2=Broad-Crested Rectangular Weir** (Weir Controls 61.83 cfs @ 2.78 fps)

**Existing System A**

City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

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**Summary for Pond A05: A05**

Inflow Area = 31.040 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 96.86 cfs @ 0.16 hrs, Volume= 1.331 af  
 Outflow = 96.86 cfs @ 0.16 hrs, Volume= 1.331 af, Atten= 0%, Lag= 0.0 min  
 Primary = 24.95 cfs @ 0.05 hrs, Volume= 0.532 af  
 Secondary = 74.31 cfs @ 0.16 hrs, Volume= 0.799 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 55.02' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	49.52'	<b>30.0" Round CB A05</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 49.52' / 48.03' S= 0.0075 '/' Cc= 0.900 n= 0.013, Flow Area= 4.91 sf
#2	Secondary	53.75'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=24.92 cfs @ 0.05 hrs HW=54.07' TW=52.72' (Dynamic Tailwater)  
 ↖1=CB A05 (Outlet Controls 24.92 cfs @ 5.08 fps)

**Secondary OutFlow** Max=73.99 cfs @ 0.16 hrs HW=55.02' TW=53.92' (Dynamic Tailwater)  
 ↖2=Broad-Crested Rectangular Weir (Weir Controls 73.99 cfs @ 2.92 fps)

**Summary for Pond A06: A06**

Inflow Area = 39.500 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 120.95 cfs @ 0.16 hrs, Volume= 1.679 af  
 Outflow = 120.95 cfs @ 0.16 hrs, Volume= 1.679 af, Atten= 0%, Lag= 0.0 min  
 Primary = 26.21 cfs @ 0.03 hrs, Volume= 0.477 af  
 Secondary = 101.31 cfs @ 0.16 hrs, Volume= 1.206 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 53.93' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	48.03'	<b>30.0" Round CB A06</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 48.03' / 47.71' S= 0.0016 '/' Cc= 0.900 n= 0.013, Flow Area= 4.91 sf
#2	Secondary	52.18'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=25.48 cfs @ 0.03 hrs HW=52.07' TW=50.67' (Dynamic Tailwater)  
 ↖1=CB A06 (Outlet Controls 25.48 cfs @ 5.19 fps)

**Secondary OutFlow** Max=101.27 cfs @ 0.16 hrs HW=53.93' TW=53.10' (Dynamic Tailwater)  
 ↖2=Broad-Crested Rectangular Weir (Weir Controls 101.27 cfs @ 2.89 fps)

**Existing System A**

City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

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**Summary for Pond A07: A07**

Inflow Area = 45.290 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 138.97 cfs @ 0.16 hrs, Volume= 1.931 af  
 Outflow = 138.97 cfs @ 0.16 hrs, Volume= 1.931 af, Atten= 0%, Lag= 0.0 min  
 Primary = 39.35 cfs @ 0.09 hrs, Volume= 0.882 af  
 Secondary = 100.36 cfs @ 0.16 hrs, Volume= 1.050 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 53.10' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	47.71'	<b>30.0" Round CB A07</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 47.71' / 43.96' S= 0.0187 '/' Cc= 0.900 n= 0.013, Flow Area= 4.91 sf
#2	Secondary	51.56'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=39.33 cfs @ 0.09 hrs HW=52.57' TW=49.23' (Dynamic Tailwater)  
 ↑1=**CB A07** (Outlet Controls 39.33 cfs @ 8.01 fps)

**Secondary OutFlow** Max=100.31 cfs @ 0.16 hrs HW=53.10' TW=49.88' (Dynamic Tailwater)  
 ↑2=**Broad-Crested Rectangular Weir** (Weir Controls 100.31 cfs @ 3.26 fps)

**Summary for Pond A08: A08**

Inflow Area = 49.260 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 151.33 cfs @ 0.16 hrs, Volume= 2.102 af  
 Outflow = 151.33 cfs @ 0.16 hrs, Volume= 2.102 af, Atten= 0%, Lag= 0.0 min  
 Primary = 26.51 cfs @ 0.03 hrs, Volume= 0.552 af  
 Secondary = 127.69 cfs @ 0.16 hrs, Volume= 1.549 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 49.88' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	42.86'	<b>30.0" Round CB A08</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 42.86' / 42.56' S= 0.0015 '/' Cc= 0.900 n= 0.013, Flow Area= 4.91 sf
#2	Secondary	47.95'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=26.16 cfs @ 0.03 hrs HW=48.18' TW=46.70' (Dynamic Tailwater)  
 ↑1=**CB A08** (Outlet Controls 26.16 cfs @ 5.33 fps)

**Secondary OutFlow** Max=126.64 cfs @ 0.16 hrs HW=49.88' TW=48.71' (Dynamic Tailwater)  
 ↑2=**Broad-Crested Rectangular Weir** (Weir Controls 126.64 cfs @ 3.28 fps)

**Existing System A**

City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

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**Summary for Pond A09: A09**

Inflow Area = 52.290 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 160.77 cfs @ 0.16 hrs, Volume= 2.231 af  
 Outflow = 160.77 cfs @ 0.16 hrs, Volume= 2.231 af, Atten= 0%, Lag= 0.0 min  
 Primary = 32.37 cfs @ 0.05 hrs, Volume= 0.600 af  
 Secondary = 130.34 cfs @ 0.16 hrs, Volume= 1.632 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 48.72' @ 0.17 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	42.55'	<b>36.0" Round CB A09</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 42.55' / 42.25' S= 0.0015 '/' Cc= 0.900 n= 0.013, Flow Area= 7.07 sf
#2	Secondary	46.55'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=32.35 cfs @ 0.05 hrs HW=47.06' TW=46.10' (Dynamic Tailwater)  
 ↑1=**CB A09** (Outlet Controls 32.35 cfs @ 4.58 fps)

**Secondary OutFlow** Max=130.16 cfs @ 0.16 hrs HW=48.71' TW=47.86' (Dynamic Tailwater)  
 ↑2=**Broad-Crested Rectangular Weir** (Weir Controls 130.16 cfs @ 3.02 fps)

**Summary for Pond A10: A10**

Inflow Area = 59.110 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 182.00 cfs @ 0.16 hrs, Volume= 2.524 af  
 Outflow = 182.00 cfs @ 0.16 hrs, Volume= 2.524 af, Atten= 0%, Lag= 0.0 min  
 Primary = 30.81 cfs @ 0.13 hrs, Volume= 0.519 af  
 Secondary = 152.43 cfs @ 0.16 hrs, Volume= 2.005 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 47.90' @ 0.17 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	42.24'	<b>36.0" Round Culvert</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 42.24' / 41.94' S= 0.0015 '/' Cc= 0.900 n= 0.013, Flow Area= 7.07 sf
#2	Secondary	45.35'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=30.80 cfs @ 0.13 hrs HW=47.52' TW=46.65' (Dynamic Tailwater)  
 ↑1=**Culvert** (Outlet Controls 30.80 cfs @ 4.36 fps)

**Secondary OutFlow** Max=152.06 cfs @ 0.16 hrs HW=47.86' TW=47.07' (Dynamic Tailwater)  
 ↑2=**Broad-Crested Rectangular Weir** (Weir Controls 152.06 cfs @ 3.02 fps)

**Existing System A**

City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

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**Summary for Pond A11/12: A11/12 POND**

Inflow Area = 94.950 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 290.86 cfs @ 0.16 hrs, Volume= 4.046 af  
 Outflow = 62.12 cfs @ 0.29 hrs, Volume= 4.050 af, Atten= 79%, Lag= 7.4 min  
 Primary = 62.12 cfs @ 0.29 hrs, Volume= 4.050 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 47.74' @ 0.29 hrs Surf.Area= 92,722 sf Storage= 117,702 cf

Plug-Flow detention time= 17.1 min calculated for 4.046 af (100% of inflow)  
 Center-of-Mass det. time= 17.1 min ( 26.7 - 9.5 )

Volume	Invert	Avail.Storage	Storage Description
#1	44.40'	1,991,741 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
44.40	10	0	0
45.20	100	44	44
48.00	102,400	143,500	143,544
50.00	433,110	535,510	679,054
52.00	879,577	1,312,687	1,991,741

Device	Routing	Invert	Outlet Devices
#1	Primary	41.93'	<b>34.0" Round CB A12-Out</b> L= 376.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 41.93' / 38.20' S= 0.0099 '/ Cc= 0.900 n= 0.013, Flow Area= 6.31 sf

**Primary OutFlow** Max=62.12 cfs @ 0.29 hrs HW=47.73' TW=0.00' (Dynamic Tailwater)  
 ←**1=CB A12-Out** (Barrel Controls 62.12 cfs @ 9.85 fps)

**Summary for Subcatchment EA01: E-A01**

Runoff = 10.43 cfs @ 0.13 hrs, Volume= 0.144 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
 City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description			
3.350	0.95	Paved parking, HSG D			
3.350		100.00% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.6					<b>Direct Entry, EA01</b>

**Existing System A**

City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

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**Summary for Subcatchment EA02: E-A02**

Runoff = 17.22 cfs @ 0.12 hrs, Volume= 0.237 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
5.530	0.95	Paved parking, HSG D
5.530		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.9					Direct Entry, E-A02

**Summary for Subcatchment EA03: E-A03**

Runoff = 24.19 cfs @ 0.13 hrs, Volume= 0.333 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
7.770	0.95	Paved parking, HSG D
7.770		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.7					Direct Entry, E-A3

**Summary for Subcatchment EA04: E-A04**

Runoff = 25.24 cfs @ 0.16 hrs, Volume= 0.345 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
8.040	0.95	Paved parking, HSG D
8.040		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.0					Direct Entry, E-A04

**Summary for Subcatchment EA05: E-A05**

Runoff = 19.78 cfs @ 0.16 hrs, Volume= 0.272 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

**Existing System A**

City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

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Area (ac)	C	Description
6.350	0.95	Paved parking, HSG D
6.350		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.1					<b>Direct Entry, E-A05</b>

**Summary for Subcatchment EA06: E-A06**

Runoff = 24.88 cfs @ 0.17 hrs, Volume= 0.349 af, Depth= 0.49"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
8.460	0.95	Paved parking, HSG D
8.460		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.4					<b>Direct Entry, A-A06</b>

**Summary for Subcatchment EA07: E-A07**

Runoff = 18.03 cfs @ 0.16 hrs, Volume= 0.248 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
5.790	0.95	Paved parking, HSG D
5.790		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.3					<b>Direct Entry, E-A07</b>

**Summary for Subcatchment EA08: E-A08**

Runoff = 12.36 cfs @ 0.14 hrs, Volume= 0.170 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
3.970	0.95	Paved parking, HSG D
3.970		100.00% Impervious Area

**Existing System A**

City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.1					<b>Direct Entry, E-A08</b>

**Summary for Subcatchment EA09: E-A09**

Runoff = 9.43 cfs @ 0.14 hrs, Volume= 0.130 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
3.030	0.95	Paved parking, HSG D
3.030		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.9					<b>Direct Entry, E-A9</b>

**Summary for Subcatchment EA10: E-A10**

Runoff = 21.25 cfs @ 0.16 hrs, Volume= 0.292 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
6.820	0.95	Paved parking, HSG D
6.820		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.4					<b>Direct Entry, E-A10</b>

**Summary for Subcatchment EA11: E-A11**

Runoff = 60.82 cfs @ 0.16 hrs, Volume= 0.837 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
19.520	0.95	Paved parking, HSG D
19.520		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.4					<b>Direct Entry, E-A11</b>



**Existing System A**

City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

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**Summary for Subcatchment EA12: E-A12**

Runoff = 48.94 cfs @ 0.17 hrs, Volume= 0.686 af, Depth= 0.50"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
16.320	0.95	Paved parking, HSG D
16.320		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.2					<b>Direct Entry, E-A12</b>

**Summary for Pond OA: OUTFALL A**

Inflow Area = 94.950 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event

Inflow = 62.12 cfs @ 0.29 hrs, Volume= 4.050 af

Primary = 62.12 cfs @ 0.29 hrs, Volume= 4.050 af, Atten= 0%, Lag= 0.0 min

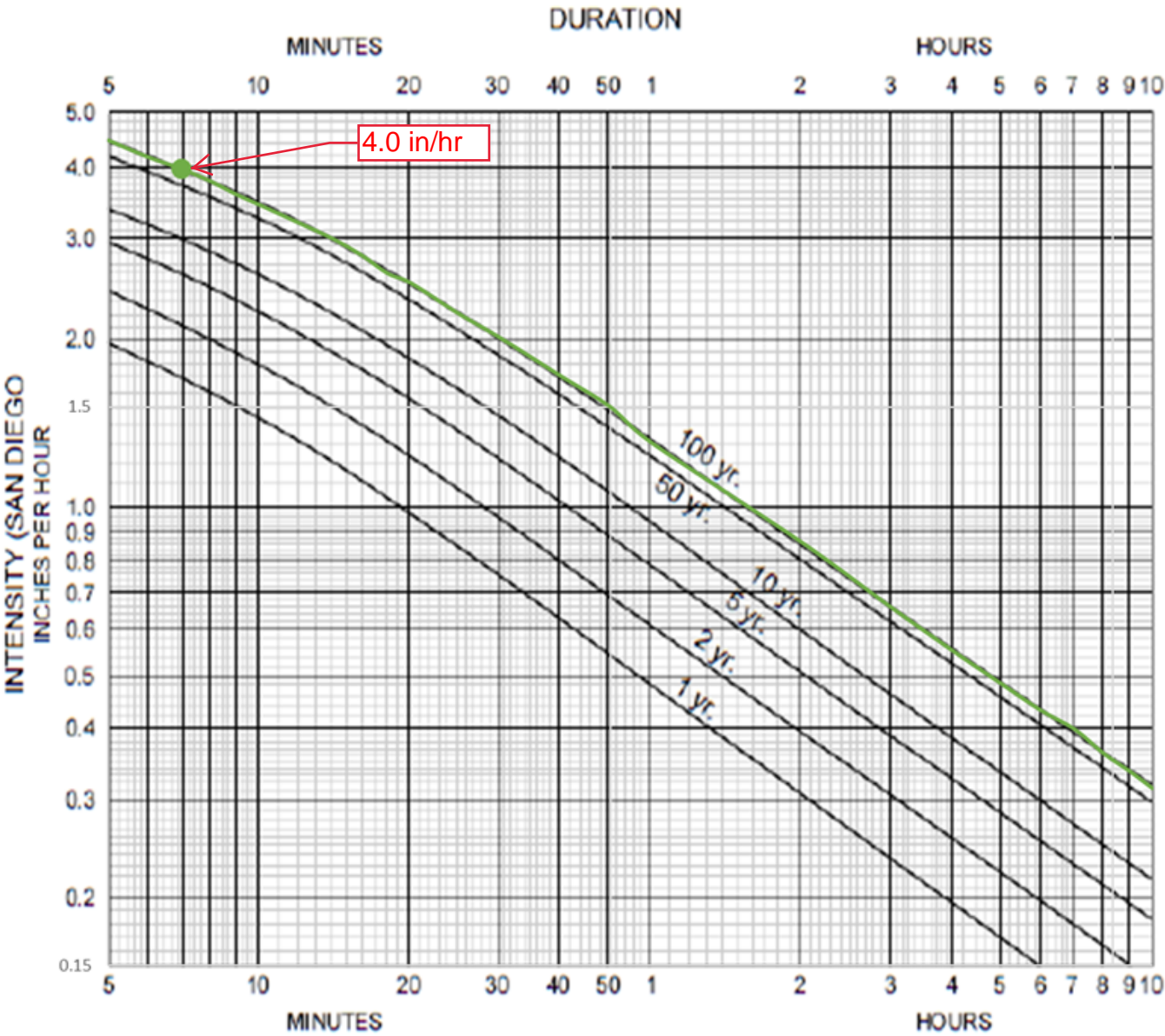
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

100-year Rainfall Intensity

ELEV.	FACTOR
0-1500	1.00
1500-3000	1.25
3000-4000	1.42
4000-5000	1.60
5000-6000	1.7
DESERT	1.25

TO OBTAIN CORRECT INTENSITY, MULTIPLY INTENSITY ON CHART BY FACTOR FOR DESIGN ELEVATION.

INTENSITY - DURATION - FREQUENCY CURVES  
 RAINFALL  
 FOR  
 COUNTY OF SAN DIEGO



— 100 year    ● 100 year event

Figure A-1. Intensity-Duration-Frequency Design Chart

**Existing System A**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points x 3

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Pond A01: A01</b>	Peak Elev=63.48' Inflow=11.07 cfs 0.152 af Primary=9.18 cfs 0.143 af Secondary=1.90 cfs 0.010 af Outflow=11.07 cfs 0.152 af
<b>Pond A02: A02</b>	Peak Elev=60.69' Inflow=29.35 cfs 0.404 af Primary=8.07 cfs 0.161 af Secondary=21.94 cfs 0.243 af Outflow=29.35 cfs 0.404 af
<b>Pond A03: A03</b>	Peak Elev=58.86' Inflow=55.03 cfs 0.758 af Primary=17.37 cfs 0.355 af Secondary=38.18 cfs 0.402 af Outflow=55.03 cfs 0.758 af
<b>Pond A04: A04</b>	Peak Elev=56.63' Inflow=81.86 cfs 1.124 af Primary=18.91 cfs 0.354 af Secondary=66.95 cfs 0.769 af Outflow=81.82 cfs 1.124 af
<b>Pond A05: A05</b>	Peak Elev=55.11' Inflow=102.82 cfs 1.412 af Primary=24.95 cfs 0.534 af Secondary=80.44 cfs 0.879 af Outflow=102.82 cfs 1.412 af
<b>Pond A06: A06</b>	Peak Elev=54.03' Inflow=128.39 cfs 1.783 af Primary=26.66 cfs 0.479 af Secondary=108.62 cfs 1.304 af Outflow=128.39 cfs 1.783 af
<b>Pond A07: A07</b>	Peak Elev=53.19' Inflow=147.52 cfs 2.046 af Primary=39.36 cfs 0.883 af Secondary=109.09 cfs 1.163 af Outflow=147.52 cfs 2.046 af
<b>Pond A08: A08</b>	Peak Elev=50.00' Inflow=160.65 cfs 2.227 af Primary=26.36 cfs 0.548 af Secondary=137.06 cfs 1.678 af Outflow=160.65 cfs 2.227 af
<b>Pond A09: A09</b>	Peak Elev=48.85' Inflow=170.66 cfs 2.364 af Primary=32.45 cfs 0.603 af Secondary=139.99 cfs 1.762 af Outflow=170.66 cfs 2.364 af
<b>Pond A10: A10</b>	Peak Elev=48.02' Inflow=193.20 cfs 2.675 af Primary=31.51 cfs 0.528 af Secondary=162.95 cfs 2.146 af Outflow=193.20 cfs 2.675 af
<b>Pond A11/12: A11/12 POND</b>	Peak Elev=47.84' Storage=127,620 cf Inflow=308.76 cfs 4.291 af 34.0" Round Culvert n=0.013 L=376.0' S=0.0099 '/' Outflow=62.61 cfs 4.304 af
<b>Subcatchment EA01: E-A01</b>	Runoff Area=3.350 ac 100.00% Impervious Runoff Depth=0.55" Tc=7.6 min C=0.95 Runoff=11.07 cfs 0.152 af
<b>Subcatchment EA02: E-A02</b>	Runoff Area=5.530 ac 100.00% Impervious Runoff Depth=0.55" Tc=6.9 min C=0.95 Runoff=18.28 cfs 0.252 af
<b>Subcatchment EA03: E-A03</b>	Runoff Area=7.770 ac 100.00% Impervious Runoff Depth=0.55" Tc=7.7 min C=0.95 Runoff=25.68 cfs 0.354 af
<b>Subcatchment EA04: E-A04</b>	Runoff Area=8.040 ac 100.00% Impervious Runoff Depth=0.55" Tc=9.0 min C=0.95 Runoff=26.79 cfs 0.366 af
<b>Subcatchment EA05: E-A05</b>	Runoff Area=6.350 ac 100.00% Impervious Runoff Depth=0.55" Tc=9.1 min C=0.95 Runoff=21.00 cfs 0.289 af
<b>Subcatchment EA06: E-A06</b>	Runoff Area=8.460 ac 100.00% Impervious Runoff Depth=0.53" Tc=10.4 min C=0.95 Runoff=26.41 cfs 0.370 af

**Existing System A***City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr*

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<b>Subcatchment EA07: E-A07</b>	Runoff Area=5.790 ac 100.00% Impervious Runoff Depth=0.55" Tc=9.3 min C=0.95 Runoff=19.14 cfs 0.263 af
<b>Subcatchment EA08: E-A08</b>	Runoff Area=3.970 ac 100.00% Impervious Runoff Depth=0.55" Tc=8.1 min C=0.95 Runoff=13.12 cfs 0.181 af
<b>Subcatchment EA09: E-A09</b>	Runoff Area=3.030 ac 100.00% Impervious Runoff Depth=0.55" Tc=7.9 min C=0.95 Runoff=10.01 cfs 0.138 af
<b>Subcatchment EA10: E-A10</b>	Runoff Area=6.820 ac 100.00% Impervious Runoff Depth=0.55" Tc=9.4 min C=0.95 Runoff=22.56 cfs 0.310 af
<b>Subcatchment EA11: E-A11</b>	Runoff Area=19.520 ac 100.00% Impervious Runoff Depth=0.55" Tc=9.4 min C=0.95 Runoff=64.57 cfs 0.888 af
<b>Subcatchment EA12: E-A12</b>	Runoff Area=16.320 ac 100.00% Impervious Runoff Depth=0.54" Tc=10.2 min C=0.95 Runoff=51.95 cfs 0.728 af
<b>Pond OA: OUTFALLA</b>	Inflow=62.61 cfs 4.304 af Primary=62.61 cfs 4.304 af

**Total Runoff Area = 94.950 ac Runoff Volume = 4.291 af Average Runoff Depth = 0.54"**  
**0.00% Pervious = 0.000 ac 100.00% Impervious = 94.950 ac**

**Existing System A**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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**Summary for Pond A01: A01**

Inflow Area = 3.350 ac, 100.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 11.07 cfs @ 0.13 hrs, Volume= 0.152 af  
 Outflow = 11.07 cfs @ 0.14 hrs, Volume= 0.152 af, Atten= 0%, Lag= 0.6 min  
 Primary = 9.18 cfs @ 0.18 hrs, Volume= 0.143 af  
 Secondary = 1.90 cfs @ 0.16 hrs, Volume= 0.010 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 63.48' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	59.12'	<b>18.0" Round CB A1</b> L= 282.8' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 59.12' / 56.16' S= 0.0105 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#2	Secondary	63.37'	<b>20.0' long x 282.8' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=9.18 cfs @ 0.18 hrs HW=63.43' TW=60.63' (Dynamic Tailwater)  
 ↑1=**CB A1** (Outlet Controls 9.18 cfs @ 5.19 fps)

**Secondary OutFlow** Max=1.90 cfs @ 0.16 hrs HW=63.48' TW=60.69' (Dynamic Tailwater)  
 ↑2=**Broad-Crested Rectangular Weir** (Weir Controls 1.90 cfs @ 0.88 fps)

**Summary for Pond A02: A02**

Inflow Area = 8.880 ac, 100.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 29.35 cfs @ 0.14 hrs, Volume= 0.404 af  
 Outflow = 29.35 cfs @ 0.15 hrs, Volume= 0.404 af, Atten= 0%, Lag= 0.6 min  
 Primary = 8.07 cfs @ 0.04 hrs, Volume= 0.161 af  
 Secondary = 21.94 cfs @ 0.16 hrs, Volume= 0.243 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 60.69' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.16'	<b>18.0" Round CB A02</b> L= 285.8' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.16' / 54.30' S= 0.0065 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf
#2	Secondary	60.14'	<b>20.0' long x 285.8' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=7.97 cfs @ 0.04 hrs HW=60.27' TW=58.15' (Dynamic Tailwater)  
 ↑1=**CB A02** (Outlet Controls 7.97 cfs @ 4.51 fps)

**Secondary OutFlow** Max=21.94 cfs @ 0.16 hrs HW=60.69' TW=58.86' (Dynamic Tailwater)  
 ↑2=**Broad-Crested Rectangular Weir** (Weir Controls 21.94 cfs @ 2.00 fps)

**Existing System A**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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**Summary for Pond A03: A03**

Inflow Area = 16.650 ac, 100.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 55.03 cfs @ 0.13 hrs, Volume= 0.758 af  
 Outflow = 55.03 cfs @ 0.13 hrs, Volume= 0.758 af, Atten= 0%, Lag= 0.0 min  
 Primary = 17.37 cfs @ 0.05 hrs, Volume= 0.355 af  
 Secondary = 38.18 cfs @ 0.16 hrs, Volume= 0.402 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 58.86' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	54.30'	<b>24.0" Round CB A03</b> L= 279.8' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 54.30' / 51.21' S= 0.0110 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Secondary	58.05'	<b>20.0' long x 279.8' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=17.36 cfs @ 0.05 hrs HW=58.25' TW=55.89' (Dynamic Tailwater)  
 ↑1=**CB A03** (Outlet Controls 17.36 cfs @ 5.53 fps)

**Secondary OutFlow** Max=38.18 cfs @ 0.16 hrs HW=58.86' TW=56.63' (Dynamic Tailwater)  
 ↑2=**Broad-Crested Rectangular Weir** (Weir Controls 38.18 cfs @ 2.37 fps)

**Summary for Pond A04: A04**

Inflow Area = 24.690 ac, 100.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 81.86 cfs @ 0.16 hrs, Volume= 1.124 af  
 Outflow = 81.82 cfs @ 0.16 hrs, Volume= 1.124 af, Atten= 0%, Lag= 0.0 min  
 Primary = 18.91 cfs @ 0.03 hrs, Volume= 0.354 af  
 Secondary = 66.95 cfs @ 0.16 hrs, Volume= 0.769 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 56.63' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	51.21'	<b>24.0" Round CB A04</b> L= 231.1' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 51.21' / 49.52' S= 0.0073 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Secondary	55.46'	<b>20.0' long x 231.1' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=16.92 cfs @ 0.03 hrs HW=55.48' TW=53.51' (Dynamic Tailwater)  
 ↑1=**CB A04** (Outlet Controls 16.92 cfs @ 5.39 fps)

**Secondary OutFlow** Max=66.69 cfs @ 0.16 hrs HW=56.63' TW=55.10' (Dynamic Tailwater)  
 ↑2=**Broad-Crested Rectangular Weir** (Weir Controls 66.69 cfs @ 2.85 fps)

**Existing System A**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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**Summary for Pond A05: A05**

Inflow Area = 31.040 ac, 100.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 102.82 cfs @ 0.16 hrs, Volume= 1.412 af  
 Outflow = 102.82 cfs @ 0.16 hrs, Volume= 1.412 af, Atten= 0%, Lag= 0.0 min  
 Primary = 24.95 cfs @ 0.05 hrs, Volume= 0.534 af  
 Secondary = 80.44 cfs @ 0.16 hrs, Volume= 0.879 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 55.11' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	49.52'	<b>30.0" Round CB A05</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 49.52' / 48.03' S= 0.0075 '/' Cc= 0.900 n= 0.013, Flow Area= 4.91 sf
#2	Secondary	53.75'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=24.92 cfs @ 0.05 hrs HW=54.06' TW=52.72' (Dynamic Tailwater)  
 ↑1=**CB A05** (Outlet Controls 24.92 cfs @ 5.08 fps)

**Secondary OutFlow** Max=80.09 cfs @ 0.16 hrs HW=55.10' TW=54.02' (Dynamic Tailwater)  
 ↑2=**Broad-Crested Rectangular Weir** (Weir Controls 80.09 cfs @ 2.96 fps)

**Summary for Pond A06: A06**

Inflow Area = 39.500 ac, 100.00% Impervious, Inflow Depth = 0.54" for 100-Year event  
 Inflow = 128.39 cfs @ 0.16 hrs, Volume= 1.783 af  
 Outflow = 128.39 cfs @ 0.16 hrs, Volume= 1.783 af, Atten= 0%, Lag= 0.0 min  
 Primary = 26.66 cfs @ 0.03 hrs, Volume= 0.479 af  
 Secondary = 108.62 cfs @ 0.16 hrs, Volume= 1.304 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 54.03' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	48.03'	<b>30.0" Round CB A06</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 48.03' / 47.71' S= 0.0016 '/' Cc= 0.900 n= 0.013, Flow Area= 4.91 sf
#2	Secondary	52.18'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=26.19 cfs @ 0.03 hrs HW=52.28' TW=50.80' (Dynamic Tailwater)  
 ↑1=**CB A06** (Outlet Controls 26.19 cfs @ 5.33 fps)

**Secondary OutFlow** Max=108.58 cfs @ 0.16 hrs HW=54.03' TW=53.19' (Dynamic Tailwater)  
 ↑2=**Broad-Crested Rectangular Weir** (Weir Controls 108.58 cfs @ 2.94 fps)

**Existing System A**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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**Summary for Pond A07: A07**

Inflow Area = 45.290 ac, 100.00% Impervious, Inflow Depth = 0.54" for 100-Year event  
 Inflow = 147.52 cfs @ 0.16 hrs, Volume= 2.046 af  
 Outflow = 147.52 cfs @ 0.16 hrs, Volume= 2.046 af, Atten= 0%, Lag= 0.0 min  
 Primary = 39.36 cfs @ 0.09 hrs, Volume= 0.883 af  
 Secondary = 109.09 cfs @ 0.16 hrs, Volume= 1.163 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 53.19' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	47.71'	<b>30.0" Round CB A07</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 47.71' / 43.96' S= 0.0187 '/' Cc= 0.900 n= 0.013, Flow Area= 4.91 sf
#2	Secondary	51.56'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=39.33 cfs @ 0.09 hrs HW=52.56' TW=49.22' (Dynamic Tailwater)  
 ↑1=CB A07 (Outlet Controls 39.33 cfs @ 8.01 fps)

**Secondary OutFlow** Max=109.04 cfs @ 0.16 hrs HW=53.19' TW=50.00' (Dynamic Tailwater)  
 ↑2=Broad-Crested Rectangular Weir (Weir Controls 109.04 cfs @ 3.35 fps)

**Summary for Pond A08: A08**

Inflow Area = 49.260 ac, 100.00% Impervious, Inflow Depth = 0.54" for 100-Year event  
 Inflow = 160.65 cfs @ 0.16 hrs, Volume= 2.227 af  
 Outflow = 160.65 cfs @ 0.16 hrs, Volume= 2.227 af, Atten= 0%, Lag= 0.0 min  
 Primary = 26.36 cfs @ 0.03 hrs, Volume= 0.548 af  
 Secondary = 137.06 cfs @ 0.16 hrs, Volume= 1.678 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 50.00' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	42.86'	<b>30.0" Round CB A08</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 42.86' / 42.56' S= 0.0015 '/' Cc= 0.900 n= 0.013, Flow Area= 4.91 sf
#2	Secondary	47.95'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=26.16 cfs @ 0.03 hrs HW=48.23' TW=46.75' (Dynamic Tailwater)  
 ↑1=CB A08 (Outlet Controls 26.16 cfs @ 5.33 fps)

**Secondary OutFlow** Max=135.85 cfs @ 0.16 hrs HW=50.00' TW=48.83' (Dynamic Tailwater)  
 ↑2=Broad-Crested Rectangular Weir (Weir Controls 135.85 cfs @ 3.31 fps)



**Existing System A**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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**Summary for Pond A09: A09**

Inflow Area = 52.290 ac, 100.00% Impervious, Inflow Depth = 0.54" for 100-Year event  
 Inflow = 170.66 cfs @ 0.16 hrs, Volume= 2.364 af  
 Outflow = 170.66 cfs @ 0.16 hrs, Volume= 2.364 af, Atten= 0%, Lag= 0.0 min  
 Primary = 32.45 cfs @ 0.04 hrs, Volume= 0.603 af  
 Secondary = 139.99 cfs @ 0.16 hrs, Volume= 1.762 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 48.85' @ 0.17 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	42.55'	<b>36.0" Round CB A09</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 42.55' / 42.25' S= 0.0015 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf
#2	Secondary	46.55'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=32.45 cfs @ 0.04 hrs HW=47.00' TW=46.03' (Dynamic Tailwater)  
 ↑1=**CB A09** (Outlet Controls 32.45 cfs @ 4.59 fps)

**Secondary OutFlow** Max=139.82 cfs @ 0.16 hrs HW=48.83' TW=47.97' (Dynamic Tailwater)  
 ↑2=**Broad-Crested Rectangular Weir** (Weir Controls 139.82 cfs @ 3.06 fps)

**Summary for Pond A10: A10**

Inflow Area = 59.110 ac, 100.00% Impervious, Inflow Depth = 0.54" for 100-Year event  
 Inflow = 193.20 cfs @ 0.16 hrs, Volume= 2.675 af  
 Outflow = 193.20 cfs @ 0.16 hrs, Volume= 2.675 af, Atten= 0%, Lag= 0.0 min  
 Primary = 31.51 cfs @ 0.13 hrs, Volume= 0.528 af  
 Secondary = 162.95 cfs @ 0.16 hrs, Volume= 2.146 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 48.02' @ 0.17 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	42.24'	<b>36.0" Round Culvert</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 42.24' / 41.94' S= 0.0015 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf
#2	Secondary	45.35'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=31.50 cfs @ 0.13 hrs HW=47.63' TW=46.72' (Dynamic Tailwater)  
 ↑1=**Culvert** (Outlet Controls 31.50 cfs @ 4.46 fps)

**Secondary OutFlow** Max=162.56 cfs @ 0.16 hrs HW=47.98' TW=47.15' (Dynamic Tailwater)  
 ↑2=**Broad-Crested Rectangular Weir** (Weir Controls 162.56 cfs @ 3.09 fps)

**Existing System A**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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**Summary for Pond A11/12: A11/12 POND**

Inflow Area = 94.950 ac, 100.00% Impervious, Inflow Depth = 0.54" for 100-Year event  
 Inflow = 308.76 cfs @ 0.16 hrs, Volume= 4.291 af  
 Outflow = 62.61 cfs @ 0.29 hrs, Volume= 4.304 af, Atten= 80%, Lag= 7.4 min  
 Primary = 62.61 cfs @ 0.29 hrs, Volume= 4.304 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 47.84' @ 0.29 hrs Surf.Area= 96,551 sf Storage= 127,620 cf

Plug-Flow detention time= 18.4 min calculated for 4.290 af (100% of inflow)  
 Center-of-Mass det. time= 18.5 min ( 28.0 - 9.5 )

Volume	Invert	Avail.Storage	Storage Description
#1	44.40'	1,991,741 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
44.40	10	0	0
45.20	100	44	44
48.00	102,400	143,500	143,544
50.00	433,110	535,510	679,054
52.00	879,577	1,312,687	1,991,741

Device	Routing	Invert	Outlet Devices
#1	Primary	41.93'	<b>34.0" Round CB A12-Out</b> L= 376.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 41.93' / 38.20' S= 0.0099 '/ Cc= 0.900 n= 0.013, Flow Area= 6.31 sf

**Primary OutFlow** Max=62.60 cfs @ 0.29 hrs HW=47.84' TW=0.00' (Dynamic Tailwater)  
 ←**1=CB A12-Out** (Barrel Controls 62.60 cfs @ 9.93 fps)

**Summary for Subcatchment EA01: E-A01**

Runoff = 11.07 cfs @ 0.13 hrs, Volume= 0.152 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
 City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description			
3.350	0.95	Paved parking, HSG D			
3.350		100.00% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.6					<b>Direct Entry, EA01</b>

**Existing System A**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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**Summary for Subcatchment EA02: E-A02**

Runoff = 18.28 cfs @ 0.12 hrs, Volume= 0.252 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
5.530	0.95	Paved parking, HSG D
5.530		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.9					Direct Entry, E-A02

**Summary for Subcatchment EA03: E-A03**

Runoff = 25.68 cfs @ 0.13 hrs, Volume= 0.354 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
7.770	0.95	Paved parking, HSG D
7.770		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.7					Direct Entry, E-A3

**Summary for Subcatchment EA04: E-A04**

Runoff = 26.79 cfs @ 0.16 hrs, Volume= 0.366 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
8.040	0.95	Paved parking, HSG D
8.040		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.0					Direct Entry, E-A04

**Summary for Subcatchment EA05: E-A05**

Runoff = 21.00 cfs @ 0.16 hrs, Volume= 0.289 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

**Existing System A**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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Area (ac)	C	Description
6.350	0.95	Paved parking, HSG D
6.350		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.1					<b>Direct Entry, E-A05</b>

**Summary for Subcatchment EA06: E-A06**

Runoff = 26.41 cfs @ 0.17 hrs, Volume= 0.370 af, Depth= 0.53"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
8.460	0.95	Paved parking, HSG D
8.460		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.4					<b>Direct Entry, A-A06</b>

**Summary for Subcatchment EA07: E-A07**

Runoff = 19.14 cfs @ 0.16 hrs, Volume= 0.263 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
5.790	0.95	Paved parking, HSG D
5.790		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.3					<b>Direct Entry, E-A07</b>

**Summary for Subcatchment EA08: E-A08**

Runoff = 13.12 cfs @ 0.14 hrs, Volume= 0.181 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
3.970	0.95	Paved parking, HSG D
3.970		100.00% Impervious Area

**Existing System A**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.1					<b>Direct Entry, E-A08</b>

**Summary for Subcatchment EA09: E-A09**

Runoff = 10.01 cfs @ 0.14 hrs, Volume= 0.138 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
3.030	0.95	Paved parking, HSG D
3.030		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.9					<b>Direct Entry, E-A9</b>

**Summary for Subcatchment EA10: E-A10**

Runoff = 22.56 cfs @ 0.16 hrs, Volume= 0.310 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
6.820	0.95	Paved parking, HSG D
6.820		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.4					<b>Direct Entry, E-A10</b>

**Summary for Subcatchment EA11: E-A11**

Runoff = 64.57 cfs @ 0.16 hrs, Volume= 0.888 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
19.520	0.95	Paved parking, HSG D
19.520		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.4					<b>Direct Entry, E-A11</b>

**Existing System A**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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**Summary for Subcatchment EA12: E-A12**

Runoff = 51.95 cfs @ 0.17 hrs, Volume= 0.728 af, Depth= 0.54"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
16.320	0.95	Paved parking, HSG D
16.320		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.2					<b>Direct Entry, E-A12</b>

**Summary for Pond OA: OUTFALL A**

Inflow Area = 94.950 ac, 100.00% Impervious, Inflow Depth = 0.54" for 100-Year event

Inflow = 62.61 cfs @ 0.29 hrs, Volume= 4.304 af

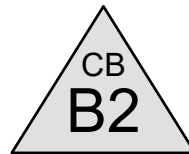
Primary = 62.61 cfs @ 0.29 hrs, Volume= 4.304 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

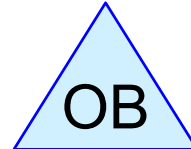


E-B1

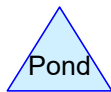
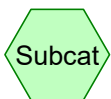
B1



B2



OUTFALL B



## Existing System B

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### Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	B1	49.75	47.88	758.6	0.0025	0.013	24.0	0.0	0.0
2	B2	47.60	47.46	130.0	0.0011	0.013	34.0	0.0	0.0



**Existing System B***City of San Diego 50-Year Duration=7 min, Inten=3.80 in/hr*

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Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**Pond B1: B1**Peak Elev=52.27' Storage=7,859 cf Inflow=36.94 cfs 0.355 af  
24.0" Round Culvert n=0.013 L=758.6' S=0.0025 '/ Outflow=12.77 cfs 0.357 af**Pond B2: B2**Peak Elev=49.51' Inflow=12.77 cfs 0.357 af  
34.0" Round Culvert n=0.013 L=130.0' S=0.0011 '/ Outflow=12.77 cfs 0.357 af**Subcatchment EB1: E-B1**Runoff Area=10.110 ac 100.00% Impervious Runoff Depth=0.42"  
Tc=6.6 min C=0.95 Runoff=36.94 cfs 0.355 af**Pond OB: OUTFALL B**Inflow=12.77 cfs 0.357 af  
Primary=12.77 cfs 0.357 af**Total Runoff Area = 10.110 ac Runoff Volume = 0.355 af Average Runoff Depth = 0.42"**  
**0.00% Pervious = 0.000 ac 100.00% Impervious = 10.110 ac**

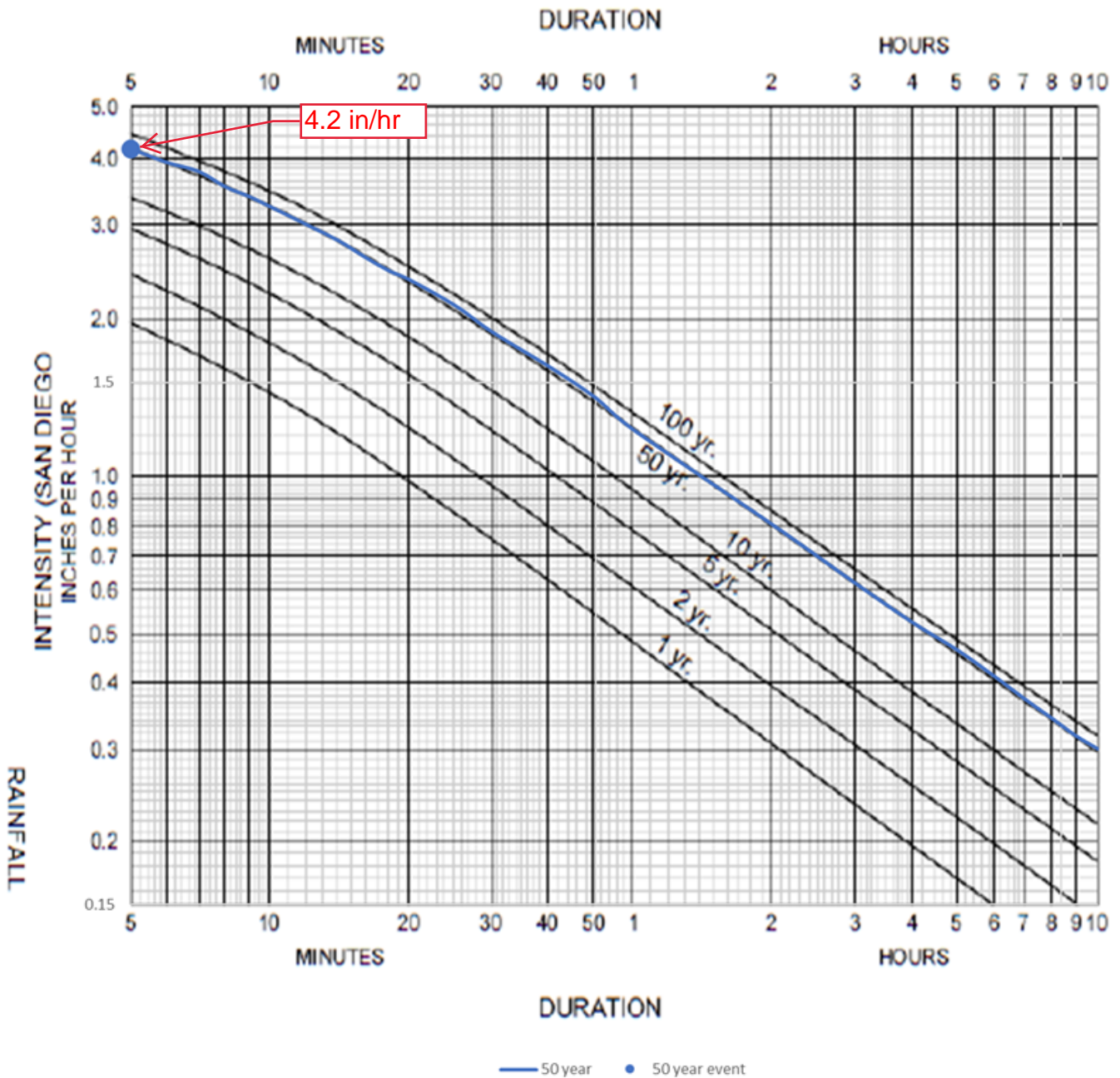
50-year Rainfall Intensity

ELEV.	FACTOR
0-1500	1.00
1500-3000	1.25
3000-4000	1.42
4000-5000	1.60
5000-6000	1.7
DESERT	1.25

Figure A-1. Intensity-Duration-Frequency Design Chart

TO OBTAIN CORRECT INTENSITY, MULTIPLY INTENSITY ON CHART BY FACTOR FOR DESIGN ELEVATION.

INTENSITY - DURATION - FREQUENCY CURVES  
FOR  
COUNTY OF SAN DIEGO  
RAINFALL



**Existing System B**

City of San Diego 50-Year Duration=7 min, Inten=3.80 in/hr

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**Summary for Pond B1: B1**

Inflow Area = 10.110 ac, 100.00% Impervious, Inflow Depth = 0.42" for 50-Year event  
 Inflow = 36.94 cfs @ 0.11 hrs, Volume= 0.355 af  
 Outflow = 12.77 cfs @ 0.19 hrs, Volume= 0.357 af, Atten= 65%, Lag= 4.6 min  
 Primary = 12.77 cfs @ 0.19 hrs, Volume= 0.357 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
 Peak Elev= 52.27' @ 0.19 hrs Surf.Area= 23,651 sf Storage= 7,859 cf

Plug-Flow detention time= 5.9 min calculated for 0.355 af (100% of inflow)  
 Center-of-Mass det. time= 5.9 min ( 12.7 - 6.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	51.00'	141,125 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
51.00	100	0	0
52.00	7,383	3,742	3,742
54.00	130,000	137,383	141,125

Device	Routing	Invert	Outlet Devices
#1	Primary	49.75'	<b>24.0" Round CB B1</b> L= 758.6' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 49.75' / 47.88' S= 0.0025 ' / Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=12.77 cfs @ 0.19 hrs HW=52.27' TW=49.51' (Dynamic Tailwater)  
 ↑1=CB B1 (Barrel Controls 12.77 cfs @ 4.16 fps)

**Summary for Pond B2: B2**

Inflow Area = 10.110 ac, 100.00% Impervious, Inflow Depth = 0.42" for 50-Year event  
 Inflow = 12.77 cfs @ 0.19 hrs, Volume= 0.357 af  
 Outflow = 12.77 cfs @ 0.19 hrs, Volume= 0.357 af, Atten= 0%, Lag= 0.0 min  
 Primary = 12.77 cfs @ 0.19 hrs, Volume= 0.357 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
 Peak Elev= 49.51' @ 0.19 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	47.60'	<b>34.0" Round Junction B2</b> L= 130.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 47.60' / 47.46' S= 0.0011 ' / Cc= 0.900 n= 0.013, Flow Area= 6.31 sf

**Primary OutFlow** Max=12.77 cfs @ 0.19 hrs HW=49.51' TW=0.00' (Dynamic Tailwater)  
 ↑1=Junction B2 (Barrel Controls 12.77 cfs @ 3.98 fps)

**Existing System B**

City of San Diego 50-Year Duration=7 min, Inten=3.80 in/hr

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**Summary for Subcatchment EB1: E-B1**

Runoff = 36.94 cfs @ 0.11 hrs, Volume= 0.355 af, Depth= 0.42"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=7 min, Inten=3.80 in/hr

Area (ac)	C	Description
10.110	0.95	Paved parking, HSG D
10.110		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.6					<b>Direct Entry, EB1</b>

**Summary for Pond OB: OUTFALL B**

Inflow Area = 10.110 ac, 100.00% Impervious, Inflow Depth = 0.42" for 50-Year event

Inflow = 12.77 cfs @ 0.19 hrs, Volume= 0.357 af

Primary = 12.77 cfs @ 0.19 hrs, Volume= 0.357 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

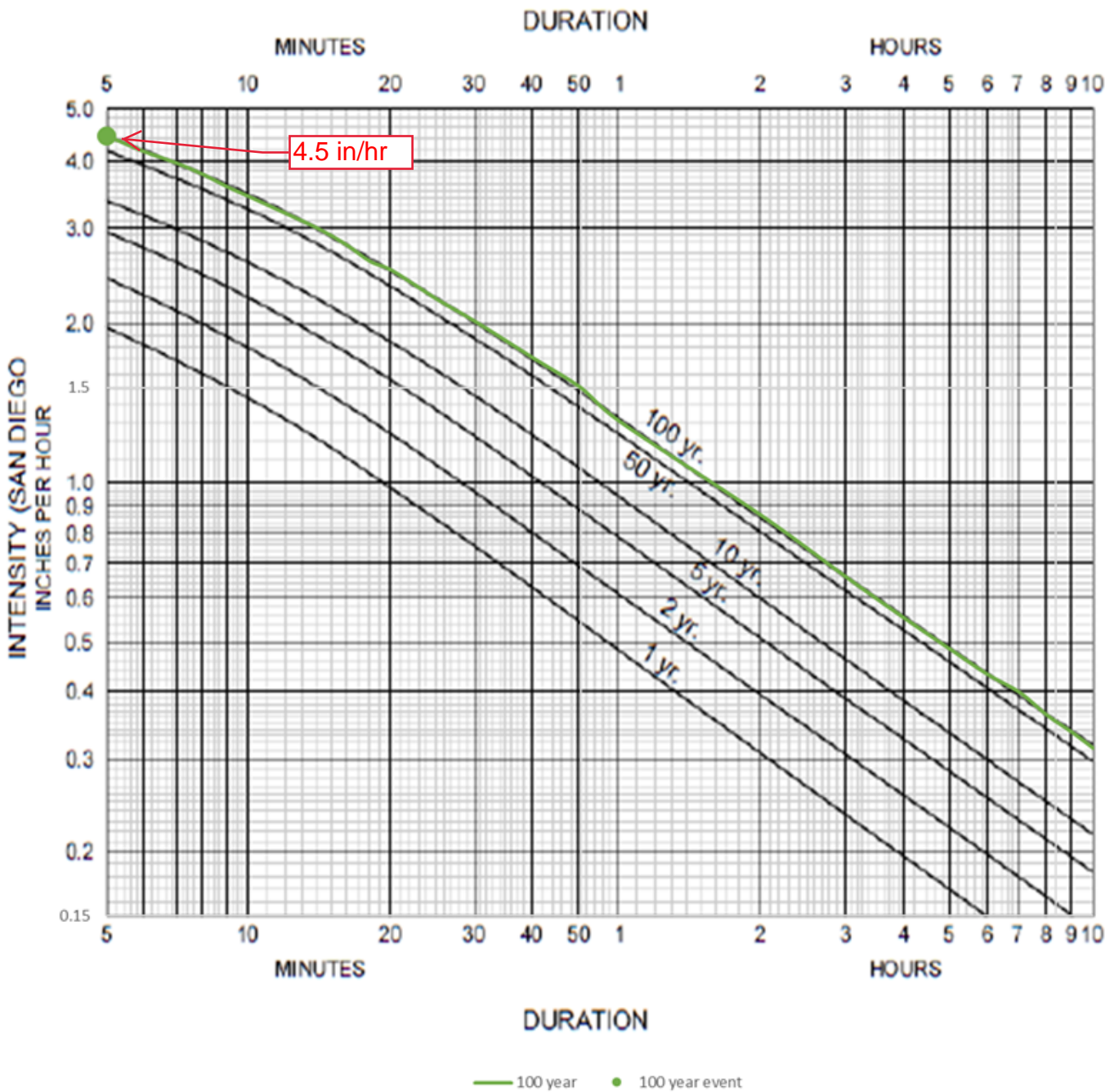
100-year Rainfall Intensity

Figure A-1. Intensity-Duration-Frequency Design Chart

ELEV.	FACTOR
0-1500	1.00
1500-3000	1.25
3000-4000	1.42
4000-5000	1.60
5000-6000	1.7
DESERT	1.25

TO OBTAIN CORRECT INTENSITY, MULTIPLY INTENSITY ON CHART BY FACTOR FOR DESIGN ELEVATION.

INTENSITY - DURATION - FREQUENCY CURVES FOR COUNTY OF SAN DIEGO RAINFALL



— 100 year    ● 100 year event

**Existing System B***City of San Diego 100-Year Duration=7 min, Inten=4.00 in/hr*

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Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**Pond B1: B1**Peak Elev=52.29' Storage=8,556 cf Inflow=38.88 cfs 0.374 af  
24.0" Round Culvert n=0.013 L=758.6' S=0.0025 '/ Outflow=12.78 cfs 0.374 af**Pond B2: B2**Peak Elev=49.51' Inflow=12.78 cfs 0.374 af  
34.0" Round Culvert n=0.013 L=130.0' S=0.0011 '/ Outflow=12.78 cfs 0.374 af**Subcatchment EB1: E-B1**Runoff Area=10.110 ac 100.00% Impervious Runoff Depth=0.44"  
Tc=6.6 min C=0.95 Runoff=38.88 cfs 0.374 af**Pond OB: OUTFALL B**Inflow=12.78 cfs 0.374 af  
Primary=12.78 cfs 0.374 af**Total Runoff Area = 10.110 ac Runoff Volume = 0.374 af Average Runoff Depth = 0.44"**  
**0.00% Pervious = 0.000 ac 100.00% Impervious = 10.110 ac**

**Existing System B**

City of San Diego 100-Year Duration=7 min, Inten=4.00 in/hr

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**Summary for Pond B1: B1**

Inflow Area = 10.110 ac, 100.00% Impervious, Inflow Depth = 0.44" for 100-Year event  
 Inflow = 38.88 cfs @ 0.11 hrs, Volume= 0.374 af  
 Outflow = 12.78 cfs @ 0.19 hrs, Volume= 0.374 af, Atten= 67%, Lag= 4.8 min  
 Primary = 12.78 cfs @ 0.19 hrs, Volume= 0.374 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
 Peak Elev= 52.29' @ 0.19 hrs Surf.Area= 25,394 sf Storage= 8,556 cf

Plug-Flow detention time= 6.3 min calculated for 0.373 af (100% of inflow)  
 Center-of-Mass det. time= 6.3 min ( 13.1 - 6.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	51.00'	141,125 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
51.00	100	0	0
52.00	7,383	3,742	3,742
54.00	130,000	137,383	141,125

Device	Routing	Invert	Outlet Devices
#1	Primary	49.75'	<b>24.0" Round CB B1</b> L= 758.6' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 49.75' / 47.88' S= 0.0025 ' / Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=12.78 cfs @ 0.19 hrs HW=52.29' TW=49.51' (Dynamic Tailwater)  
 ↑**1=CB B1** (Barrel Controls 12.78 cfs @ 4.14 fps)

**Summary for Pond B2: B2**

Inflow Area = 10.110 ac, 100.00% Impervious, Inflow Depth = 0.44" for 100-Year event  
 Inflow = 12.78 cfs @ 0.19 hrs, Volume= 0.374 af  
 Outflow = 12.78 cfs @ 0.19 hrs, Volume= 0.374 af, Atten= 0%, Lag= 0.0 min  
 Primary = 12.78 cfs @ 0.19 hrs, Volume= 0.374 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
 Peak Elev= 49.51' @ 0.19 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	47.60'	<b>34.0" Round Junction B2</b> L= 130.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 47.60' / 47.46' S= 0.0011 ' / Cc= 0.900 n= 0.013, Flow Area= 6.31 sf

**Primary OutFlow** Max=12.78 cfs @ 0.19 hrs HW=49.51' TW=0.00' (Dynamic Tailwater)  
 ↑**1=Junction B2** (Barrel Controls 12.78 cfs @ 3.99 fps)

**Existing System B**

City of San Diego 100-Year Duration=7 min, Inten=4.00 in/hr

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**Summary for Subcatchment EB1: E-B1**

Runoff = 38.88 cfs @ 0.11 hrs, Volume= 0.374 af, Depth= 0.44"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=7 min, Inten=4.00 in/hr

Area (ac)	C	Description
10.110	0.95	Paved parking, HSG D
10.110		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.6					<b>Direct Entry, EB1</b>

**Summary for Pond OB: OUTFALL B**

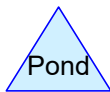
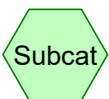
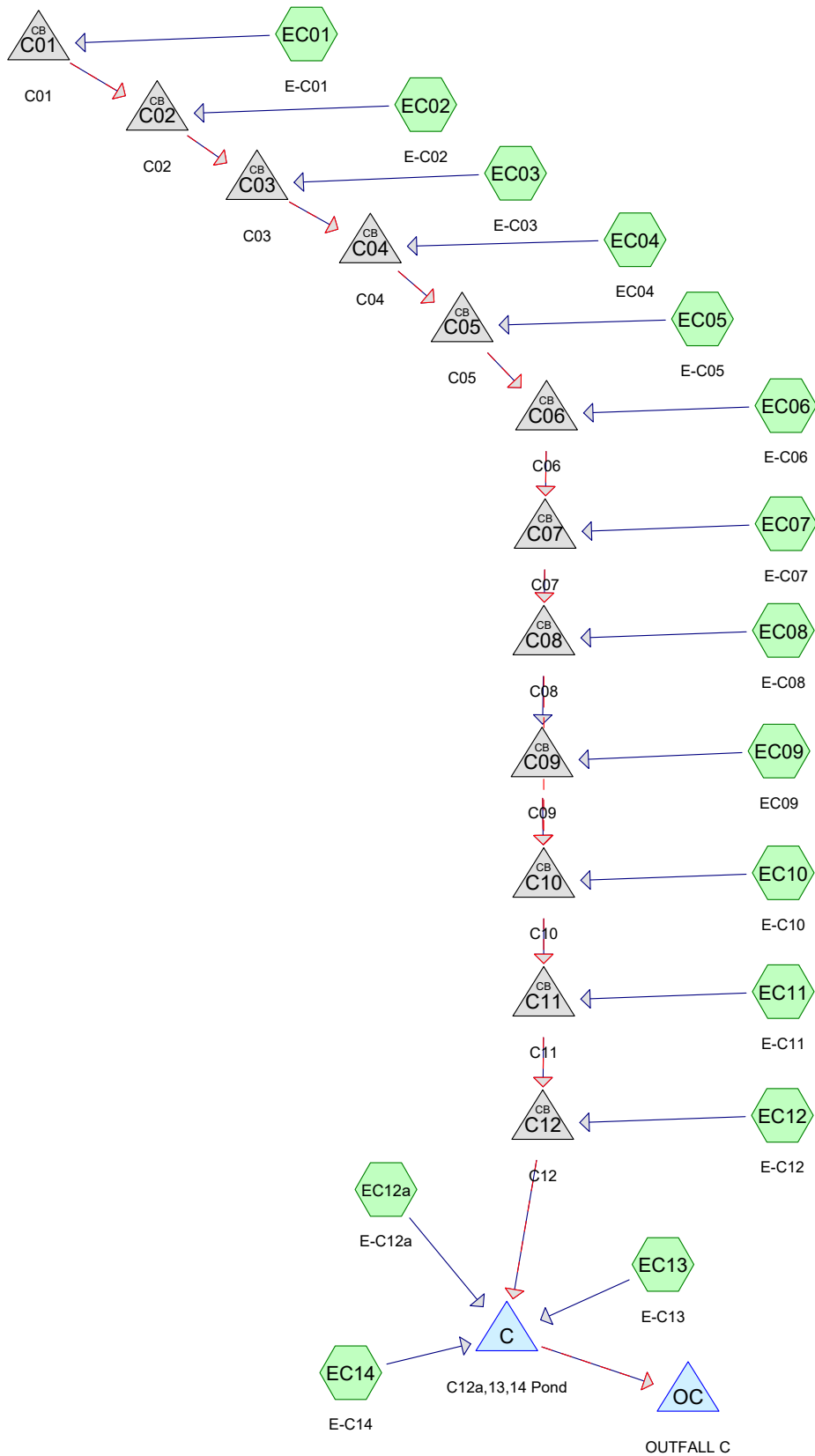
Inflow Area = 10.110 ac, 100.00% Impervious, Inflow Depth = 0.44" for 100-Year event

Inflow = 12.78 cfs @ 0.19 hrs, Volume= 0.374 af

Primary = 12.78 cfs @ 0.19 hrs, Volume= 0.374 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs





**Routing Diagram for Existing System C**  
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## Existing System C

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### Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	C	48.22	47.68	202.0	0.0027	0.013	34.0	0.0	0.0
2	C01	60.46	58.41	200.0	0.0103	0.013	24.0	0.0	0.0
3	C02	58.41	56.77	200.0	0.0082	0.013	24.0	0.0	0.0
4	C03	56.77	56.14	200.0	0.0032	0.013	24.0	0.0	0.0
5	C04	56.14	54.98	200.0	0.0058	0.013	24.0	0.0	0.0
6	C05	54.98	53.56	314.2	0.0045	0.013	24.0	0.0	0.0
7	C06	53.56	52.63	200.0	0.0046	0.013	30.0	0.0	0.0
8	C07	52.63	52.40	200.0	0.0012	0.013	30.0	0.0	0.0
9	C08	52.40	51.69	200.0	0.0036	0.013	30.0	0.0	0.0
10	C09	51.69	50.87	200.0	0.0041	0.013	30.0	0.0	0.0
11	C10	50.87	49.90	200.0	0.0048	0.013	30.0	0.0	0.0
12	C11	49.90	48.28	200.0	0.0081	0.013	36.0	0.0	0.0
13	C12	48.28	48.22	107.0	0.0006	0.013	36.0	0.0	0.0

**Existing System C**

City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

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Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points x 3

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Pond C: C12a,13,14 Pond</b>	Peak Elev=54.31' Storage=67,752 cf Inflow=199.84 cfs 2.755 af 34.0" Round Culvert n=0.013 L=202.0' S=0.0027 '/' Outflow=56.15 cfs 2.758 af
<b>Pond C01: C01</b>	Peak Elev=62.95' Inflow=11.99 cfs 0.165 af Primary=11.99 cfs 0.165 af Secondary=0.00 cfs 0.000 af Outflow=11.99 cfs 0.165 af
<b>Pond C02: C02</b>	Peak Elev=62.10' Inflow=20.55 cfs 0.283 af Primary=14.36 cfs 0.240 af Secondary=6.29 cfs 0.042 af Outflow=20.55 cfs 0.283 af
<b>Pond C03: C03</b>	Peak Elev=60.82' Inflow=32.75 cfs 0.451 af Primary=9.66 cfs 0.193 af Secondary=23.91 cfs 0.258 af Outflow=32.75 cfs 0.451 af
<b>Pond C04: C04</b>	Peak Elev=60.33' Inflow=45.39 cfs 0.625 af Primary=12.03 cfs 0.253 af Secondary=33.94 cfs 0.372 af Outflow=45.39 cfs 0.625 af
<b>Pond C05: C05</b>	Peak Elev=59.51' Inflow=52.96 cfs 0.729 af Primary=12.66 cfs 0.223 af Secondary=43.99 cfs 0.506 af Outflow=52.96 cfs 0.729 af
<b>Pond C06: C06</b>	Peak Elev=58.83' Inflow=67.84 cfs 0.934 af Primary=16.69 cfs 0.318 af Secondary=54.05 cfs 0.616 af Outflow=67.84 cfs 0.934 af
<b>Pond C07: C07</b>	Peak Elev=58.42' Inflow=91.22 cfs 1.255 af Primary=17.59 cfs 0.394 af Secondary=74.19 cfs 0.861 af Outflow=91.22 cfs 1.255 af
<b>Pond C08: C08</b>	Peak Elev=57.80' Inflow=103.49 cfs 1.423 af Primary=18.98 cfs 0.419 af Secondary=85.15 cfs 1.004 af Outflow=103.49 cfs 1.423 af
<b>Pond C09: C09</b>	Peak Elev=57.07' Inflow=31.26 cfs 0.596 af Primary=18.03 cfs 0.203 af Secondary=26.79 cfs 0.393 af Outflow=31.26 cfs 0.596 af
<b>Pond C10: C10</b>	Peak Elev=57.03' Inflow=131.86 cfs 1.813 af Primary=21.03 cfs 0.447 af Secondary=112.31 cfs 1.366 af Outflow=131.86 cfs 1.813 af
<b>Pond C11: C11</b>	Peak Elev=56.22' Inflow=144.10 cfs 1.981 af Primary=34.09 cfs 0.697 af Secondary=110.16 cfs 1.284 af Outflow=144.10 cfs 1.981 af
<b>Pond C12: C12</b>	Peak Elev=55.16' Inflow=150.72 cfs 2.072 af Primary=35.66 cfs 0.601 af Secondary=115.47 cfs 1.470 af Outflow=150.72 cfs 2.072 af
<b>Subcatchment EC01: E-C01</b>	Runoff Area=3.850 ac 100.00% Impervious Runoff Depth=0.51" Tc=7.4 min C=0.95 Runoff=11.99 cfs 0.165 af
<b>Subcatchment EC02: E-C02</b>	Runoff Area=2.750 ac 100.00% Impervious Runoff Depth=0.51" Tc=7.2 min C=0.95 Runoff=8.56 cfs 0.118 af
<b>Subcatchment EC03: E-C03</b>	Runoff Area=3.920 ac 100.00% Impervious Runoff Depth=0.51" Tc=7.6 min C=0.95 Runoff=12.20 cfs 0.168 af
<b>Subcatchment EC04: EC04</b>	Runoff Area=4.060 ac 100.00% Impervious Runoff Depth=0.51" Tc=7.8 min C=0.95 Runoff=12.64 cfs 0.174 af

**Existing System C***City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr*

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<b>Subcatchment EC05: E-C05</b>	Runoff Area=2.430 ac 100.00% Impervious Runoff Depth=0.51" Tc=8.2 min C=0.95 Runoff=7.57 cfs 0.104 af
<b>Subcatchment EC06: E-C06</b>	Runoff Area=4.780 ac 100.00% Impervious Runoff Depth=0.51" Tc=7.2 min C=0.95 Runoff=14.88 cfs 0.205 af
<b>Subcatchment EC07: E-C07</b>	Runoff Area=7.480 ac 100.00% Impervious Runoff Depth=0.51" Tc=9.3 min C=0.95 Runoff=23.29 cfs 0.321 af
<b>Subcatchment EC08: E-C08</b>	Runoff Area=3.930 ac 100.00% Impervious Runoff Depth=0.51" Tc=8.0 min C=0.95 Runoff=12.23 cfs 0.168 af
<b>Subcatchment EC09: EC09</b>	Runoff Area=4.120 ac 100.00% Impervious Runoff Depth=0.51" Tc=8.1 min C=0.95 Runoff=12.83 cfs 0.177 af
<b>Subcatchment EC10: E-C10</b>	Runoff Area=4.970 ac 100.00% Impervious Runoff Depth=0.51" Tc=8.2 min C=0.95 Runoff=15.47 cfs 0.213 af
<b>Subcatchment EC11: E-C11</b>	Runoff Area=3.920 ac 100.00% Impervious Runoff Depth=0.51" Tc=8.3 min C=0.95 Runoff=12.20 cfs 0.168 af
<b>Subcatchment EC12: E-C12</b>	Runoff Area=2.120 ac 100.00% Impervious Runoff Depth=0.51" Tc=7.3 min C=0.95 Runoff=6.60 cfs 0.091 af
<b>Subcatchment EC12a: E-C12a</b>	Runoff Area=5.560 ac 100.00% Impervious Runoff Depth=0.51" Tc=8.4 min C=0.95 Runoff=17.31 cfs 0.238 af
<b>Subcatchment EC13: E-C13</b>	Runoff Area=10.350 ac 100.00% Impervious Runoff Depth=0.51" Tc=9.7 min C=0.95 Runoff=32.04 cfs 0.444 af
<b>Subcatchment EC14: E-C14</b>	Runoff Area=0.040 ac 100.00% Impervious Runoff Depth=0.51" Tc=5.0 min C=0.95 Runoff=0.12 cfs 0.002 af
<b>Pond OC: OUTFALL C</b>	Inflow=56.15 cfs 2.758 af Primary=56.15 cfs 2.758 af

**Total Runoff Area = 64.280 ac Runoff Volume = 2.755 af Average Runoff Depth = 0.51"**  
**0.00% Pervious = 0.000 ac 100.00% Impervious = 64.280 ac**

50-year Rainfall Intensity

ELEV.	FACTOR
0-1500	1.00
1500-3000	1.25
3000-4000	1.42
4000-5000	1.60
5000-6000	1.7
DESERT	1.25

TO OBTAIN CORRECT INTENSITY, MULTIPLY INTENSITY ON CHART BY FACTOR FOR DESIGN ELEVATION.

INTENSITY - DURATION - FREQUENCY CURVES FOR COUNTY OF SAN DIEGO RAINFALL

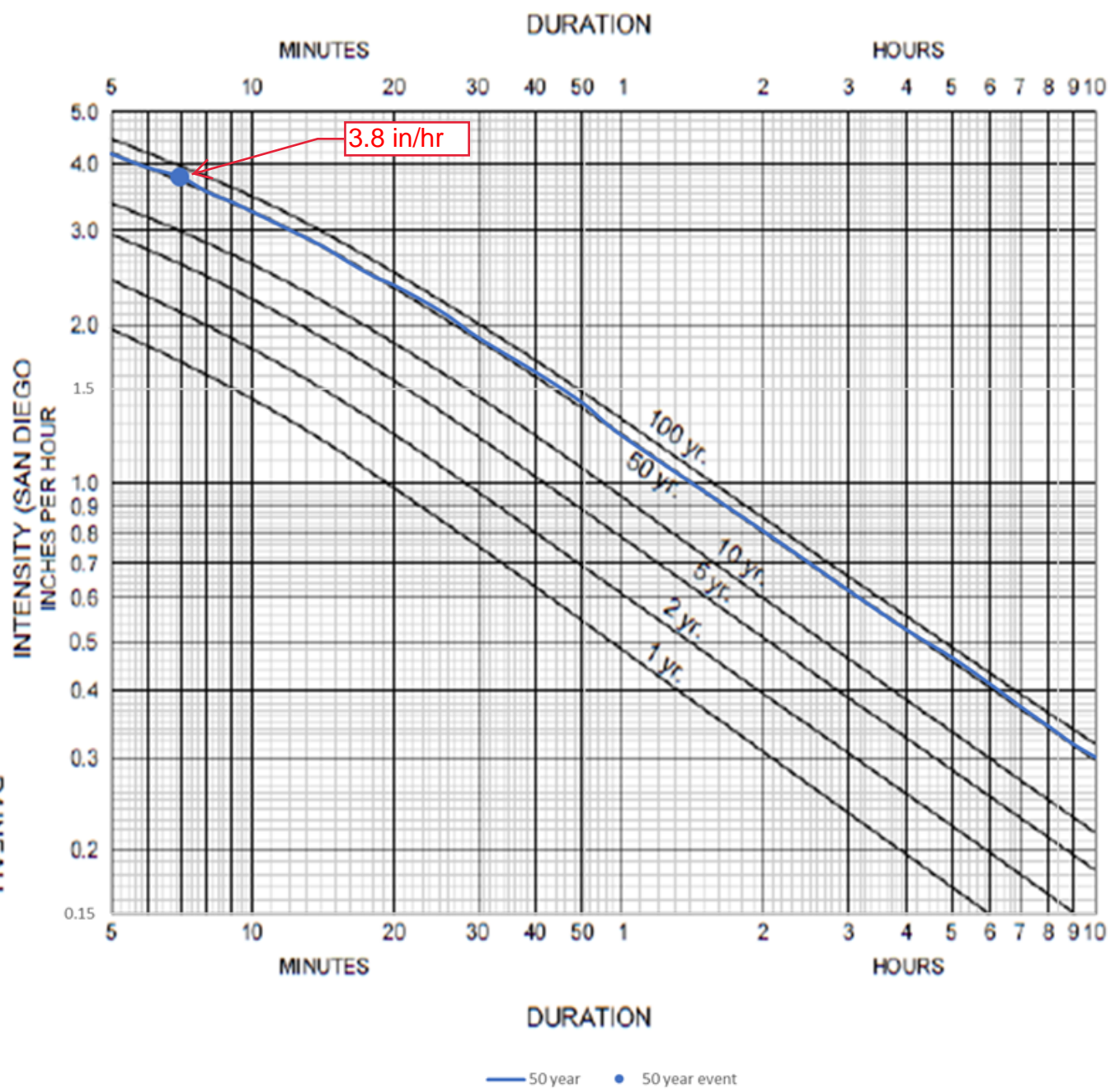


Figure A-1. Intensity-Duration-Frequency Design Chart

— 50 year • 50 year event

**Existing System C**

City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

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**Summary for Pond C: C12a,13,14 Pond**

Inflow Area = 64.280 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 199.84 cfs @ 0.16 hrs, Volume= 2.755 af  
 Outflow = 56.15 cfs @ 0.27 hrs, Volume= 2.758 af, Atten= 72%, Lag= 6.4 min  
 Primary = 56.15 cfs @ 0.27 hrs, Volume= 2.758 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 54.31' @ 0.27 hrs Surf.Area= 151,007 sf Storage= 67,752 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 10.5 min ( 19.6 - 9.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	52.72'	1,636,408 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
52.72	0	0	0
53.53	100	41	41
54.00	113,857	26,780	26,820
56.00	354,249	468,106	494,926
58.00	787,233	1,141,482	1,636,408

Device	Routing	Invert	Outlet Devices
#1	Primary	48.22'	<b>34.0" Round CB C14-Out</b> L= 202.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 48.22' / 47.68' S= 0.0027 '/ Cc= 0.900 n= 0.013, Flow Area= 6.31 sf

**Primary OutFlow** Max=56.14 cfs @ 0.27 hrs HW=54.31' TW=0.00' (Dynamic Tailwater)  
 ←**1=CB C14-Out** (Barrel Controls 56.14 cfs @ 8.90 fps)

**Summary for Pond C01: C01**

Inflow Area = 3.850 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 11.99 cfs @ 0.13 hrs, Volume= 0.165 af  
 Outflow = 11.99 cfs @ 0.13 hrs, Volume= 0.165 af, Atten= 0%, Lag= 0.0 min  
 Primary = 11.99 cfs @ 0.13 hrs, Volume= 0.165 af  
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 62.95' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	60.46'	<b>24.0" Round CB C01</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 60.46' / 58.41' S= 0.0103 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Secondary	63.92'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Existing System C**

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**Primary OutFlow** Max=11.98 cfs @ 0.13 hrs HW=62.95' TW=62.10' (Dynamic Tailwater)  
 ↑**1=CB C01** (Outlet Controls 11.98 cfs @ 3.93 fps)

**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=60.46' TW=58.41' (Dynamic Tailwater)  
 ↑**2=Broad-Crested Rectangular Weir** ( Controls 0.00 cfs)

**Summary for Pond C02: C02**

Inflow Area = 6.600 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 20.55 cfs @ 0.13 hrs, Volume= 0.283 af  
 Outflow = 20.55 cfs @ 0.13 hrs, Volume= 0.283 af, Atten= 0%, Lag= 0.0 min  
 Primary = 14.36 cfs @ 0.11 hrs, Volume= 0.240 af  
 Secondary = 6.29 cfs @ 0.16 hrs, Volume= 0.042 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 62.10' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	58.41'	<b>24.0" Round CB C02</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 58.41' / 56.77' S= 0.0082 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Secondary	61.86'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=14.35 cfs @ 0.11 hrs HW=62.04' TW=60.75' (Dynamic Tailwater)  
 ↑**1=CB C02** (Outlet Controls 14.35 cfs @ 4.57 fps)

**Secondary OutFlow** Max=6.29 cfs @ 0.16 hrs HW=62.10' TW=60.82' (Dynamic Tailwater)  
 ↑**2=Broad-Crested Rectangular Weir** (Weir Controls 6.29 cfs @ 1.31 fps)

**Summary for Pond C03: C03**

Inflow Area = 10.520 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 32.75 cfs @ 0.13 hrs, Volume= 0.451 af  
 Outflow = 32.75 cfs @ 0.14 hrs, Volume= 0.451 af, Atten= 0%, Lag= 0.6 min  
 Primary = 9.66 cfs @ 0.04 hrs, Volume= 0.193 af  
 Secondary = 23.91 cfs @ 0.16 hrs, Volume= 0.258 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 60.82' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.77'	<b>24.0" Round CB C03</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.77' / 56.14' S= 0.0032 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Secondary	60.23'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Existing System C**

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**Primary OutFlow** Max=9.60 cfs @ 0.04 hrs HW=60.30' TW=59.72' (Dynamic Tailwater)↑**1=CB C03** (Outlet Controls 9.60 cfs @ 3.05 fps)**Secondary OutFlow** Max=23.91 cfs @ 0.16 hrs HW=60.82' TW=60.33' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Weir Controls 23.91 cfs @ 2.02 fps)**Summary for Pond C04: C04**

Inflow Area = 14.580 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 45.39 cfs @ 0.14 hrs, Volume= 0.625 af  
 Outflow = 45.39 cfs @ 0.15 hrs, Volume= 0.625 af, Atten= 0%, Lag= 0.6 min  
 Primary = 12.03 cfs @ 0.05 hrs, Volume= 0.253 af  
 Secondary = 33.94 cfs @ 0.16 hrs, Volume= 0.372 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 60.33' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.14'	<b>24.0" Round CB C04</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.14' / 54.98' S= 0.0058 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Secondary	59.59'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=12.01 cfs @ 0.05 hrs HW=59.82' TW=58.92' (Dynamic Tailwater)↑**1=CB C04** (Outlet Controls 12.01 cfs @ 3.82 fps)**Secondary OutFlow** Max=33.94 cfs @ 0.16 hrs HW=60.33' TW=59.51' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Weir Controls 33.94 cfs @ 2.29 fps)**Summary for Pond C05: C05**

Inflow Area = 17.010 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 52.96 cfs @ 0.15 hrs, Volume= 0.729 af  
 Outflow = 52.96 cfs @ 0.15 hrs, Volume= 0.729 af, Atten= 0%, Lag= 0.0 min  
 Primary = 12.66 cfs @ 0.03 hrs, Volume= 0.223 af  
 Secondary = 43.99 cfs @ 0.16 hrs, Volume= 0.506 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 59.51' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	54.98'	<b>24.0" Round CB C05</b> L= 314.2' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 54.98' / 53.56' S= 0.0045 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Secondary	58.59'	<b>20.0' long x 314.2' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63



**Existing System C**

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**Primary OutFlow** Max=8.55 cfs @ 0.03 hrs HW=58.14' TW=57.52' (Dynamic Tailwater)↑**1=CB C05** (Outlet Controls 8.55 cfs @ 2.72 fps)**Secondary OutFlow** Max=43.96 cfs @ 0.16 hrs HW=59.51' TW=58.83' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Weir Controls 43.96 cfs @ 2.39 fps)**Summary for Pond C06: C06**

Inflow Area = 21.790 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 67.84 cfs @ 0.15 hrs, Volume= 0.934 af  
 Outflow = 67.84 cfs @ 0.15 hrs, Volume= 0.934 af, Atten= 0%, Lag= 0.0 min  
 Primary = 16.69 cfs @ 0.03 hrs, Volume= 0.318 af  
 Secondary = 54.05 cfs @ 0.16 hrs, Volume= 0.616 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 58.83' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	53.56'	<b>30.0" Round CB C6</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 53.56' / 52.63' S= 0.0046 '/' Cc= 0.900 n= 0.013, Flow Area= 4.91 sf
#2	Secondary	57.58'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=13.41 cfs @ 0.03 hrs HW=57.58' TW=57.19' (Dynamic Tailwater)↑**1=CB C6** (Outlet Controls 13.41 cfs @ 2.73 fps)**Secondary OutFlow** Max=53.99 cfs @ 0.16 hrs HW=58.83' TW=58.42' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Weir Controls 53.99 cfs @ 2.17 fps)**Summary for Pond C07: C07**

Inflow Area = 29.270 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 91.22 cfs @ 0.16 hrs, Volume= 1.255 af  
 Outflow = 91.22 cfs @ 0.16 hrs, Volume= 1.255 af, Atten= 0%, Lag= 0.0 min  
 Primary = 17.59 cfs @ 0.09 hrs, Volume= 0.394 af  
 Secondary = 74.19 cfs @ 0.16 hrs, Volume= 0.861 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 58.42' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	52.63'	<b>30.0" Round CB C07</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 52.63' / 52.40' S= 0.0012 '/' Cc= 0.900 n= 0.013, Flow Area= 4.91 sf
#2	Secondary	56.97'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Existing System C**

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**Primary OutFlow** Max=17.39 cfs @ 0.09 hrs HW=57.91' TW=57.26' (Dynamic Tailwater)  
 ↑**1=CB C07** (Outlet Controls 17.39 cfs @ 3.54 fps)

**Secondary OutFlow** Max=73.83 cfs @ 0.16 hrs HW=58.41' TW=57.79' (Dynamic Tailwater)  
 ↑**2=Broad-Crested Rectangular Weir** (Weir Controls 73.83 cfs @ 2.56 fps)

**Summary for Pond C08: C08**

Inflow Area = 33.200 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 103.49 cfs @ 0.16 hrs, Volume= 1.423 af  
 Outflow = 103.49 cfs @ 0.16 hrs, Volume= 1.423 af, Atten= 0%, Lag= 0.0 min  
 Primary = 18.98 cfs @ 0.09 hrs, Volume= 0.419 af  
 Secondary = 85.15 cfs @ 0.16 hrs, Volume= 1.004 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 57.80' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	52.40'	<b>30.0" Round CB C08</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 52.40' / 51.69' S= 0.0036 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf
#2	Secondary	56.25'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=18.69 cfs @ 0.09 hrs HW=57.27' TW=56.52' (Dynamic Tailwater)  
 ↑**1=CB C08** (Outlet Controls 18.69 cfs @ 3.81 fps)

**Secondary OutFlow** Max=84.81 cfs @ 0.16 hrs HW=57.79' TW=57.03' (Dynamic Tailwater)  
 ↑**2=Broad-Crested Rectangular Weir** (Weir Controls 84.81 cfs @ 2.75 fps)

**Summary for Pond C09: C09**

Inflow Area = 37.320 ac, 100.00% Impervious, Inflow Depth = 0.19" for 50-Year event  
 Inflow = 31.26 cfs @ 0.14 hrs, Volume= 0.596 af  
 Outflow = 31.26 cfs @ 0.14 hrs, Volume= 0.596 af, Atten= 0%, Lag= 0.0 min  
 Primary = 18.03 cfs @ 0.02 hrs, Volume= 0.203 af  
 Secondary = 26.79 cfs @ 0.16 hrs, Volume= 0.393 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 57.07' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	51.69'	<b>30.0" Round CB C09</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 51.69' / 50.87' S= 0.0041 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf
#2	Secondary	55.64'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

## Existing System C

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**Primary OutFlow** Max=12.61 cfs @ 0.02 hrs HW=55.06' TW=54.71' (Dynamic Tailwater)

↳**1=CB C09** (Outlet Controls 12.61 cfs @ 2.57 fps)

**Secondary OutFlow** Max=26.53 cfs @ 0.16 hrs HW=57.07' TW=57.03' (Dynamic Tailwater)

↳**2=Broad-Crested Rectangular Weir** (Weir Controls 26.53 cfs @ 0.93 fps)

### Summary for Pond C10: C10

Inflow Area = 42.290 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
Inflow = 131.86 cfs @ 0.16 hrs, Volume= 1.813 af  
Outflow = 131.86 cfs @ 0.16 hrs, Volume= 1.813 af, Atten= 0%, Lag= 0.0 min  
Primary = 21.03 cfs @ 0.03 hrs, Volume= 0.447 af  
Secondary = 112.31 cfs @ 0.16 hrs, Volume= 1.366 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 57.03' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	50.87'	<b>30.0" Round CB C10</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 50.87' / 49.90' S= 0.0048 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf
#2	Secondary	55.12'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=20.74 cfs @ 0.03 hrs HW=55.43' TW=54.50' (Dynamic Tailwater)

↳**1=CB C10** (Outlet Controls 20.74 cfs @ 4.23 fps)

**Secondary OutFlow** Max=111.60 cfs @ 0.16 hrs HW=57.03' TW=56.21' (Dynamic Tailwater)

↳**2=Broad-Crested Rectangular Weir** (Weir Controls 111.60 cfs @ 2.92 fps)

### Summary for Pond C11: C11

Inflow Area = 46.210 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
Inflow = 144.10 cfs @ 0.16 hrs, Volume= 1.981 af  
Outflow = 144.10 cfs @ 0.16 hrs, Volume= 1.981 af, Atten= 0%, Lag= 0.0 min  
Primary = 34.09 cfs @ 0.14 hrs, Volume= 0.697 af  
Secondary = 110.16 cfs @ 0.16 hrs, Volume= 1.284 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 56.22' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	49.90'	<b>36.0" Round CB C11</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 49.90' / 48.28' S= 0.0081 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf
#2	Secondary	54.45'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Existing System C**

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**Primary OutFlow** Max=34.08 cfs @ 0.14 hrs HW=56.17' TW=55.10' (Dynamic Tailwater)  
 ↳1=CB C11 (Outlet Controls 34.08 cfs @ 4.82 fps)

**Secondary OutFlow** Max=109.73 cfs @ 0.16 hrs HW=56.21' TW=55.15' (Dynamic Tailwater)  
 ↳2=Broad-Crested Rectangular Weir (Weir Controls 109.73 cfs @ 3.12 fps)

**Summary for Pond C12: C12**

Inflow Area = 48.330 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 150.72 cfs @ 0.16 hrs, Volume= 2.072 af  
 Outflow = 150.72 cfs @ 0.16 hrs, Volume= 2.072 af, Atten= 0%, Lag= 0.0 min  
 Primary = 35.66 cfs @ 0.14 hrs, Volume= 0.601 af  
 Secondary = 115.47 cfs @ 0.16 hrs, Volume= 1.470 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 55.16' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	48.28'	<b>36.0" Round C12</b> L= 107.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 48.28' / 48.22' S= 0.0006 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf
#2	Secondary	53.33'	<b>20.0' long x 107.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=35.63 cfs @ 0.14 hrs HW=55.10' TW=54.00' (Dynamic Tailwater)  
 ↳1=C12 (Inlet Controls 35.63 cfs @ 5.04 fps)

**Secondary OutFlow** Max=115.13 cfs @ 0.16 hrs HW=55.16' TW=54.09' (Dynamic Tailwater)  
 ↳2=Broad-Crested Rectangular Weir (Weir Controls 115.13 cfs @ 3.15 fps)

**Summary for Subcatchment EC01: E-C01**

Runoff = 11.99 cfs @ 0.13 hrs, Volume= 0.165 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
 City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
3.850	0.95	Paved parking, HSG D
3.850		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.4					<b>Direct Entry, EC01</b>

**Existing System C**

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**Summary for Subcatchment EC02: E-C02**

Runoff = 8.56 cfs @ 0.12 hrs, Volume= 0.118 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
2.750	0.95	Paved parking, HSG D
2.750		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.2					Direct Entry, E-C02

**Summary for Subcatchment EC03: E-C03**

Runoff = 12.20 cfs @ 0.13 hrs, Volume= 0.168 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
3.920	0.95	Paved parking, HSG D
3.920		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.6					Direct Entry, E-C03

**Summary for Subcatchment EC04: EC04**

Runoff = 12.64 cfs @ 0.13 hrs, Volume= 0.174 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
4.060	0.95	Paved parking, HSG D
4.060		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.8					Direct Entry, EC04

**Summary for Subcatchment EC05: E-C05**

Runoff = 7.57 cfs @ 0.14 hrs, Volume= 0.104 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

**Existing System C**

City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

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Area (ac)	C	Description
2.430	0.95	Paved parking, HSG D
2.430		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.2					<b>Direct Entry, EC05</b>

**Summary for Subcatchment EC06: E-C06**

Runoff = 14.88 cfs @ 0.12 hrs, Volume= 0.205 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
4.780	0.95	Paved parking, HSG D
4.780		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.2					<b>Direct Entry, E-C6</b>

**Summary for Subcatchment EC07: E-C07**

Runoff = 23.29 cfs @ 0.16 hrs, Volume= 0.321 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
7.480	0.95	Paved parking, HSG D
7.480		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.3					<b>Direct Entry, E-A07</b>

**Summary for Subcatchment EC08: E-C08**

Runoff = 12.23 cfs @ 0.14 hrs, Volume= 0.168 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
3.930	0.95	Paved parking, HSG D
3.930		100.00% Impervious Area

**Existing System C**

City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					<b>Direct Entry, E-C08</b>

**Summary for Subcatchment EC09: EC09**

Runoff = 12.83 cfs @ 0.14 hrs, Volume= 0.177 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
4.120	0.95	Paved parking, HSG D
4.120		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.1					<b>Direct Entry, E-C09</b>

**Summary for Subcatchment EC10: E-C10**

Runoff = 15.47 cfs @ 0.14 hrs, Volume= 0.213 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
4.970	0.95	Paved parking, HSG D
4.970		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.2					<b>Direct Entry, E-C10</b>

**Summary for Subcatchment EC11: E-C11**

Runoff = 12.20 cfs @ 0.14 hrs, Volume= 0.168 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
3.920	0.95	Paved parking, HSG D
3.920		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.3					<b>Direct Entry, E-C11</b>

**Existing System C**

City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

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**Summary for Subcatchment EC12: E-C12**

Runoff = 6.60 cfs @ 0.13 hrs, Volume= 0.091 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
2.120	0.95	Paved parking, HSG D
2.120		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.3					Direct Entry, E-C12

**Summary for Subcatchment EC12a: E-C12a**

Runoff = 17.31 cfs @ 0.14 hrs, Volume= 0.238 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
5.560	0.95	Paved parking, HSG D
5.560		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.4					Direct Entry, E-C12

**Summary for Subcatchment EC13: E-C13**

Runoff = 32.04 cfs @ 0.16 hrs, Volume= 0.444 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

Area (ac)	C	Description
10.350	0.95	Paved parking, HSG D
10.350		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.7					Direct Entry, E-A13

**Summary for Subcatchment EC14: E-C14**

Runoff = 0.12 cfs @ 0.09 hrs, Volume= 0.002 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr



**Existing System C**

City of San Diego 50-Year Duration=10 min, Inten=3.25 in/hr

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Area (ac)	C	Description
0.040	0.95	Paved parking, HSG D
0.040		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, EC14</b>

**Summary for Pond OC: OUTFALL C**

Inflow Area = 64.280 ac, 100.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
 Inflow = 56.15 cfs @ 0.27 hrs, Volume= 2.758 af  
 Primary = 56.15 cfs @ 0.27 hrs, Volume= 2.758 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

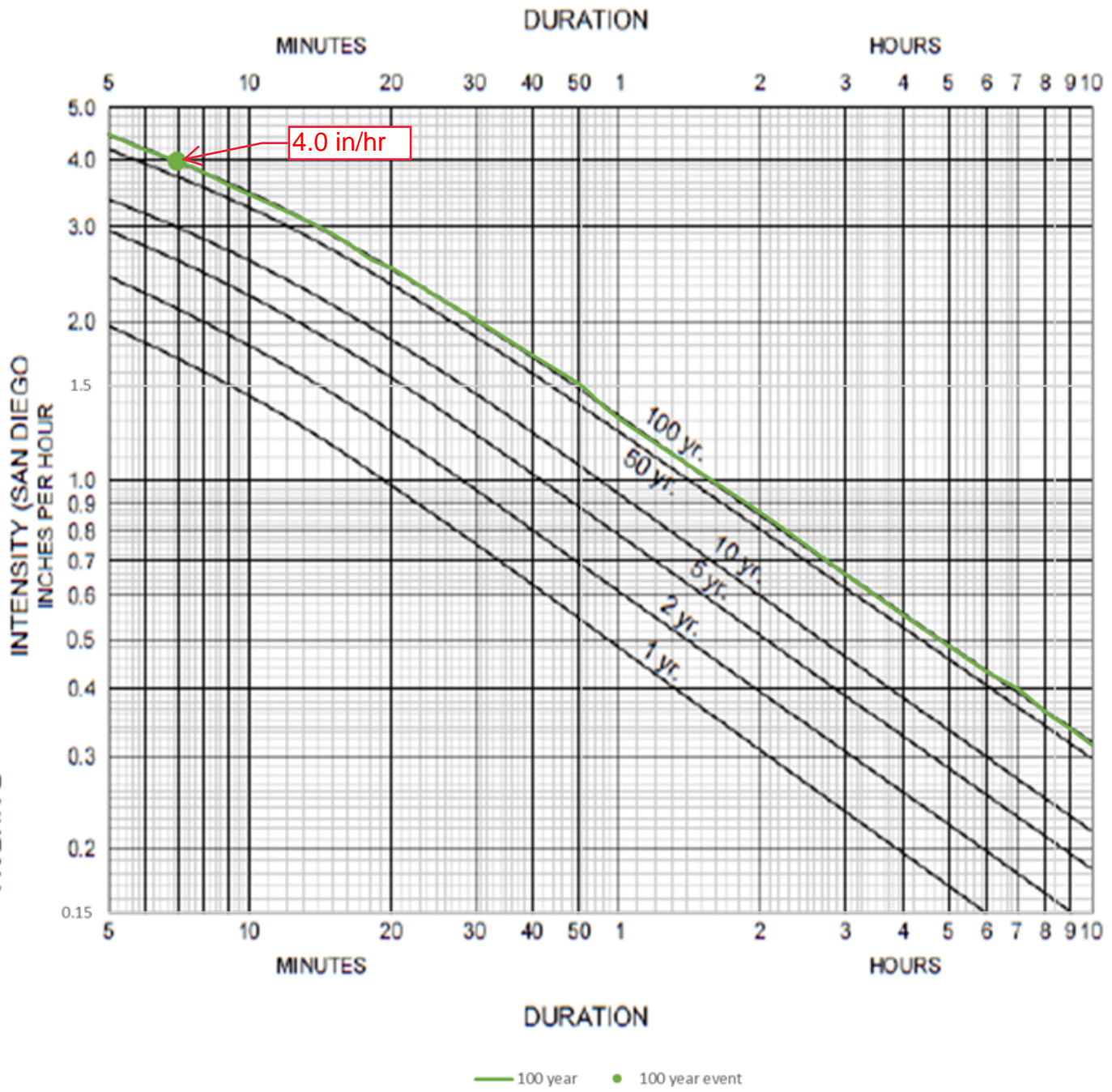
100-year Rainfall Intensity

Figure A-1. Intensity-Duration-Frequency Design Chart

ELEV.	FACTOR
0-1500	1.00
1500-3000	1.25
3000-4000	1.42
4000-5000	1.60
5000-6000	1.7
DESERT	1.25

TO OBTAIN CORRECT INTENSITY, MULTIPLY INTENSITY ON CHART BY FACTOR FOR DESIGN ELEVATION.

INTENSITY - DURATION - FREQUENCY CURVES FOR COUNTY OF SAN DIEGO RAINFALL



— 100 year    ● 100 year event

**Existing System C**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points x 3

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Pond C: C12a,13,14 Pond</b>	Peak Elev=54.35' Storage=74,536 cf Inflow=212.14 cfs 2.926 af 34.0" Round Culvert n=0.013 L=202.0' S=0.0027 '/' Outflow=56.47 cfs 2.945 af
<b>Pond C01: C01</b>	Peak Elev=63.15' Inflow=12.72 cfs 0.175 af Primary=12.72 cfs 0.175 af Secondary=0.00 cfs 0.000 af Outflow=12.72 cfs 0.175 af
<b>Pond C02: C02</b>	Peak Elev=62.13' Inflow=21.81 cfs 0.300 af Primary=14.36 cfs 0.246 af Secondary=7.61 cfs 0.054 af Outflow=21.81 cfs 0.300 af
<b>Pond C03: C03</b>	Peak Elev=60.86' Inflow=34.77 cfs 0.479 af Primary=9.63 cfs 0.194 af Secondary=25.96 cfs 0.284 af Outflow=34.77 cfs 0.479 af
<b>Pond C04: C04</b>	Peak Elev=60.38' Inflow=48.18 cfs 0.663 af Primary=12.03 cfs 0.254 af Secondary=36.88 cfs 0.410 af Outflow=48.18 cfs 0.663 af
<b>Pond C05: C05</b>	Peak Elev=59.58' Inflow=56.21 cfs 0.774 af Primary=13.37 cfs 0.223 af Secondary=47.42 cfs 0.551 af Outflow=56.21 cfs 0.774 af
<b>Pond C06: C06</b>	Peak Elev=58.92' Inflow=72.01 cfs 0.992 af Primary=16.38 cfs 0.318 af Secondary=58.35 cfs 0.674 af Outflow=72.01 cfs 0.992 af
<b>Pond C07: C07</b>	Peak Elev=58.52' Inflow=96.84 cfs 1.333 af Primary=17.61 cfs 0.395 af Secondary=79.86 cfs 0.937 af Outflow=96.84 cfs 1.333 af
<b>Pond C08: C08</b>	Peak Elev=57.90' Inflow=109.86 cfs 1.511 af Primary=19.00 cfs 0.422 af Secondary=91.56 cfs 1.089 af Outflow=109.86 cfs 1.511 af
<b>Pond C09: C09</b>	Peak Elev=57.18' Inflow=32.03 cfs 0.609 af Primary=18.90 cfs 0.197 af Secondary=27.77 cfs 0.413 af Outflow=32.03 cfs 0.609 af
<b>Pond C10: C10</b>	Peak Elev=57.15' Inflow=139.97 cfs 1.925 af Primary=22.24 cfs 0.449 af Secondary=120.33 cfs 1.476 af Outflow=139.97 cfs 1.925 af
<b>Pond C11: C11</b>	Peak Elev=56.32' Inflow=152.96 cfs 2.103 af Primary=34.40 cfs 0.703 af Secondary=118.71 cfs 1.400 af Outflow=152.96 cfs 2.103 af
<b>Pond C12: C12</b>	Peak Elev=55.25' Inflow=159.99 cfs 2.200 af Primary=36.59 cfs 0.616 af Secondary=123.84 cfs 1.584 af Outflow=159.99 cfs 2.200 af
<b>Subcatchment EC01: E-C01</b>	Runoff Area=3.850 ac 100.00% Impervious Runoff Depth=0.55" Tc=7.4 min C=0.95 Runoff=12.72 cfs 0.175 af
<b>Subcatchment EC02: E-C02</b>	Runoff Area=2.750 ac 100.00% Impervious Runoff Depth=0.55" Tc=7.2 min C=0.95 Runoff=9.09 cfs 0.125 af
<b>Subcatchment EC03: E-C03</b>	Runoff Area=3.920 ac 100.00% Impervious Runoff Depth=0.55" Tc=7.6 min C=0.95 Runoff=12.95 cfs 0.178 af
<b>Subcatchment EC04: EC04</b>	Runoff Area=4.060 ac 100.00% Impervious Runoff Depth=0.55" Tc=7.8 min C=0.95 Runoff=13.42 cfs 0.185 af

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<b>Subcatchment EC05: E-C05</b>	Runoff Area=2.430 ac 100.00% Impervious Runoff Depth=0.55" Tc=8.2 min C=0.95 Runoff=8.03 cfs 0.111 af
<b>Subcatchment EC06: E-C06</b>	Runoff Area=4.780 ac 100.00% Impervious Runoff Depth=0.55" Tc=7.2 min C=0.95 Runoff=15.80 cfs 0.218 af
<b>Subcatchment EC07: E-C07</b>	Runoff Area=7.480 ac 100.00% Impervious Runoff Depth=0.55" Tc=9.3 min C=0.95 Runoff=24.73 cfs 0.340 af
<b>Subcatchment EC08: E-C08</b>	Runoff Area=3.930 ac 100.00% Impervious Runoff Depth=0.55" Tc=8.0 min C=0.95 Runoff=12.99 cfs 0.179 af
<b>Subcatchment EC09: EC09</b>	Runoff Area=4.120 ac 100.00% Impervious Runoff Depth=0.55" Tc=8.1 min C=0.95 Runoff=13.62 cfs 0.187 af
<b>Subcatchment EC10: E-C10</b>	Runoff Area=4.970 ac 100.00% Impervious Runoff Depth=0.55" Tc=8.2 min C=0.95 Runoff=16.42 cfs 0.226 af
<b>Subcatchment EC11: E-C11</b>	Runoff Area=3.920 ac 100.00% Impervious Runoff Depth=0.55" Tc=8.3 min C=0.95 Runoff=12.95 cfs 0.178 af
<b>Subcatchment EC12: E-C12</b>	Runoff Area=2.120 ac 100.00% Impervious Runoff Depth=0.55" Tc=7.3 min C=0.95 Runoff=7.01 cfs 0.096 af
<b>Subcatchment EC12a: E-C12a</b>	Runoff Area=5.560 ac 100.00% Impervious Runoff Depth=0.55" Tc=8.4 min C=0.95 Runoff=18.37 cfs 0.253 af
<b>Subcatchment EC13: E-C13</b>	Runoff Area=10.350 ac 100.00% Impervious Runoff Depth=0.55" Tc=9.7 min C=0.95 Runoff=34.01 cfs 0.471 af
<b>Subcatchment EC14: E-C14</b>	Runoff Area=0.040 ac 100.00% Impervious Runoff Depth=0.55" Tc=5.0 min C=0.95 Runoff=0.13 cfs 0.002 af
<b>Pond OC: OUTFALL C</b>	Inflow=56.47 cfs 2.945 af Primary=56.47 cfs 2.945 af

**Total Runoff Area = 64.280 ac Runoff Volume = 2.925 af Average Runoff Depth = 0.55"**  
**0.00% Pervious = 0.000 ac 100.00% Impervious = 64.280 ac**

**Existing System C**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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**Summary for Pond C: C12a,13,14 Pond**

Inflow Area = 64.280 ac, 100.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 212.14 cfs @ 0.16 hrs, Volume= 2.926 af  
 Outflow = 56.47 cfs @ 0.27 hrs, Volume= 2.945 af, Atten= 73%, Lag= 6.5 min  
 Primary = 56.47 cfs @ 0.27 hrs, Volume= 2.945 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 54.35' @ 0.27 hrs Surf.Area= 156,313 sf Storage= 74,536 cf

Plug-Flow detention time= 11.4 min calculated for 2.926 af (100% of inflow)  
 Center-of-Mass det. time= 11.6 min ( 20.7 - 9.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	52.72'	1,636,408 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
52.72	0	0	0
53.53	100	41	41
54.00	113,857	26,780	26,820
56.00	354,249	468,106	494,926
58.00	787,233	1,141,482	1,636,408

Device	Routing	Invert	Outlet Devices
#1	Primary	48.22'	<b>34.0" Round CB C14-Out</b> L= 202.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 48.22' / 47.68' S= 0.0027 '/ Cc= 0.900 n= 0.013, Flow Area= 6.31 sf

**Primary OutFlow** Max=56.47 cfs @ 0.27 hrs HW=54.35' TW=0.00' (Dynamic Tailwater)  
 ←**1=CB C14-Out** (Barrel Controls 56.47 cfs @ 8.96 fps)

**Summary for Pond C01: C01**

Inflow Area = 3.850 ac, 100.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 12.72 cfs @ 0.13 hrs, Volume= 0.175 af  
 Outflow = 12.72 cfs @ 0.13 hrs, Volume= 0.175 af, Atten= 0%, Lag= 0.0 min  
 Primary = 12.72 cfs @ 0.13 hrs, Volume= 0.175 af  
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 63.15' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	60.46'	<b>24.0" Round CB C01</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 60.46' / 58.41' S= 0.0103 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Secondary	63.92'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Existing System C**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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**Primary OutFlow** Max=12.72 cfs @ 0.13 hrs HW=63.15' TW=62.13' (Dynamic Tailwater)  
 ↑**1=CB C01** (Outlet Controls 12.72 cfs @ 4.05 fps)

**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=60.46' TW=58.41' (Dynamic Tailwater)  
 ↑**2=Broad-Crested Rectangular Weir** ( Controls 0.00 cfs)

**Summary for Pond C02: C02**

Inflow Area = 6.600 ac, 100.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 21.81 cfs @ 0.13 hrs, Volume= 0.300 af  
 Outflow = 21.81 cfs @ 0.14 hrs, Volume= 0.300 af, Atten= 0%, Lag= 0.6 min  
 Primary = 14.36 cfs @ 0.10 hrs, Volume= 0.246 af  
 Secondary = 7.61 cfs @ 0.16 hrs, Volume= 0.054 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 62.13' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	58.41'	<b>24.0" Round CB C02</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 58.41' / 56.77' S= 0.0082 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Secondary	61.86'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=14.35 cfs @ 0.10 hrs HW=62.04' TW=60.75' (Dynamic Tailwater)  
 ↑**1=CB C02** (Outlet Controls 14.35 cfs @ 4.57 fps)

**Secondary OutFlow** Max=7.61 cfs @ 0.16 hrs HW=62.13' TW=60.86' (Dynamic Tailwater)  
 ↑**2=Broad-Crested Rectangular Weir** (Weir Controls 7.61 cfs @ 1.40 fps)

**Summary for Pond C03: C03**

Inflow Area = 10.520 ac, 100.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 34.77 cfs @ 0.16 hrs, Volume= 0.479 af  
 Outflow = 34.77 cfs @ 0.13 hrs, Volume= 0.479 af, Atten= 0%, Lag= 0.0 min  
 Primary = 9.63 cfs @ 0.04 hrs, Volume= 0.194 af  
 Secondary = 25.96 cfs @ 0.16 hrs, Volume= 0.284 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 60.86' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.77'	<b>24.0" Round CB C03</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.77' / 56.14' S= 0.0032 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Secondary	60.23'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Existing System C**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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**Primary OutFlow** Max=9.60 cfs @ 0.04 hrs HW=60.33' TW=59.75' (Dynamic Tailwater)↑**1=CB C03** (Outlet Controls 9.60 cfs @ 3.06 fps)**Secondary OutFlow** Max=25.96 cfs @ 0.16 hrs HW=60.86' TW=60.38' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Weir Controls 25.96 cfs @ 2.05 fps)**Summary for Pond C04: C04**

Inflow Area = 14.580 ac, 100.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 48.18 cfs @ 0.16 hrs, Volume= 0.663 af  
 Outflow = 48.18 cfs @ 0.16 hrs, Volume= 0.663 af, Atten= 0%, Lag= 0.0 min  
 Primary = 12.03 cfs @ 0.05 hrs, Volume= 0.254 af  
 Secondary = 36.88 cfs @ 0.16 hrs, Volume= 0.410 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 60.38' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.14'	<b>24.0" Round CB C04</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.14' / 54.98' S= 0.0058 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Secondary	59.59'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=12.00 cfs @ 0.05 hrs HW=59.81' TW=58.91' (Dynamic Tailwater)↑**1=CB C04** (Outlet Controls 12.00 cfs @ 3.82 fps)**Secondary OutFlow** Max=36.88 cfs @ 0.16 hrs HW=60.38' TW=59.58' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Weir Controls 36.88 cfs @ 2.34 fps)**Summary for Pond C05: C05**

Inflow Area = 17.010 ac, 100.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 56.21 cfs @ 0.16 hrs, Volume= 0.774 af  
 Outflow = 56.21 cfs @ 0.16 hrs, Volume= 0.774 af, Atten= 0%, Lag= 0.0 min  
 Primary = 13.37 cfs @ 0.03 hrs, Volume= 0.223 af  
 Secondary = 47.42 cfs @ 0.16 hrs, Volume= 0.551 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 59.58' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	54.98'	<b>24.0" Round CB C05</b> L= 314.2' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 54.98' / 53.56' S= 0.0045 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf
#2	Secondary	58.59'	<b>20.0' long x 314.2' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63



**Existing System C**

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**Primary OutFlow** Max=9.97 cfs @ 0.03 hrs HW=58.54' TW=57.69' (Dynamic Tailwater)↑**1=CB C05** (Outlet Controls 9.97 cfs @ 3.17 fps)**Secondary OutFlow** Max=47.38 cfs @ 0.16 hrs HW=59.58' TW=58.92' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Weir Controls 47.38 cfs @ 2.40 fps)**Summary for Pond C06: C06**

Inflow Area = 21.790 ac, 100.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 72.01 cfs @ 0.16 hrs, Volume= 0.992 af  
 Outflow = 72.01 cfs @ 0.16 hrs, Volume= 0.992 af, Atten= 0%, Lag= 0.0 min  
 Primary = 16.38 cfs @ 0.03 hrs, Volume= 0.318 af  
 Secondary = 58.35 cfs @ 0.16 hrs, Volume= 0.674 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 58.92' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	53.56'	<b>30.0" Round CB C6</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 53.56' / 52.63' S= 0.0046 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf
#2	Secondary	57.58'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=15.29 cfs @ 0.03 hrs HW=57.74' TW=57.23' (Dynamic Tailwater)↑**1=CB C6** (Outlet Controls 15.29 cfs @ 3.11 fps)**Secondary OutFlow** Max=58.26 cfs @ 0.16 hrs HW=58.92' TW=58.52' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Weir Controls 58.26 cfs @ 2.18 fps)**Summary for Pond C07: C07**

Inflow Area = 29.270 ac, 100.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 96.84 cfs @ 0.16 hrs, Volume= 1.333 af  
 Outflow = 96.84 cfs @ 0.16 hrs, Volume= 1.333 af, Atten= 0%, Lag= 0.0 min  
 Primary = 17.61 cfs @ 0.08 hrs, Volume= 0.395 af  
 Secondary = 79.86 cfs @ 0.16 hrs, Volume= 0.937 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 58.52' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	52.63'	<b>30.0" Round CB C07</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 52.63' / 52.40' S= 0.0012 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf
#2	Secondary	56.97'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Existing System C**

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**Primary OutFlow** Max=17.39 cfs @ 0.08 hrs HW=57.91' TW=57.25' (Dynamic Tailwater)  
 ↳1=CB C07 (Outlet Controls 17.39 cfs @ 3.54 fps)

**Secondary OutFlow** Max=79.47 cfs @ 0.16 hrs HW=58.51' TW=57.89' (Dynamic Tailwater)  
 ↳2=Broad-Crested Rectangular Weir (Weir Controls 79.47 cfs @ 2.57 fps)

**Summary for Pond C08: C08**

Inflow Area = 33.200 ac, 100.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 109.86 cfs @ 0.16 hrs, Volume= 1.511 af  
 Outflow = 109.86 cfs @ 0.16 hrs, Volume= 1.511 af, Atten= 0%, Lag= 0.0 min  
 Primary = 19.00 cfs @ 0.07 hrs, Volume= 0.422 af  
 Secondary = 91.56 cfs @ 0.16 hrs, Volume= 1.089 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 57.90' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	52.40'	<b>30.0" Round CB C08</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 52.40' / 51.69' S= 0.0036 '/' Cc= 0.900 n= 0.013, Flow Area= 4.91 sf
#2	Secondary	56.25'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=18.79 cfs @ 0.07 hrs HW=57.07' TW=56.31' (Dynamic Tailwater)  
 ↳1=CB C08 (Outlet Controls 18.79 cfs @ 3.83 fps)

**Secondary OutFlow** Max=91.18 cfs @ 0.16 hrs HW=57.89' TW=57.14' (Dynamic Tailwater)  
 ↳2=Broad-Crested Rectangular Weir (Weir Controls 91.18 cfs @ 2.77 fps)

**Summary for Pond C09: C09**

Inflow Area = 37.320 ac, 100.00% Impervious, Inflow Depth = 0.20" for 100-Year event  
 Inflow = 32.03 cfs @ 0.14 hrs, Volume= 0.609 af  
 Outflow = 32.03 cfs @ 0.14 hrs, Volume= 0.609 af, Atten= 0%, Lag= 0.0 min  
 Primary = 18.90 cfs @ 0.02 hrs, Volume= 0.197 af  
 Secondary = 27.77 cfs @ 0.16 hrs, Volume= 0.413 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 57.18' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	51.69'	<b>30.0" Round CB C09</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 51.69' / 50.87' S= 0.0041 '/' Cc= 0.900 n= 0.013, Flow Area= 4.91 sf
#2	Secondary	55.64'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Existing System C**

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**Primary OutFlow** Max=13.97 cfs @ 0.02 hrs HW=55.22' TW=54.80' (Dynamic Tailwater)  
 ↑**1=CB C09** (Outlet Controls 13.97 cfs @ 2.85 fps)

**Secondary OutFlow** Max=27.44 cfs @ 0.16 hrs HW=57.18' TW=57.14' (Dynamic Tailwater)  
 ↑**2=Broad-Crested Rectangular Weir** (Weir Controls 27.44 cfs @ 0.89 fps)

**Summary for Pond C10: C10**

Inflow Area = 42.290 ac, 100.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 139.97 cfs @ 0.16 hrs, Volume= 1.925 af  
 Outflow = 139.97 cfs @ 0.16 hrs, Volume= 1.925 af, Atten= 0%, Lag= 0.0 min  
 Primary = 22.24 cfs @ 0.02 hrs, Volume= 0.449 af  
 Secondary = 120.33 cfs @ 0.16 hrs, Volume= 1.476 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 57.15' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	50.87'	<b>30.0" Round CB C10</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 50.87' / 49.90' S= 0.0048 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf
#2	Secondary	55.12'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=20.66 cfs @ 0.02 hrs HW=55.02' TW=54.10' (Dynamic Tailwater)  
 ↑**1=CB C10** (Outlet Controls 20.66 cfs @ 4.21 fps)

**Secondary OutFlow** Max=119.57 cfs @ 0.16 hrs HW=57.14' TW=56.31' (Dynamic Tailwater)  
 ↑**2=Broad-Crested Rectangular Weir** (Weir Controls 119.57 cfs @ 2.96 fps)

**Summary for Pond C11: C11**

Inflow Area = 46.210 ac, 100.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 152.96 cfs @ 0.16 hrs, Volume= 2.103 af  
 Outflow = 152.96 cfs @ 0.16 hrs, Volume= 2.103 af, Atten= 0%, Lag= 0.0 min  
 Primary = 34.40 cfs @ 0.14 hrs, Volume= 0.703 af  
 Secondary = 118.71 cfs @ 0.16 hrs, Volume= 1.400 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 56.32' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	49.90'	<b>36.0" Round CB C11</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 49.90' / 48.28' S= 0.0081 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf
#2	Secondary	54.45'	<b>20.0' long x 200.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Existing System C**

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**Primary OutFlow** Max=34.39 cfs @ 0.14 hrs HW=56.27' TW=55.19' (Dynamic Tailwater)  
 ↳1=CB C11 (Outlet Controls 34.39 cfs @ 4.87 fps)

**Secondary OutFlow** Max=118.26 cfs @ 0.16 hrs HW=56.31' TW=55.24' (Dynamic Tailwater)  
 ↳2=Broad-Crested Rectangular Weir (Weir Controls 118.26 cfs @ 3.17 fps)

**Summary for Pond C12: C12**

Inflow Area = 48.330 ac, 100.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 159.99 cfs @ 0.16 hrs, Volume= 2.200 af  
 Outflow = 159.99 cfs @ 0.16 hrs, Volume= 2.200 af, Atten= 0%, Lag= 0.0 min  
 Primary = 36.59 cfs @ 0.14 hrs, Volume= 0.616 af  
 Secondary = 123.84 cfs @ 0.16 hrs, Volume= 1.584 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 55.25' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	48.28'	<b>36.0" Round C12</b> L= 107.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 48.28' / 48.22' S= 0.0006 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf
#2	Secondary	53.33'	<b>20.0' long x 107.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=36.56 cfs @ 0.14 hrs HW=55.18' TW=54.03' (Dynamic Tailwater)  
 ↳1=C12 (Inlet Controls 36.56 cfs @ 5.17 fps)

**Secondary OutFlow** Max=123.47 cfs @ 0.16 hrs HW=55.24' TW=54.12' (Dynamic Tailwater)  
 ↳2=Broad-Crested Rectangular Weir (Weir Controls 123.47 cfs @ 3.23 fps)

**Summary for Subcatchment EC01: E-C01**

Runoff = 12.72 cfs @ 0.13 hrs, Volume= 0.175 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
 City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
3.850	0.95	Paved parking, HSG D
3.850		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.4					<b>Direct Entry, EC01</b>

**Existing System C**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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**Summary for Subcatchment EC02: E-C02**

Runoff = 9.09 cfs @ 0.12 hrs, Volume= 0.125 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
2.750	0.95	Paved parking, HSG D
2.750		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.2					Direct Entry, E-C02

**Summary for Subcatchment EC03: E-C03**

Runoff = 12.95 cfs @ 0.13 hrs, Volume= 0.178 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
3.920	0.95	Paved parking, HSG D
3.920		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.6					Direct Entry, E-C03

**Summary for Subcatchment EC04: EC04**

Runoff = 13.42 cfs @ 0.13 hrs, Volume= 0.185 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
4.060	0.95	Paved parking, HSG D
4.060		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.8					Direct Entry, EC04

**Summary for Subcatchment EC05: E-C05**

Runoff = 8.03 cfs @ 0.14 hrs, Volume= 0.111 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

**Existing System C**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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Area (ac)	C	Description
2.430	0.95	Paved parking, HSG D
2.430		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.2					<b>Direct Entry, EC05</b>

**Summary for Subcatchment EC06: E-C06**

Runoff = 15.80 cfs @ 0.12 hrs, Volume= 0.218 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
4.780	0.95	Paved parking, HSG D
4.780		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.2					<b>Direct Entry, E-C6</b>

**Summary for Subcatchment EC07: E-C07**

Runoff = 24.73 cfs @ 0.16 hrs, Volume= 0.340 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
7.480	0.95	Paved parking, HSG D
7.480		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.3					<b>Direct Entry, E-A07</b>

**Summary for Subcatchment EC08: E-C08**

Runoff = 12.99 cfs @ 0.14 hrs, Volume= 0.179 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
3.930	0.95	Paved parking, HSG D
3.930		100.00% Impervious Area

**Existing System C**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					<b>Direct Entry, E-C08</b>

**Summary for Subcatchment EC09: EC09**

Runoff = 13.62 cfs @ 0.14 hrs, Volume= 0.187 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
4.120	0.95	Paved parking, HSG D
4.120		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.1					<b>Direct Entry, E-C09</b>

**Summary for Subcatchment EC10: E-C10**

Runoff = 16.42 cfs @ 0.14 hrs, Volume= 0.226 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
4.970	0.95	Paved parking, HSG D
4.970		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.2					<b>Direct Entry, E-C10</b>

**Summary for Subcatchment EC11: E-C11**

Runoff = 12.95 cfs @ 0.14 hrs, Volume= 0.178 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
3.920	0.95	Paved parking, HSG D
3.920		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.3					<b>Direct Entry, E-C11</b>



**Existing System C**

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**Summary for Subcatchment EC12: E-C12**

Runoff = 7.01 cfs @ 0.13 hrs, Volume= 0.096 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
2.120	0.95	Paved parking, HSG D
2.120		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.3					Direct Entry, E-C12

**Summary for Subcatchment EC12a: E-C12a**

Runoff = 18.37 cfs @ 0.14 hrs, Volume= 0.253 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
5.560	0.95	Paved parking, HSG D
5.560		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.4					Direct Entry, E-C12

**Summary for Subcatchment EC13: E-C13**

Runoff = 34.01 cfs @ 0.16 hrs, Volume= 0.471 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

Area (ac)	C	Description
10.350	0.95	Paved parking, HSG D
10.350		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.7					Direct Entry, E-A13

**Summary for Subcatchment EC14: E-C14**

Runoff = 0.13 cfs @ 0.09 hrs, Volume= 0.002 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

**Existing System C**

City of San Diego 100-Year Duration=10 min, Inten=3.45 in/hr

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Page 30

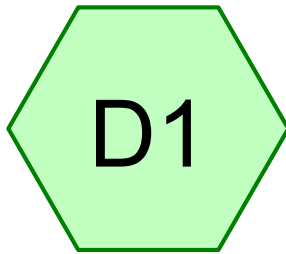
Area (ac)	C	Description
0.040	0.95	Paved parking, HSG D
0.040		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, EC14</b>

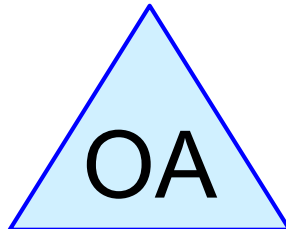
**Summary for Pond OC: OUTFALL C**

Inflow Area = 64.280 ac, 100.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 56.47 cfs @ 0.27 hrs, Volume= 2.945 af  
 Primary = 56.47 cfs @ 0.27 hrs, Volume= 2.945 af, Atten= 0%, Lag= 0.0 min

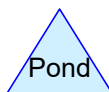
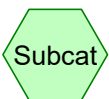
Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3



D1



OUTFALL A



**Routing Diagram for Existing System D**

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**Existing System D**

City of San Diego 50-Year Duration=20 min, Inten=2.35 in/hr

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Page 2

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment D1: D1**

Runoff Area=2.914 ac 0.00% Impervious Runoff Depth=0.41"

Tc=20.0 min C=0.52 Runoff=3.56 cfs 0.099 af

**Pond OA: OUTFALL A**

Inflow=3.56 cfs 0.099 af

Primary=3.56 cfs 0.099 af

**Total Runoff Area = 2.914 ac Runoff Volume = 0.099 af Average Runoff Depth = 0.41"**

**100.00% Pervious = 2.914 ac 0.00% Impervious = 0.000 ac**

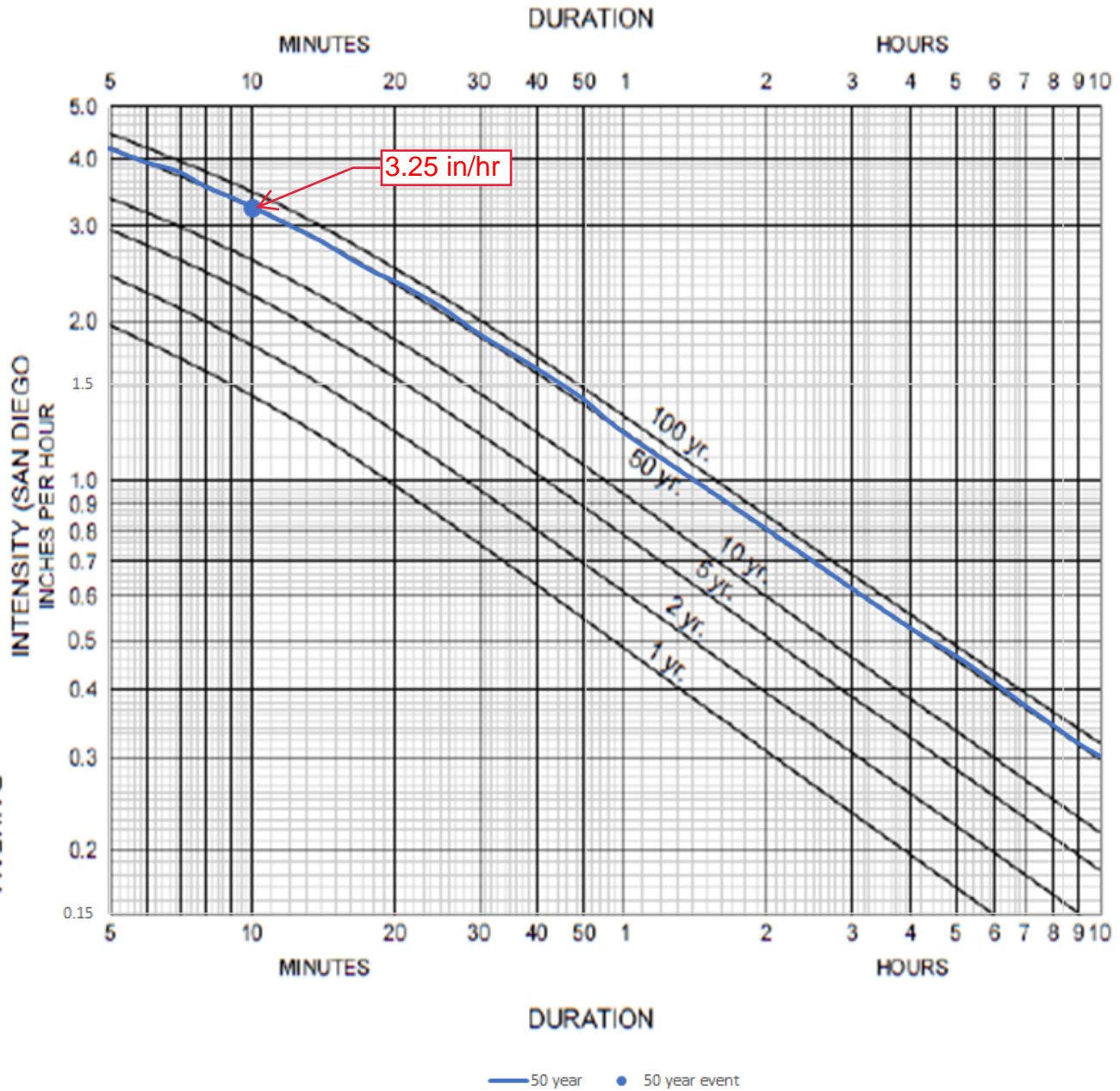
50-year Rainfall Intensity

Figure A-1. Intensity-Duration-Frequency Design Chart

ELEV.	FACTOR
0-1500	1.00
1500-3000	1.25
3000-4000	1.42
4000-5000	1.60
5000-6000	1.7
DESERT	1.25

TO OBTAIN CORRECT INTENSITY, MULTIPLY INTENSITY ON CHART BY FACTOR FOR DESIGN ELEVATION.

INTENSITY - DURATION - FREQUENCY CURVES  
FOR  
COUNTY OF SAN DIEGO  
RAINFALL





**Existing System D**

City of San Diego 50-Year Duration=20 min, Inten=2.35 in/hr

Prepared by Geosyntec Consultants

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Page 3

**Summary for Subcatchment D1: D1**

Runoff = 3.56 cfs @ 0.33 hrs, Volume= 0.099 af, Depth= 0.41"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=20 min, Inten=2.35 in/hr

Area (ac)	C	Description
2.914	0.52	Mixed Use, HSG D
2.914		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.0					<b>Direct Entry, P-A02</b>

**Summary for Pond OA: OUTFALL A**

Inflow Area = 2.914 ac, 0.00% Impervious, Inflow Depth = 0.41" for 50-Year event

Inflow = 3.56 cfs @ 0.33 hrs, Volume= 0.099 af

Primary = 3.56 cfs @ 0.33 hrs, Volume= 0.099 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

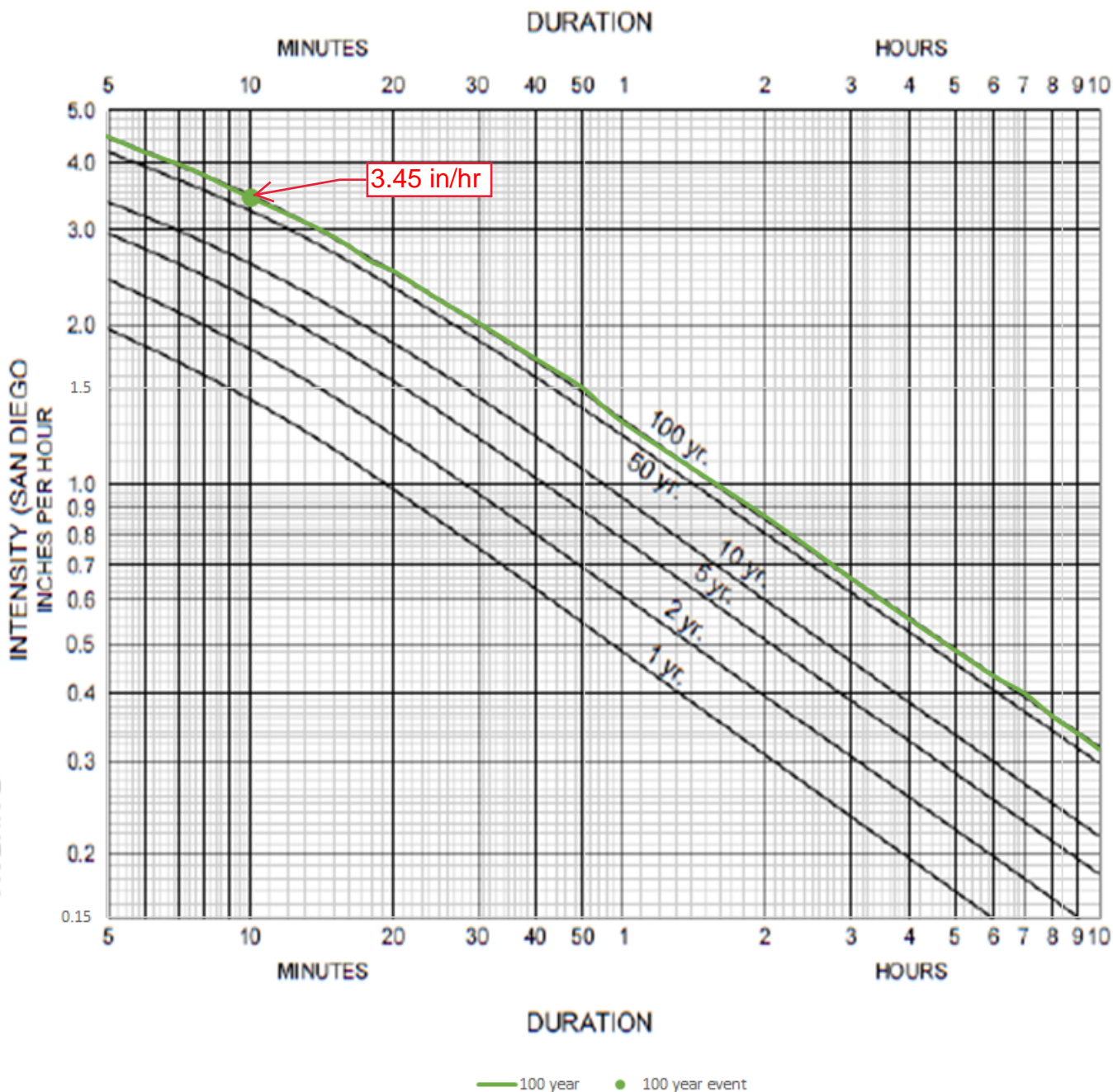
100-year Rainfall Intensity

Figure A-1. Intensity-Duration-Frequency Design Chart

ELEV.	FACTOR
0-1500	1.00
1500-3000	1.25
3000-4000	1.42
4000-5000	1.60
5000-6000	1.7
DESERT	1.25

TO OBTAIN CORRECT INTENSITY, MULTIPLY INTENSITY ON CHART BY FACTOR FOR DESIGN ELEVATION.

INTENSITY - DURATION - FREQUENCY RAINFALL CURVES FOR COUNTY OF SAN DIEGO



**Existing System D***City of San Diego 100-Year Duration=20 min, Inten=2.50 in/hr*

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Page 4

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment D1: D1**

Runoff Area=2.914 ac 0.00% Impervious Runoff Depth=0.43"

Tc=20.0 min C=0.52 Runoff=3.79 cfs 0.105 af

**Pond OA: OUTFALL A**

Inflow=3.79 cfs 0.105 af

Primary=3.79 cfs 0.105 af

**Total Runoff Area = 2.914 ac Runoff Volume = 0.105 af Average Runoff Depth = 0.43"****100.00% Pervious = 2.914 ac 0.00% Impervious = 0.000 ac**

**Existing System D**

City of San Diego 100-Year Duration=20 min, Inten=2.50 in/hr

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Page 5

**Summary for Subcatchment D1: D1**

Runoff = 3.79 cfs @ 0.33 hrs, Volume= 0.105 af, Depth= 0.43"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=20 min, Inten=2.50 in/hr

Area (ac)	C	Description
2.914	0.52	Mixed Use, HSG D
2.914		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.0					<b>Direct Entry, P-A02</b>

**Summary for Pond OA: OUTFALL A**

Inflow Area = 2.914 ac, 0.00% Impervious, Inflow Depth = 0.43" for 100-Year event

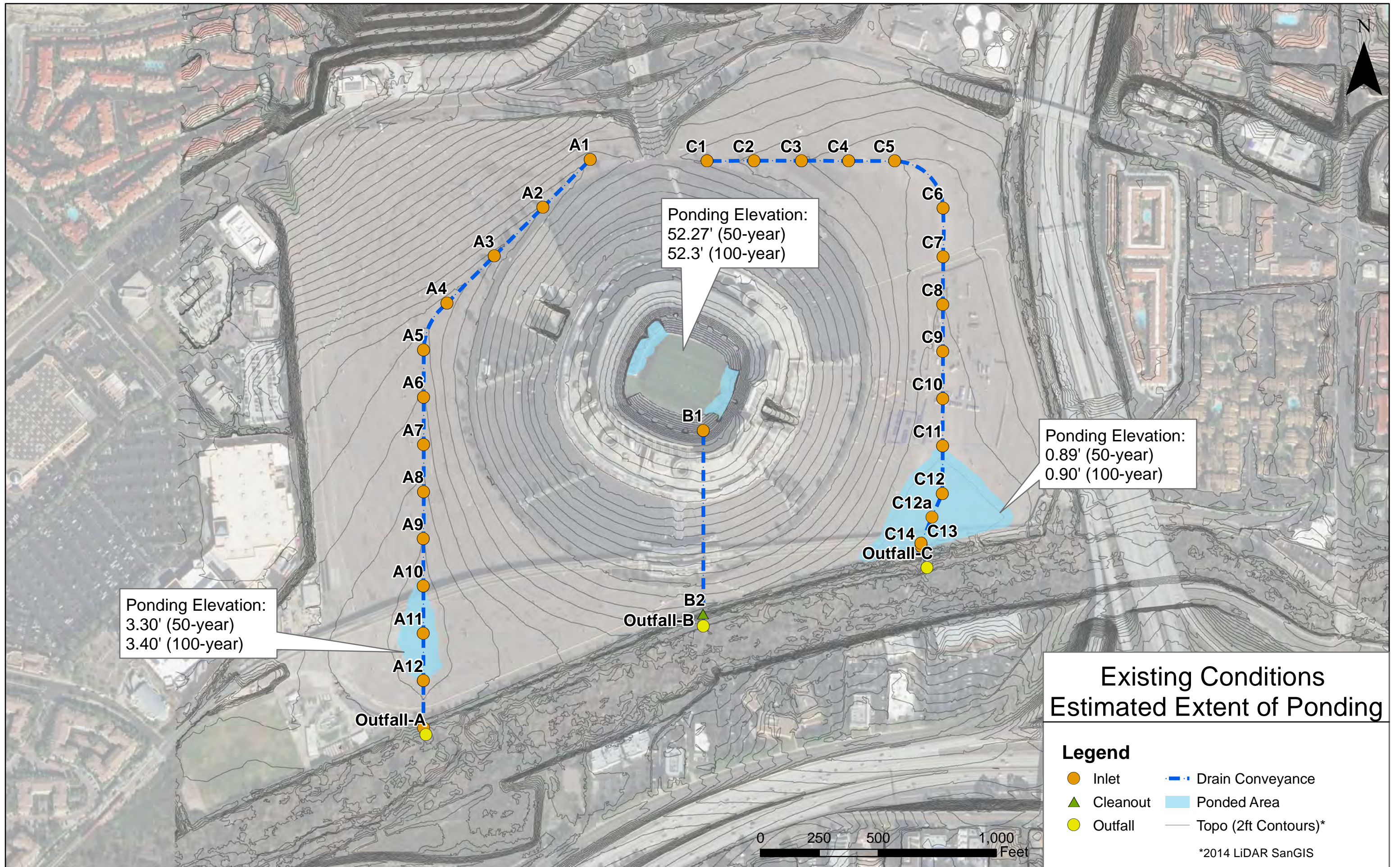
Inflow = 3.79 cfs @ 0.33 hrs, Volume= 0.105 af

Primary = 3.79 cfs @ 0.33 hrs, Volume= 0.105 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

**APPENDIX A.6**  
Existing Conditions: Estimated Extent  
of Ponding Exhibit





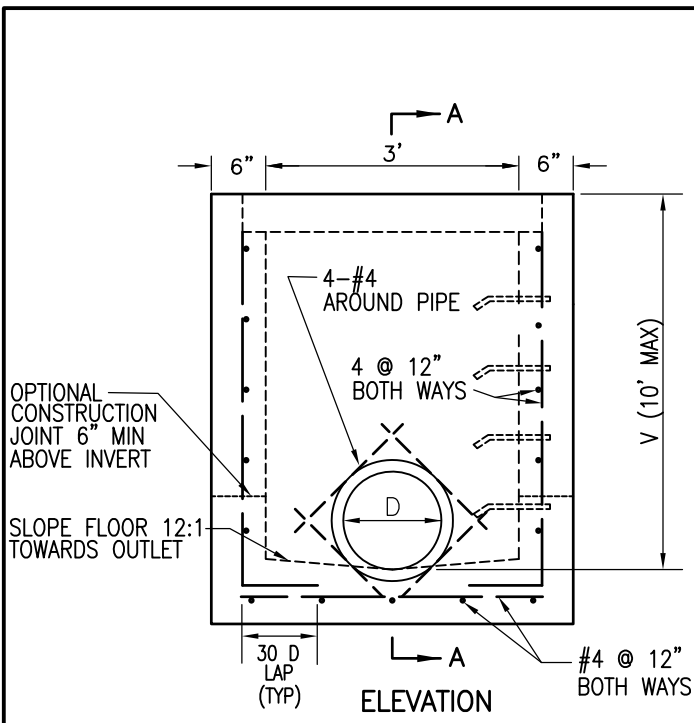


# **APPENDIX B**

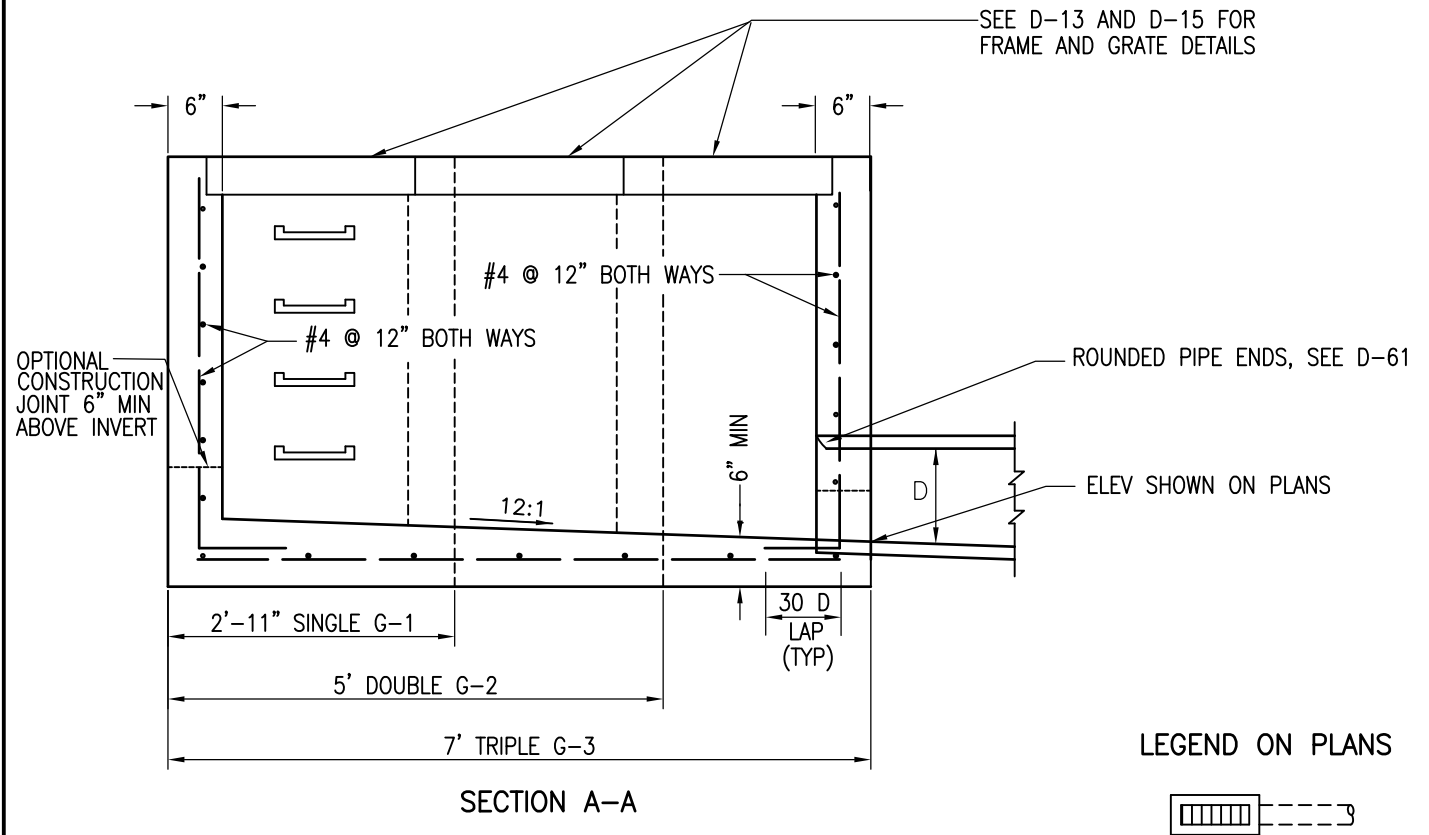
## **Proposed Conditions Supporting Material**

# **APPENDIX B.1**

## **Proposed Conditions: Storm Drain Network, Standard Drawings, & Grading Plans**



- NOTES**
1. SEE D-11A AND D-11B FOR ADDITIONAL NOTES AND DETAILS.
  2. STEPS SHALL BE INSTALLED WHEN V EXCEEDS 4'. SEE D-11A FOR DETAILS.
  3. MAINTAIN 1-1/2" CLEAR SPACING BETWEEN REINFORCING AND CONCRETE SURFACE.
  4. INCREASE IN ALLOWABLE DEPTH SUBJECT TO APPROVAL BY LOCAL AGENCY.
  5. SECTION A-A SHOWS 3 SIZES AND SHALL NOT IMPLY THAT AN INTERIOR WALL IS TO BE BUILT FOR THE STRUCTURES WITH DOUBLE OR TRIPLE FRAME AND GRATE.
  6. EXPOSED EDGES OF CONCRETE SHALL BE ROUNDED WITH RADIUS OF 1/2".
  7. DESIGNATE TYPES AS FOLLOWS: SINGLE G-1, DOUBLE G-2 OR TRIPLE G-3.
  8. ONLY END BEARING GRATES SHALL BE USED. SEE D-15.



Revision	By	Approved	Date
ORIGINAL		Kercheval	12/75
Reformatted		T. Stanton	04/06
Edited		T. Stanton	02/09
Edited	S.S.	T. Regello	03/11
Edited	T.R.	T. Regello	10/15

**SAN DIEGO REGIONAL STANDARD DRAWING**

**CATCH BASIN - TYPE G**

RECOMMENDED BY THE SAN DIEGO REGIONAL STANDARDS COMMITTEE

*T. Stanton* 12/17/2015  
 Chairperson R.C.E. 19246 Date

DRAWING NUMBER **D-08**



Reference: Carrier Johnson Planning Documents





# SAN DIEGO STATE UNIVERSITY MISSION VALLEY 75% DD SITE DEVELOPMENT 2/12/19

## WORK TO BE DONE

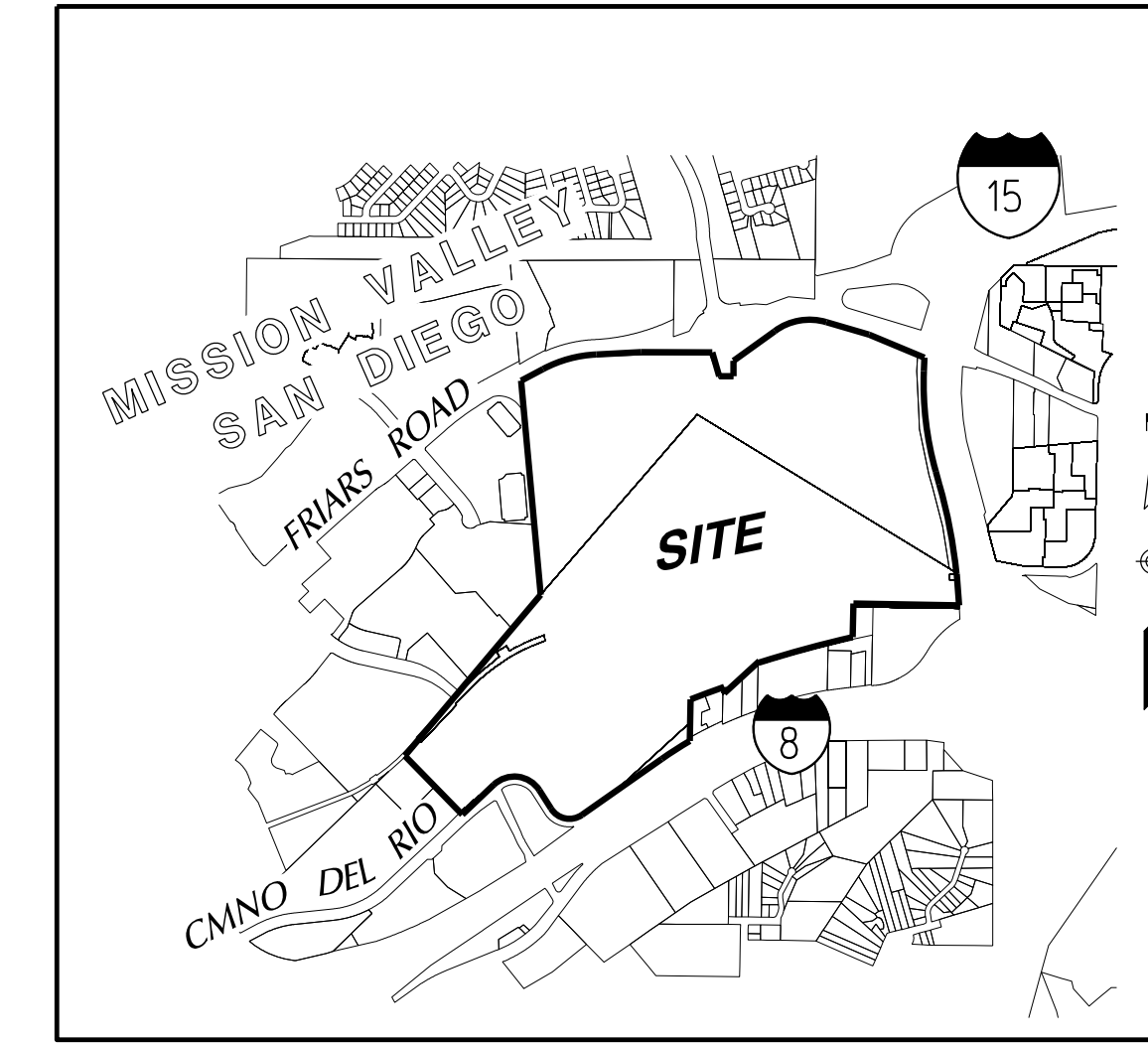
THE IMPROVEMENTS CONSIST OF THE FOLLOWING WORK TO BE DONE ACCORDING TO THESE PLANS AND THE SPECIFICATIONS AND STANDARD DRAWINGS OF THE CITY OF SAN DIEGO, GRADING, UTILITIES INCLUDING SEWER, WATER, AND STORM DRAIN, IN ADDITION TO THE INSTALLATION OF IMPROVEMENT WITHIN PROJECT LIMITS ARE TO BE DONE ACCORDING TO THESE PLANS, THE CURRENT SAN DIEGO AREA REGIONAL STANDARD DRAWING, THE SPECIFICATION FOR PUBLIC WORKS CONSTRUCTION, THE CALIFORNIA BUILDING CODE, CALIFORNIA PLUMBING CODE AND CALIFORNIA FIRE CODE.

STANDARD SPECIFICATIONS DOCUMENT NO. PW107016-01

DESCRIPTION  
STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION (GREENBOOK), 2016 EDITION  
2018 CALIFORNIA BUILDING CODE (CBC)  
VOLUMES 1 AND 2, (PART 2, TITLE 24 CCR)  
2018 CITY OF SAN DIEGO STANDARD DRAWINGS FOR PUBLIC WORKS

## LEGEND

PROPERTY BOUNDARY	STANDARD DWGS.	SYMBOL
EXISTING EDGE OF PAVEMENT		---
EXISTING BUILDING		[Symbol]
EXISTING EASEMENTS		---
EXISTING FENCE		---
EXISTING SPOT ELEVATION		x 495.8
EXISTING MAJOR CONTOUR		550
EXISTING MINOR CONTOUR		---
EXISTING SEWER MAIN		S
EXISTING SEWER MANHOLE		(M)
EXISTING WATER MAIN		W
EXISTING FUEL LINE		OIL
EXISTING CATV		CATV
EXISTING ELECTRICAL		E
EXISTING STORM DRAIN MAIN		S
EXISTING STORM DRAIN STRUCTURE		(S)
EXISTING GAS		G
EXISTING TELEPHONE		T
PROPOSED EASEMENTS		---
PROPOSED MAJOR CONTOUR		550
PROPOSED MINOR CONTOUR		---
PROPOSED CUT/FILL LINE		C
DAYLIGHT LINE		
PROPOSED TOP/TOE OF SLOPE		---
PROPOSED SLOPE (2:1 MAX.)		---
PROPOSED SWALE (SEE DETAIL, SHT. C-N2)		---
PROPOSED (P.V.T.) BROW DITCH (TYPE-B) RSD D-75		---
PROPOSED RETAINING WALL		---
PROPOSED SEWER MAIN (SDR-18, SDR-35)		S
PROPOSED SEWER MANHOLE (SDS-107)		(M)
PROPOSED WATER MAIN (12" PVC (C-900))		W
PROPOSED FIRE SERVICE (12" PVC (C-900))		FS
PROP. (P.V.T.) TYPE-1 CATCH BASIN RSD D-29		(CB)
PROP. (P.V.T.) STORM DRAIN RSD D-09 (CLEANOUT PER PLAN (UNLESS OTHERWISE NOTED))		(SD)
PROP. (P.V.T.) TYPE-B CURB INLET RSD D-02 (UNLESS OTHERWISE NOTED)		(CI)
PROP. (P.V.T.) STORM DRAIN (RCP 2000-DI)		(SD)
PROP. (P.V.T.) STORM DRAIN (PVC)		(SD)
HEADWALL D-34 THRU D-35B		(HW)
PROP. (P.V.T.) RIP-RAP ENERGY DISSIPATOR RSD D-40		(ER)
WATER METER (SIZE PER PLAN)		(W)
12" BACKFLOW		(BF)
12" FIRE SERVICE		(F)
IRRIGATION METER (SIZE PER PLAN)		(I)
PROPOSED AC PAVEMENT SCH. J SDG-113		(PA)
PROPOSED 6" CURB & GUTTER		(CG)
PROPOSED ZERO CURB		(ZC)
PROPOSED JOINT TRENCH BY OTHERS		(JT)
PROPOSED STREET LIGHT SD-101		(SL)
PROPOSED FIRE HYDRANT SDW-104		(FH)
PROPOSED MODULAR WETLAND BY OTHERS		(MW)



VICINITY MAP  
NO SCALE

## SITE ADDRESS

9449 FRIARS ROAD  
SAN DIEGO, CA 92108

## APN #

433-250-16-00, 433-250-13-00,  
433-250-14-00, 433-250-19-00

## BENCHMARK

THE BENCHMARK FOR THIS SURVEY IS THE FOUND BRASS PLUG LOCATED IN THE SOUTHEASTERLY CORNER OF THE BRIDGE AT THE INTERSECTION OF MISSION VILLAGE DRIVE AND FRIARS ROAD PER THE CITY OF SAN DIEGO VERTICAL BENCHMARK.

BM ELEVATION: 90.926, NAVD 88 (ADDED 2.15' TP 88.776 NGVD29)

## TOPOGRAPHY SOURCE

AERIAL TOPOGRAPHY PREPARED BY  
RICK ENGINEERING COMPANY DATED  
MARCH 28, 2014

## BASIS OF ELEVATION

THE BASIS OF ELEVATION FOR THIS SURVEY WAS CALCULATED BY ADDING 2.15' TO THE PUBLISHED NGVD 29 ELEVATION OF THE FOUND BRASS PLUG LOCATED IN THE SOUTHEASTERLY CORNER OF THE BRIDGE AT THE INTERSECTION OF MISSION VILLAGE DRIVE AND FRIARS ROAD PER THE CITY OF SAN DIEGO VERTICAL BENCHMARK.

BM ELEVATION: 90.926, NAVD 88 (ADDED 2.15' TO THE PUBLISHED VALUE OF 88.776 NGVD29)

## LEGAL DESCRIPTION

THOSE PORTIONS OF LOTS 31, 35, 42 AND 45 IN THE CITY OF SAN DIEGO, STATE OF CALIFORNIA, ACCORDING TO PARTITION MAP THEREOF ON FILE IN THE OFFICE OF THE COUNTY CLERK OF SAID SAN DIEGO COUNTY, IN ACTION NO. 348 ENTITLED "JUAN M. LUCO, ET AL VS. THE COMMERCIAL BANK OF SAN DIEGO, ET AL."

## BASIS OF BEARINGS

THE BASIS OF BEARINGS FOR THIS SURVEY IS THE CALIFORNIA COORDINATE SYSTEM CCS83 ZONE 6, EPOCH 1991.35 AND IS DETERMINED BY RTK G.P.S. MEASUREMENTS USING THE CALVRS NETWORK, MEASUREMENTS TAKEN ON FEBRUARY 13, 2018 AT POINT 986 AND POINT 1525 AS SHOWN HEREON, PER ROS 14492.

BEARING POINT \*986 TO POINT \*1525: N 08° 45' 13" W

## GEOTECHNICAL RECOMMENDATIONS

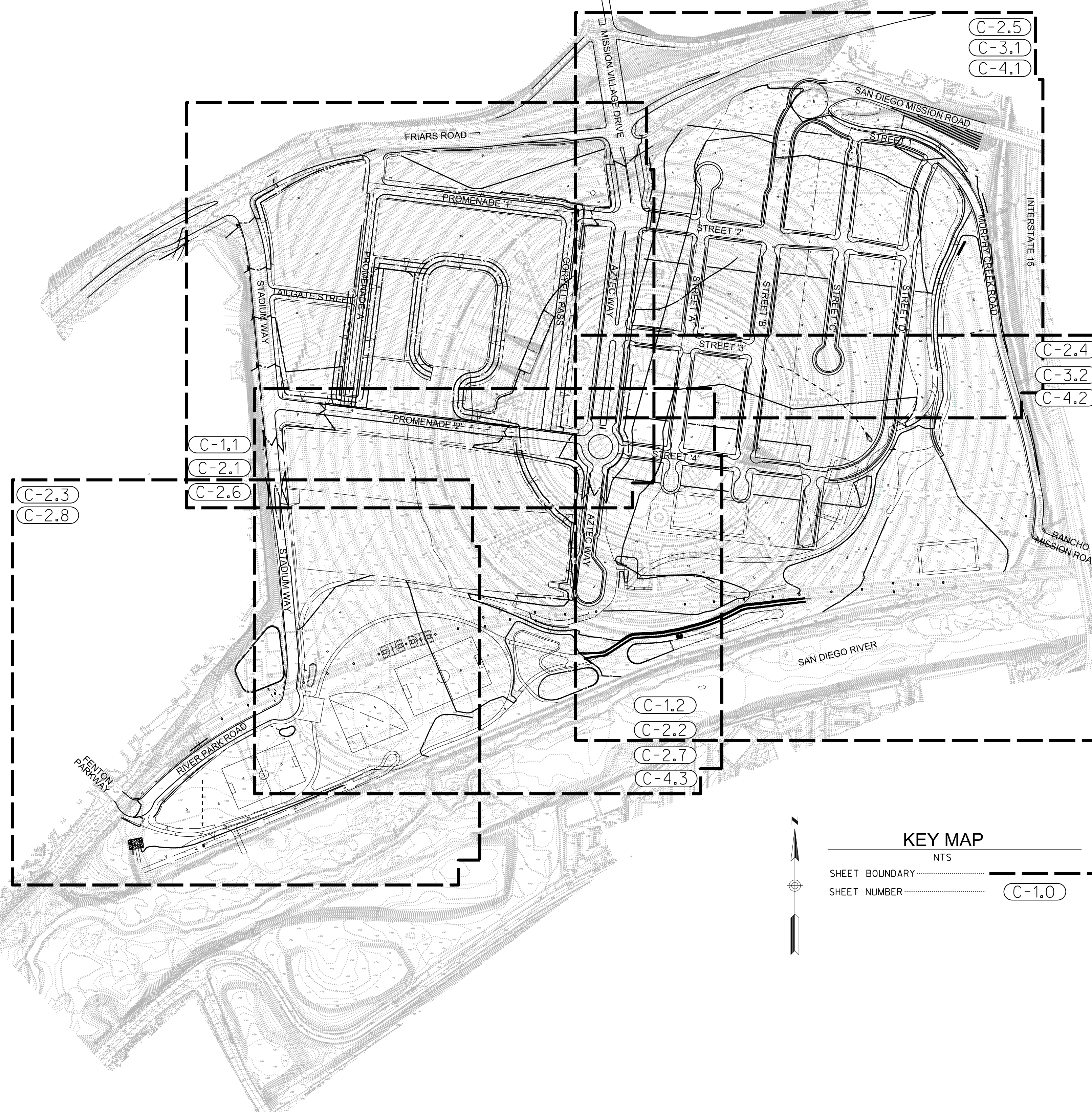
ALL GRADING SHALL BE DONE IN ACCORDANCE WITH THE APPROVED GEOTECHNICAL REPORT FOR THIS PROJECT.

## CIVIL SHEET INDEX

TITLE SHEET	SHEET	C-N1
DETAILS	SHEET	C-N2
CROSS-SECTIONS	SHEET	C-N3
CROSS-SECTIONS	SHEET	C-N4
PHASE 1A GRADING PLANS	SHEETS	C-1-1-C-1,2
PHASE 1A UTILITY PLANS	SHEETS	C-1-1-C-1,2
PHASE 1A EROSION CONTROL PLAN	SHEET	C-1-3
PHASE 1B GRADING PLANS	SHEETS	C-2-1-C-2,5
PHASE 1B UTILITY PLANS	SHEETS	C-2-6-C-2,8
PHASE 1B EROSION CONTROL PLAN	SHEET	C-2-9
PHASE 3 GRADING PLANS	SHEETS	C-3-1-C-3,2
PHASE 3 UTILITY PLANS	SHEETS	C-3-1-C-3,2
PHASE 3 EROSION CONTROL PLAN	SHEET	C-3-3
PHASE 4 GRADING PLANS	SHEETS	C-4-1-C-4,3
PHASE 4 UTILITY PLANS	SHEETS	C-4-1-C-4,3

## ABBREVIATIONS

APN	ASSESSOR'S PARCEL NUMBER	MOD	MODIFIED
BC	BEGIN CURVE	NO	NUMBER
BM	BEGIN CURVE	NTS	NOT TO SCALE
BVC	BEGIN VERTICAL CURVE	PI	POINT OF INTERSECTION
CB	CATCH BASIN	PL	PROPERTY LINE
CL	CENTERLINE	PUB	PUBLIC
CO	CLEAN OUT	PVC	POLYVINYL CHLORIDE
CONT	CONTINUATION	PVT	PRIVATE
CYD	CUBIC YARD	R	RADIUS
EC	END CURVE	RCP	REINFORCED CONCRETE PIPE
EG	EXISTING GROUND	RT	RIGHT ELEVATION
FF	FINISHED FLOOR	RT	RIGHT
FH	FIRE HYDRANT	ROW	RIGHT-OF-WAY
FL/ELEV.	ELEVATION	ROW	RIGHT-OF-WAY
FS	FIRE SERVICE	S	SANITARY SEWER
EVC	END VERTICAL CURVE	SD	STORM DRAIN
EX	EXISTING	SDRS	SAN DIEGO REGIONAL STANDARD DWG
FG	FINISH GRADE	SHT	SHEET
FL	FLOW LINE	STA	STATION
FS	FINISH SURFACE	TC	TOP OF CURB
GB	GRADE BREAK	TEMP	TEMPORARY
HP	HIGH POINT	TF	TOP OF FOOTING
IE	INVERT ELEVATION	TG	TOP OF GRATE
LP	LOW POINT	TP	TOP OF PIPE
LT	LEFT	TW	TOP OF WALL
MAX	MAXIMUM	TYP	TYPICAL
MIN	MINIMUM	VC	VERTICAL CURVE
		W/	WITH



## KEY MAP

NTS  
SHEET BOUNDARY  
SHEET NUMBER C-1.0

## RAW EARTHWORK QUANTITIES CUT/FILL

	CUT (CY)	FILL (CY)	IMPORT/BORROW (CY)
PHASE 1a	180,000	270,000	90,000
PHASE 1b	410,000	410,000	270,000 CY (SEE NOTE 3)
PHASE 3	160,000	385,000	225,000
OPENING DAY	750,000	1,065,000	315,000
OVERALL TOTAL	750,000	1,065,000	315,000
FUTURE PHASE 4	25,000	127,000	102,000

## NOTES:

- BASED ON RAW QUANTITIES 90,000 CY TO BE IMPORTED FROM OFFSITE SOURCES TO IMPLEMENT PHASE 1, NOTE: EXACT IMPORT REQUIREMENTS TO BE DETERMINED BY STADIUM ARCHITECT.
- APPROXIMATE OVER EXCAVATIONS FOR PHASE 1b INCLUDE 30,000 CY FOR 1.5 FOOT STREET STRUCTURAL SECTION UNDERCUT, 5,000 CY FOR 4 FOOT FOOTBALL FIELD UNDERCUT, 5,000 CY FOR FOUNDATION SPOILS, 25,000 CY FOR MISCELLANEOUS SPOILS (TREES, UTILITIES, BIO-BASIN, FLATWORK ETC).
- A PORTION OF THIS CUT ASSUMES COMPLETE STADIUM DEMOLITION AND BORROW OF EARTHWORK FOR PHASE 1b FILL SHOULD THIS BORROW MATERIAL NOT BE AVAILABLE, THEN AN IMPORT OF MATERIAL WILL BE REQUIRED TO IMPLEMENT PHASE 1b, PHASE 3 GRADING SHALL BE EVALUATED FOR SHEET GRADE FINAL CONDITIONS TO DETERMINE IF SITE GRADE ADJUSTMENTS FOR OVERALL EARTHWORK BALANCING WILL BE REQUIRED.
- APPROXIMATE OVER EXCAVATIONS FOR PHASE 4 INCLUDE 34,000 CY FOR 1.5 FOOT STREET STRUCTURAL SECTION UNDERCUT, 30,000 CY FOR MISCELLANEOUS SPOILS (TREES, UTILITIES, BIO-BASIN, FLATWORK ETC) AND 5,000 CY FOR FOUNDATION SPOILS.
- IMPORT FROM OFFSITE SOURCES TO IMPLEMENT PHASE 3 WILL BE REQUIRED. NOTE: EXACT IMPORT REQUIREMENTS SHALL BE VALIDATED IN FINAL ENGINEERING.
- QUANTITY ADJUSTMENTS DO NOT INCLUDE ADJUSTMENTS FOR THE EXISTING 4.5" PAVING - THIS IS APPROXIMATELY 59,000 CY.
- DOES NOT INCLUDE UTILIZING THE STADIUM DEMOLITION WHICH IS ESTIMATED TO BE 60,000 CY (PROVIDED BY OTHERS)
- EARTHWORK QUANTITIES SHOWN ARE RAW CUT/FILL NUMBERS AND ARE CONCEPTUAL AND AN ORDER OF MAGNITUDE QUANTITY ESTIMATE, THESE QUANTITIES WILL CHANGE AS A RESULT OF ADDITIONAL DESIGN DETAIL, FINAL ENGINEERING AND GEOTECHNICAL/SOILS REPORT RECOMMENDATIONS.
- EARTHWORK QUANTITIES DO NOT INCLUDE REMEDIAL GRADING OR BULK/SHRINK ADJUSTMENTS OR REFLECT GEOTECHNICAL GRADING REQUIREMENTS.
- THESE QUANTITIES REFLECT GRADING TO THE 55 ELEVATION FOR THE SOUTH PRE-CAMPUS PARKING (DOES NOT INCLUDE EXCAVATION TO THE P3 PARKING LEVEL AT ELEVATION 44, THE QUANTITY TO EXCAVATE THE P3 LEVEL ON THE SOUTH CAMPUS IS APPROXIMATELY 120K-140K CY WHICH WILL EITHER BE USED FOR FILL ON SITE IN FUTURE PHASES OR EXPORTED AT THE TIME OF CONSTRUCTION).
- EARTHWORK VOLUMES ARE BASED UPON THE 2018 AERIAL TOPOGRAPHY.
- GRADING PER 75% DESIGN DEVELOPMENT DRAWINGS DATED 1/18/19.
- ALL EARTHWORK QUANTITIES TO BE VALIDATED WITH FINAL ENGINEERING TO MINIMIZE THE REQUIRED IMPORT TO THE PROJECT.

5620 FRIARS ROAD  
SAN DIEGO, CA 92110  
619-291-0707  
(FAX) 619-291-4165



SAN DIEGO STATE UNIVERSITY  
MISSION VALLEY  
75% DD SITE DEVELOPMENT PACKAGE

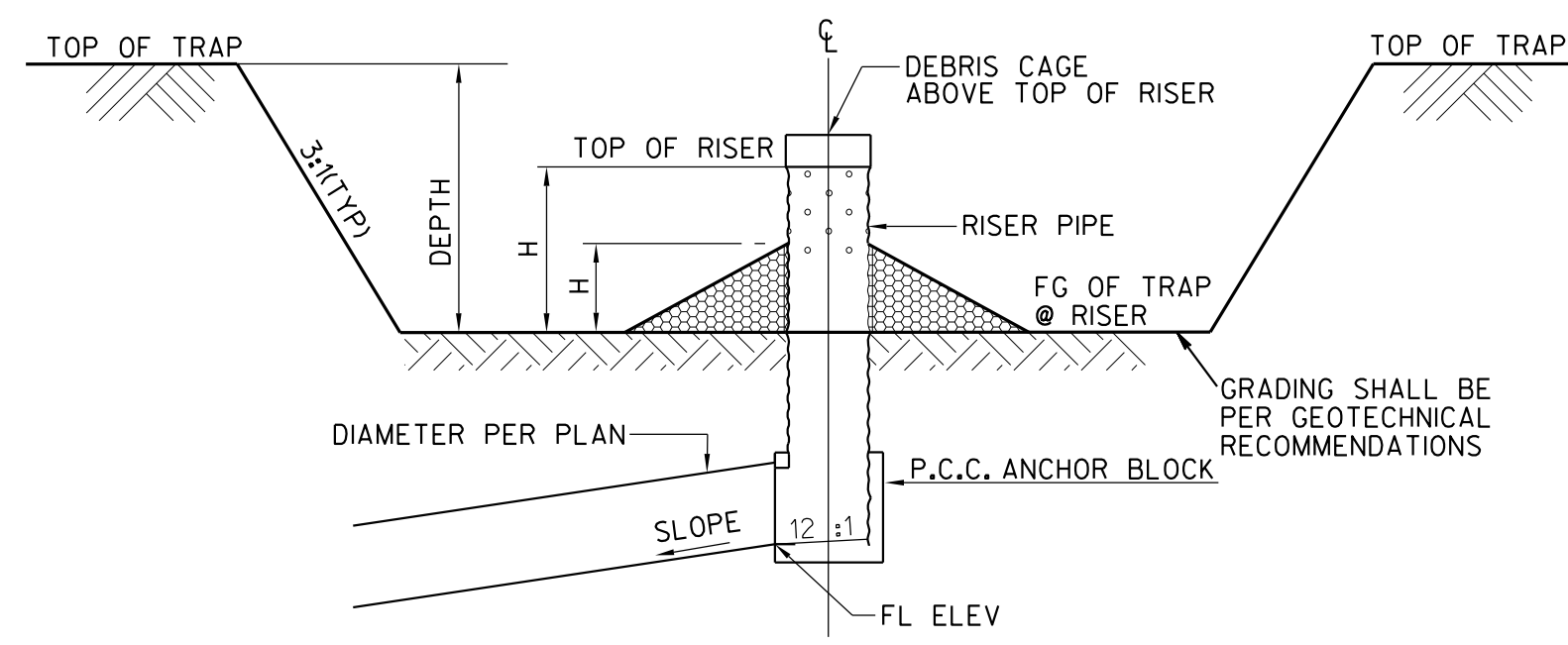
ISSUED: 01/18/19-RFP  
REV. 2/12/19

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FILENAME:  
DRAWN BY: CHECKED BY:  
PLOT DATE:

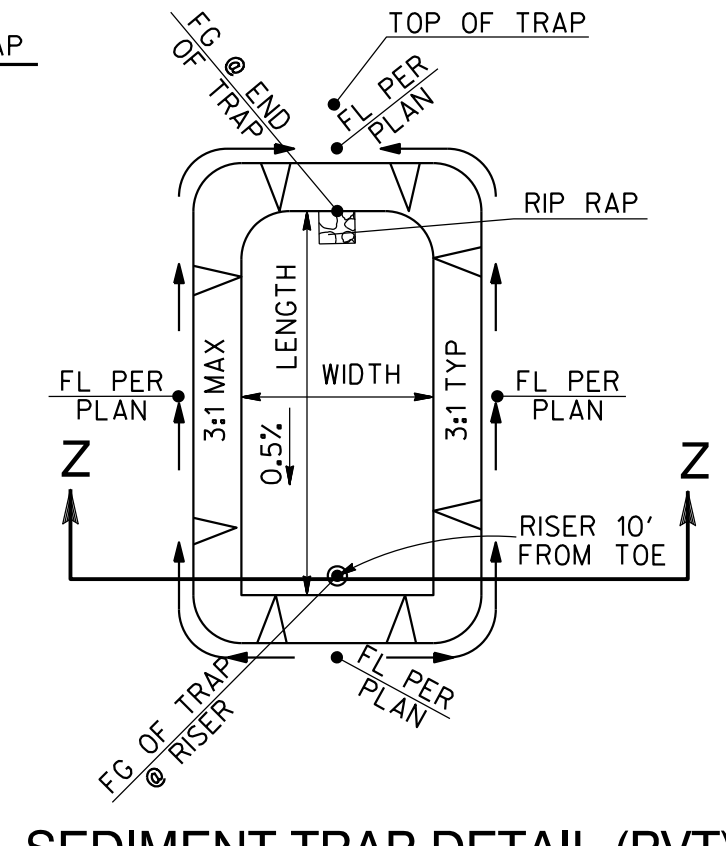
TITLE SHEET

1 OF 23  
DRAWING NO:  
C-N1

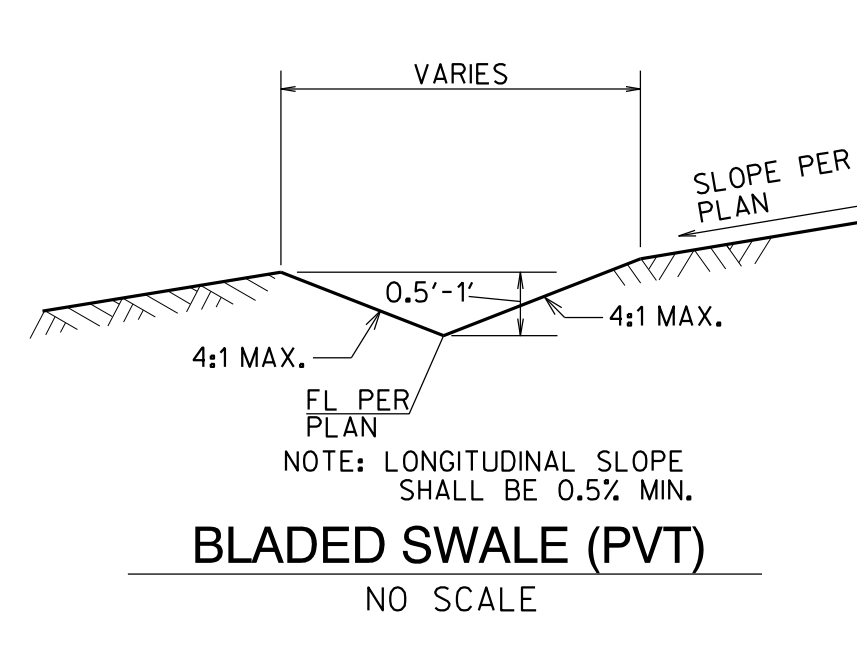




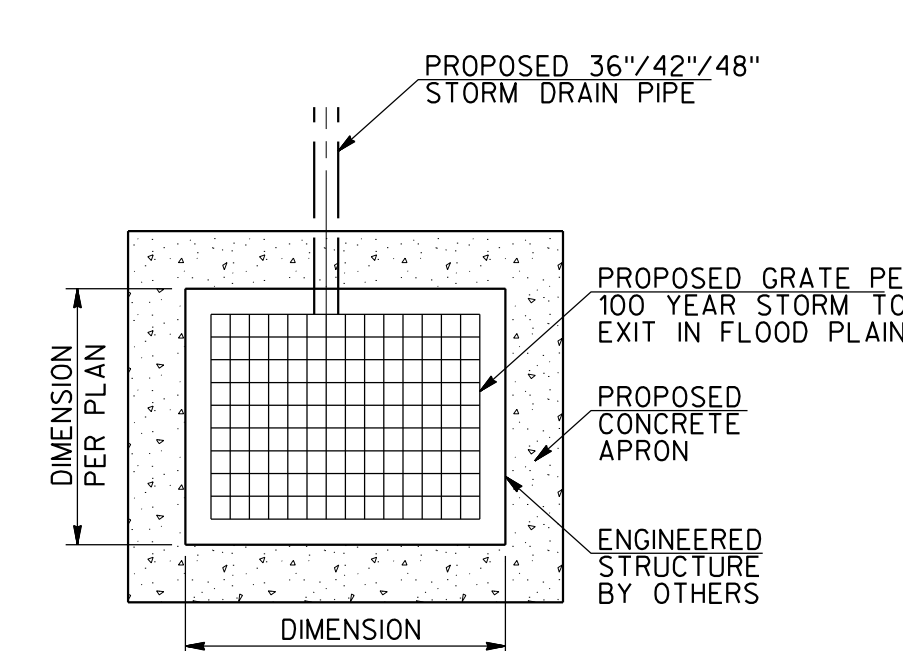
SECTION Z-Z SEDIMENT TRAP  
NO SCALE



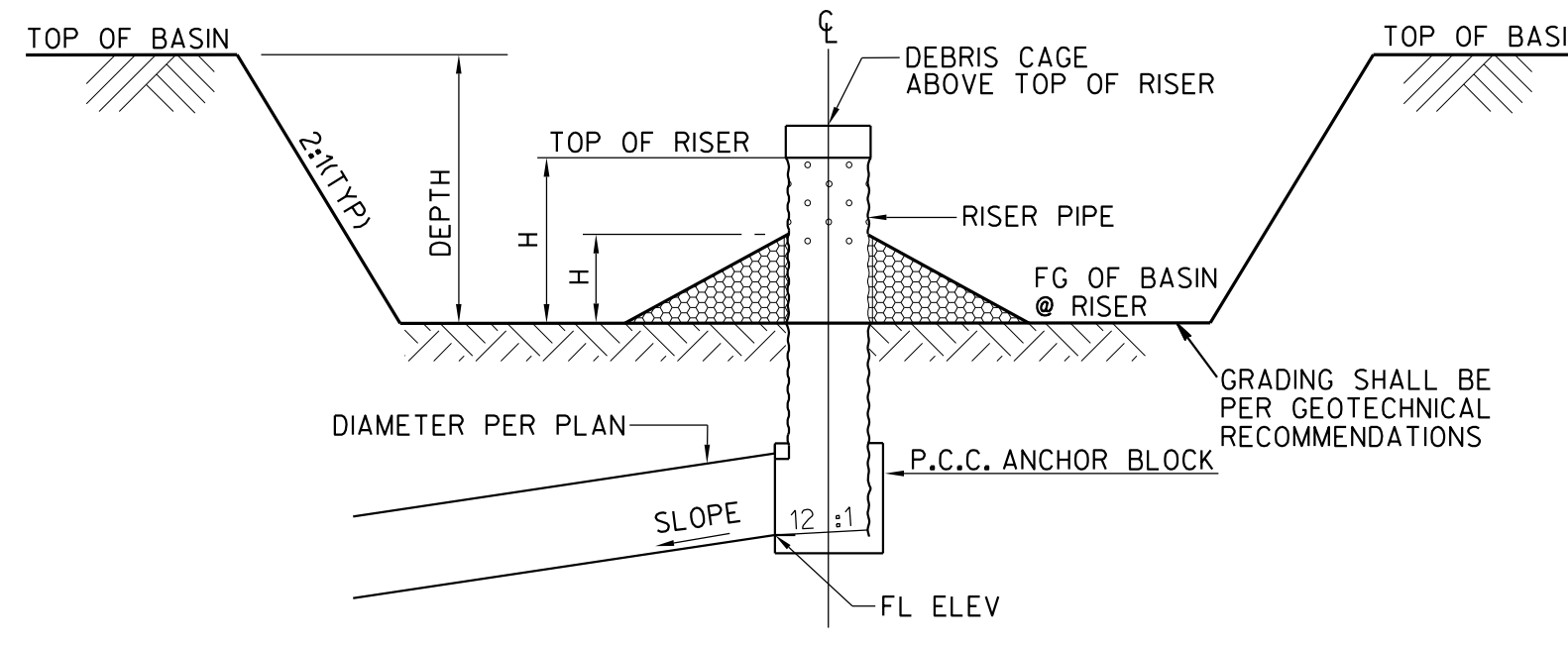
SEDIMENT TRAP DETAIL (PVT)  
TYPICAL  
NO SCALE



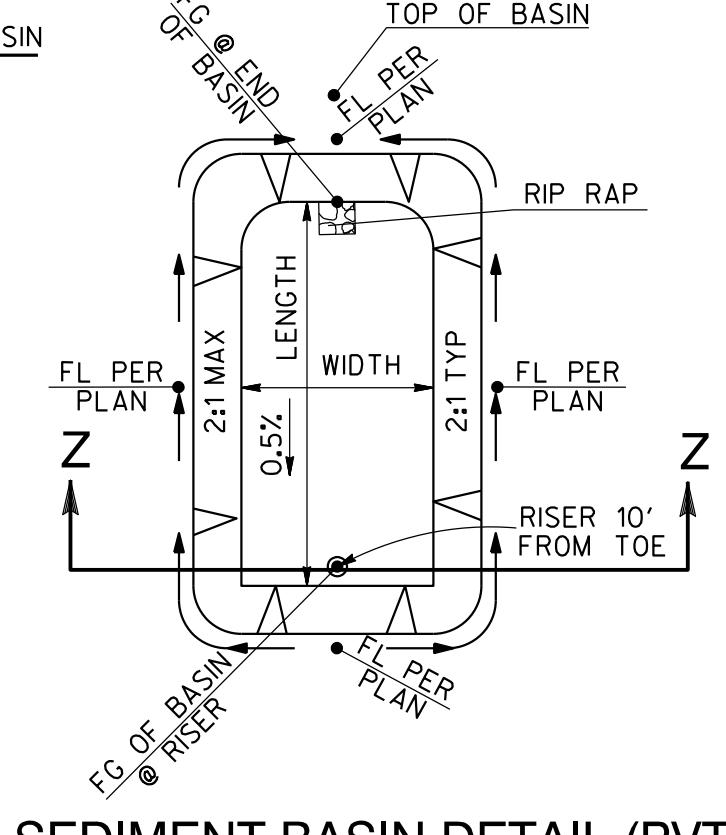
BLADED SWALE (PVT)  
NO SCALE



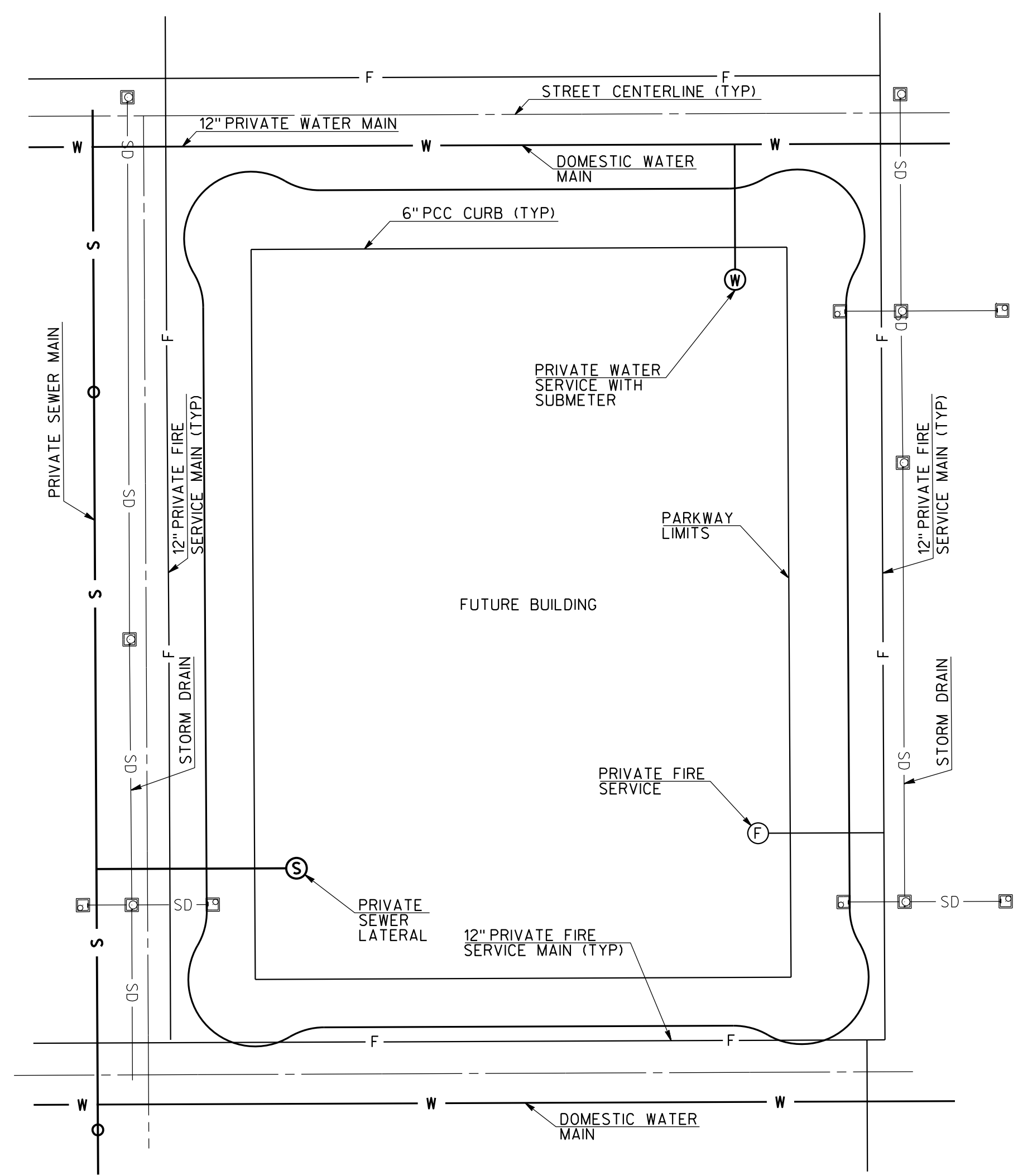
TYPICAL EMERGENCY OUTLET  
BUBBLER STRUCTURE PER  
100 YEAR STORM  
NO SCALE



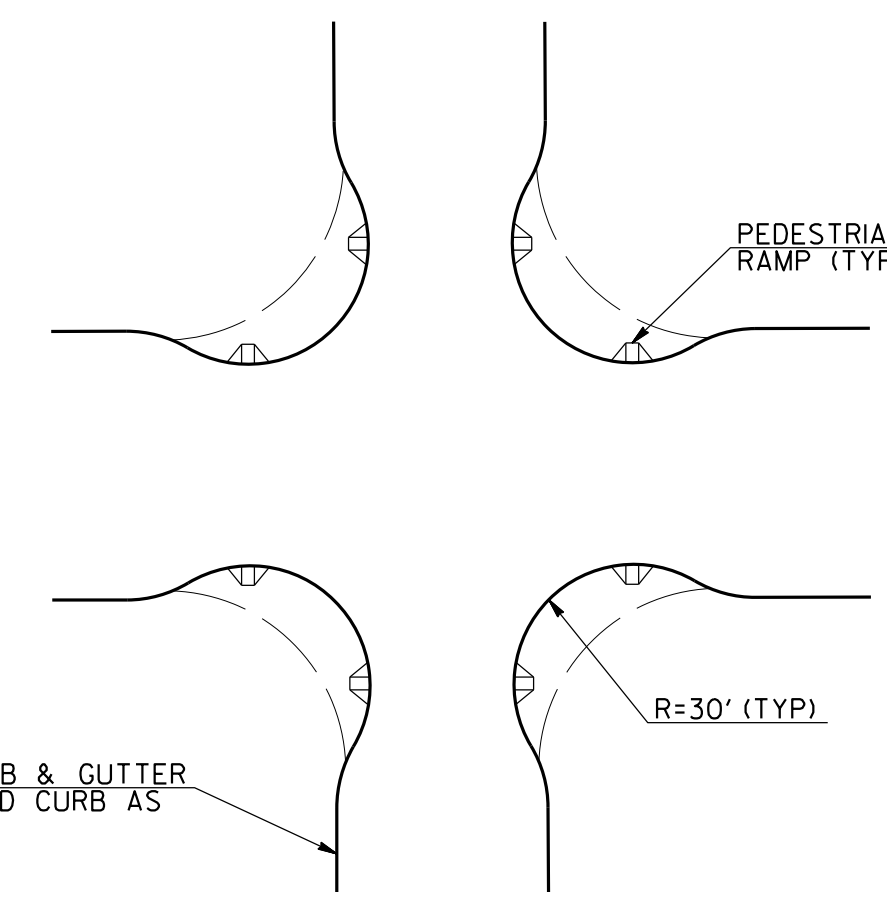
SECTION Z-Z SEDIMENT BASIN  
NO SCALE



SEDIMENT BASIN DETAIL (PVT)  
TYPICAL  
NO SCALE

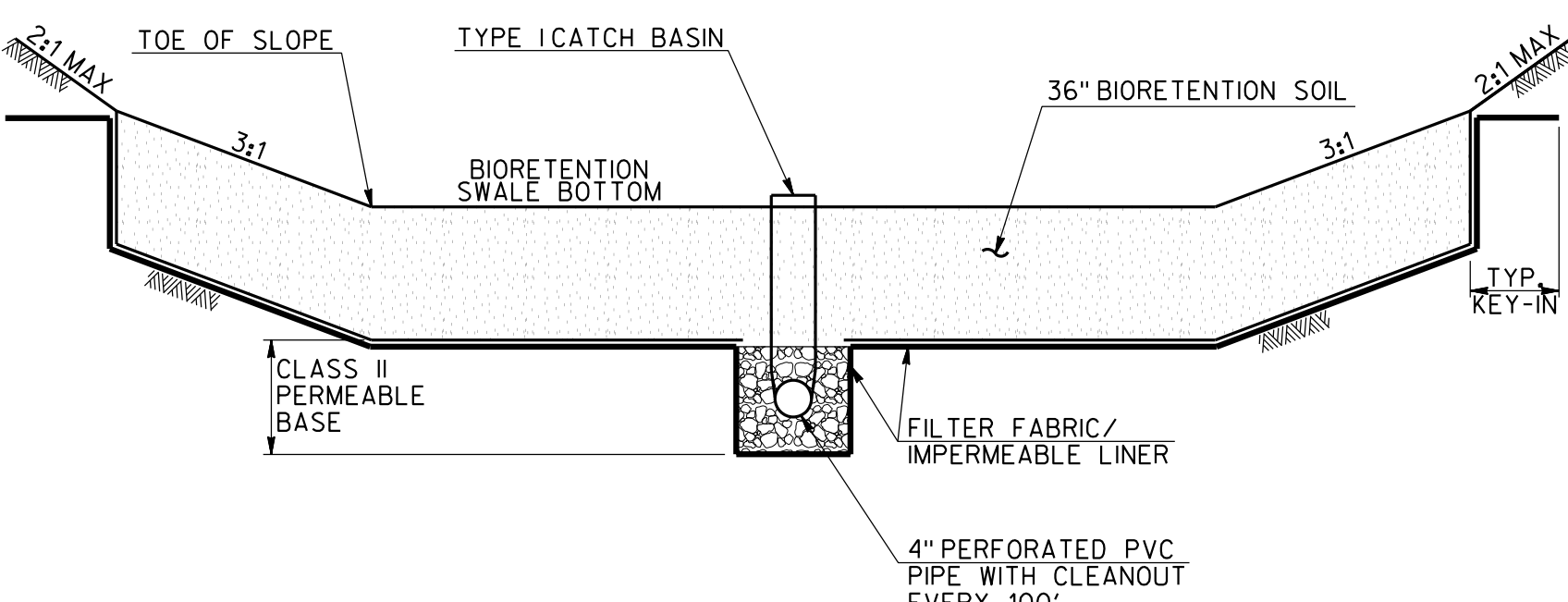


TYPICAL FUTURE RESIDENTIAL  
BLOCK UTILITY LATERAL  
DIAGRAM  
NO SCALE

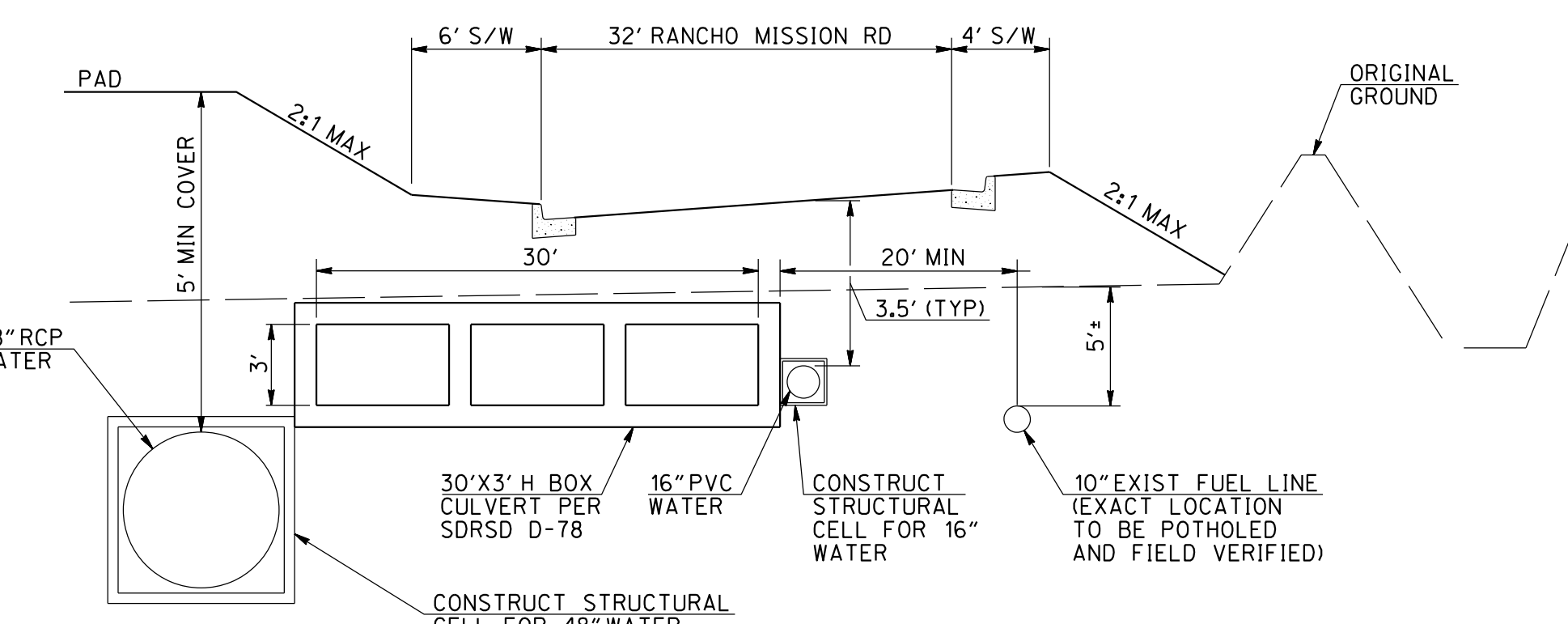


TYPICAL INTERSECTION CURB POPOUT  
NO SCALE

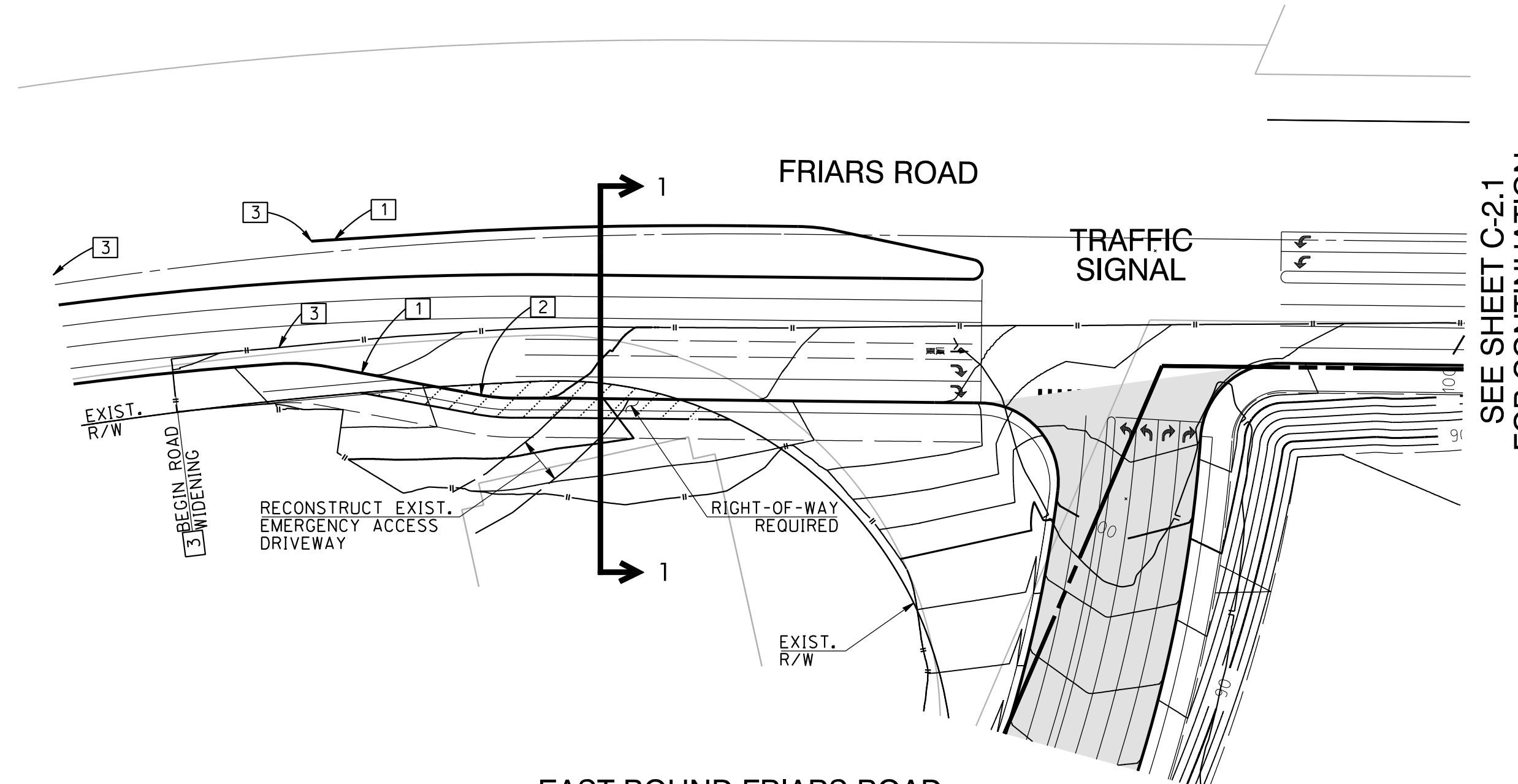
NOTE: CURB POPOUTS TO MEET MUTCD STANDARDS & FIRE ACCESSIBILITY STANDARDS



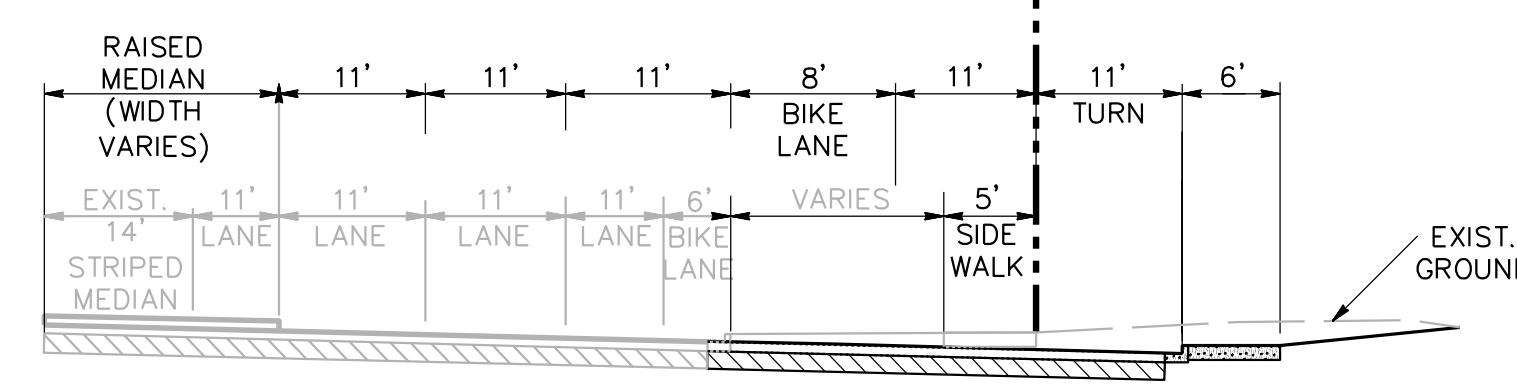
TYPICAL BIORETENTION SWALE DETAIL (PVT)  
NO SCALE



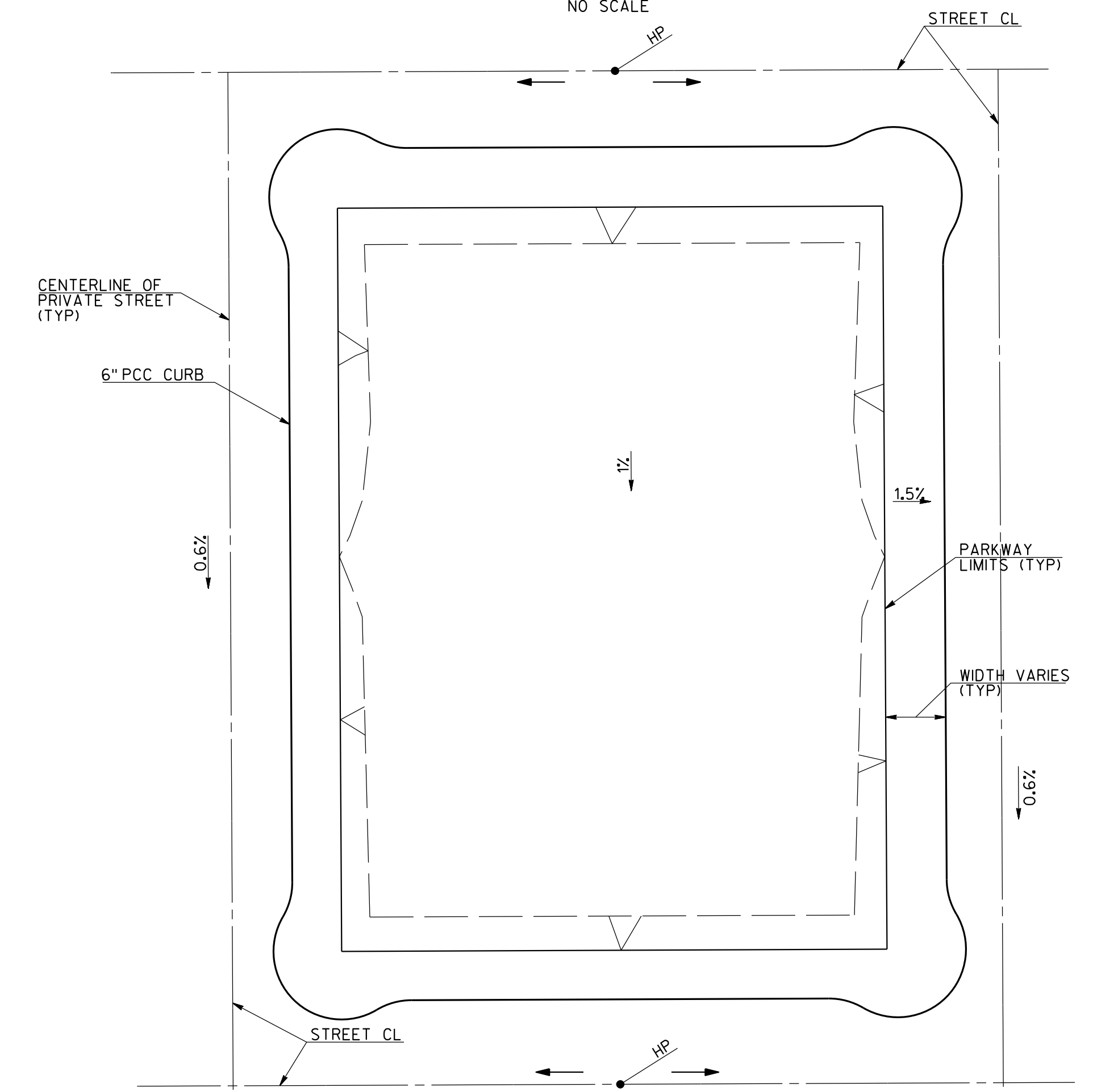
BOX CULVERT DETAIL (PVT)  
NO SCALE



EAST BOUND FRIARS ROAD  
WIDENING AT STADIUM WAY  
1\"/>



SECTION 1-1  
NOT TO SCALE



FUTURE RESIDENTIAL BLOCK  
PAD DESIGN  
NO SCALE

NOTE: PAD ELEVATION SET AT BLOCK MID POINT ELEVATION AT ROW

**CONTRACTOR NOTES**

- 1) ALL EXISTING DRY AND WET UTILITIES TO BE RELOCATED AS NEEDED TO ACCOMMODATE PROPOSED IMPROVEMENTS
- 2) ADDITIONAL RIGHT-OF-WAY REQUIRED
- 3) OFFSITE LETTER OF PERMISSION TO GRADE REQUIRED

**IMPROVEMENT CONSTRUCTION NOTES**

- 1) INSTALL 6\"/>

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(FAX) 619-291-4165



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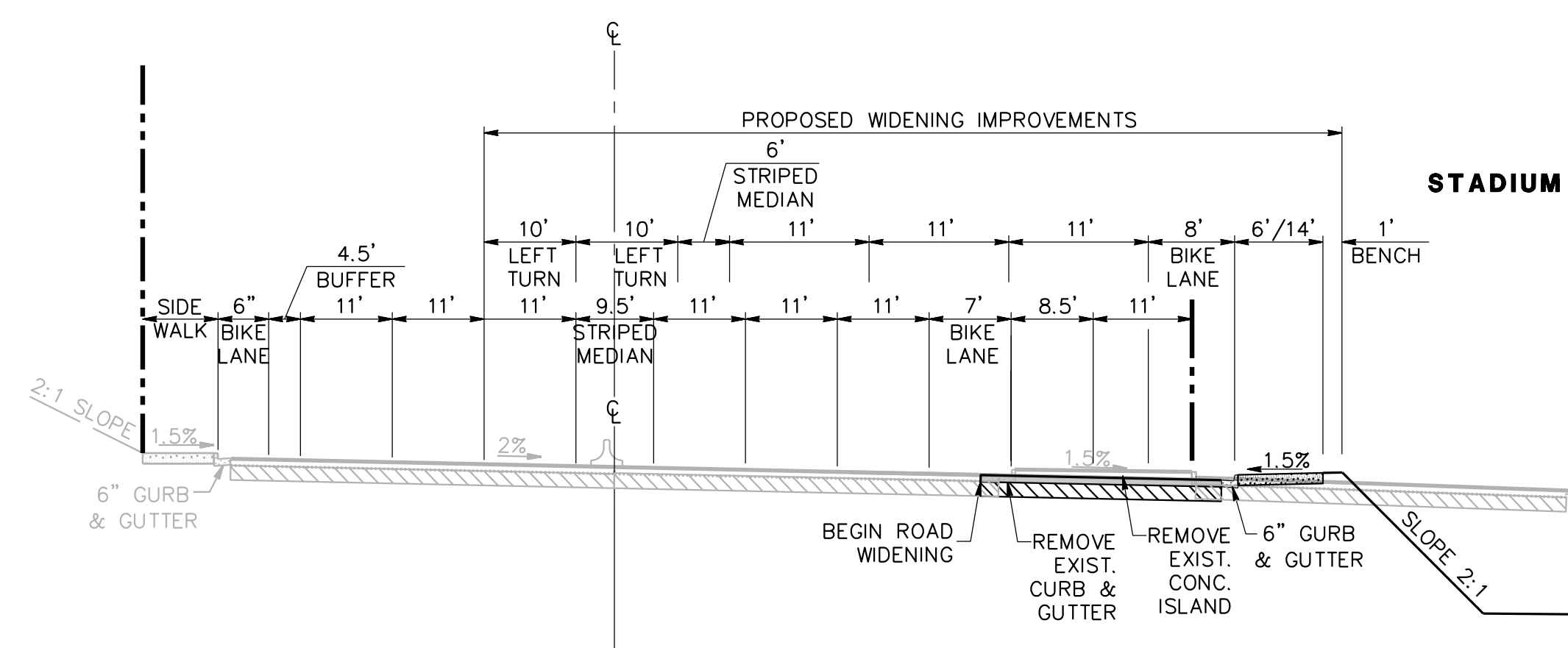
ISSUED: 01/18/19-RFP  
REV: 2/12/19

PROJECT NO:  
FILENAME:  
DRAWN BY: CHECKED BY:  
PLOT DATE:  
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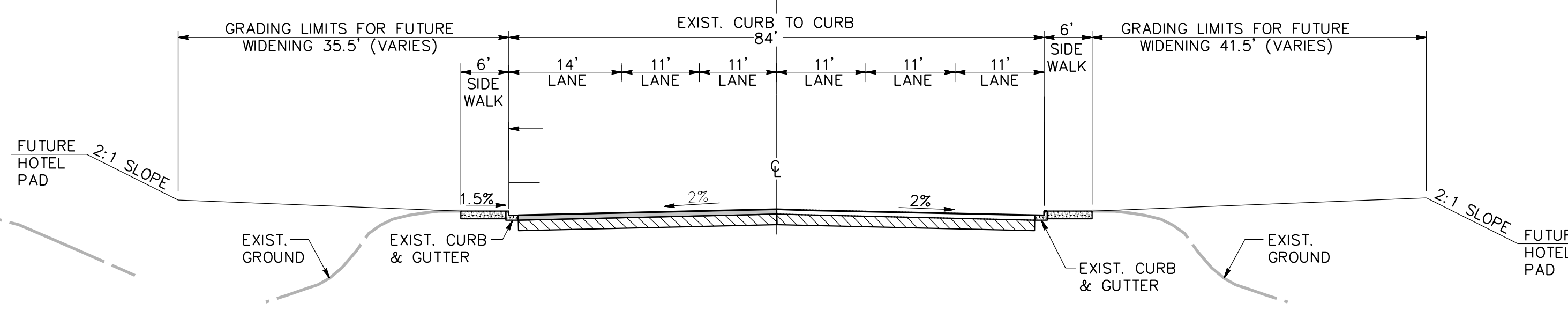
**DETAILS**



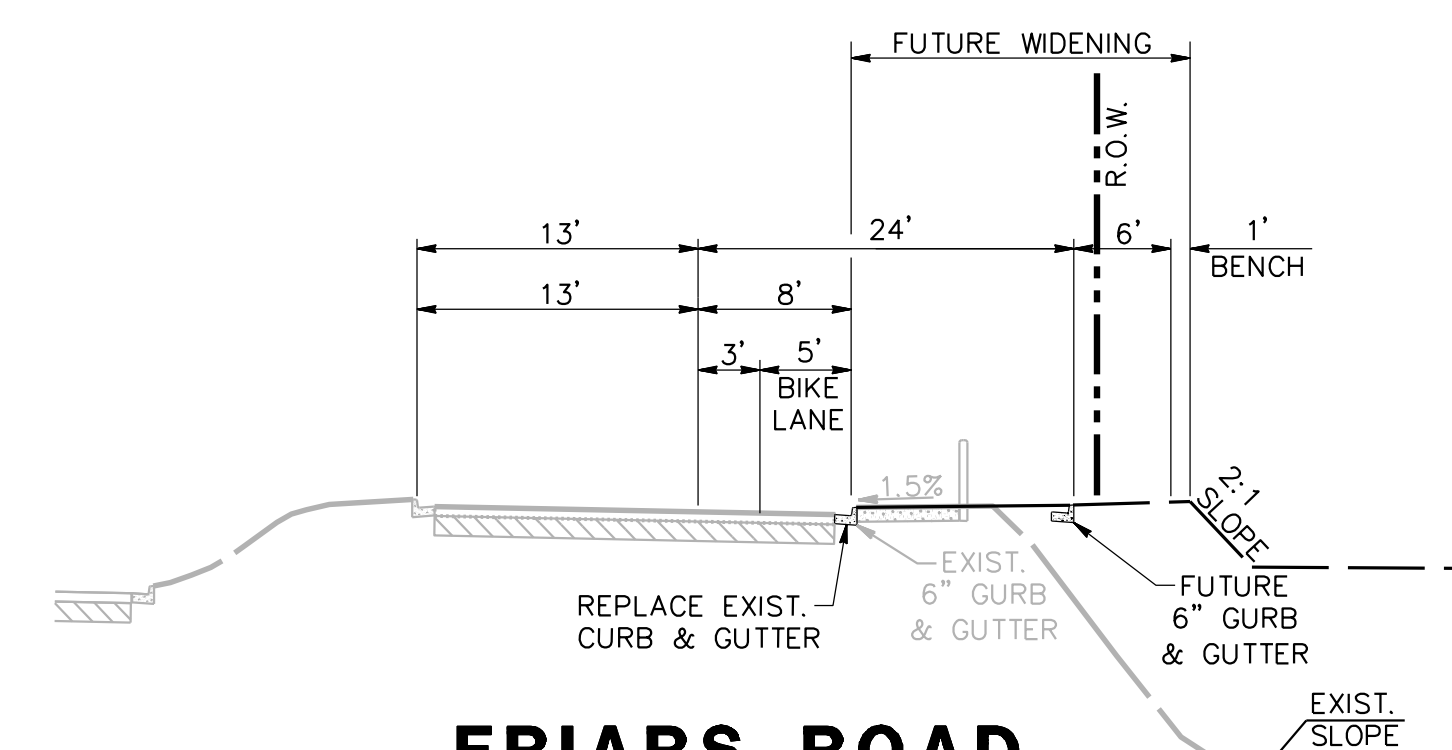
**PHASE 1A/1B  
(OPENING DAY TYPICAL STREET SECTIONS)**  
(SEE SHEET C-N5 FOR PHASE 4 STREET SECTIONS)



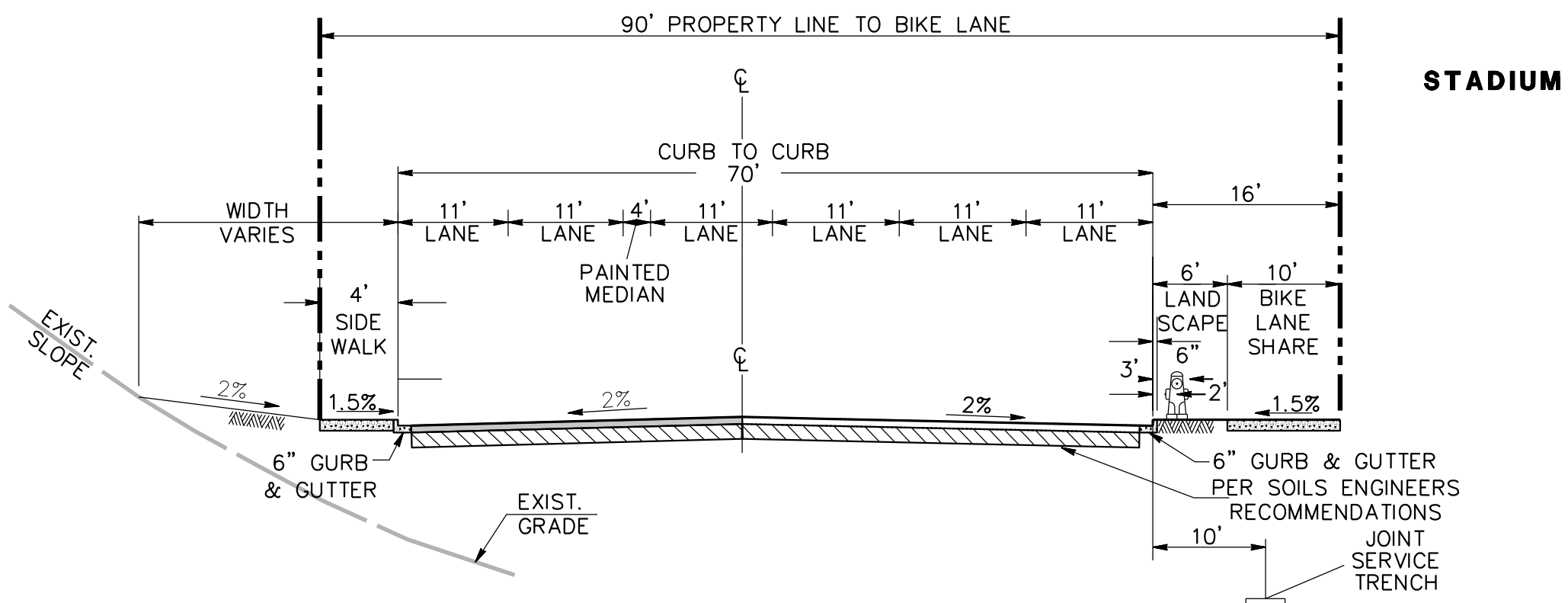
**FRIARS ROAD**  
MODIFIED 6-LANE COLLECTOR  
DESIGN SPEED: 45 MPH  
NOT TO SCALE



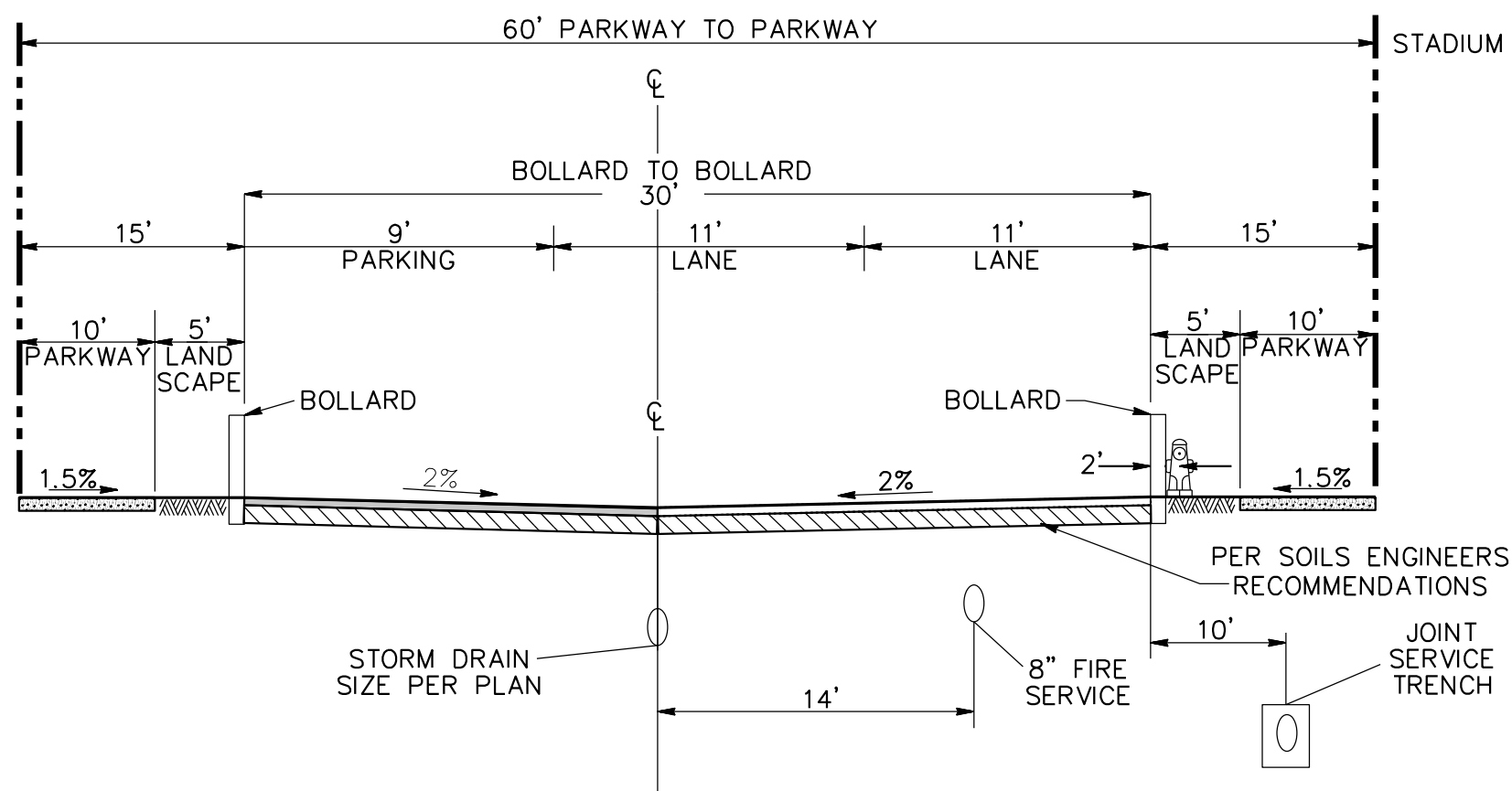
**MISSION VILLAGE DRIVE**  
SITE ENTRANCE  
NOT TO SCALE



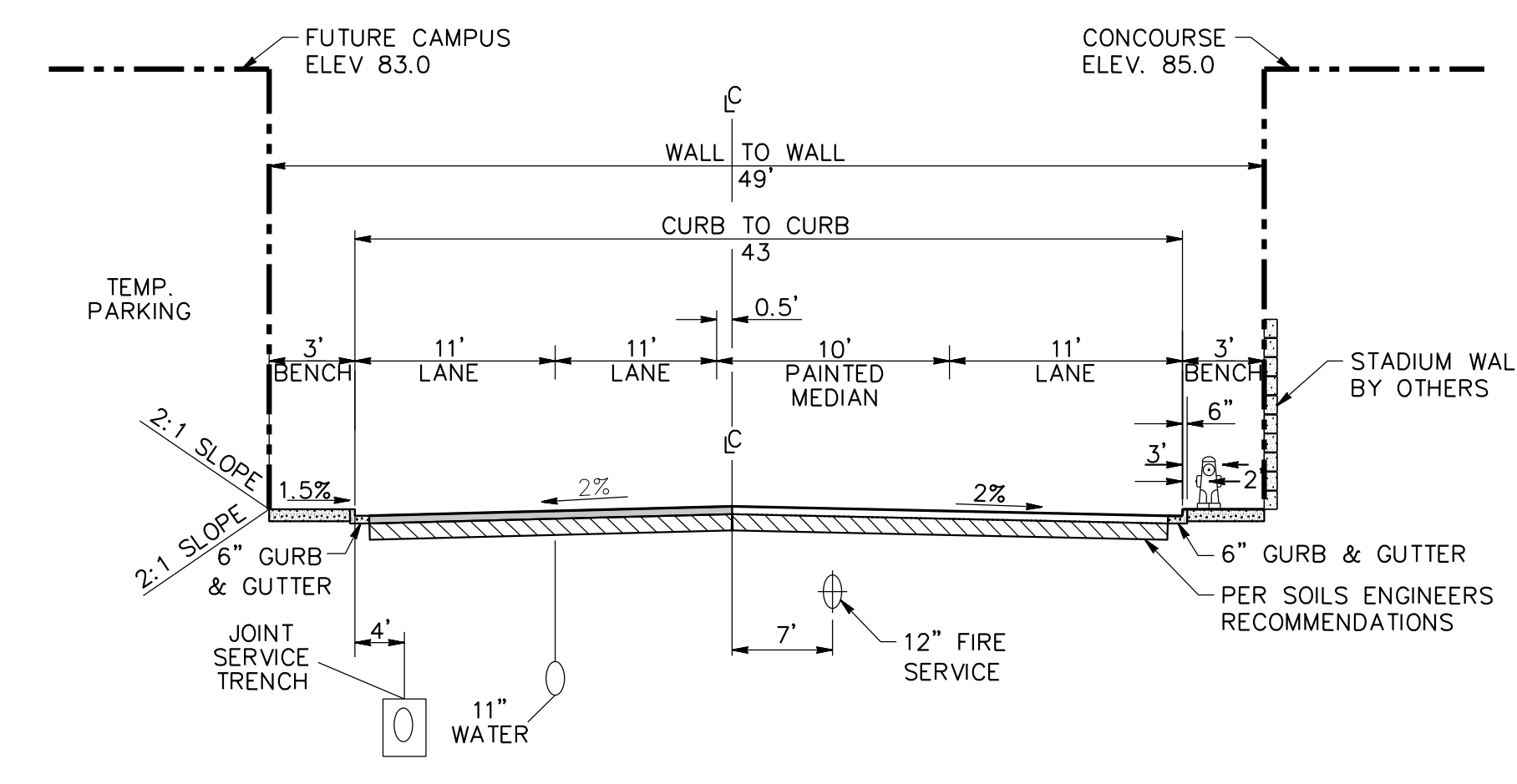
**FRIARS ROAD**  
EB OFFRAMP  
(SECTION 2-2)  
NOT TO SCALE



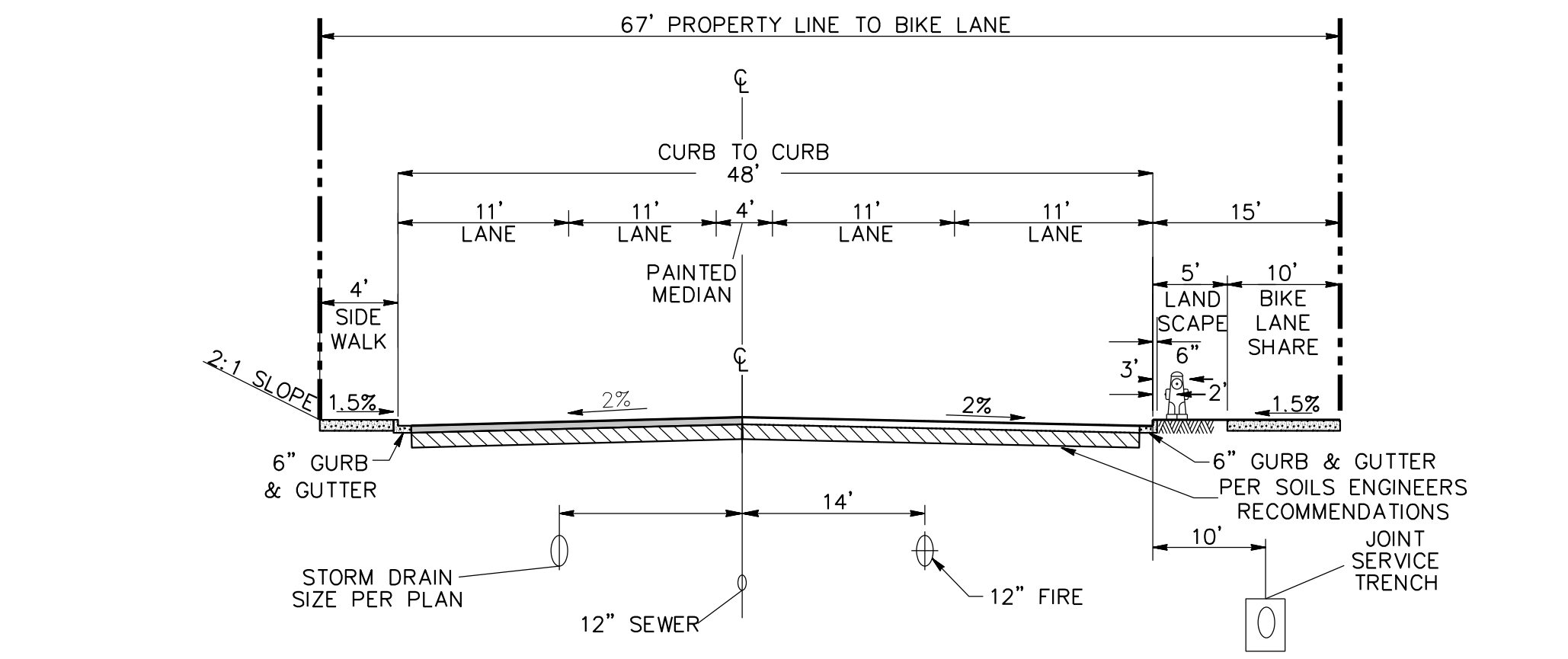
**STADIUM WAY**  
(BETWEEN FRIARS ROAD AND PROMENADE "2")  
MODIFIED 6-LANE MAJOR COLLECTOR  
DESIGN SPEED: 35 MPH  
NOT TO SCALE



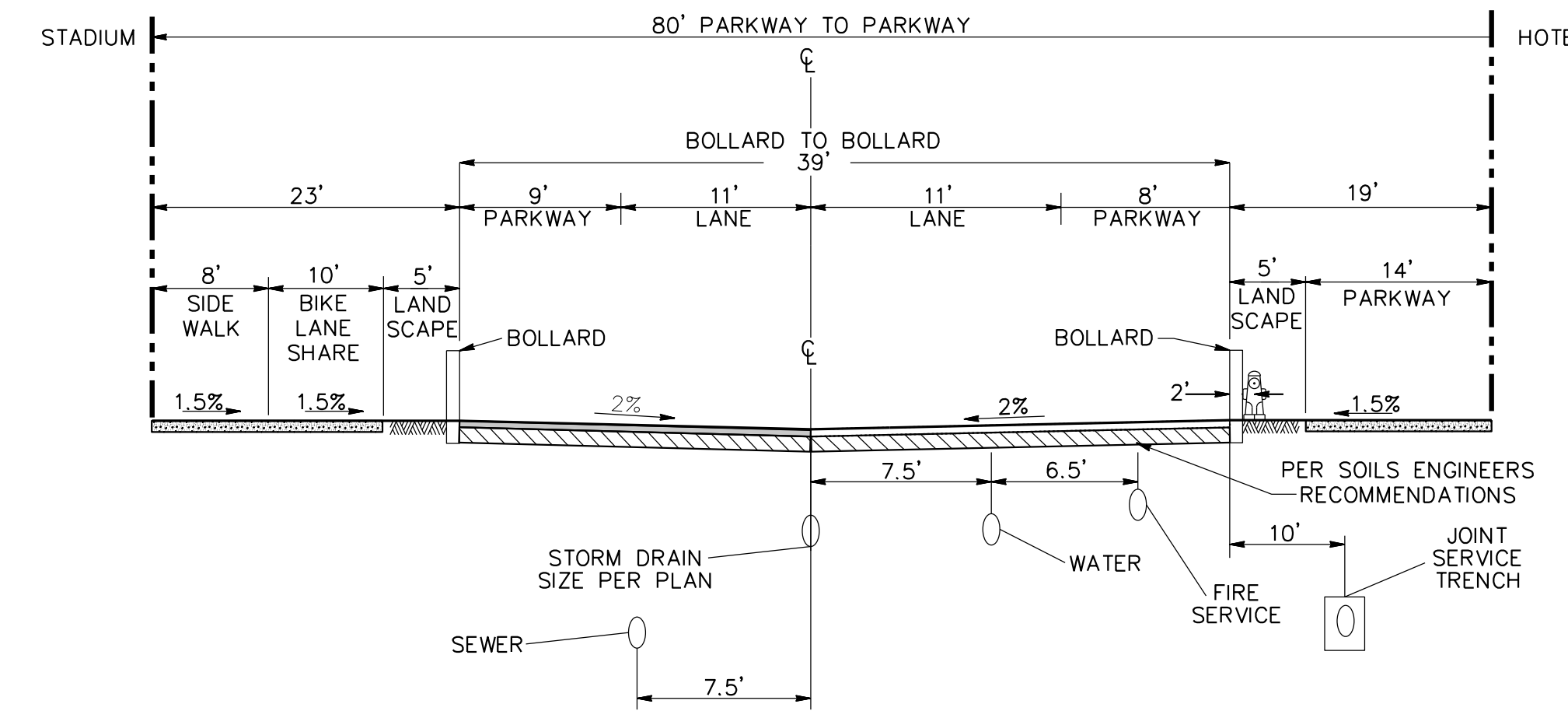
**PROMENADE "A"**  
MODIFIED 2-LANE COLLECTOR  
WITHOUT BIKE LANE  
DESIGN SPEED: 25 MPH  
NOT TO SCALE



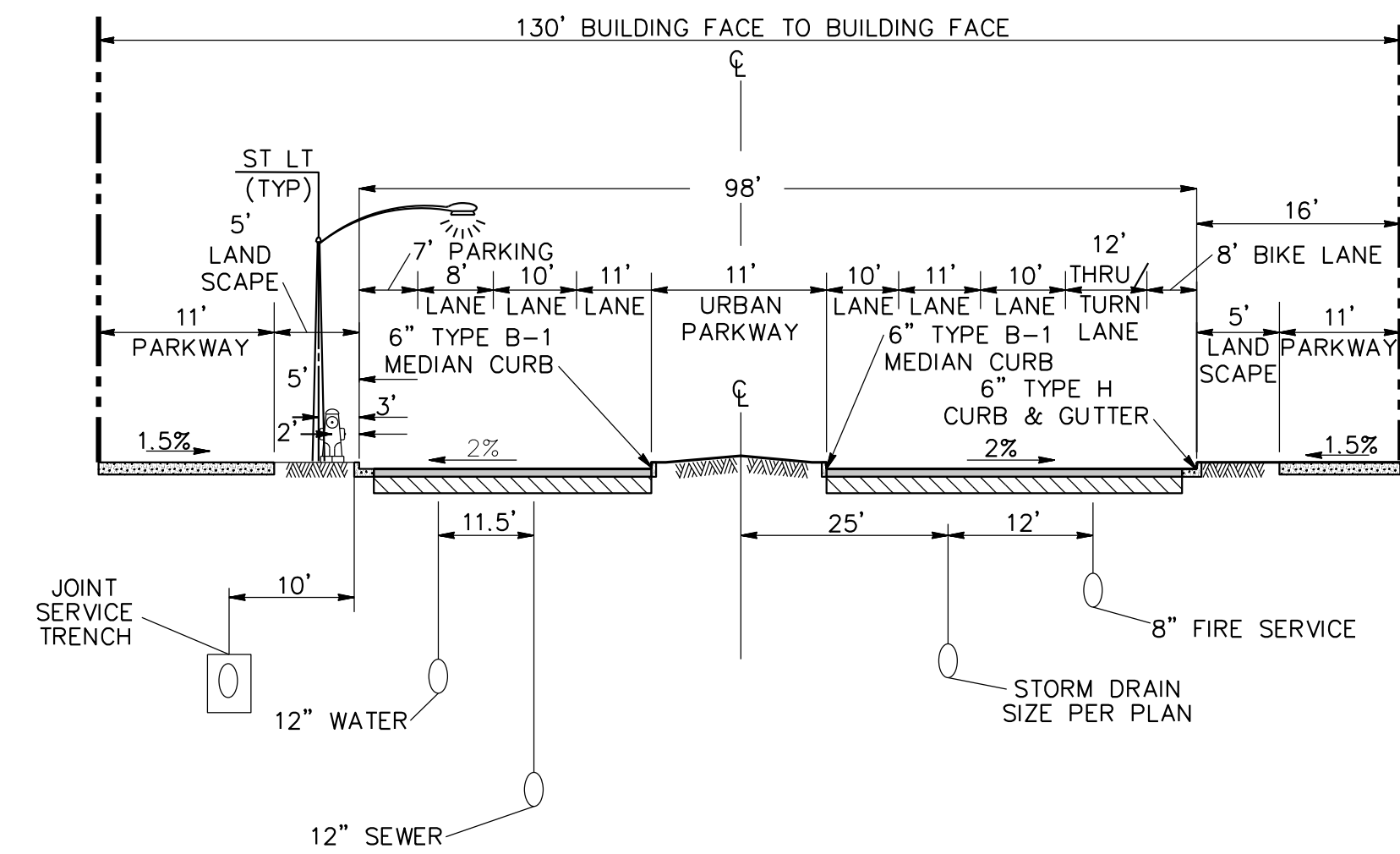
**PROMENADE 2**  
MODIFIED 2-LANE COLLECTOR  
WITHOUT BIKE LANE  
DESIGN SPEED: 25 MPH  
NOT TO SCALE



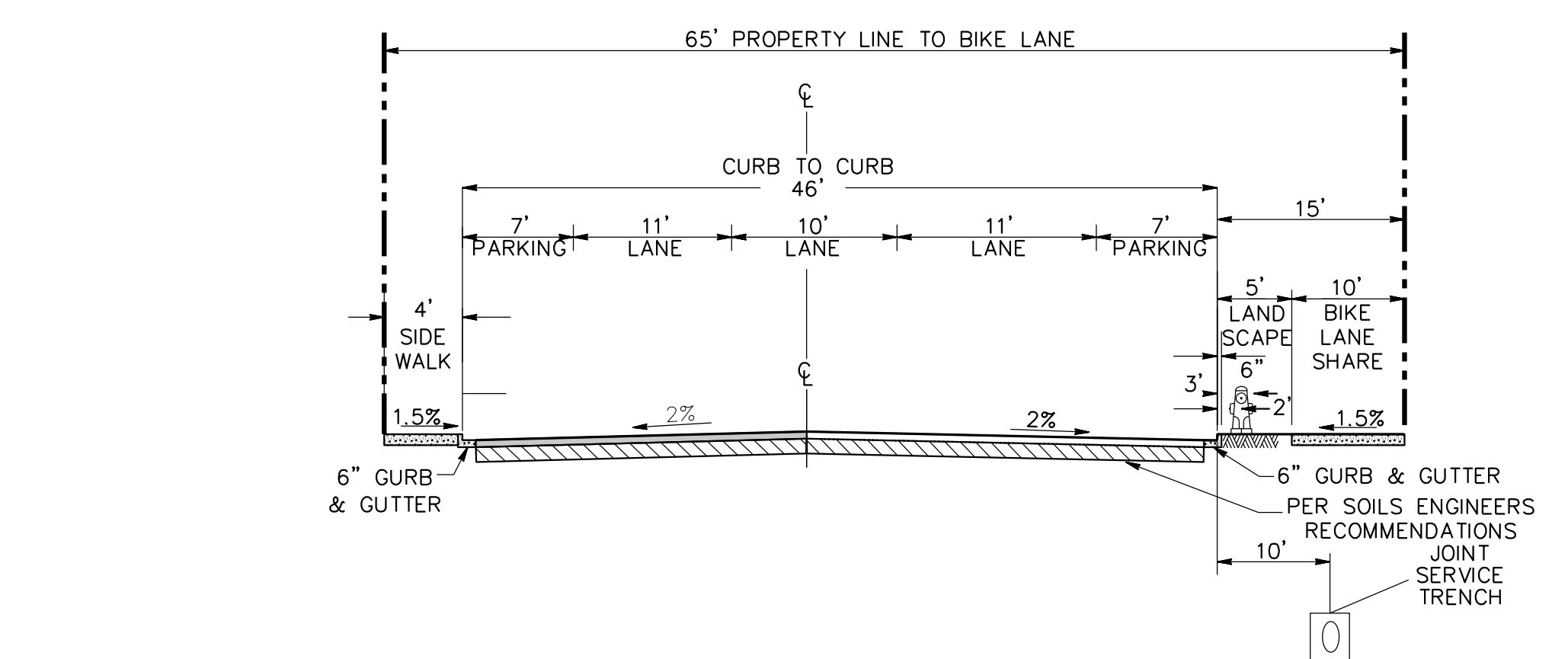
**STADIUM WAY**  
(BETWEEN PROMENADE "2" AND  
TEMPORARY PARKING ENTRANCE)  
MODIFIED 2-LANE COLLECTOR  
WITH LEFT TURN LANE  
DESIGN SPEED: 30 MPH  
NOT TO SCALE



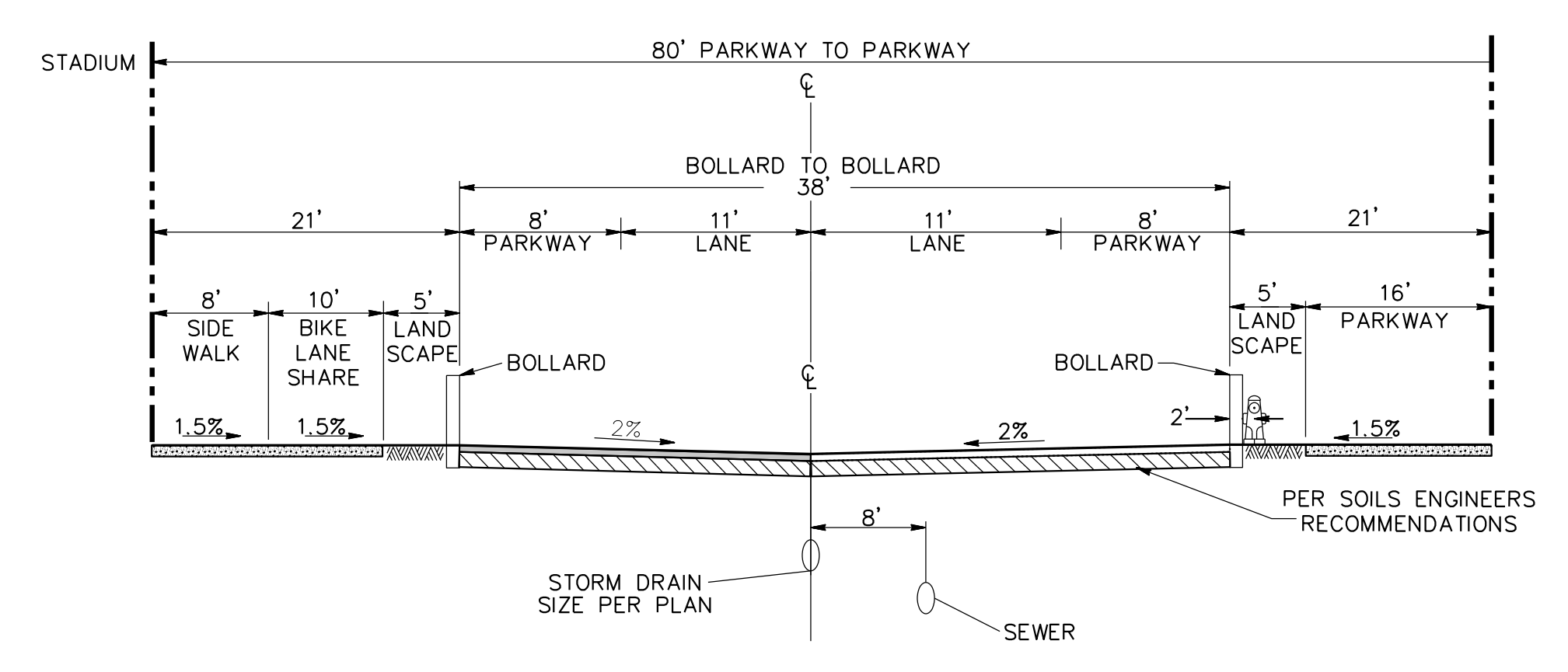
**PROMENADE "1"**  
MODIFIED 2-LANE COLLECTOR  
WITHOUT BIKE LANE  
DESIGN SPEED: 25 MPH  
NOT TO SCALE



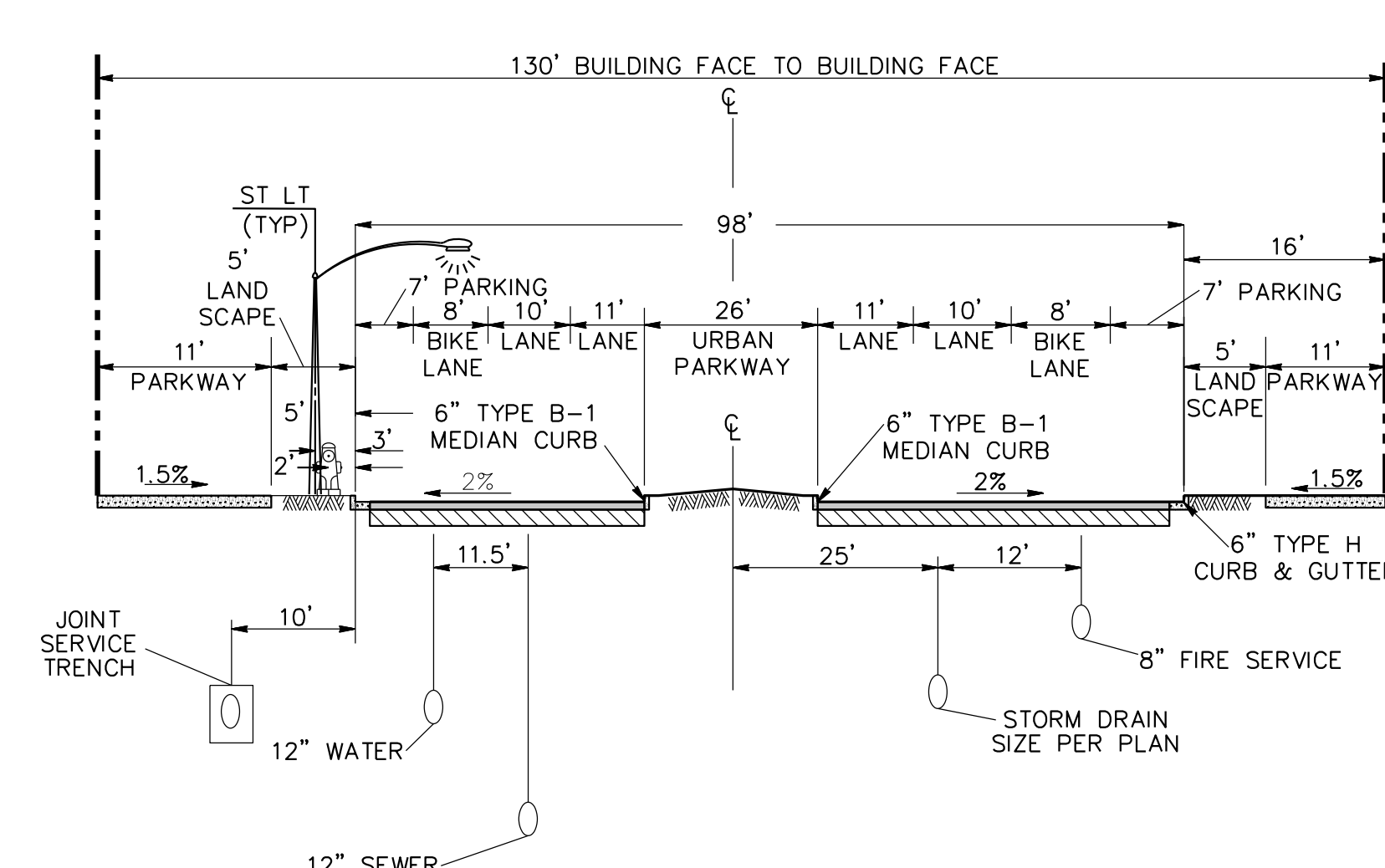
**AZTEC DRIVE AT  
PROMENADE 1 INTERSECTION**  
MODIFIED 4-LANE URBAN MAJOR  
WITH BIKE LANES  
DESIGN SPEED: 25 MPH  
NOT TO SCALE



**RIVER PARK ROAD/STADIUM WAY**  
(BETWEEN TEMPORARY PARKING AND RIVER PARK ROAD)  
MODIFIED 2-LANE COLLECTOR  
WITH TWO WAY LEFT TURN LANE  
DESIGN SPEED: 25 MPH  
NOT TO SCALE



**CORYELL PASS**  
MODIFIED 2-LANE COLLECTOR  
WITHOUT BIKE LANE  
DESIGN SPEED: 25 MPH  
NOT TO SCALE



**AZTEC DRIVE**  
MODIFIED 4-LANE URBAN COLLECTOR  
WITH LEFT TURN POCKETS  
DESIGN SPEED: 25 MPH  
NOT TO SCALE

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619-291-4165  
**RICK**  
ENGINEERING COMPANY



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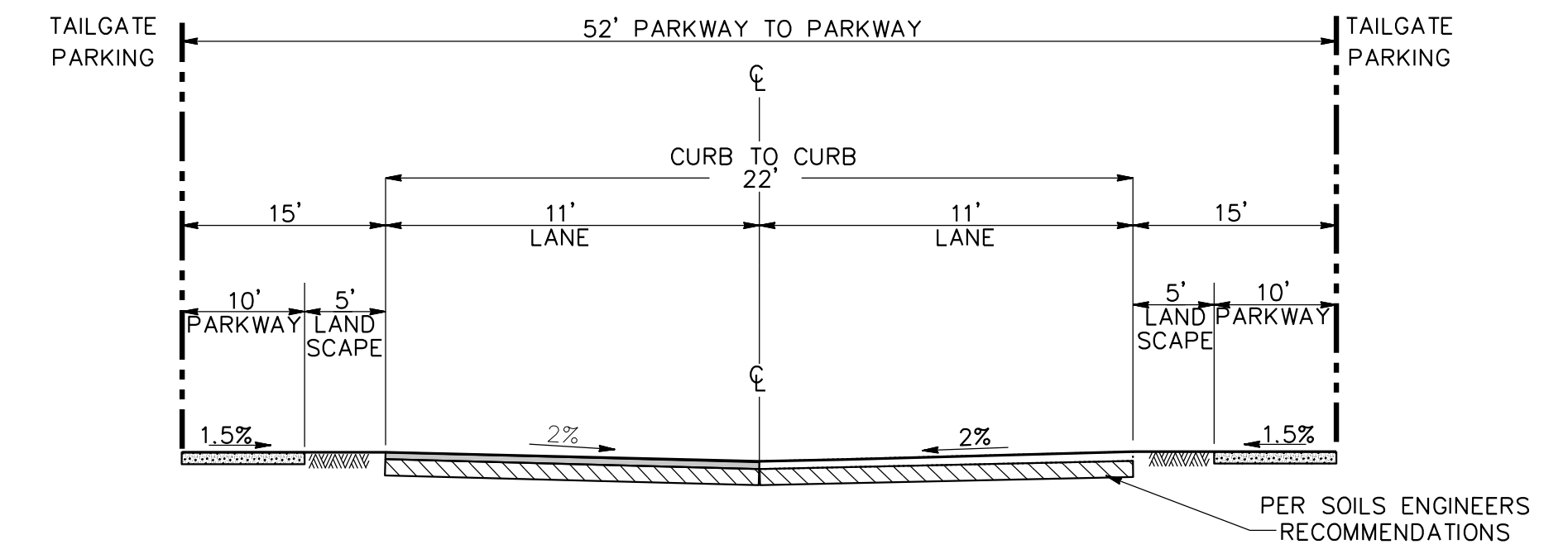
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DRAWN BY: CHECKED BY:  
PLOT DATE:  
TITLE:

**PH 1A/1B  
SECTIONS**

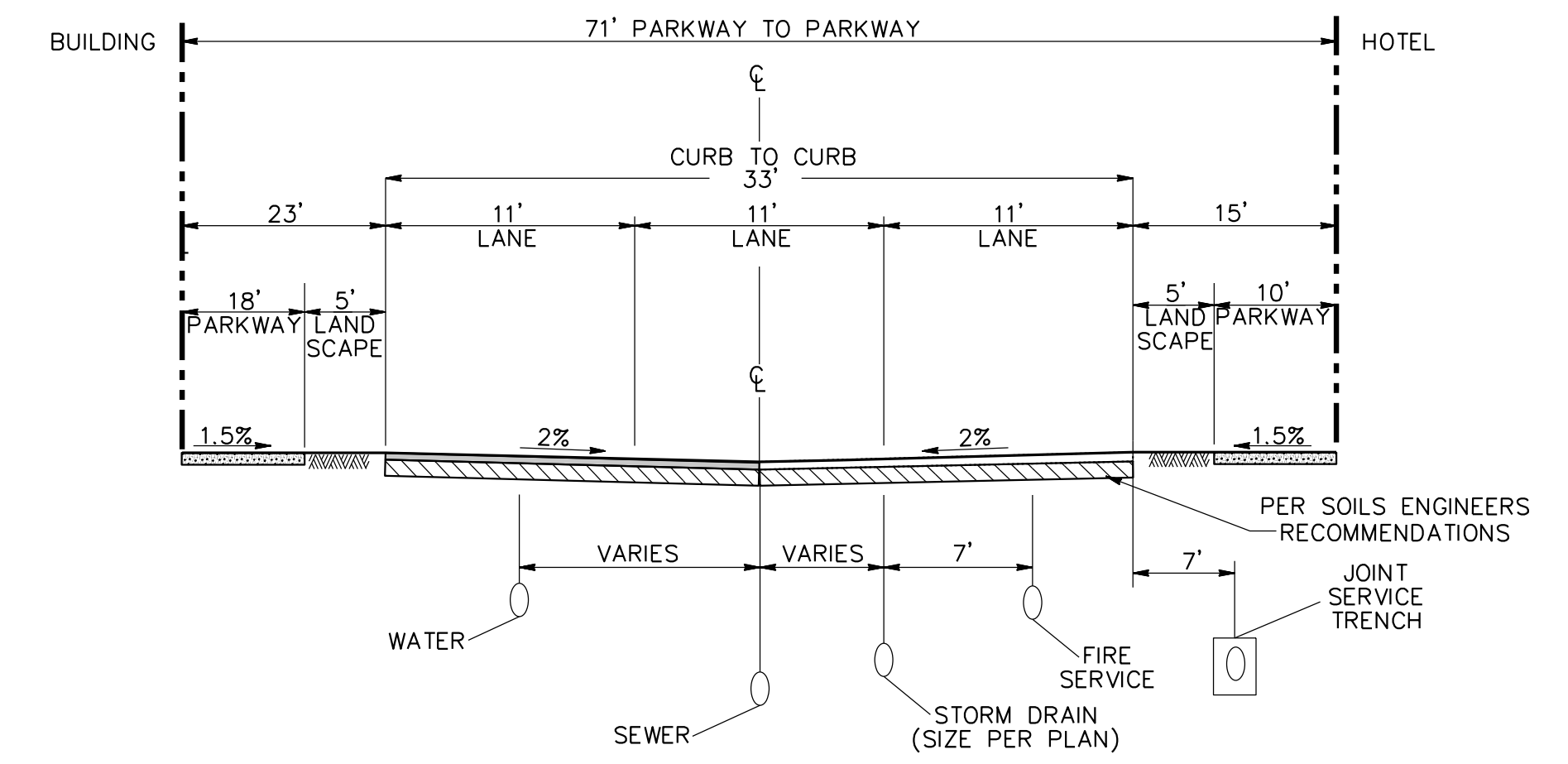
3 OF 23  
DRAWING NO:

**C-N3**

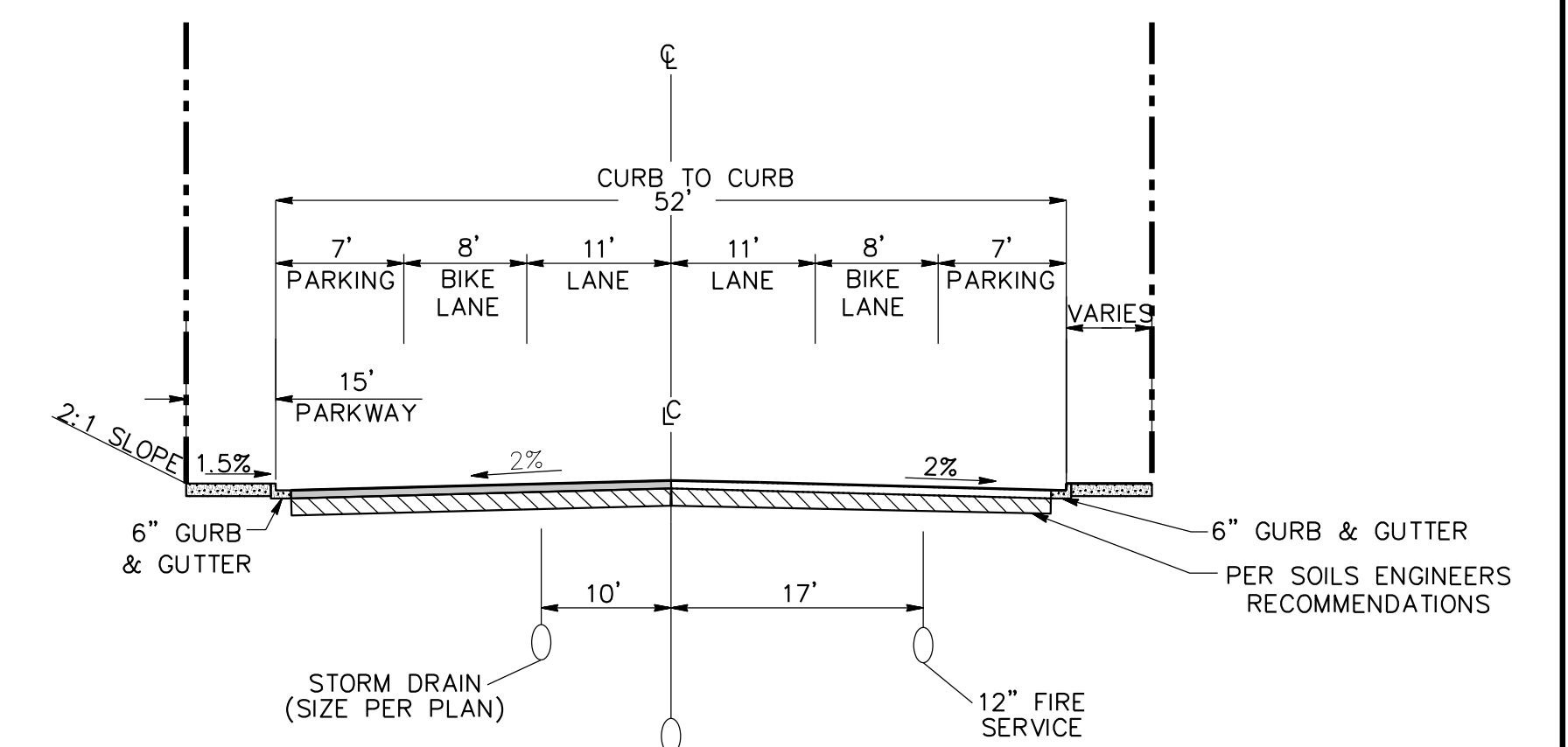
**PHASE 1A/1B**  
**(OPENING DAY TYPICAL STREET SECTIONS)**  
(SEE SHEET C-N5 FOR PHASE 4 STREET SECTIONS)



**TAILGATE STREET**  
**MODIFIED 2-LANE COLLECTOR**  
**WITHOUT BIKE LANE**  
**DESIGN SPEED: 25 MPH**  
**NOT TO SCALE**



**PROMENADE 1**  
**(BETWEEN CORYELL PASS AND AZTEC WAY)**  
**MODIFIED 2-LANE COLLECTOR**  
**WITHOUT BIKE LANE**  
**DESIGN SPEED: 25 MPH**  
**NOT TO SCALE**



**AZTEC WAY**  
**MODIFIED 2-LANE COLLECTOR**  
**WITH BIKE LANES**  
**DESIGN SPEED: 25 MPH**  
**NOT TO SCALE**

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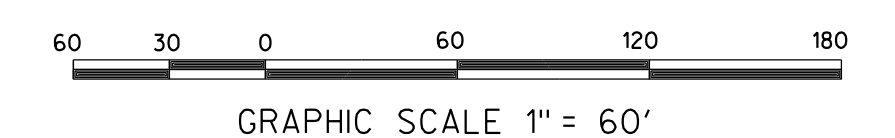
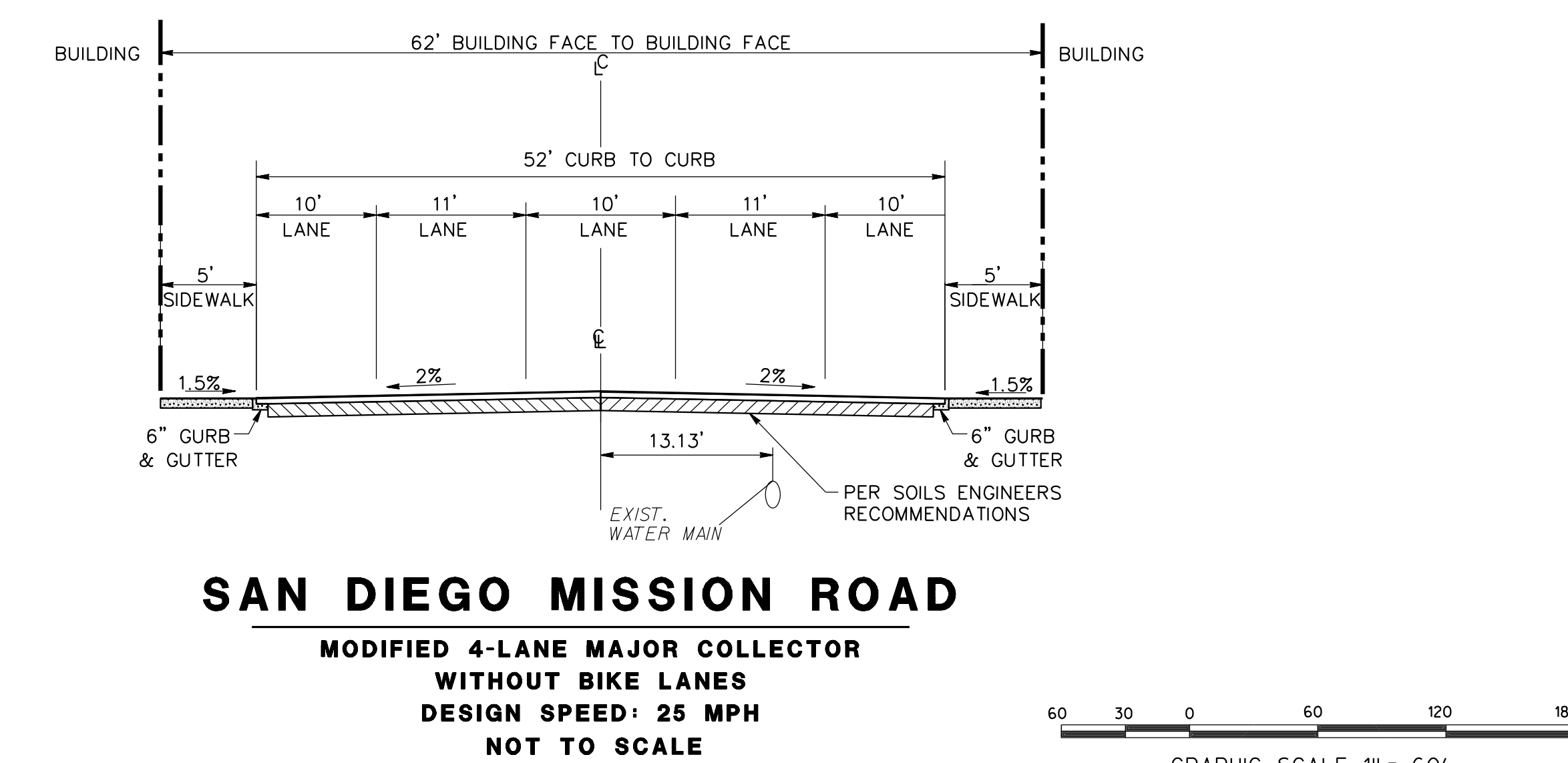
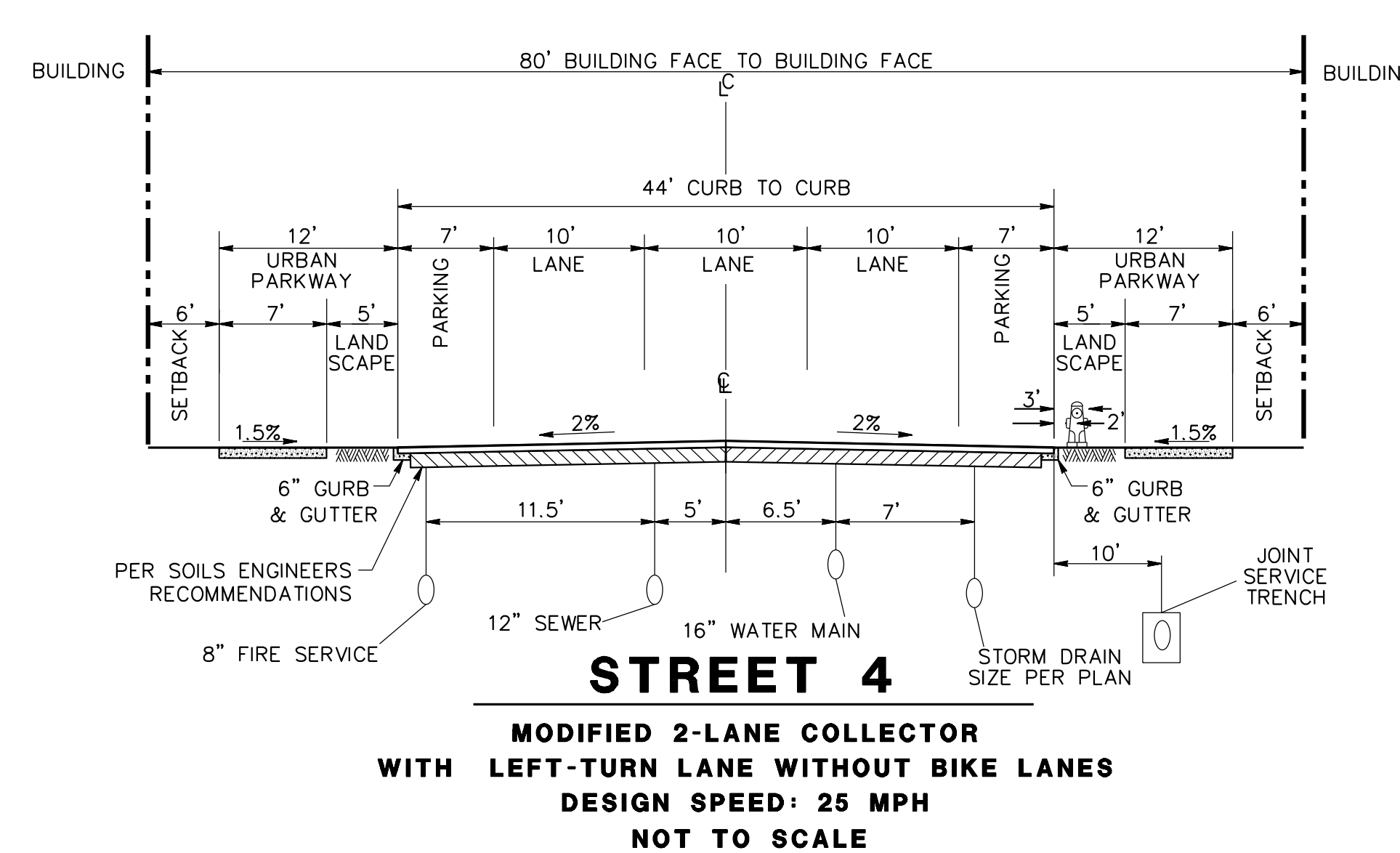
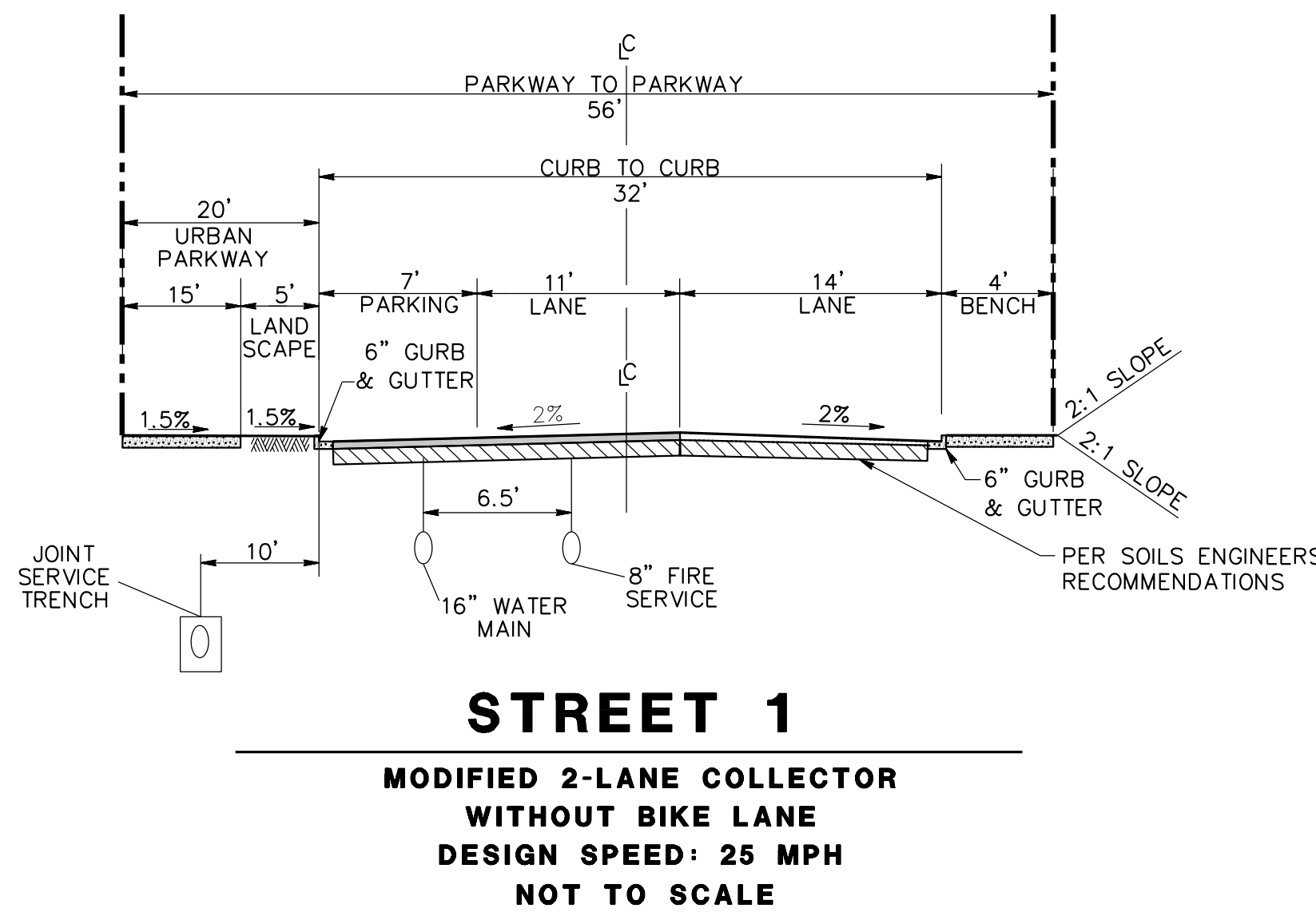
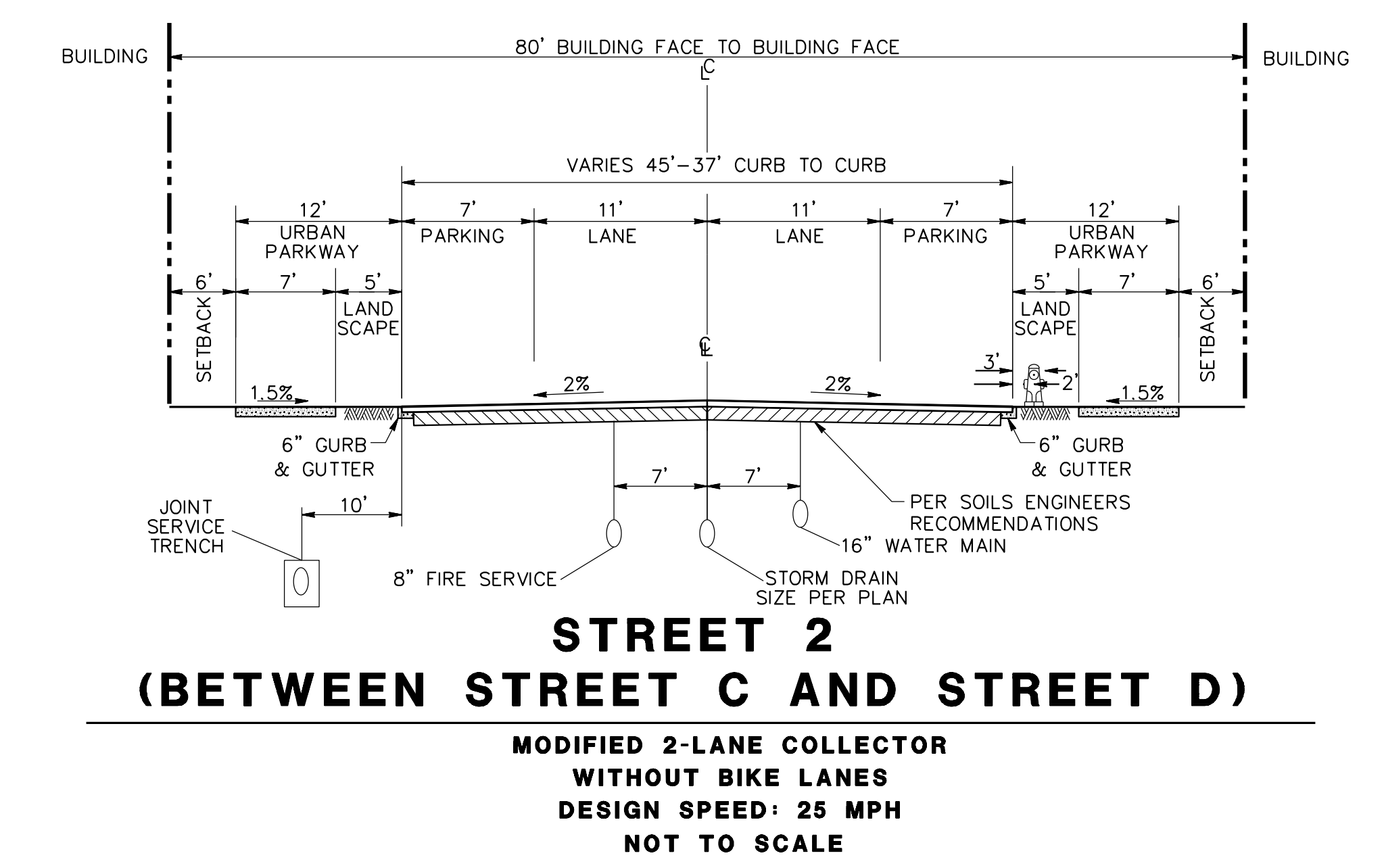
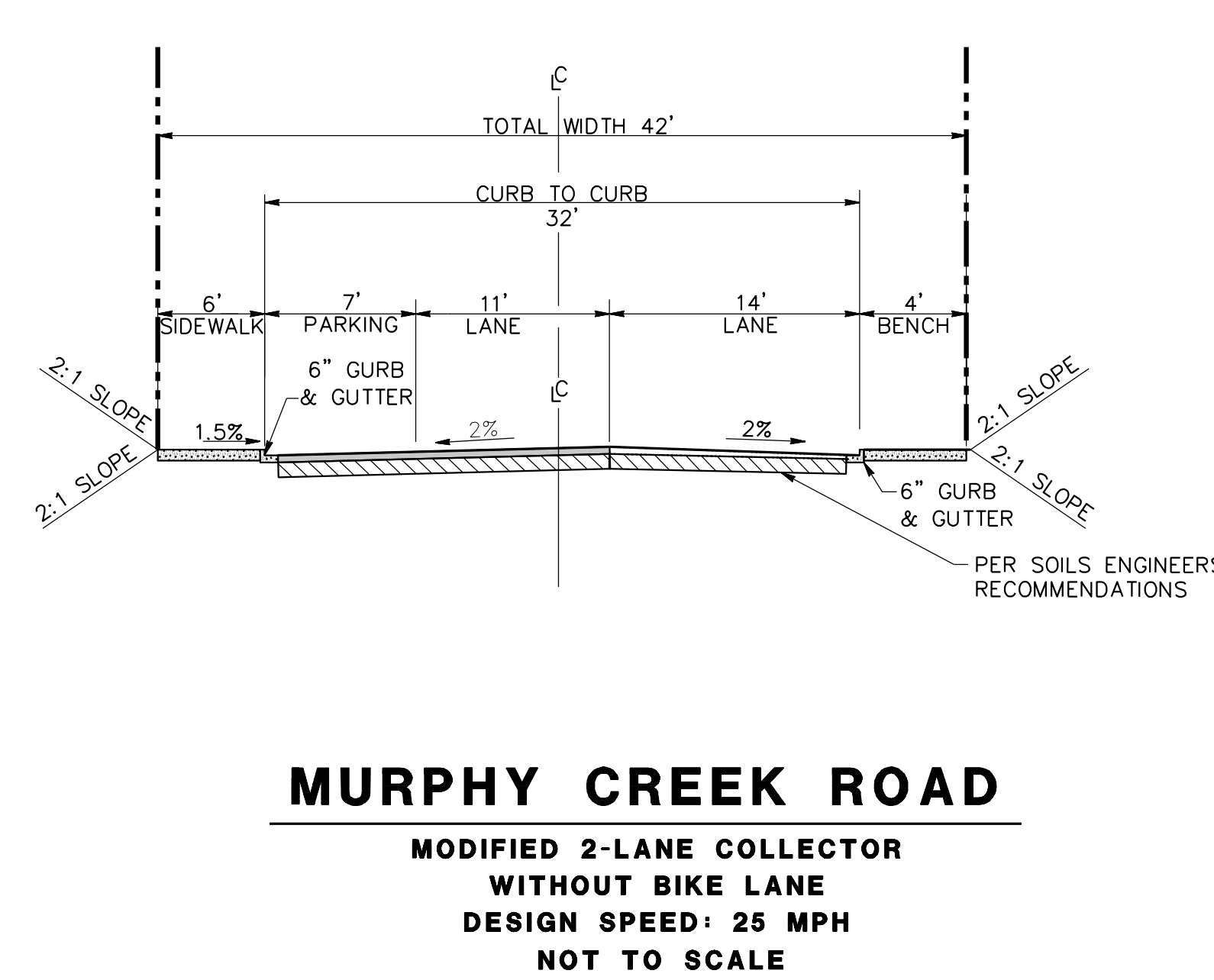
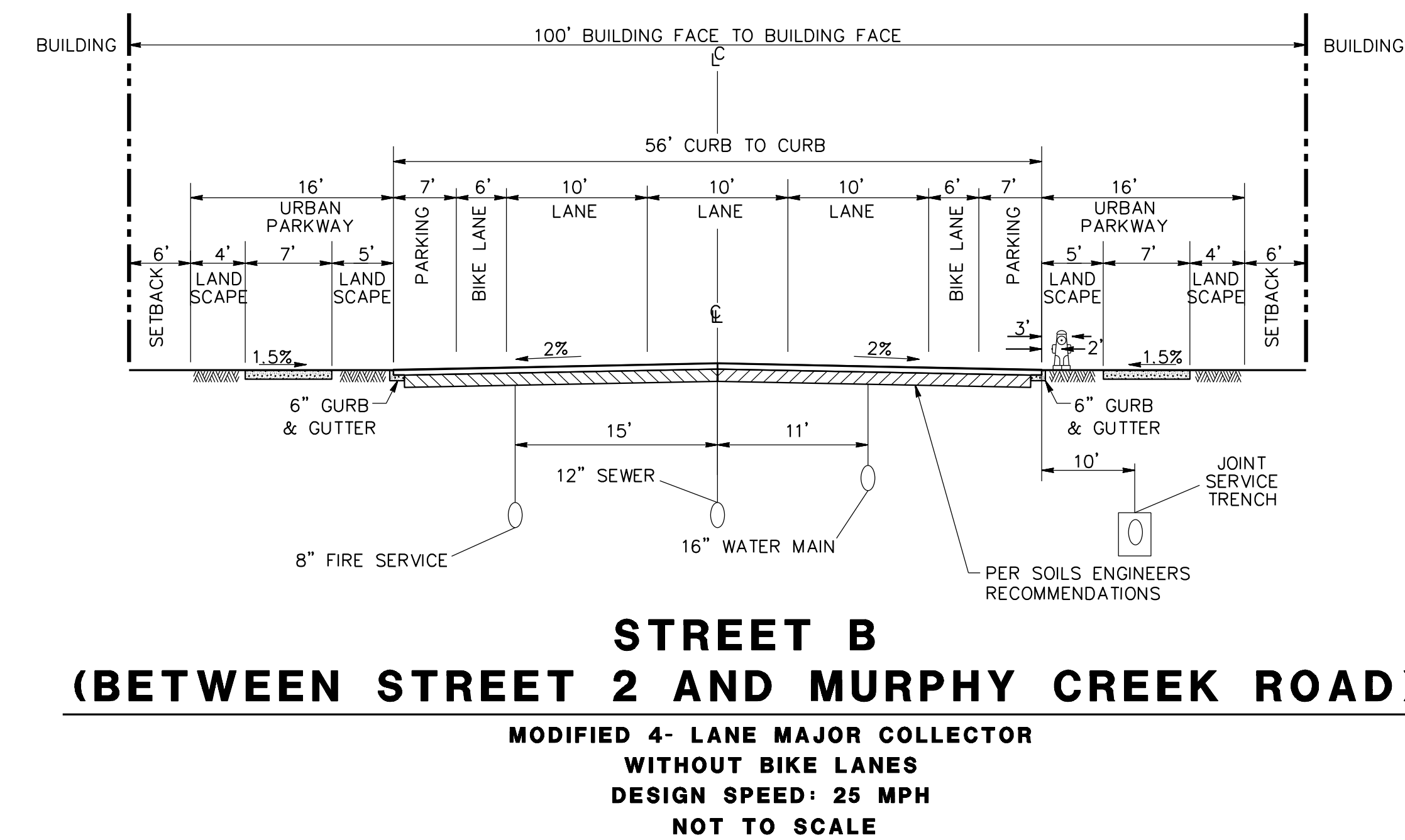
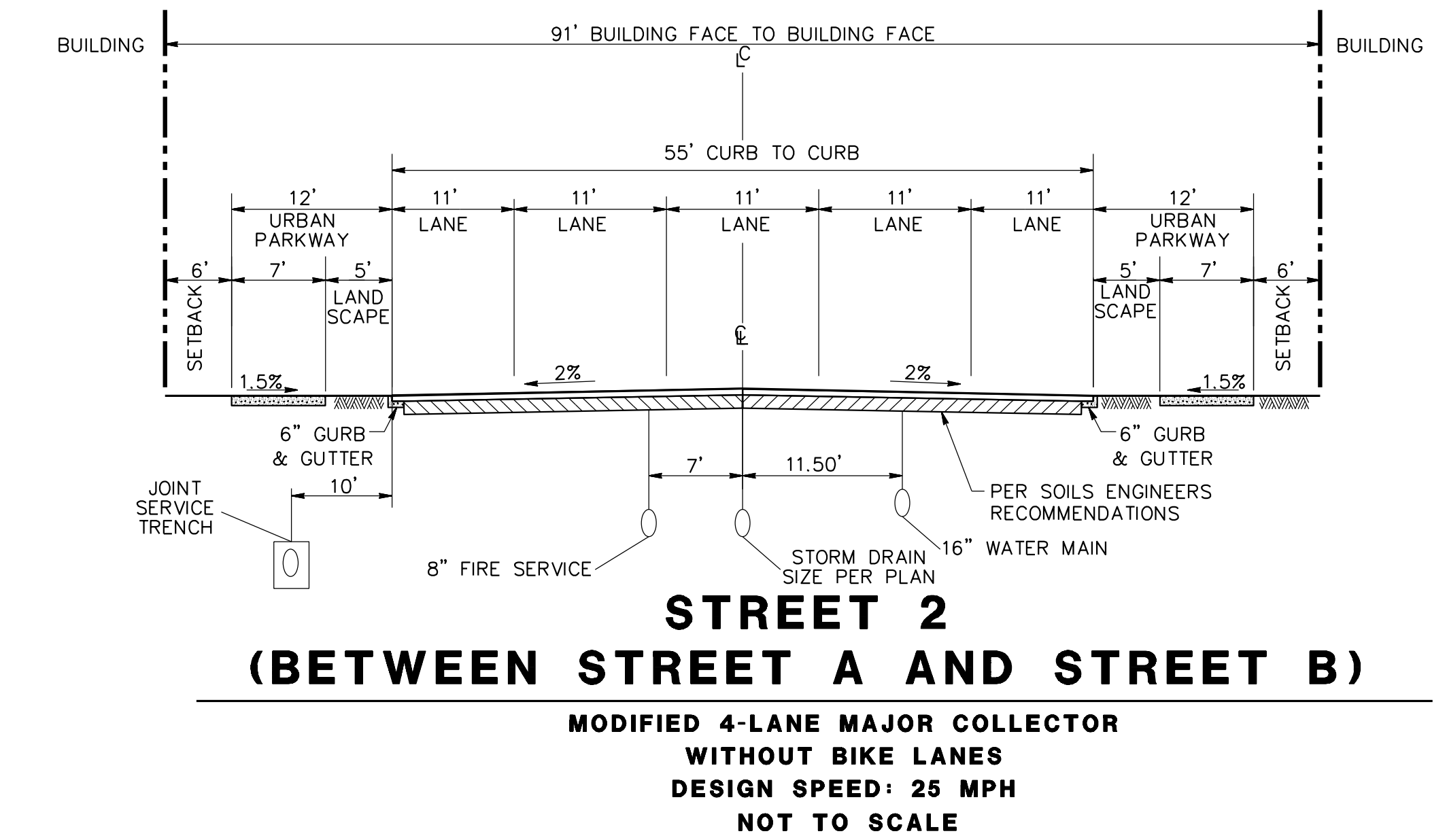
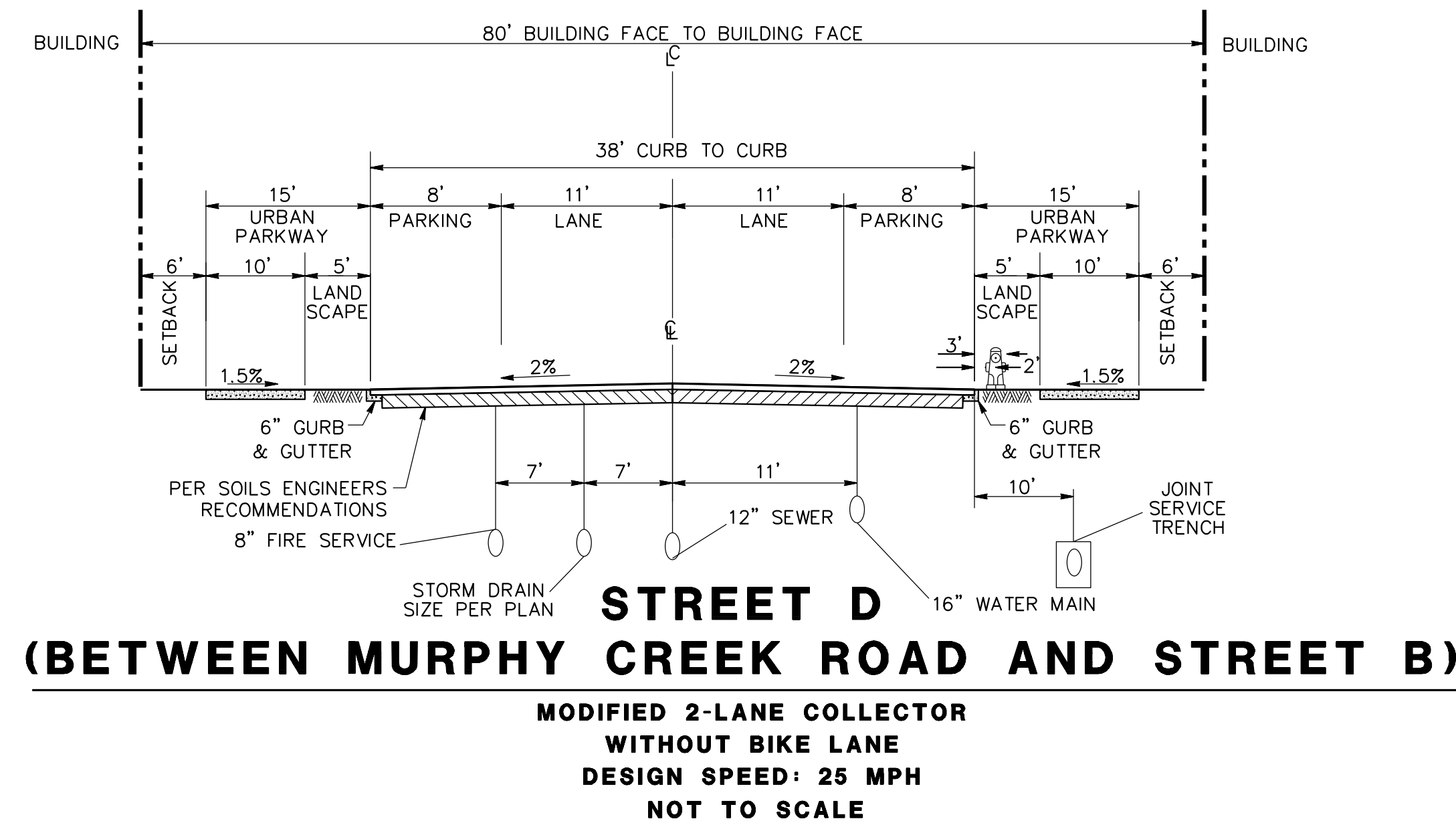
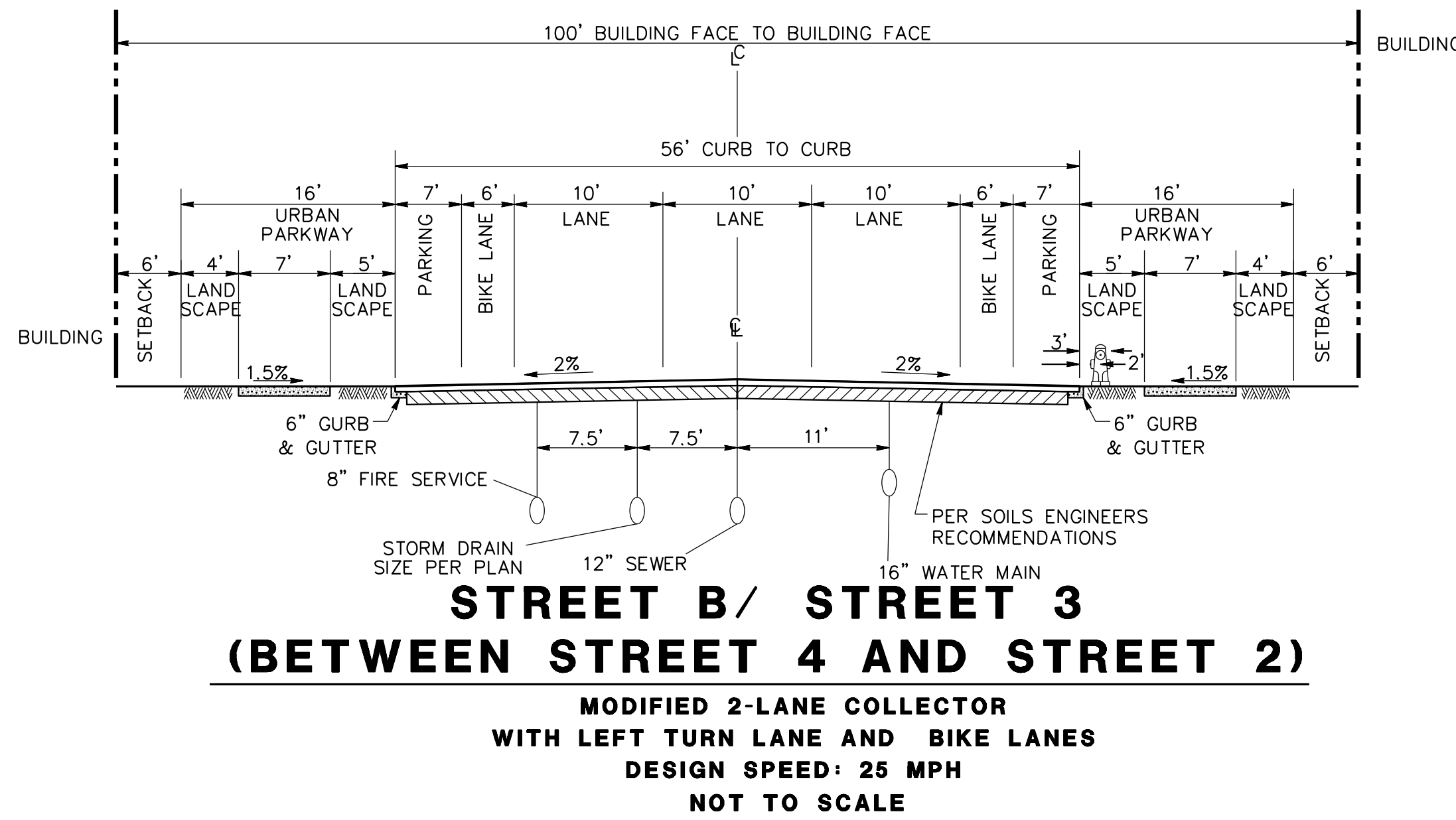
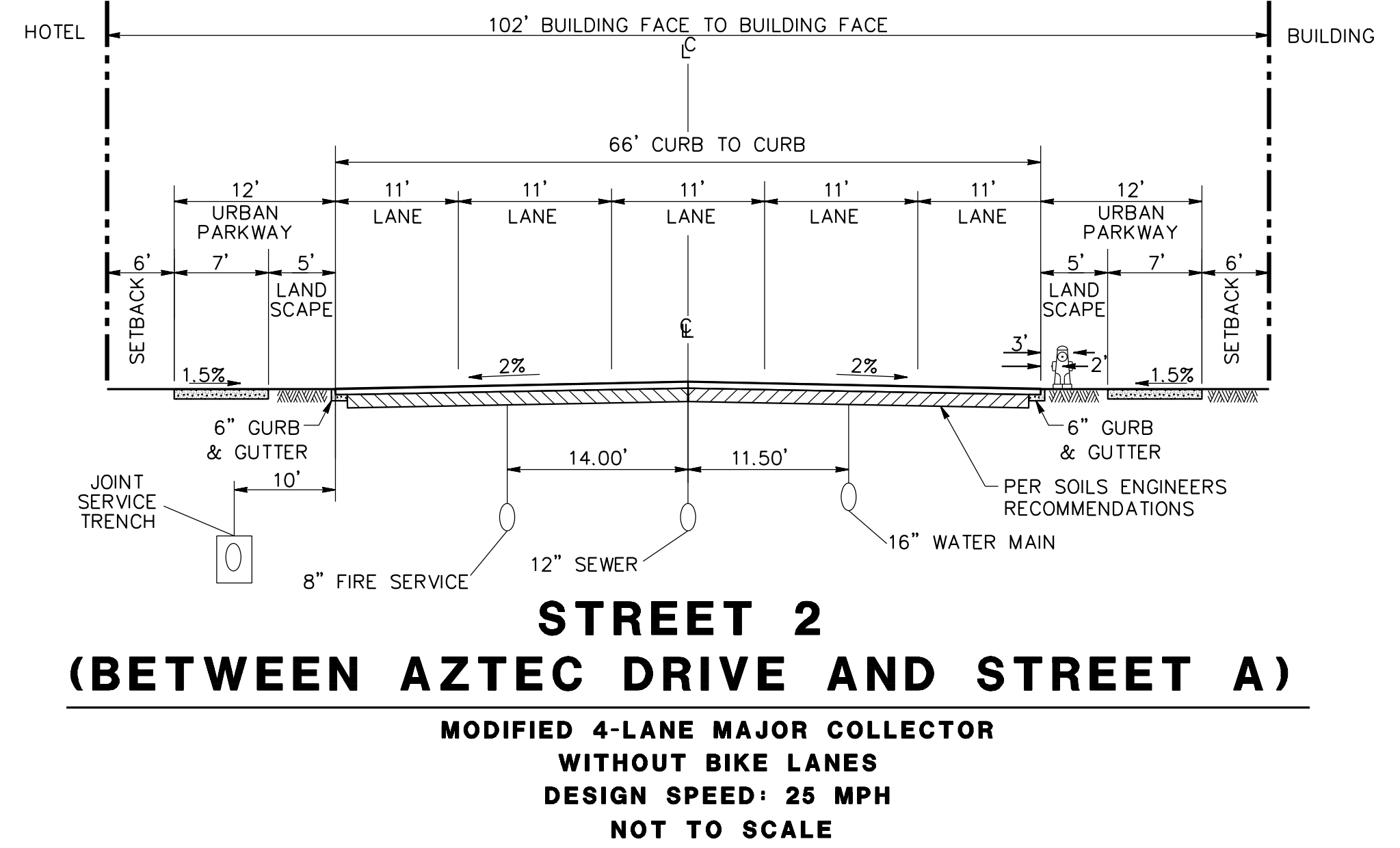
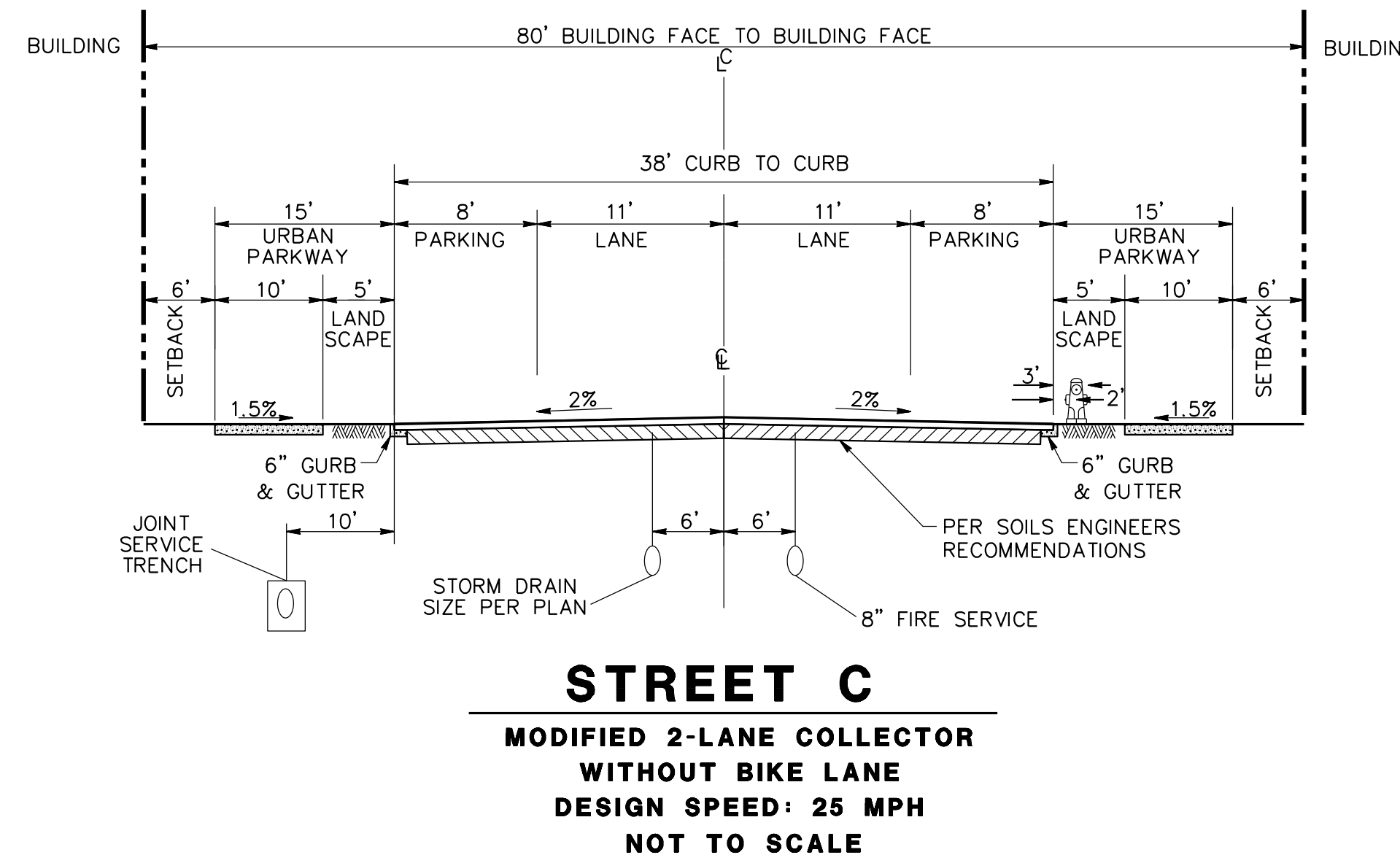
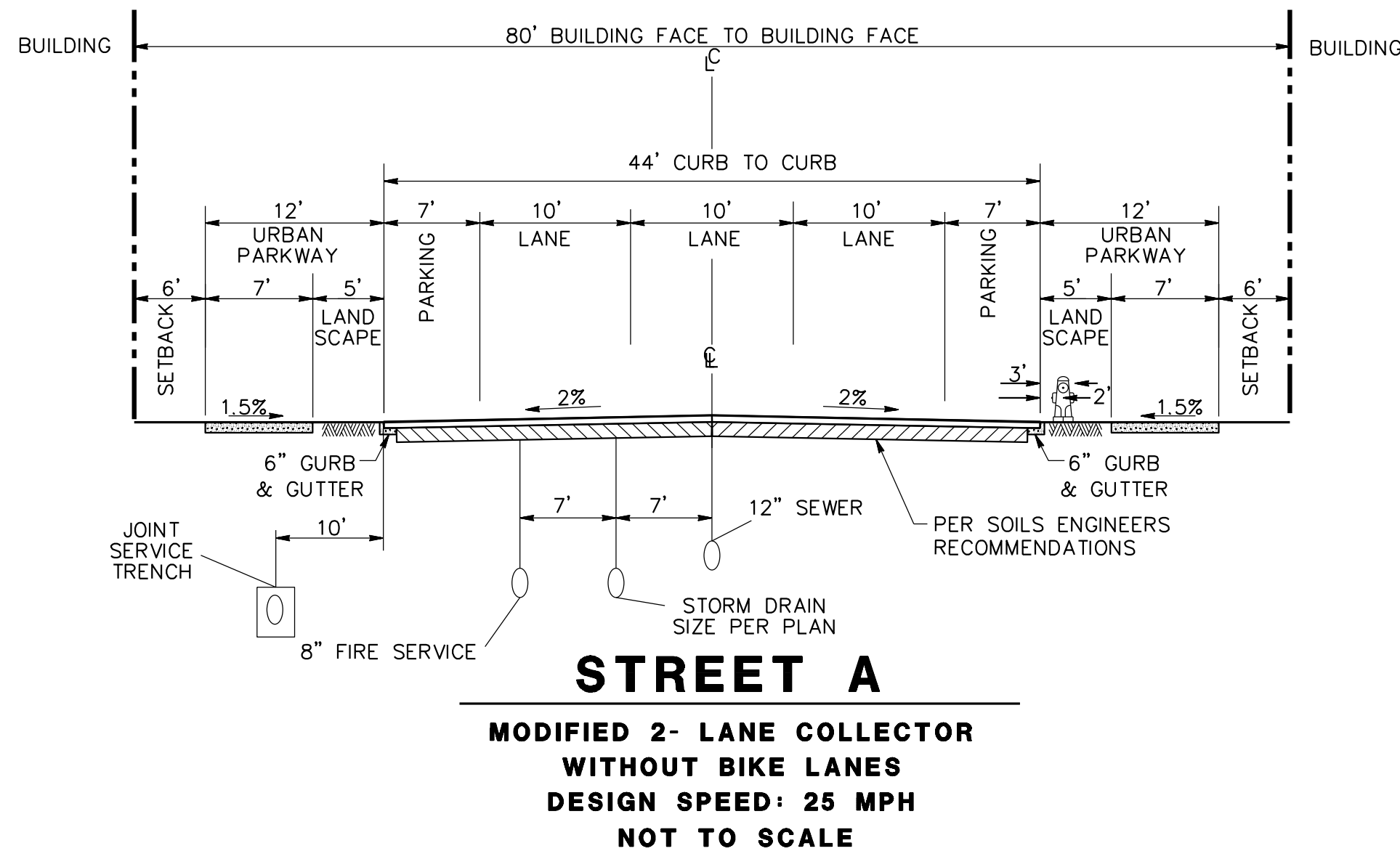
**PH 1A/IB**  
**SECTIONS**

4 OF 23  
 DRAWING NO:

**C-N4**



**PHASE 4 STREET SECTIONS**  
(SEE SHEET CN-3 AND C-N4 FOR OPENING DAY SECTIONS)



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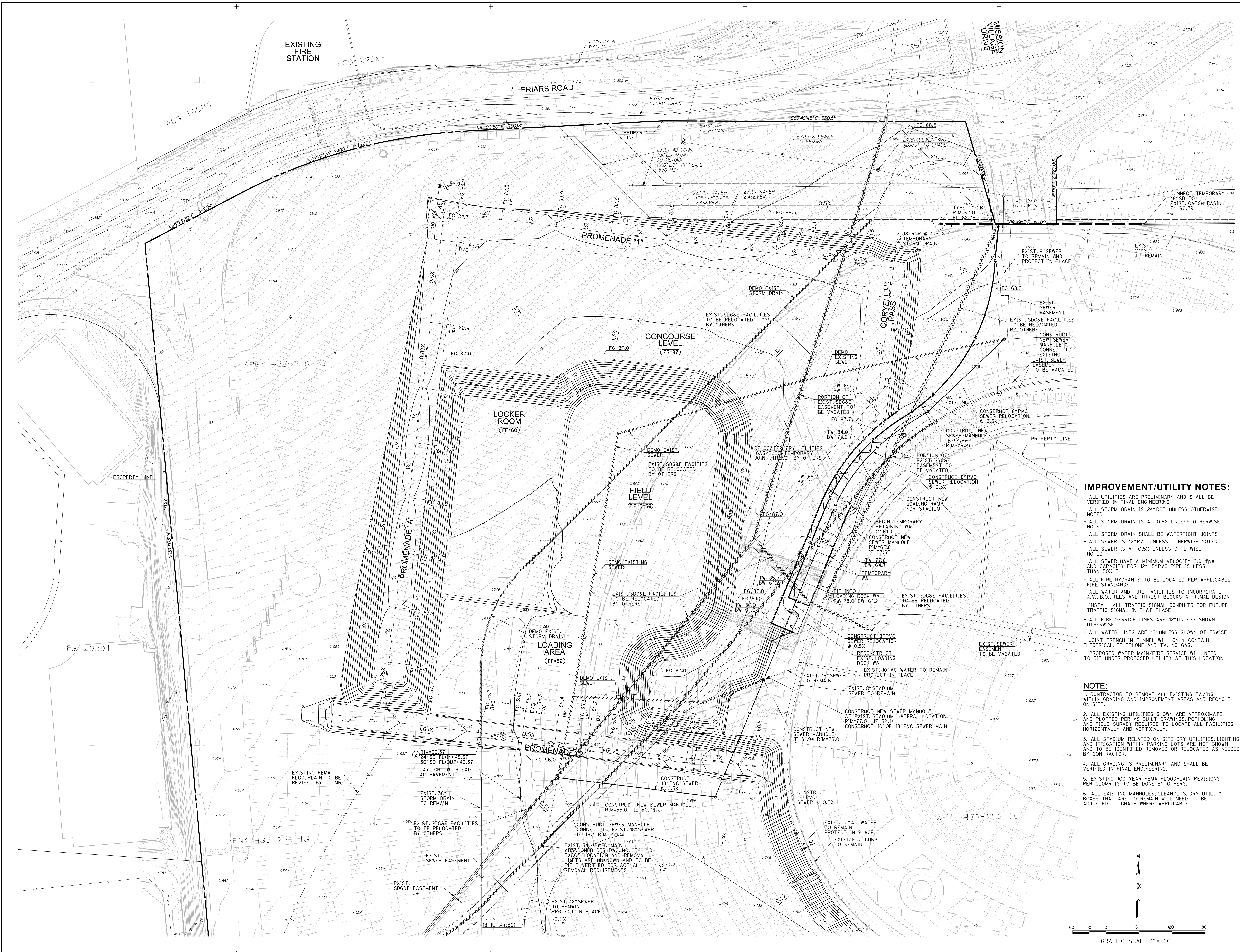
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PROJECT NO:  
FILENAME:  
DRAWN BY: CHECKED BY:  
PLOT DATE: 1/18/2019  
TITLE:

**PH 4**  
**SECTIONS**

5 OF 23  
DRAWING NO:  
**C-N5**



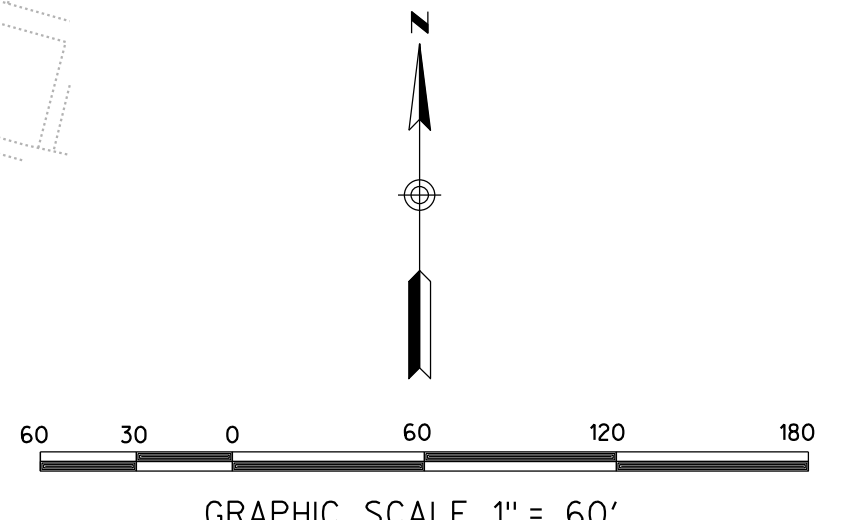


**IMPROVEMENT/UTILITY NOTES:**

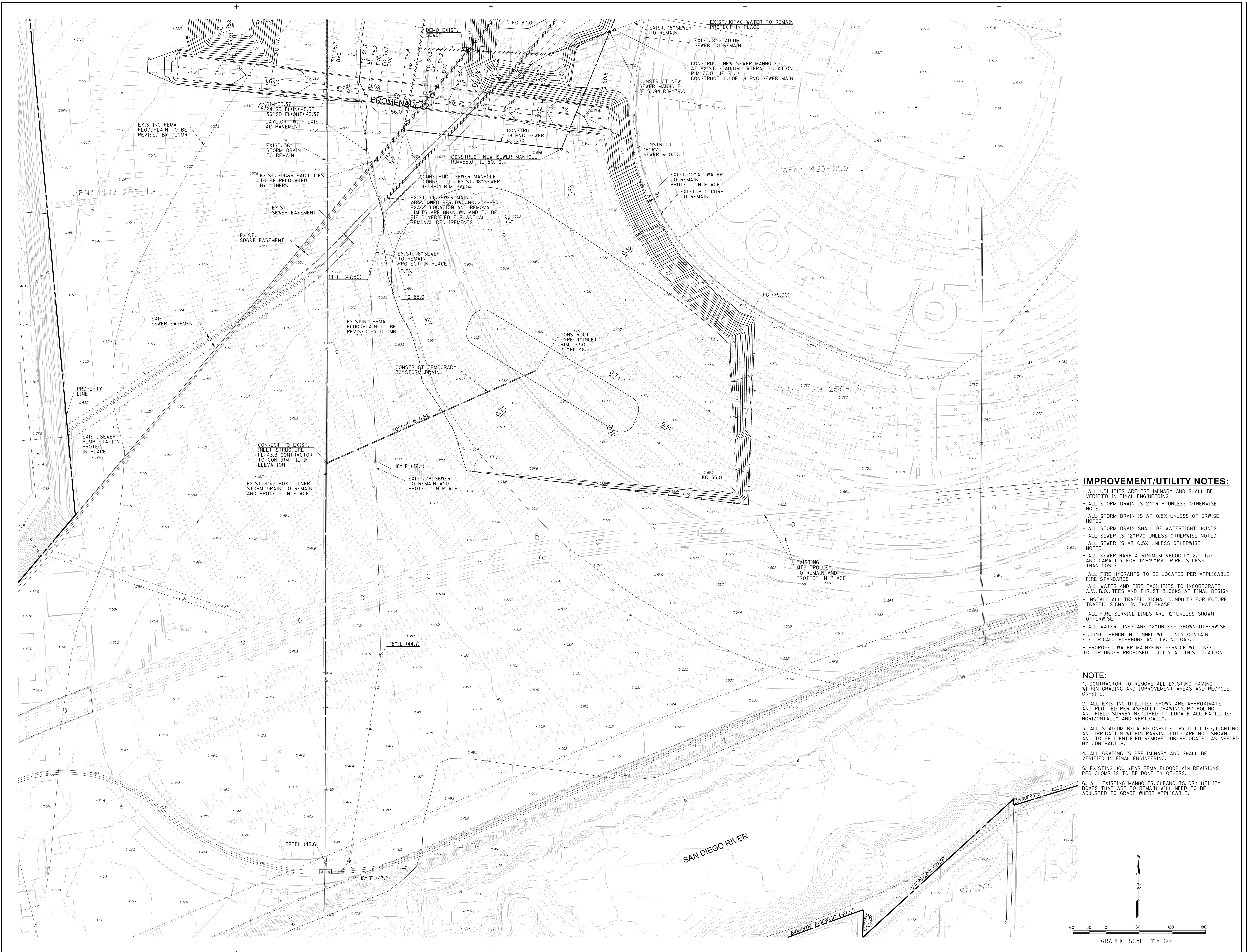
- ALL UTILITIES ARE PRELIMINARY AND SHALL BE VERIFIED IN FINAL ENGINEERING.
- ALL STORM DRAIN IS AT 0.5% UNLESS OTHERWISE NOTED.
- ALL STORM DRAIN IS AT 0.5% UNLESS OTHERWISE NOTED.
- ALL STORM DRAIN SHALL BE WATERTIGHT JOINTS.
- ALL SEWER IS AT 0.5% UNLESS OTHERWISE NOTED.
- ALL SEWER HAVE A MINIMUM VELOCITY 2.0 fps AND CAPACITY FOR 12" 15" PVC PIPE IS LESS THAN 50% FULL.
- ALL FIRE HYDRANTS TO BE LOCATED PER APPLICABLE FIRE STANDARDS.
- ALL WATER AND FIRE FACILITIES TO INCORPORATE A.V., B.O., TEES AND THRUST BLOCKS AT FINAL DESIGN.
- INSTALL ALL TRAFFIC SIGNAL CONDUITS FOR FUTURE TRAFFIC SIGNAL IN THAT PHASE.
- ALL FIRE SERVICE LINES ARE 12" UNLESS SHOWN OTHERWISE.
- ALL WATER LINES ARE 12" UNLESS SHOWN OTHERWISE.
- JOINT TRENCH IN TUNNEL WILL ONLY CONTAIN ELECTRICAL, TELEPHONE AND TV, NO GAS.
- PROPOSED WATER MAIN/FIRE SERVICE WILL NEED TO DIP UNDER PROPOSED UTILITY AT THIS LOCATION.

**NOTE:**

1. CONTRACTOR TO REMOVE ALL EXISTING PAVING WITHIN GRADING AND IMPROVEMENT AREAS AND RECYCLE ON-SITE.
2. ALL EXISTING UTILITIES SHOWN ARE APPROXIMATE AND PLOTTED PER AS-BUILT DRAWINGS, POTHOLES AND FIELD SURVEY REQUIRED TO LOCATE ALL FACILITIES HORIZONTALLY AND VERTICALLY.
3. ALL STADIUM RELATED ON-SITE DRY UTILITIES, LIGHTING AND IRRIGATION WITHIN PARKING LOTS ARE NOT SHOWN AND TO BE IDENTIFIED REMOVED OR RELOCATED AS NEEDED BY CONTRACTOR.
4. ALL GRADING IS PRELIMINARY AND SHALL BE VERIFIED IN FINAL ENGINEERING.
5. EXISTING 100 YEAR FEMA FLOODPLAIN REVISIONS PER CLMOR IS TO BE DONE BY OTHERS.
6. ALL EXISTING MANHOLES, CLEANOUTS, DRY UTILITY BOXES THAT ARE TO REMAIN WILL NEED TO BE ADJUSTED TO GRADE WHERE APPLICABLE.





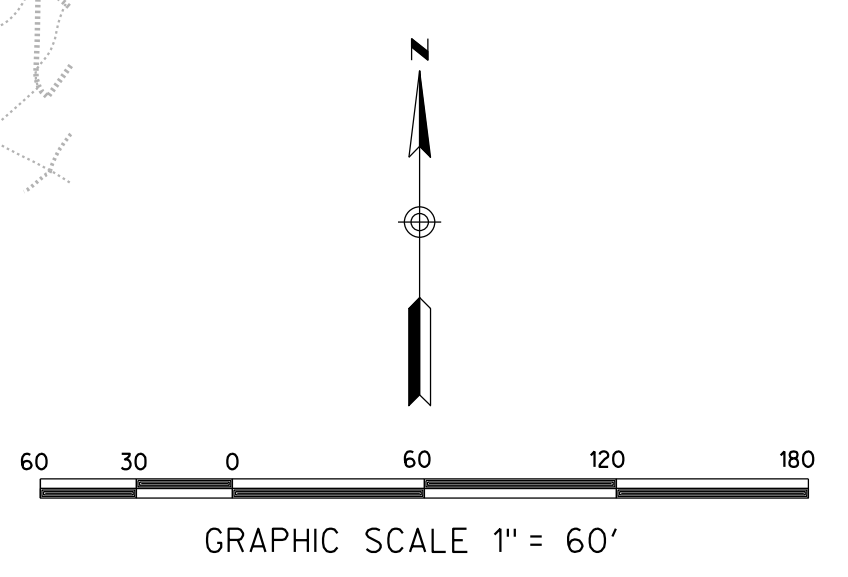


**IMPROVEMENT/UTILITY NOTES:**

- ALL UTILITIES ARE PRELIMINARY AND SHALL BE VERIFIED IN FINAL ENGINEERING
- ALL STORM DRAIN IS 24" RCP UNLESS OTHERWISE NOTED
- ALL STORM DRAIN IS AT 0.5% UNLESS OTHERWISE NOTED
- ALL STORM DRAIN SHALL BE WATERTIGHT JOINTS
- ALL SEWER IS 12" PVC UNLESS OTHERWISE NOTED
- ALL SEWER IS AT 0.5% UNLESS OTHERWISE NOTED
- ALL SEWER HAVE A MINIMUM VELOCITY 2.0 FPS AND CAPACITY FOR 12"-15" PVC PIPE IS LESS THAN 50% FULL
- ALL FIRE HYDRANTS TO BE LOCATED PER APPLICABLE FIRE STANDARDS
- ALL WATER AND FIRE FACILITIES TO INCORPORATE A.V.V. BOV. TEES AND THRUST BLOCKS AT FINAL DESIGN
- INSTALL ALL TRAFFIC SIGNAL CONDUITS FOR FUTURE TRAFFIC SIGNAL IN THAT PHASE
- ALL FIRE SERVICE LINES ARE 12" UNLESS SHOWN OTHERWISE
- ALL WATER LINES ARE 12" UNLESS SHOWN OTHERWISE
- JOINT TRENCH IN TUNNEL WILL ONLY CONTAIN ELECTRICAL, TELEPHONE AND TV. NO GAS.
- PROPOSED WATER MAIN/FIRE SERVICE WILL NEED TO DIP UNDER PROPOSED UTILITY AT THIS LOCATION

**NOTE:**

1. CONTRACTOR TO REMOVE ALL EXISTING PAVING WITHIN GRADING AND IMPROVEMENT AREAS AND RECYCLE ON-SITE.
2. ALL EXISTING UTILITIES SHOWN ARE APPROXIMATE AND PLOTTED PER AS-BUILT DRAWINGS, POT-HOLING AND FIELD SURVEY REQUIRED TO LOCATE ALL FACILITIES HORIZONTALLY AND VERTICALLY.
3. ALL STADIUM RELATED ON-SITE DRY UTILITIES, LIGHTING AND IRRIGATION WITHIN PARKING LOTS ARE NOT SHOWN AND TO BE IDENTIFIED REMOVED OR RELOCATED AS NEEDED BY CONTRACTOR.
4. ALL GRADING IS PRELIMINARY AND SHALL BE VERIFIED IN FINAL ENGINEERING.
5. EXISTING 100 YEAR FEMA FLOODPLAIN REVISIONS PER CLOMR IS TO BE DONE BY OTHERS.
6. ALL EXISTING MANHOLES, CLEANOUTS, DRY UTILITY BOXES THAT ARE TO REMAIN WILL NEED TO BE ADJUSTED TO GRADE WHERE APPLICABLE.



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PROJECT NO:  
FILENAME:  
DRAWN BY: CHECKED BY:  
PLOT DATE: 1/18/2019

TITLE:  
**PHASE 1A GRADING & UTILITIES**

7 OF 23  
DRAWING NO:  
**C-1.2**





**EROSION CONTROL LEGEND**

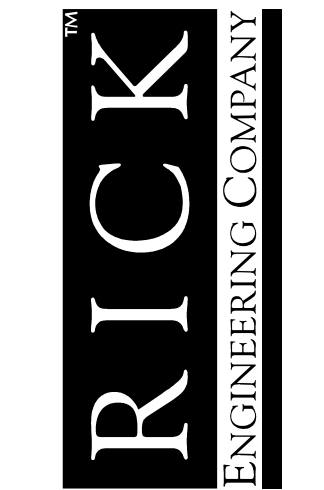
- PADS (TACKFIER)
- SLOPES (BONDED FIBER MATRIX OR EQUIVALENT)

**NOTES**

CONTRACTOR TO PROVIDE EROSION CONTROL MEASURES AS INDICATED ON CONTRACTOR PREPARED SWPPP: IE, SILT FENCING, GRAVEL BAG CHECK DAMS, FIBER ROLL, INLET PROTECTION, CONSTRUCTION ENTRANCE AND WASHOUT STATIONS.

CONTRACTOR TO INSTALL TEMPORARY STORM DRAIN AS INDICATED HEREON TO PROPOSED SEDIMENT BASINS/TRAPS. ALL TEMPORARY STORM DRAIN TO BE REMOVED AS NEEDED AS CONSTRUCTION PROGRESSES.

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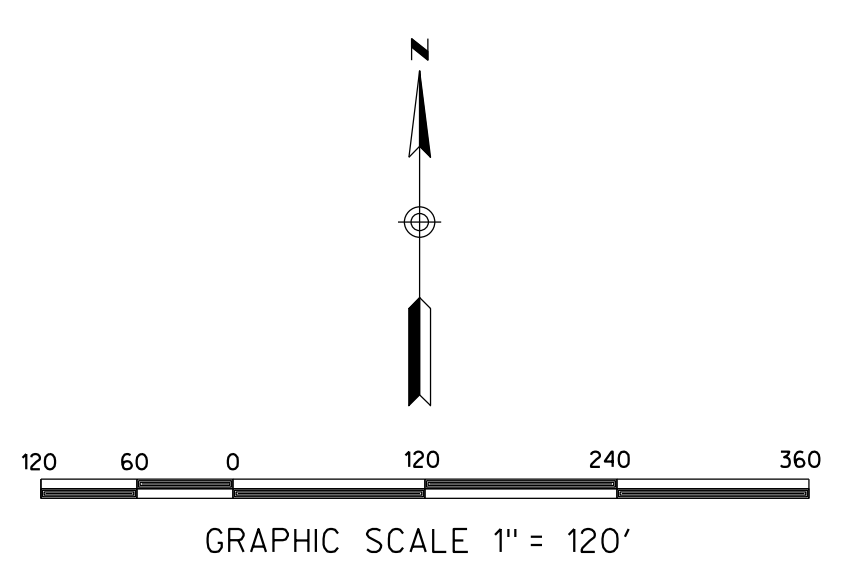
ISSUED: 01/18/19-RFP  
REV. 2/12/19

PROJECT NO: \_\_\_\_\_  
FILENAME: \_\_\_\_\_  
DRAWN BY: \_\_\_\_\_ CHECKED BY: \_\_\_\_\_  
PLOT DATE: 1/18/2019

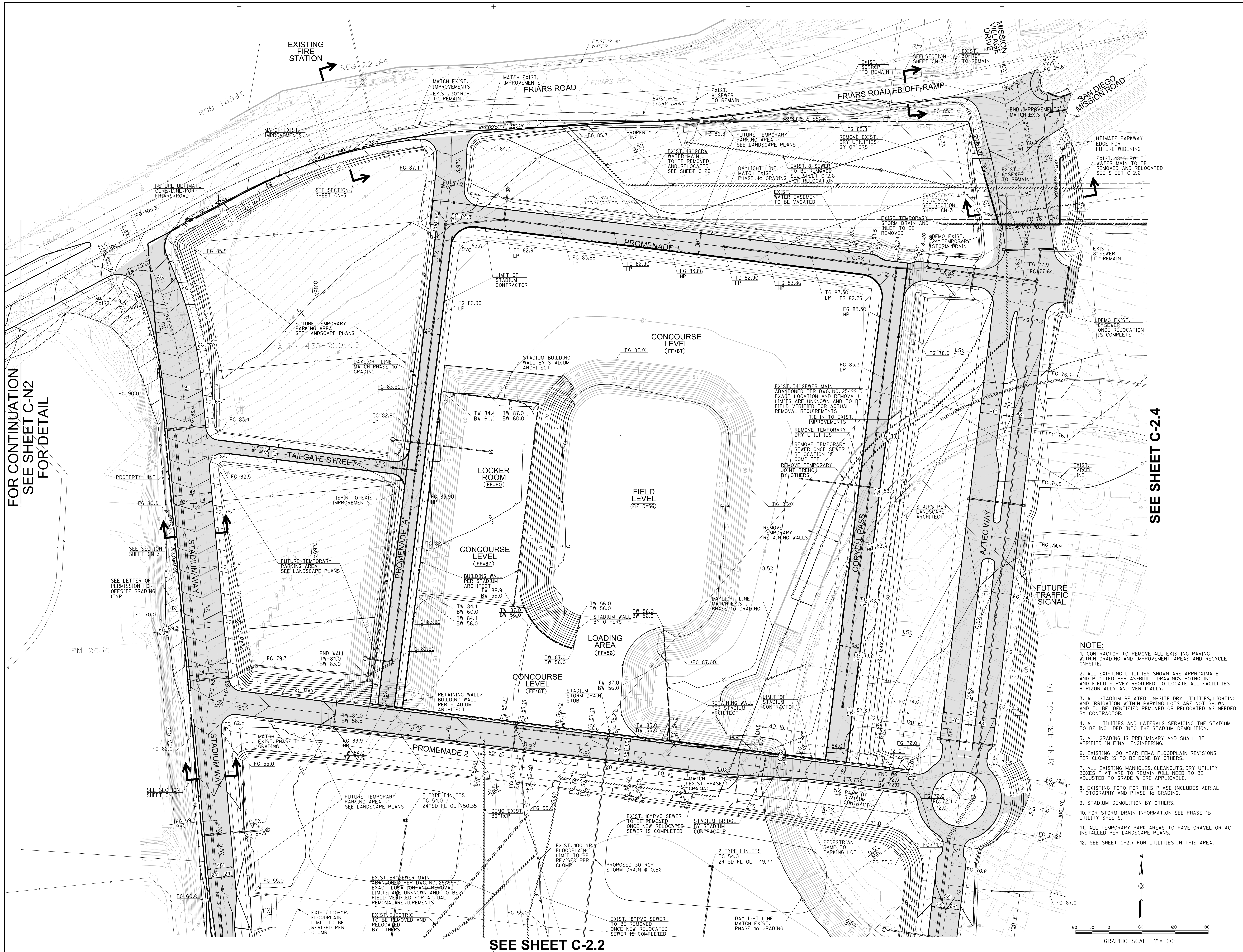
**PHASE 1A  
EROSION CONTROL**

8 OF 23  
DRAWING NO:

**C-1.3**







FOR CONTINUATION  
SEE SHEET C-N2  
FOR DETAIL

SEE SHEET C-2.4

SEE SHEET C-2.2

- NOTE:**
1. CONTRACTOR TO REMOVE ALL EXISTING PAVING WITHIN GRADING AND IMPROVEMENT AREAS AND RECYCLE ON-SITE.
  2. ALL EXISTING UTILITIES SHOWN ARE APPROXIMATE AND PLOTTED PER AS-BUILT DRAWINGS, POTHOLES AND FIELD SURVEY REQUIRED TO LOCATE ALL FACILITIES HORIZONTALLY AND VERTICALLY.
  3. ALL STADIUM RELATED ON-SITE DRY UTILITIES, LIGHTING AND IRRIGATION WITHIN PARKING LOTS ARE NOT SHOWN AND TO BE IDENTIFIED REMOVED OR RELOCATED AS NEEDED BY CONTRACTOR.
  4. ALL UTILITIES AND LATERALS SERVICING THE STADIUM TO BE INCLUDED INTO THE STADIUM DEMOLITION.
  5. ALL GRADING IS PRELIMINARY AND SHALL BE VERIFIED IN FINAL ENGINEERING.
  6. EXISTING 100 YEAR FEMA FLOODPLAIN REVISIONS PER CLOMR IS TO BE DONE BY OTHERS.
  7. ALL EXISTING MANHOLES, CLEANOUTS, DRY UTILITY BOXES THAT ARE TO REMAIN WILL NEED TO BE ADJUSTED TO GRADE WHERE APPLICABLE.
  8. EXISTING TOPO FOR THIS PHASE INCLUDES AERIAL PHOTOGRAPHY AND PHASE 1A GRADING.
  9. STADIUM DEMOLITION BY OTHERS.
  10. FOR STORM DRAIN INFORMATION SEE PHASE 1B UTILITY SHEETS.
  11. ALL TEMPORARY PARK AREAS TO HAVE GRAVEL OR AC INSTALLED PER LANDSCAPE PLANS.
  12. SEE SHEET C-2.7 FOR UTILITIES IN THIS AREA.

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PROJECT NO:  
FILENAME:  
DRAWN BY: CHECKED BY:  
PLOT DATE:

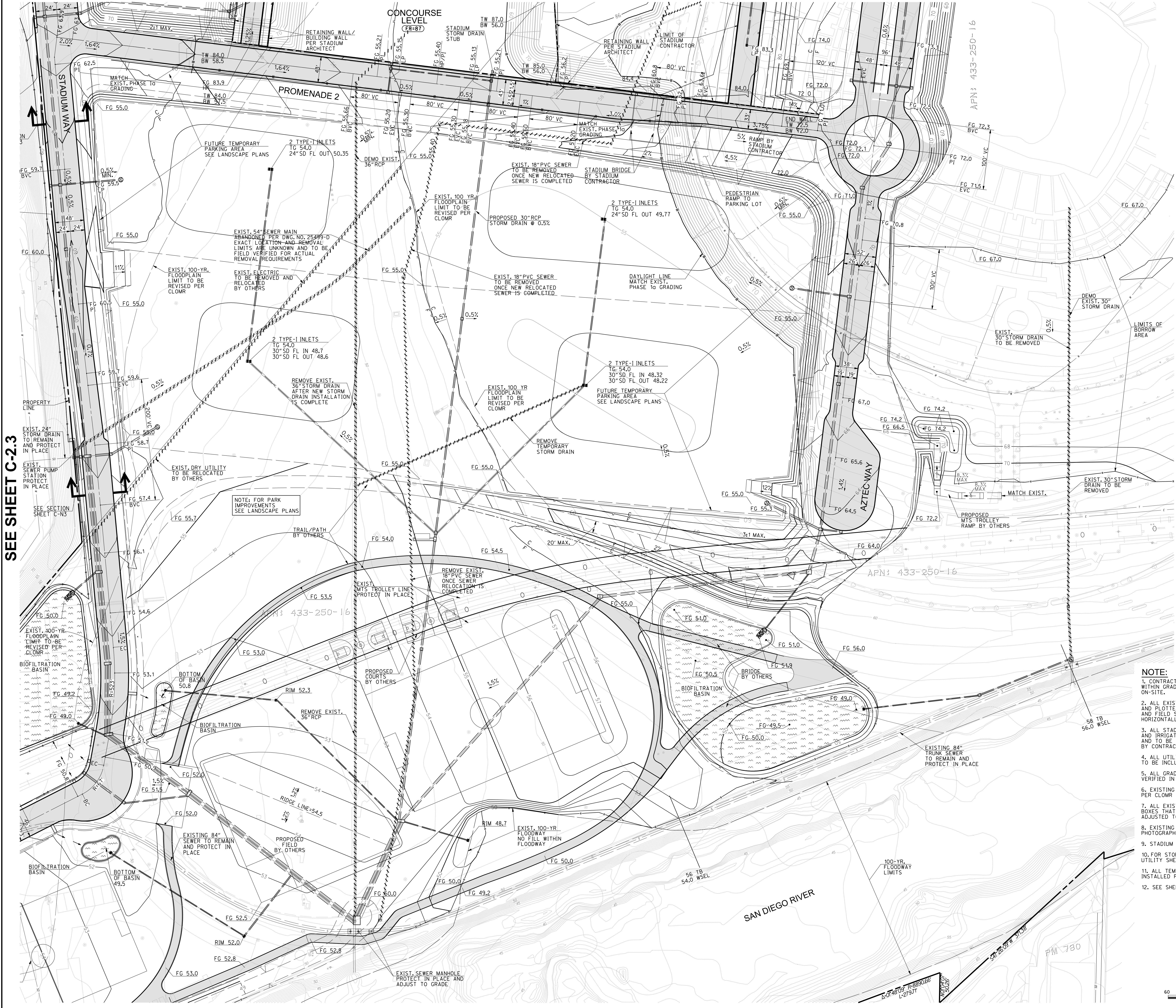
**PHASE 1B  
GRADING**

9 OF 23  
DRAWING NO:

**C-2.1**



SEE SHEET C-2.1

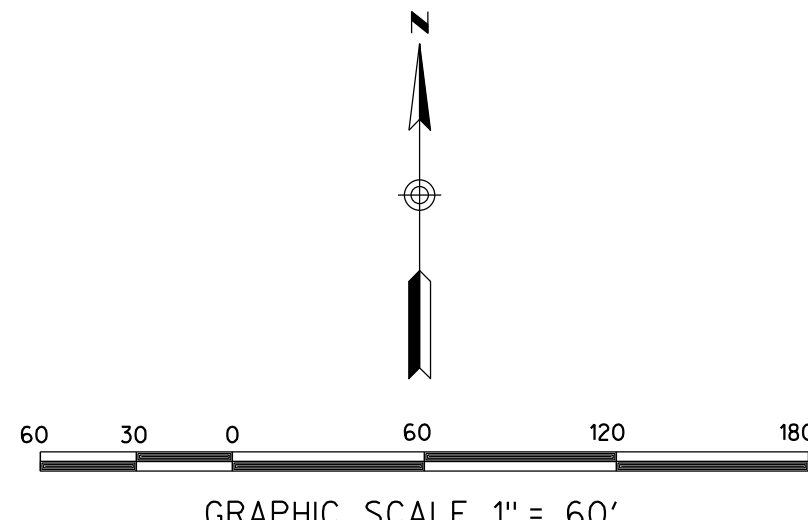


SEE SHEET C-2.3

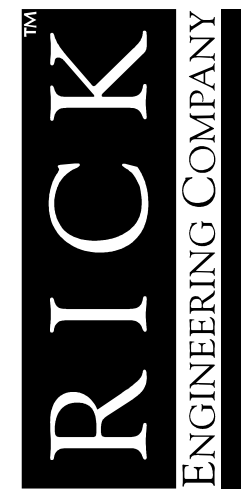
SEE SHEET C-2.5

**NOTE:**

1. CONTRACTOR TO REMOVE ALL EXISTING PAVING WITHIN GRADING AND IMPROVEMENT AREAS AND RECYCLE ON-SITE.
2. ALL EXISTING UTILITIES SHOWN ARE APPROXIMATE AND PLOTTED PER AS-BUILT DRAWINGS, POTHOLING AND FIELD SURVEY REQUIRED TO LOCATE ALL FACILITIES HORIZONTALLY AND VERTICALLY.
3. ALL STADIUM RELATED ON-SITE DRY UTILITIES, LIGHTING AND IRRIGATION WITHIN PARKING LOTS ARE NOT SHOWN AND TO BE IDENTIFIED, REMOVED OR RELOCATED AS NEEDED BY CONTRACTOR.
4. ALL UTILITIES AND LATERALS SERVICING THE STADIUM TO BE INCLUDED INTO THE STADIUM DEMOLITION.
5. ALL GRADING IS PRELIMINARY AND SHALL BE VERIFIED IN FINAL ENGINEERING.
6. EXISTING 100 YEAR FEMA FLOODPLAIN REVISIONS PER CLOMR IS TO BE DONE BY OTHERS.
7. ALL EXISTING MANHOLES, CLEANOUTS, DRY UTILITY BOXES THAT ARE TO REMAIN WILL NEED TO BE ADJUSTED TO GRADE WHERE APPLICABLE.
8. EXISTING TOPO FOR THIS PHASE INCLUDES AERIAL PHOTOGRAPHY AND PHASE 1a GRADING.
9. STADIUM DEMOLITION BY OTHERS.
10. FOR STORM DRAIN INFORMATION SEE PHASE 1b UTILITY SHEETS.
11. ALL TEMPORARY PARK AREAS TO HAVE GRAVEL OR AC INSTALLED PER LANDSCAPE PLANS.
12. SEE SHEET C-2.8 FOR UTILITIES IN THIS AREA.



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 (FAX) 619-291-4165



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 75% DD SITE DEVELOPMENT PACKAGE**

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PROJECT NO:  
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 TITLE:

**PHASE 1B  
 GRADING**

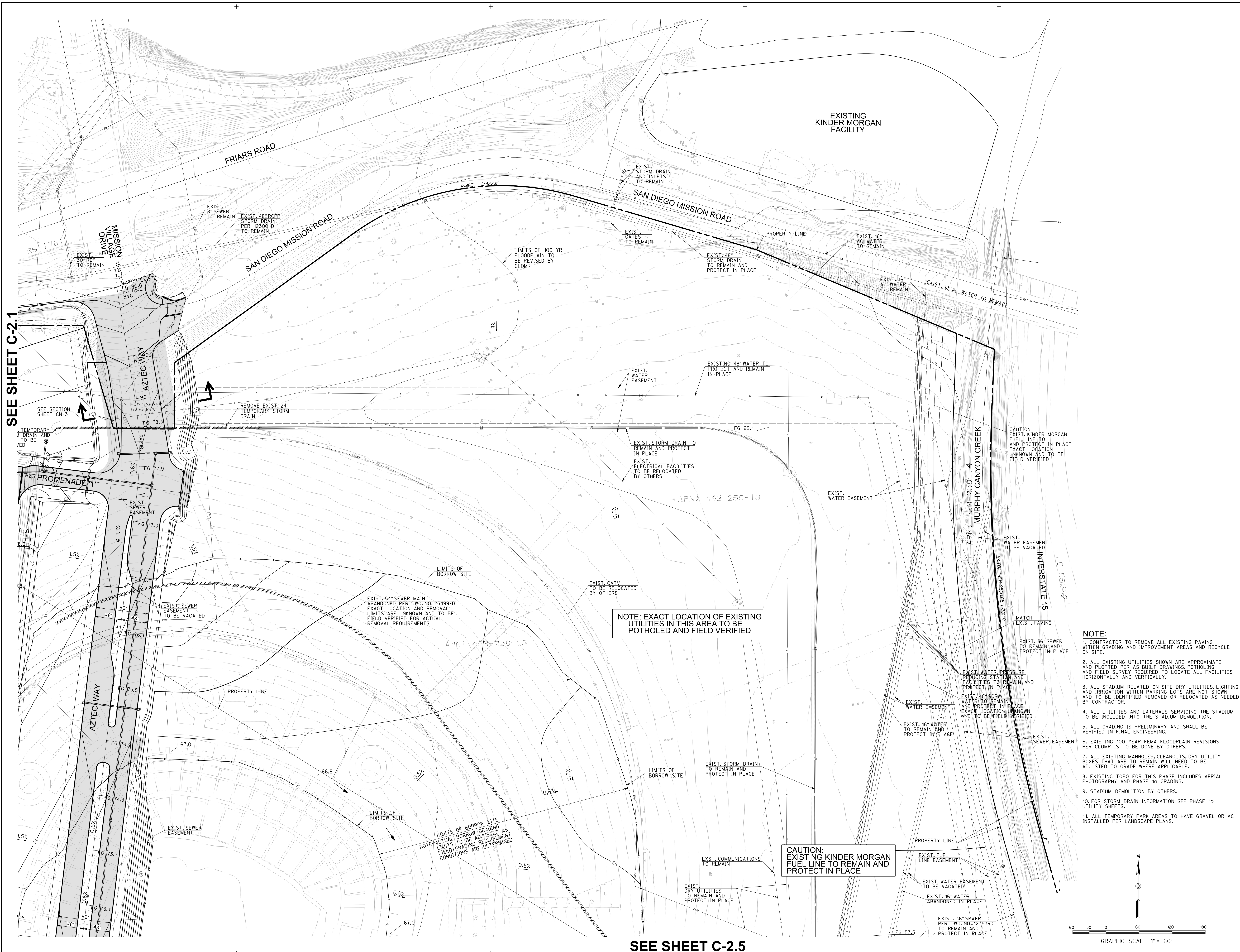
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**C-2.2**







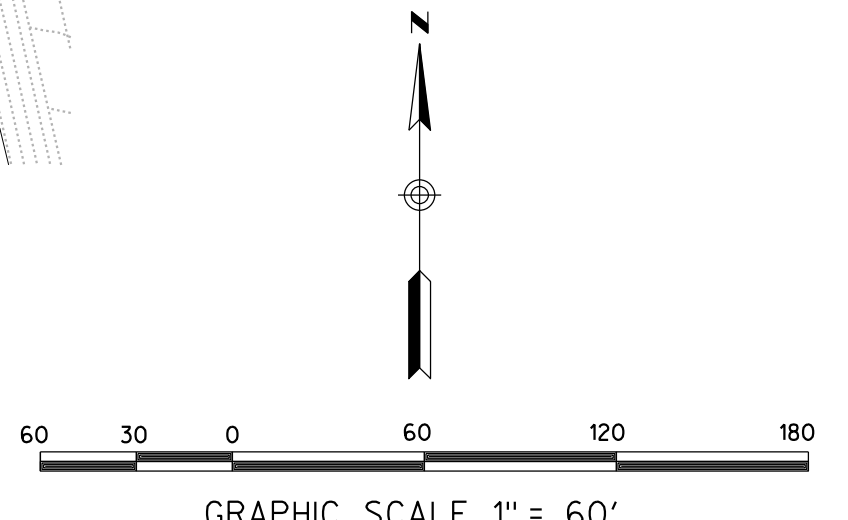


SEE SHEET C-2.1

NOTE: EXACT LOCATION OF EXISTING UTILITIES IN THIS AREA TO BE POTHOLED AND FIELD VERIFIED

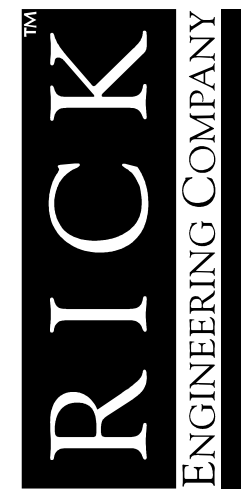
CAUTION: EXISTING KINDER MORGAN FUEL LINE TO REMAIN AND PROTECT IN PLACE

- NOTE:**
1. CONTRACTOR TO REMOVE ALL EXISTING PAVING WITHIN GRADING AND IMPROVEMENT AREAS AND RECYCLE ON-SITE.
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  3. ALL STADIUM RELATED ON-SITE DRY UTILITIES, LIGHTING AND IRRIGATION WITHIN PARKING LOTS ARE NOT SHOWN AND TO BE IDENTIFIED REMOVED OR RELOCATED AS NEEDED BY CONTRACTOR.
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  8. EXISTING TOPO FOR THIS PHASE INCLUDES AERIAL PHOTOGRAPHY AND PHASE TO GRADING.
  9. STADIUM DEMOLITION BY OTHERS.
  10. FOR STORM DRAIN INFORMATION SEE PHASE 1B UTILITY SHEETS.
  11. ALL TEMPORARY PARK AREAS TO HAVE GRAVEL OR AC INSTALLED PER LANDSCAPE PLANS.



SEE SHEET C-2.5

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PLOT DATE:  
TITLE:

**PHASE 1B  
GRADING**

12 OF 23  
DRAWING NO:

**C-2.4**



SEE SHEET C-2.4

APN: 433-250-13

EXIST. 54" SEWER MAIN  
ABANDONED PER DWG. NO. 25499-0  
EXACT LOCATION AND REMOVAL  
LIMITS ARE UNKNOWN AND TO BE  
FIELD VERIFIED FOR ACTUAL  
REMOVAL REQUIREMENTS

EXIST. SEWER  
EASEMENT  
EXIST. 36" SEWER  
TO REMAIN AND  
PROTECT IN PLACE  
PROPERTY LINE  
CAUTION!  
EXIST. KINDER MORGAN  
FUEL LINE TO REMAIN AND  
PROTECT IN PLACE  
EXIST. 16" AC WATER  
TO REMAIN AND  
PROTECT IN PLACE  
EXIST. WATER  
EASEMENT  
EXIST. WATER  
EASEMENT

EXIST. DRY UTILITIES  
TO REMAIN AND  
PROTECT IN PLACE

CAP EXIST.  
12" AC WATER

EXIST. 16" WATER  
TO BE REMOVED

NOTE: ACTUAL BORROW  
GRADING LIMITS TO BE  
ADJUSTED AS FIELD/GRADING  
REQUIREMENT AND CONDITIONS  
ARE DETERMINED

APN: 433-250-16

EXIST. 100 YR  
FLOODPLAIN  
LIMIT TO BE  
REVISED PER  
CLOMR

EXIST. STORM DRAIN  
TO REMAIN AND  
PROTECT IN PLACE

EXIST. DRY UTILITIES  
TO REMAIN AND  
PROTECT IN PLACE

EXIST. MTS. TROLLEY  
TO REMAIN AND  
PROTECT IN PLACE

PROPERTY LINE

SAN DIEGO RIVER

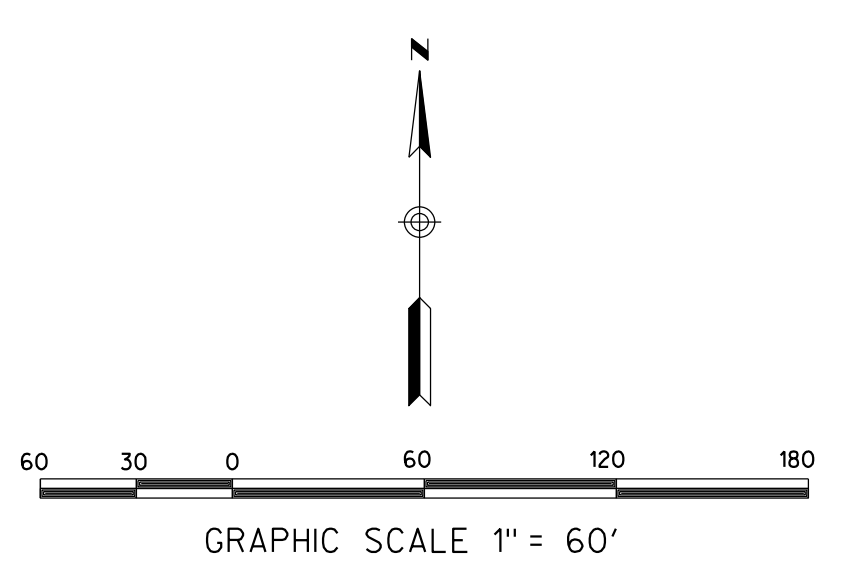
PM 8041

PM 17464

MAP 9031

MAP 6837

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  9. STADIUM DEMOLITION BY OTHERS.
  10. FOR STORM DRAIN INFORMATION SEE PHASE 1b UTILITY SHEETS.
  11. ALL TEMPORARY PARK AREAS TO HAVE GRAVEL OR AC INSTALLED PER LANDSCAPE PLANS.



APN: 433-250-16

SEE SHEET C-2.2

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# SAN DIEGO STATE UNIVERSITY MISSION VALLEY 75% DD SITE DEVELOPMENT PACKAGE

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## PHASE 1B GRADING

13 OF 23  
DRAWING NO:

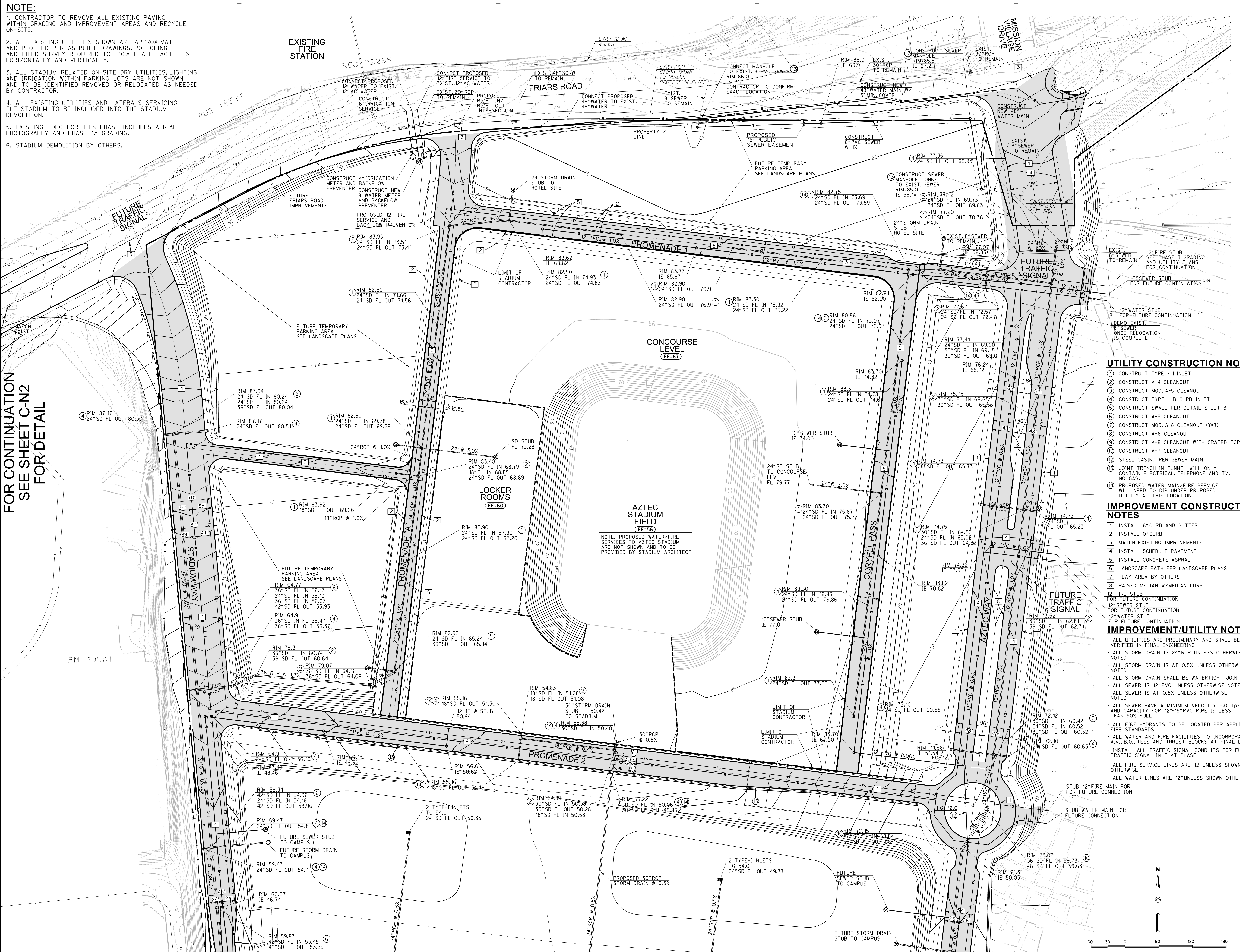
# C-2.5



**NOTE:**

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4. ALL EXISTING UTILITIES AND LATERALS SERVING THE STADIUM TO BE INCLUDED INTO THE STADIUM DEMOLITION.
5. EXISTING TOPO FOR THIS PHASE INCLUDES AERIAL PHOTOGRAPHY AND PHASE 1a GRADING.
6. STADIUM DEMOLITION BY OTHERS.

FOR CONTINUATION  
SEE SHEET C-N2  
FOR DETAIL



**UTILITY CONSTRUCTION NOTES**

1. CONSTRUCT TYPE - I INLET
2. CONSTRUCT A-4 CLEANOUT
3. CONSTRUCT MOD. A-5 CLEANOUT
4. CONSTRUCT TYPE - B CURB INLET
5. CONSTRUCT SWALE PER DETAIL SHEET 3
6. CONSTRUCT A-5 CLEANOUT
7. CONSTRUCT MOD. A-8 CLEANOUT (Y=7)
8. CONSTRUCT A-6 CLEANOUT
9. CONSTRUCT A-8 CLEANOUT WITH GRATED TOP
10. CONSTRUCT A-7 CLEANOUT
11. STEEL CASING PER SEWER MAIN
12. JOINT TRENCH IN TUNNEL WILL ONLY CONTAIN ELECTRICAL, TELEPHONE AND TV. NO GAS.
13. PROPOSED WATER MAIN/FIRE SERVICE WILL NEED TO DIP UNDER PROPOSED UTILITY AT THIS LOCATION

**IMPROVEMENT CONSTRUCTION NOTES**

1. INSTALL 6" CURB AND GUTTER
2. INSTALL 0" CURB
3. MATCH EXISTING IMPROVEMENTS
4. INSTALL SCHEDULE PAVEMENT
5. INSTALL CONCRETE ASPHALT
6. LANDSCAPE PATH PER LANDSCAPE PLANS
7. PLAY AREA BY OTHERS
8. RAISED MEDIAN W/MEDIAN CURB

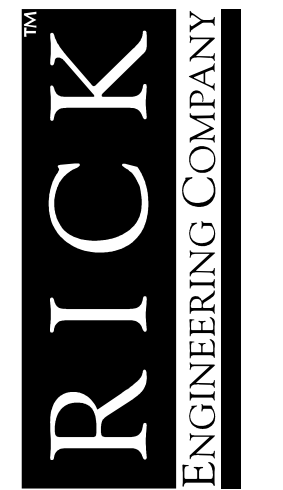
**IMPROVEMENT/UTILITY NOTES:**

- ALL UTILITIES ARE PRELIMINARY AND SHALL BE VERIFIED IN FINAL ENGINEERING
- ALL STORM DRAIN IS 24" RCP UNLESS OTHERWISE NOTED
- ALL STORM DRAIN IS AT 0.5% UNLESS OTHERWISE NOTED
- ALL STORM DRAIN SHALL BE WATERTIGHT JOINTS
- ALL SEWER IS 12" PVC UNLESS OTHERWISE NOTED
- ALL SEWER IS AT 0.5% UNLESS OTHERWISE NOTED
- ALL SEWER HAVE A MINIMUM VELOCITY 2.0 fps AND CAPACITY FOR 12"-15" PVC PIPE IS LESS THAN 50% FULL
- ALL FIRE HYDRANTS TO BE LOCATED PER APPLICABLE FIRE STANDARDS
- ALL WATER AND FIRE FACILITIES TO INCORPORATE A.V., B.O., TEES AND THRUST BLOCKS AT FINAL DESIGN
- INSTALL ALL TRAFFIC SIGNAL CONDUITS FOR FUTURE TRAFFIC SIGNAL IN THAT PHASE
- ALL FIRE SERVICE LINES ARE 12" UNLESS SHOWN OTHERWISE
- ALL WATER LINES ARE 12" UNLESS SHOWN OTHERWISE

NOTE: PROPOSED WATER/FIRE SERVICES TO AZTEC STADIUM ARE NOT SHOWN AND TO BE PROVIDED BY STADIUM ARCHITECT

SEE SHEET 2.7

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**PHASE 1B  
UTILITIES**

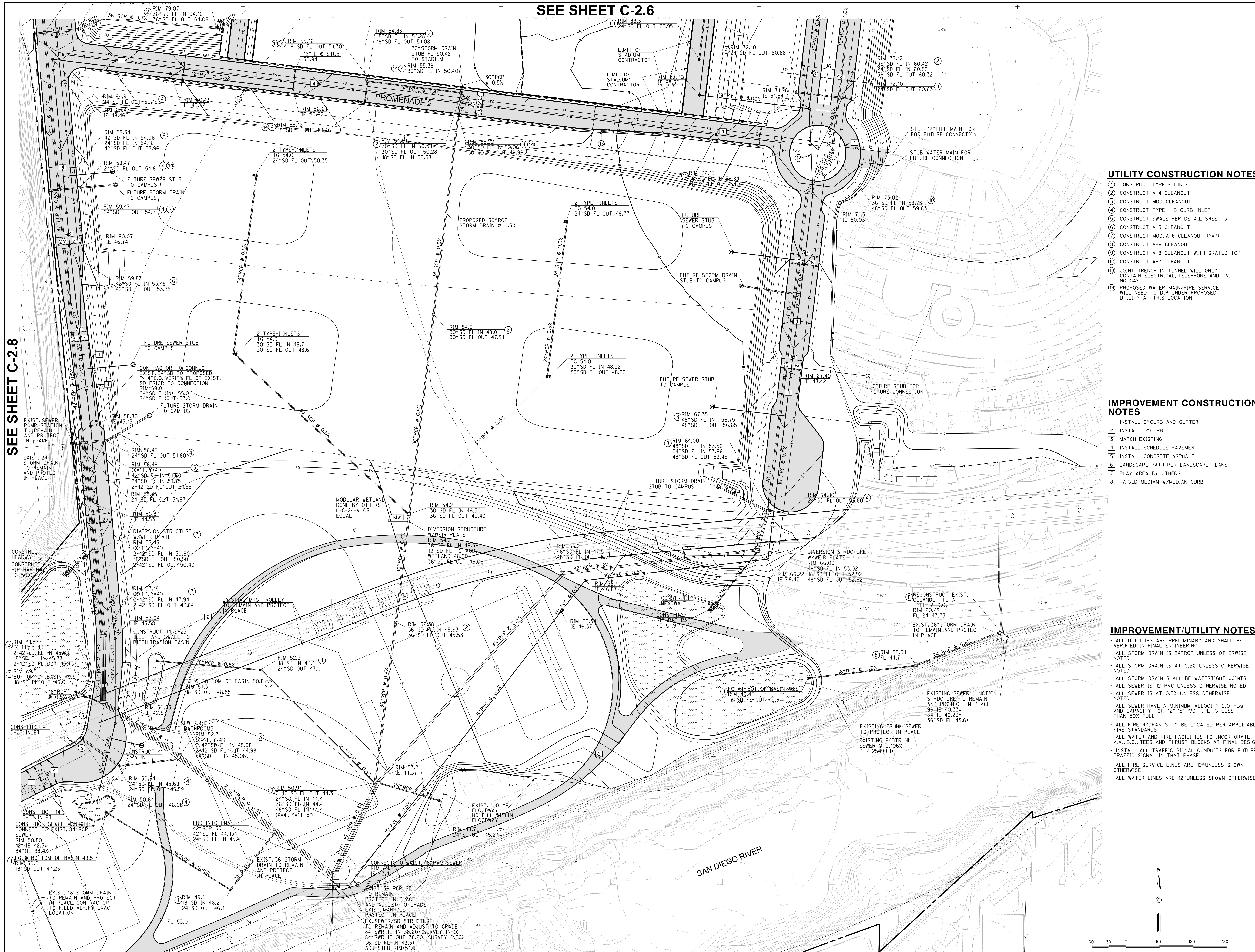
14 OF 23  
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**C-2.6**



SEE SHEET C-2.6

SEE SHEET C-2.8



**UTILITY CONSTRUCTION NOTES**

- ① CONSTRUCT TYPE - I INLET
- ② CONSTRUCT A-4 CLEANOUT
- ③ CONSTRUCT MOD. CLEANOUT
- ④ CONSTRUCT TYPE - B CURB INLET
- ⑤ CONSTRUCT SWALE PER DETAIL SHEET 3
- ⑥ CONSTRUCT A-5 CLEANOUT
- ⑦ CONSTRUCT MOD. A-8 CLEANOUT (Y=7)
- ⑧ CONSTRUCT A-6 CLEANOUT
- ⑨ CONSTRUCT A-8 CLEANOUT WITH GRATED TOP
- ⑩ CONSTRUCT A-7 CLEANOUT
- ⑬ JOINT TRENCH IN TUNNEL WILL ONLY CONTAIN ELECTRICAL, TELEPHONE AND TV. NO GAS.
- ⑭ PROPOSED WATER MAIN/FIRE SERVICE WILL NEED TO DIP UNDER PROPOSED UTILITY AT THIS LOCATION

**IMPROVEMENT CONSTRUCTION NOTES**

- ① INSTALL 6" CURB AND GUTTER
- ② INSTALL 0" CURB
- ③ MATCH EXISTING
- ④ INSTALL SCHEDULE PAVEMENT
- ⑤ INSTALL CONCRETE ASPHALT
- ⑥ LANDSCAPE PATH PER LANDSCAPE PLANS
- ⑦ PLAY AREA BY OTHERS
- ⑧ RAISED MEDIAN W/MEDIAN CURB

**IMPROVEMENT/UTILITY NOTES:**

- ALL UTILITIES ARE PRELIMINARY AND SHALL BE VERIFIED IN FINAL ENGINEERING
- ALL STORM DRAIN IS 24" RCP UNLESS OTHERWISE NOTED
- ALL STORM DRAIN IS AT 0.5% UNLESS OTHERWISE NOTED
- ALL STORM DRAIN SHALL BE WATERTIGHT JOINTS
- ALL SEWER IS 12" PVC UNLESS OTHERWISE NOTED
- ALL SEWER IS AT 0.5% UNLESS OTHERWISE NOTED
- ALL SEWER HAVE A MINIMUM VELOCITY 2.0 fps AND CAPACITY FOR 12"-15" PVC PIPE IS LESS THAN 50% FULL
- ALL FIRE HYDRANTS TO BE LOCATED PER APPLICABLE FIRE STANDARDS
- ALL WATER AND FIRE FACILITIES TO INCORPORATE A.V., B.O., TEES AND THRUST BLOCKS AT FINAL DESIGN
- INSTALL ALL TRAFFIC SIGNAL CONDUITS FOR FUTURE TRAFFIC SIGNAL IN THIS PHASE
- ALL FIRE SERVICE LINES ARE 12" UNLESS SHOWN OTHERWISE
- ALL WATER LINES ARE 12" UNLESS SHOWN OTHERWISE

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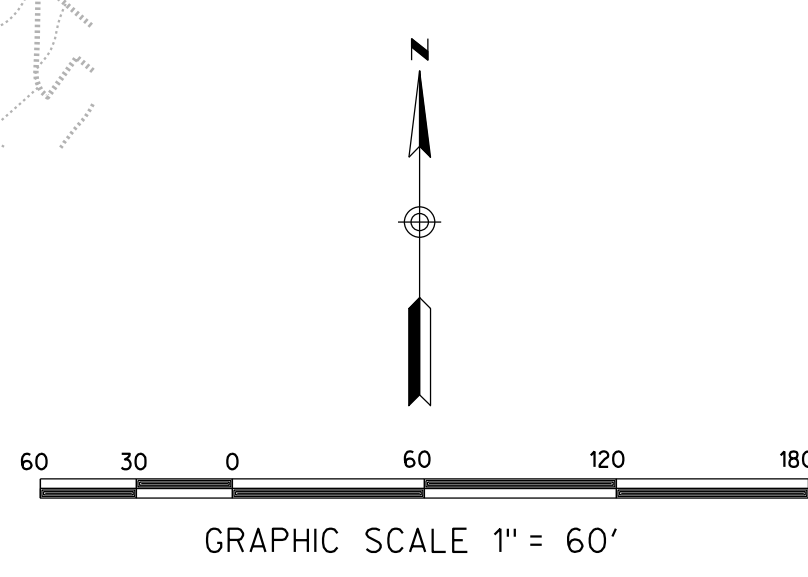
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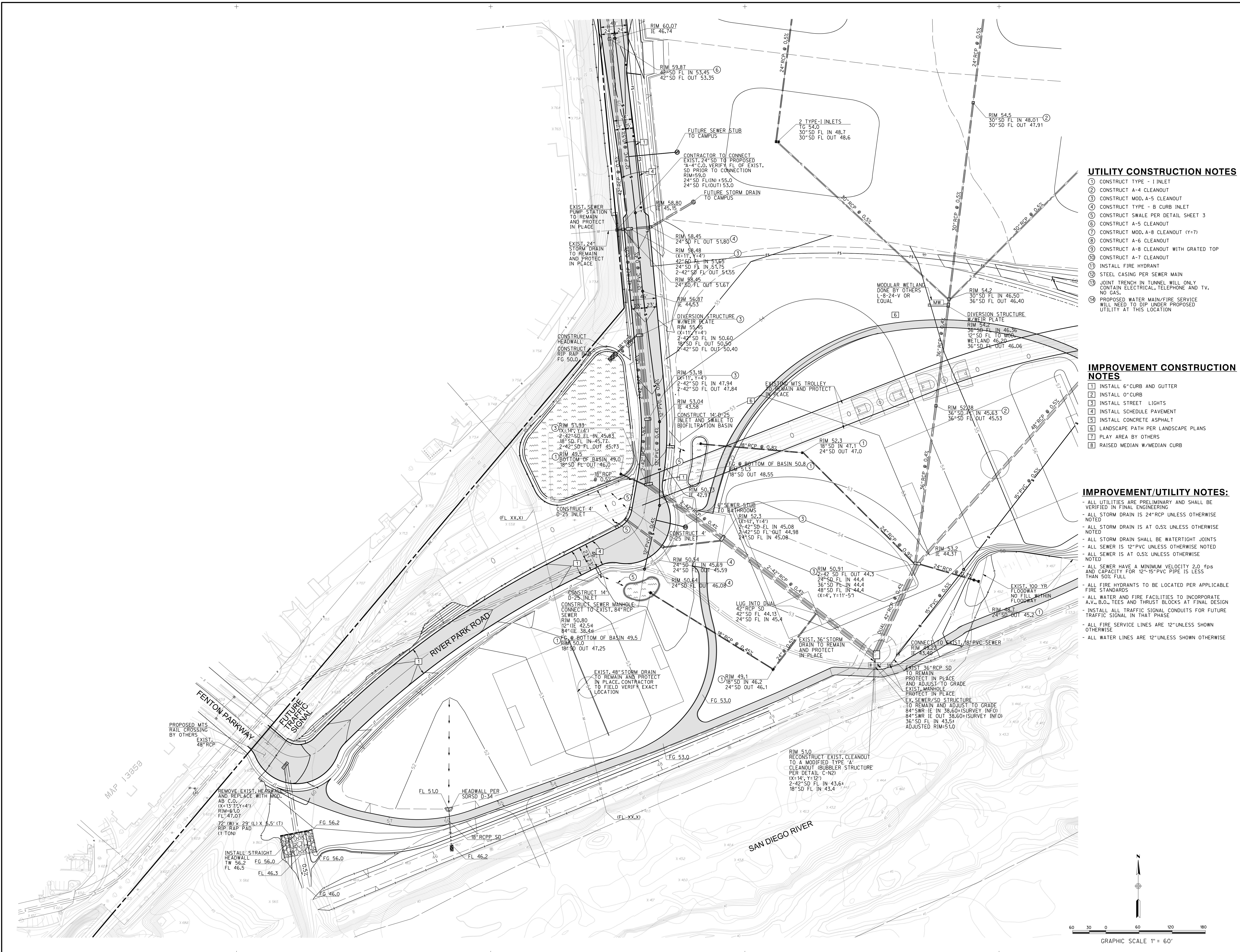
**PHASE 1B UTILITIES**

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**C-2.7**







**UTILITY CONSTRUCTION NOTES**

- 1 CONSTRUCT TYPE - I INLET
- 2 CONSTRUCT A-4 CLEANOUT
- 3 CONSTRUCT MOD. A-5 CLEANOUT
- 4 CONSTRUCT TYPE - B CURB INLET
- 5 CONSTRUCT SWALE PER DETAIL SHEET 3
- 6 CONSTRUCT A-5 CLEANOUT
- 7 CONSTRUCT MOD. A-8 CLEANOUT (Y=7)
- 8 CONSTRUCT A-6 CLEANOUT
- 9 CONSTRUCT A-8 CLEANOUT WITH GRATED TOP
- 10 CONSTRUCT A-7 CLEANOUT
- 11 INSTALL FIRE HYDRANT
- 12 STEEL CASING PER SEWER MAIN
- 13 JOINT TRENCH IN TUNNEL WILL ONLY CONTAIN ELECTRICAL, TELEPHONE AND TV. NO GAS.
- 14 PROPOSED WATER MAIN/FIRE SERVICE WILL NEED TO DIP UNDER PROPOSED UTILITY AT THIS LOCATION

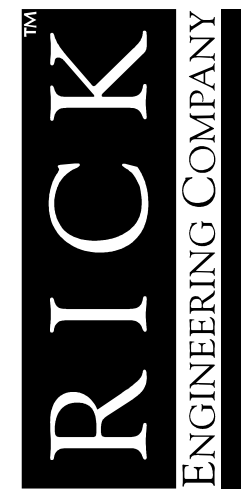
**IMPROVEMENT CONSTRUCTION NOTES**

- 1 INSTALL 6" CURB AND GUTTER
- 2 INSTALL 0" CURB
- 3 INSTALL STREET LIGHTS
- 4 INSTALL SCHEDULE PAVEMENT
- 5 INSTALL CONCRETE ASPHALT
- 6 LANDSCAPE PATH PER LANDSCAPE PLANS
- 7 PLAY AREA BY OTHERS
- 8 RAISED MEDIAN W/MEDIAN CURB

**IMPROVEMENT/UTILITY NOTES:**

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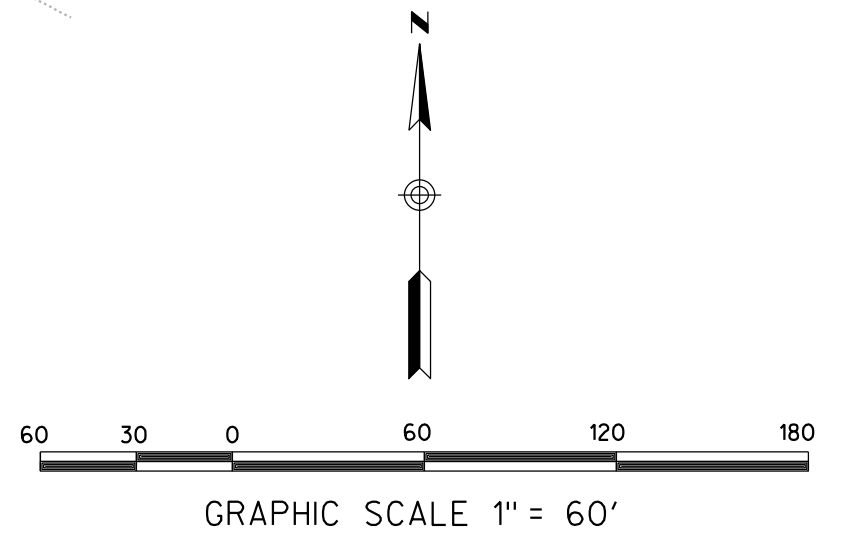
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**PHASE 1B  
UTILITIES**

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DRAWING NO:

**C-2.8**







**EROSION CONTROL LEGEND**

- PADS (TACKFIER)
- SLOPES (BONDED FIBER MATRIX OR EQUIVALENT)

**NOTES**

CONTRACTOR TO PROVIDE EROSION CONTROL MEASURES AS INDICATED ON CONTRACTOR PREPARED SWPPP: IE, SILT FENCING, GRAVEL BAG CHECK DAMS, FIBER ROLL, INLET PROTECTION, CONSTRUCTION ENTRANCE AND WASHOUT STATIONS.

CONTRACTOR TO INSTALL TEMPORARY STORM DRAIN AS INDICATED HEREON TO PROPOSED SEDIMENT BASINS/TRAPS. ALL TEMPORARY STORM DRAIN TO BE REMOVED AS NEEDED AS CONSTRUCTION PROGRESSES.

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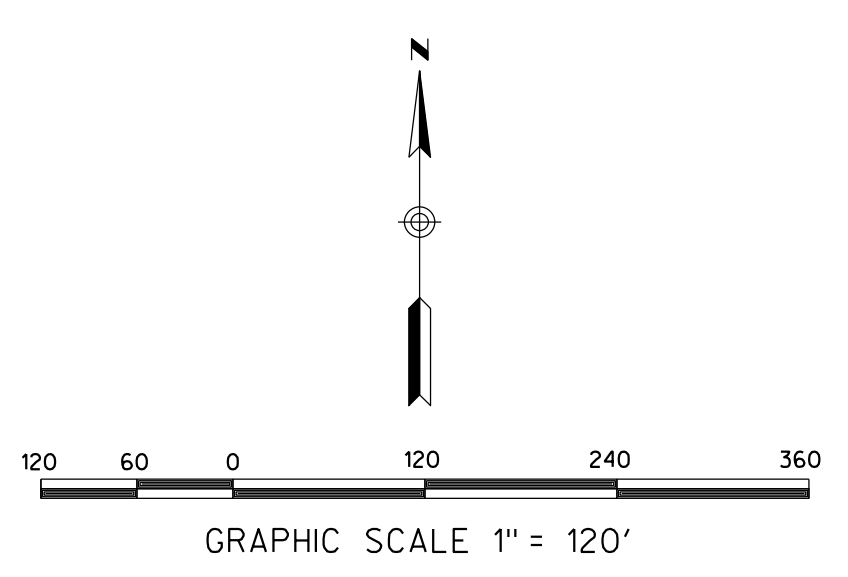
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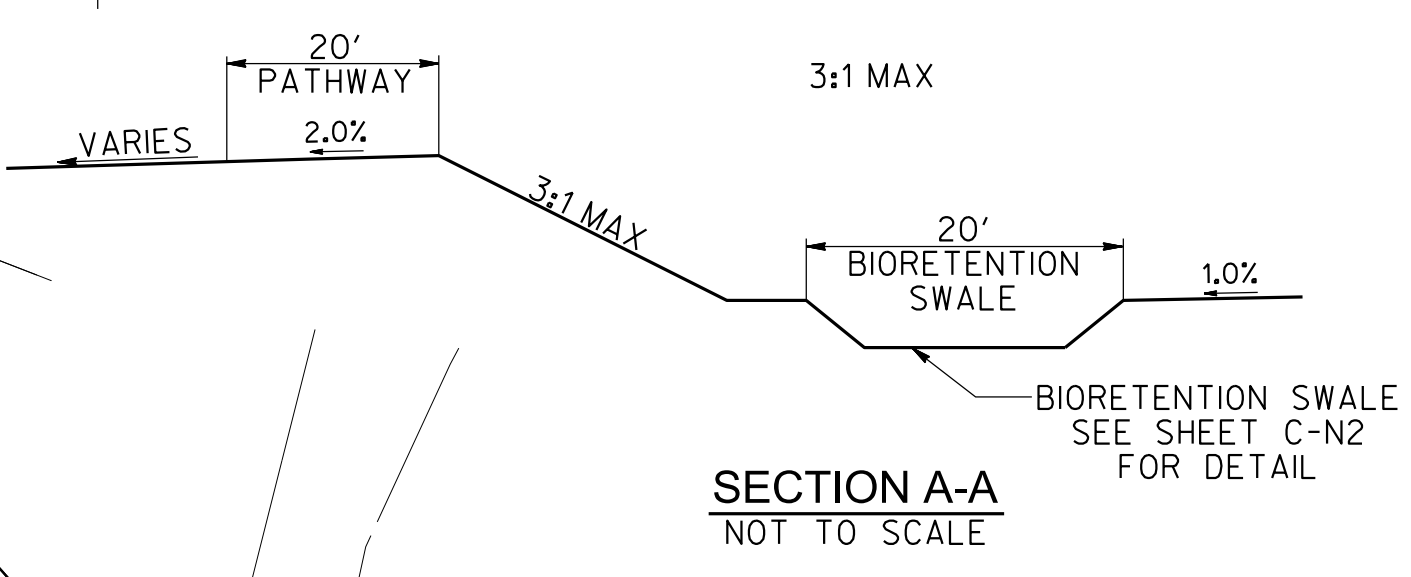
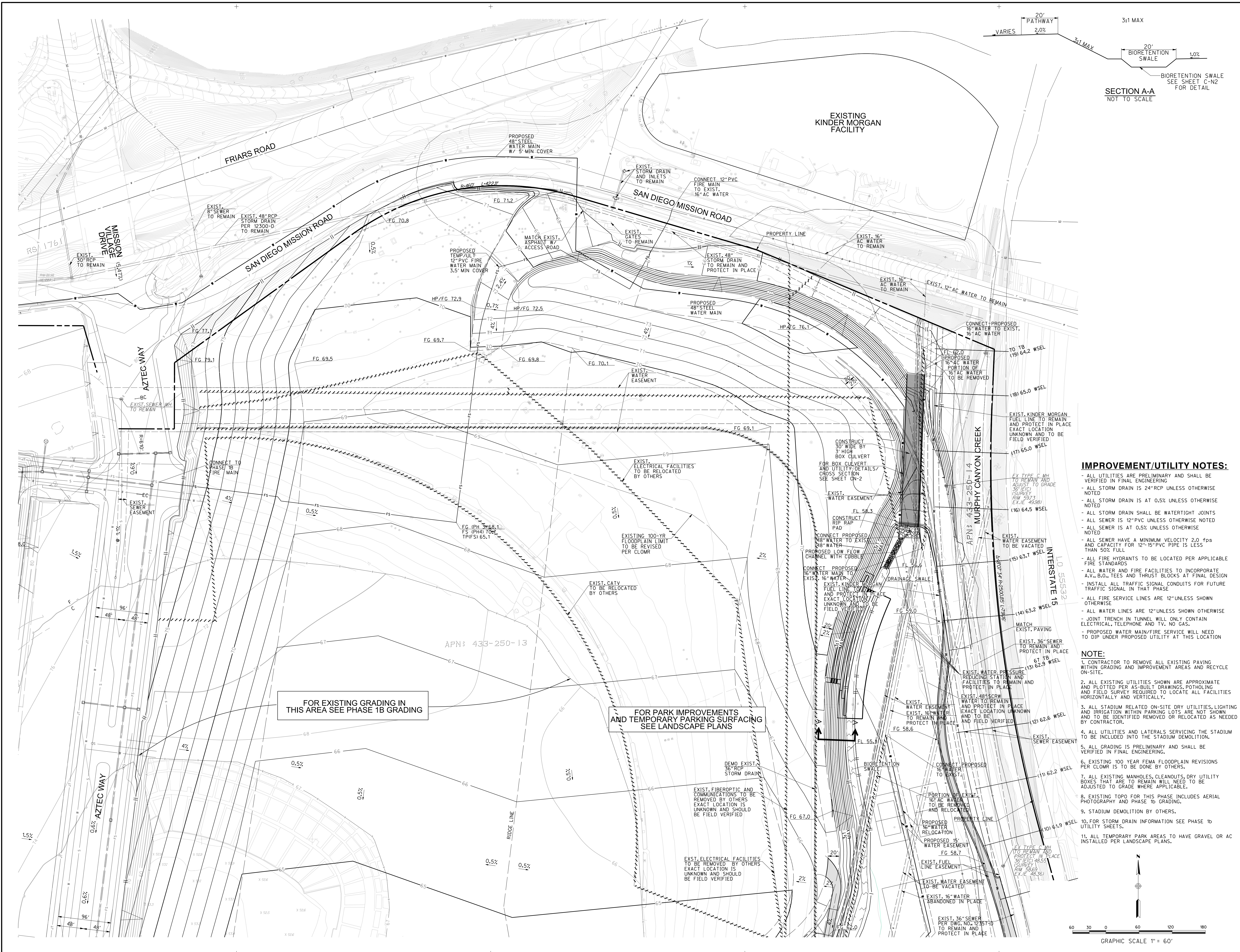
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**PHASE 1B  
EROSION  
CONTROL**

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DRAWING NO:

**C-2.9**







**IMPROVEMENT/UTILITY NOTES:**

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- INSTALL ALL TRAFFIC SIGNAL CONDUITS FOR FUTURE TRAFFIC SIGNAL IN THAT PHASE
- ALL FIRE SERVICE LINES ARE 12" UNLESS SHOWN OTHERWISE
- ALL WATER LINES ARE 12" UNLESS SHOWN OTHERWISE
- JOINT TRENCH IN TUNNEL WILL ONLY CONTAIN ELECTRICAL, TELEPHONE AND TV, NO GAS.
- PROPOSED WATER MAIN/FIRE SERVICE WILL NEED TO DIP UNDER PROPOSED UTILITY AT THIS LOCATION

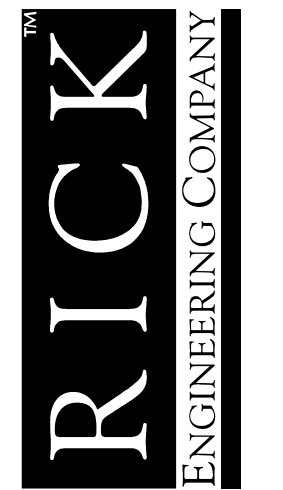
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8. EXISTING TOPO FOR THIS PHASE INCLUDES AERIAL PHOTOGRAPHY AND PHASE 1B GRADING.
9. STADIUM DEMOLITION BY OTHERS.
10. FOR STORM DRAIN INFORMATION SEE PHASE 1B UTILITY SHEETS.
11. ALL TEMPORARY PARK AREAS TO HAVE GRAVEL OR AC INSTALLED PER LANDSCAPE PLANS.

FOR EXISTING GRADING IN THIS AREA SEE PHASE 1B GRADING

FOR PARK IMPROVEMENTS AND TEMPORARY PARKING SURFACING SEE LANDSCAPE PLANS

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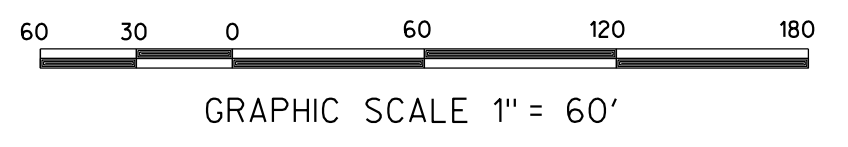
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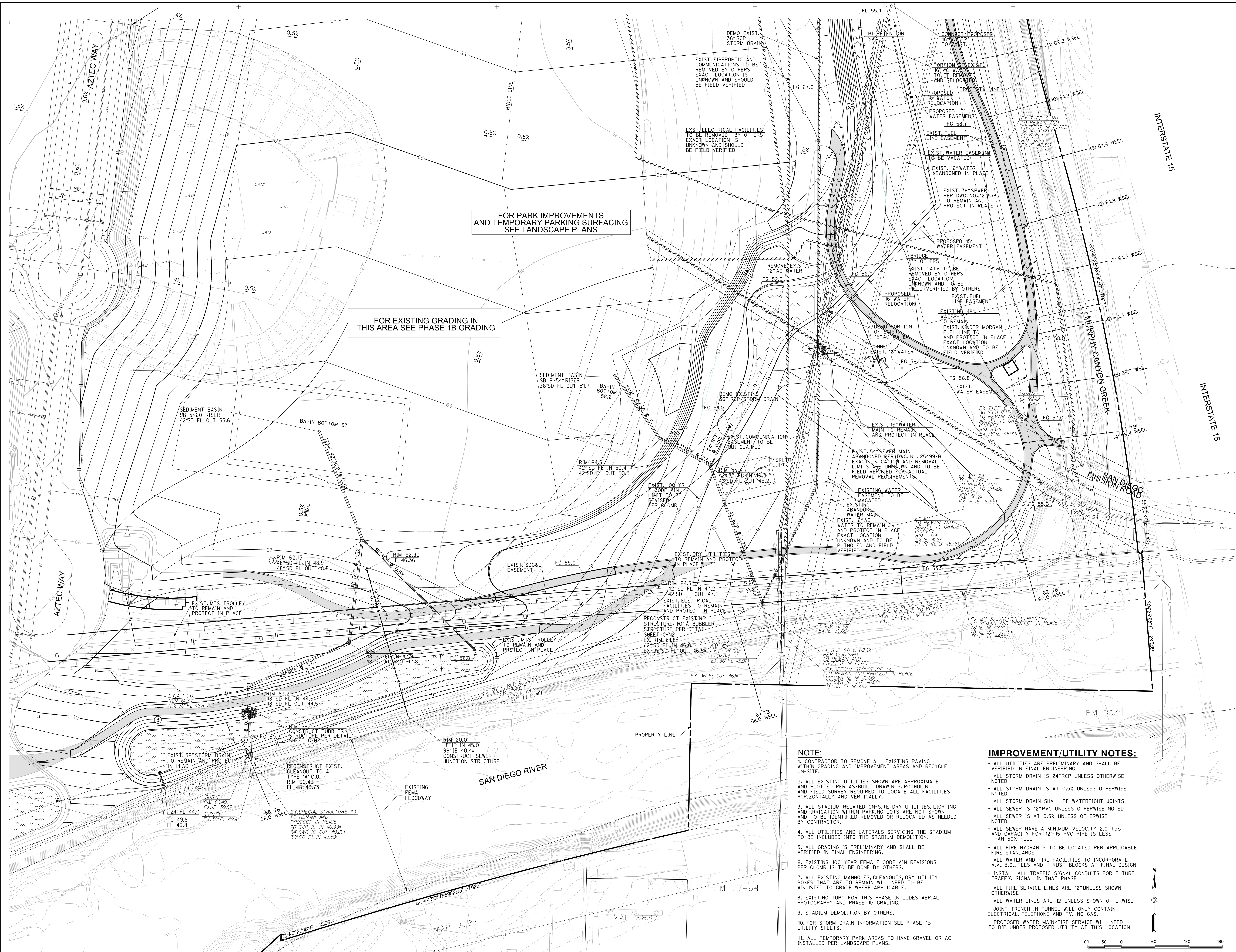
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**PHASE 3  
GRADING  
AND  
UTILITIES**  
18 OF 23  
DRAWING NO:

**C-3.1**





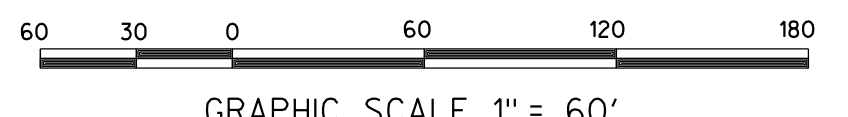


FOR PARK IMPROVEMENTS  
AND TEMPORARY PARKING SURFACING  
SEE LANDSCAPE PLANS

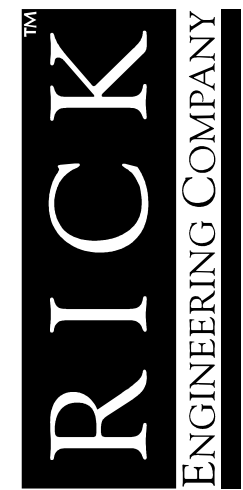
FOR EXISTING GRADING IN  
THIS AREA SEE PHASE 1B GRADING

- NOTE:**
1. CONTRACTOR TO REMOVE ALL EXISTING PAVING WITHIN GRADING AND IMPROVEMENT AREAS AND RECYCLE ON-SITE.
  2. ALL EXISTING UTILITIES SHOWN ARE APPROXIMATE AND PLOTTED PER AS-BUILT DRAWINGS, POTHOLES AND FIELD SURVEY REQUIRED TO LOCATE ALL FACILITIES HORIZONTALLY AND VERTICALLY.
  3. ALL STADIUM RELATED ON-SITE DRY UTILITIES, LIGHTING AND IRRIGATION WITHIN PARKING LOTS ARE NOT SHOWN AND TO BE IDENTIFIED REMOVED OR RELOCATED AS NEEDED BY CONTRACTOR.
  4. ALL UTILITIES AND LATERALS SERVICING THE STADIUM TO BE INCLUDED INTO THE STADIUM DEMOLITION.
  5. ALL GRADING IS PRELIMINARY AND SHALL BE VERIFIED IN FINAL ENGINEERING.
  6. EXISTING 100 YEAR FEMA FLOODPLAIN REVISIONS PER CLOMR IS TO BE DONE BY OTHERS.
  7. ALL EXISTING MANHOLES, CLEANOUTS, DRY UTILITY BOXES THAT ARE TO REMAIN WILL NEED TO BE ADJUSTED TO GRADE WHERE APPLICABLE.
  8. EXISTING TOPO FOR THIS PHASE INCLUDES AERIAL PHOTOGRAPHY AND PHASE 1B GRADING.
  9. STADIUM DEMOLITION BY OTHERS.
  10. FOR STORM DRAIN INFORMATION SEE PHASE 1b UTILITY SHEETS.
  11. ALL TEMPORARY PARK AREAS TO HAVE GRAVEL OR AC INSTALLED PER LANDSCAPE PLANS.

- IMPROVEMENT/UTILITY NOTES:**
- ALL UTILITIES ARE PRELIMINARY AND SHALL BE VERIFIED IN FINAL ENGINEERING.
  - ALL STORM DRAIN IS 24" RCP UNLESS OTHERWISE NOTED
  - ALL STORM DRAIN IS AT 0.5% UNLESS OTHERWISE NOTED
  - ALL STORM DRAIN SHALL BE WATERTIGHT JOINTS
  - ALL SEWER IS 12" PVC UNLESS OTHERWISE NOTED
  - ALL SEWER IS AT 0.5% UNLESS OTHERWISE NOTED
  - ALL SEWER HAVE A MINIMUM VELOCITY 2.0 fps AND CAPACITY FOR 12"-15" PVC PIPE IS LESS THAN 50% FULL
  - ALL FIRE HYDRANTS TO BE LOCATED PER APPLICABLE FIRE STANDARDS
  - ALL WATER AND FIRE FACILITIES TO INCORPORATE A.V., B.O., TEES AND THRUST BLOCKS AT FINAL DESIGN
  - INSTALL ALL TRAFFIC SIGNAL CONDUITS FOR FUTURE TRAFFIC SIGNAL IN THAT PHASE
  - ALL FIRE SERVICE LINES ARE 12" UNLESS SHOWN OTHERWISE
  - ALL WATER LINES ARE 12" UNLESS SHOWN OTHERWISE
  - JOINT TRENCH IN TUNNEL WILL ONLY CONTAIN ELECTRICAL, TELEPHONE AND TV. NO GAS.
  - PROPOSED WATER MAIN/FIRE SERVICE WILL NEED TO DIP UNDER PROPOSED UTILITY AT THIS LOCATION



5620 FRIARS ROAD  
SAN DIEGO, CA 92110  
619-291-0707  
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**SAN DIEGO STATE UNIVERSITY  
MISSION VALLEY  
75% DD SITE DEVELOPMENT PACKAGE**

ISSUED: 01/18/19-RFP  
REV. 2/12/19

PROJECT NO:  
FILENAME:  
DRAWN BY: CHECKED BY:  
PLOT DATE:

**PHASE 3  
GRADING  
AND  
UTILITIES**

DRAWING NO:  
**C-3.2**





**EROSION CONTROL LEGEND**

- PADS (TACKFIER)
- SLOPES (BONDED FIBER MATRIX OR EQUIVALENT)

**NOTES**

CONTRACTOR TO PROVIDE EROSION CONTROL MEASURES AS INDICATED ON CONTRACTOR PREPARED SWPPP, I.E. SILT FENCING, GRAVEL BAG CHECK DAMS, FIBER ROLL, INLET PROTECTION, CONSTRUCTION ENTRANCE AND WASHOUT STATIONS.

CONTRACTOR TO INSTALL TEMPORARY STORM DRAIN AS INDICATED HEREON TO PROPOSED SEDIMENT BASINS/TRAPS. ALL TEMPORARY STORM DRAIN TO BE REMOVED AS NEEDED AS CONSTRUCTION PROGRESSES.

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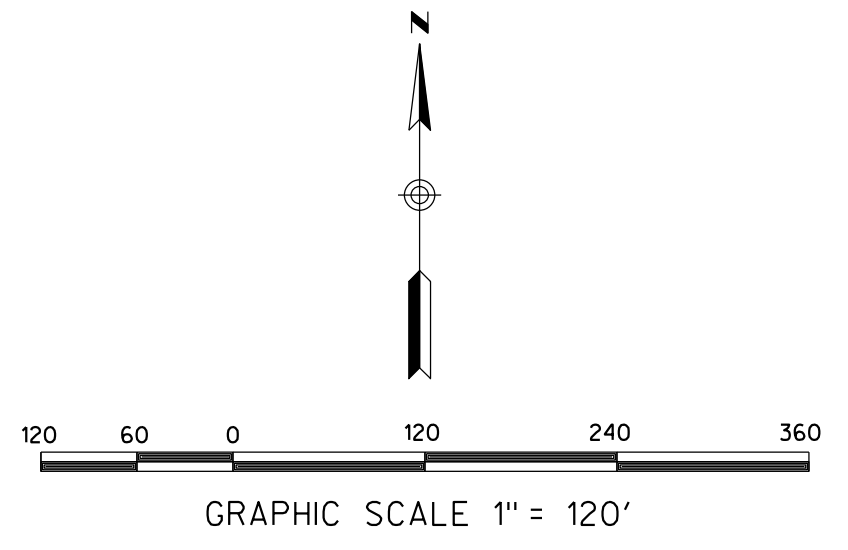
**SAN DIEGO STATE UNIVERSITY  
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ISSUED: 01/18/18-RFP  
REV. 2/12/19

PROJECT NO: \_\_\_\_\_  
FILENAME: \_\_\_\_\_  
DRAWN BY: \_\_\_\_\_ CHECKED BY: \_\_\_\_\_  
PLOT DATE: \_\_\_\_\_

TITLE:  
**PHASE 3  
AND PARK  
EROSION  
CONTROL**  
20 OF 23  
DRAWING NO:

**C-3.3**







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DRAWN BY: CHECKED BY:  
PLOT DATE:

TITLE:  
**PHASE 4  
GRADING &  
UTILITIES**

21 OF 23  
DRAWING NO:  
**C-4.1**

**NOTE:**

1. ALL EXISTING UTILITIES SHOWN ARE APPROXIMATE AND PLOTTED PER AS-BUILT DRAWINGS, POTHOLES AND FIELD SURVEY REQUIRED TO LOCATE ALL FACILITIES HORIZONTALLY AND VERTICALLY.
2. ALL STADIUM RELATED ON-SITE DRY UTILITIES, LIGHTING AND IRRIGATION WITHIN PARKING LOTS ARE NOT SHOWN AND TO BE IDENTIFIED REMOVED OR RELOCATED AS NEEDED BY CONTRACTOR.
3. EXISTING TOPO FOR THIS PHASE INCLUDES AERIAL PHOTOGRAPHY AND PHASE 3 GRADING.

**IMPROVEMENT/UTILITY NOTES:**

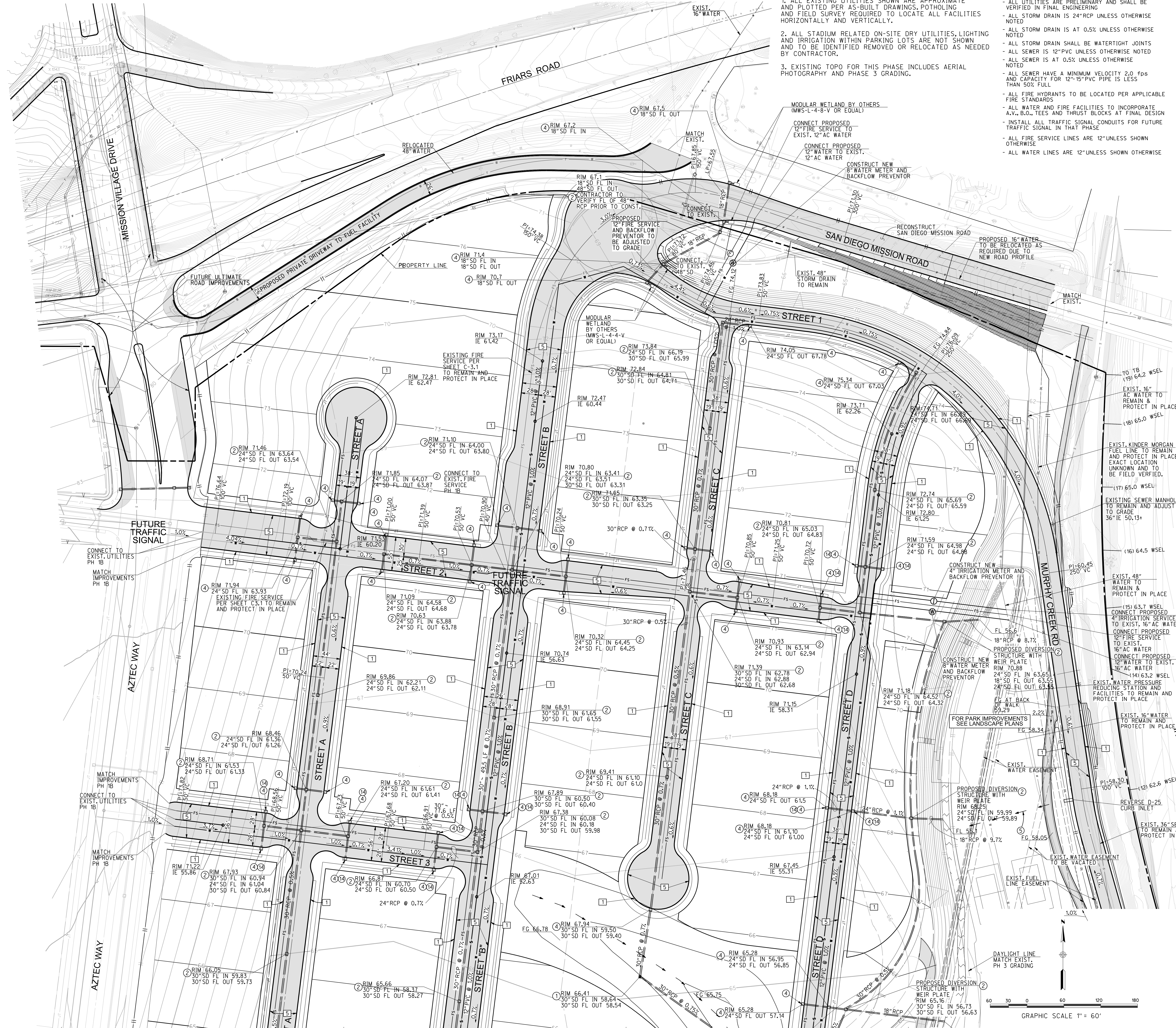
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- ALL WATER LINES ARE 12" UNLESS SHOWN OTHERWISE

**UTILITY CONSTRUCTION NOTES**

- 1 CONSTRUCT TYPE - I INLET
- 2 CONSTRUCT A-4 CLEANOUT
- 3 CONSTRUCT MOD. A-5 CLEANOUT
- 4 CONSTRUCT 5' TYPE - B INLET
- 5 CONSTRUCT SWALE PER DETAIL SHEET 3
- 6 CONSTRUCT A-5 CLEANOUT
- 7 CONSTRUCT MOD. A-8 CLEANOUT (Y=7)
- 8 CONSTRUCT A-6 CLEANOUT
- 9 CONSTRUCT A-8 CLEANOUT WITH GRATED TOP
- 10 CONSTRUCT A-7 CLEANOUT
- 11 INSTALL FIRE HYDRANT
- 12 STEEL CASING PER SEWER MAIN
- 13 JOINT TRENCH IN TUNNEL WILL ONLY CONTAIN ELECTRICAL, TELEPHONE AND TV, NO GAS.
- 14 PROPOSED WATER MAIN/FIRE SERVICE WILL NEED TO DIP UNDER PROPOSED UTILITY AT THIS LOCATION

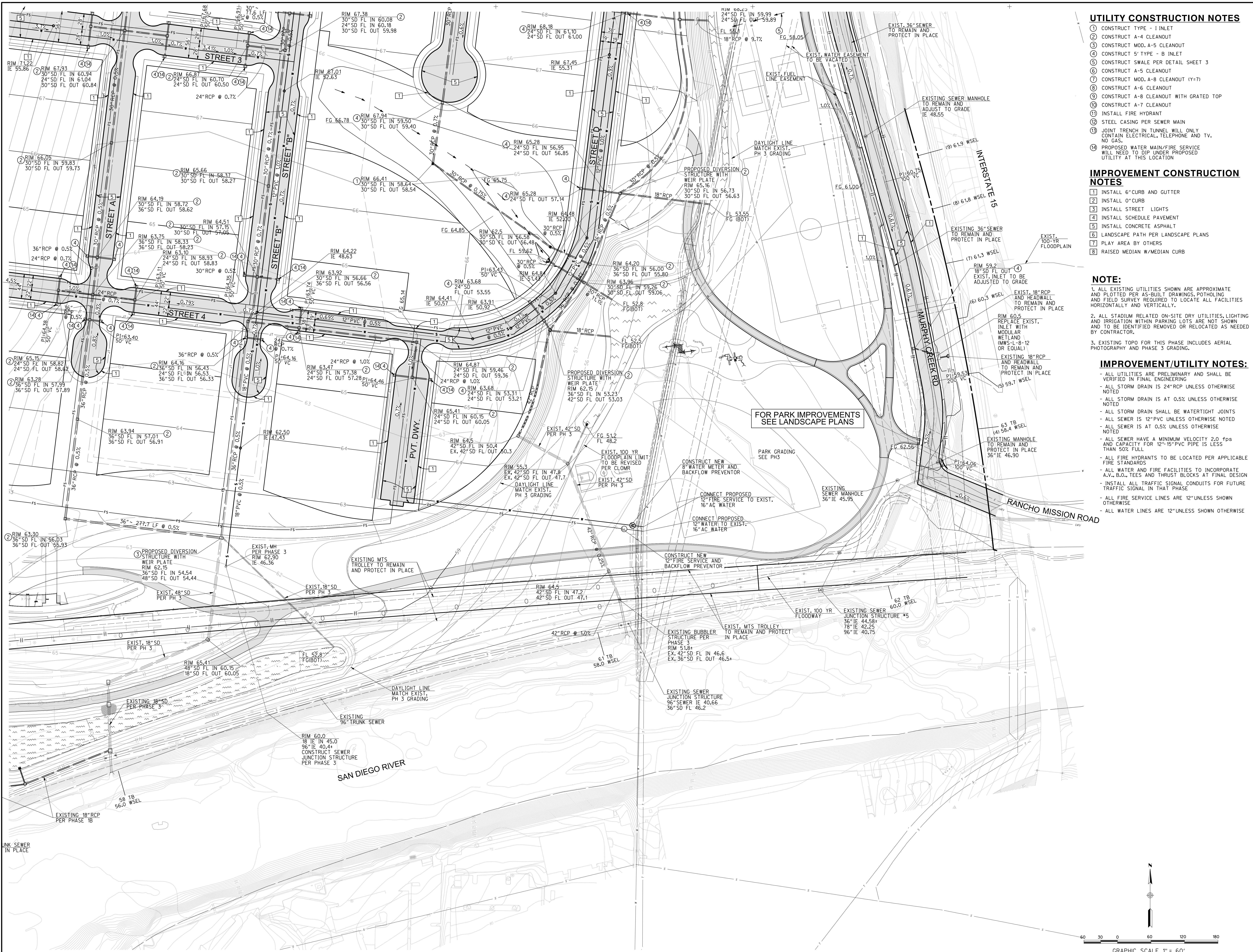
**IMPROVEMENT CONSTRUCTION NOTES**

- 1 INSTALL 6" CURB AND GUTTER
- 2 INSTALL 0" CURB
- 3 INSTALL STREET LIGHTS
- 4 INSTALL SCHEDULE PAVEMENT
- 5 INSTALL CONCRETE ASPHALT
- 6 LANDSCAPE PATH PER LANDSCAPE PLANS
- 7 PLAY AREA BY OTHERS
- 8 RAISED MEDIAN W/ MEDIAN CURB



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**UTILITY CONSTRUCTION NOTES**

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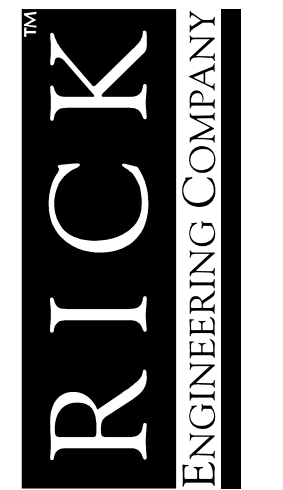
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- 3. EXISTING TOPO FOR THIS PHASE INCLUDES AERIAL PHOTOGRAPHY AND PHASE 3 GRADING.

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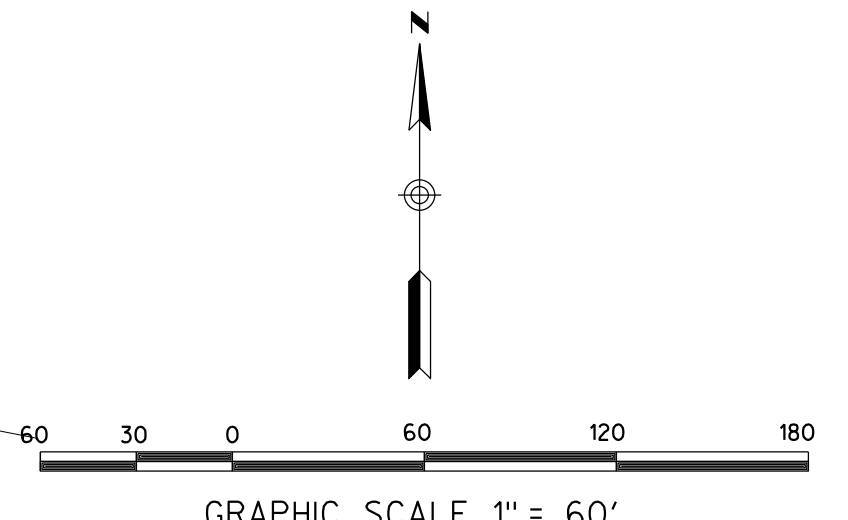
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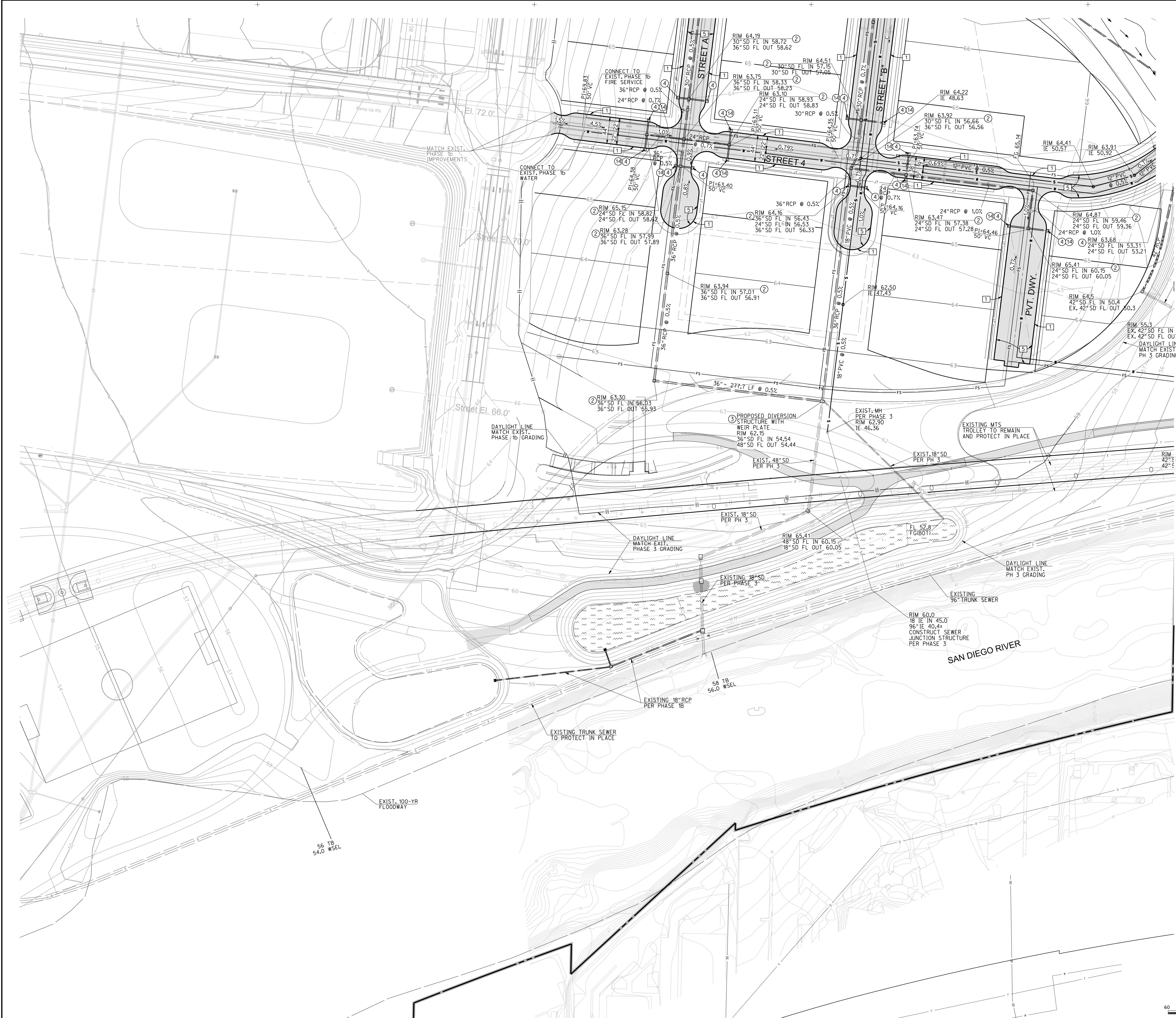
**PHASE 4  
GRADING &  
UTILITIES**

22 OF 23  
DRAWING NO:

**C-4.2**







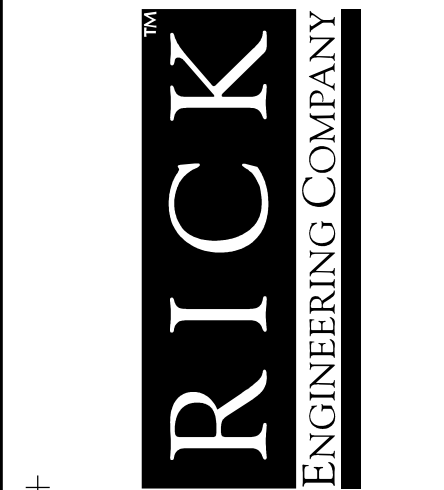
- UTILITY CONSTRUCTION NOTES**
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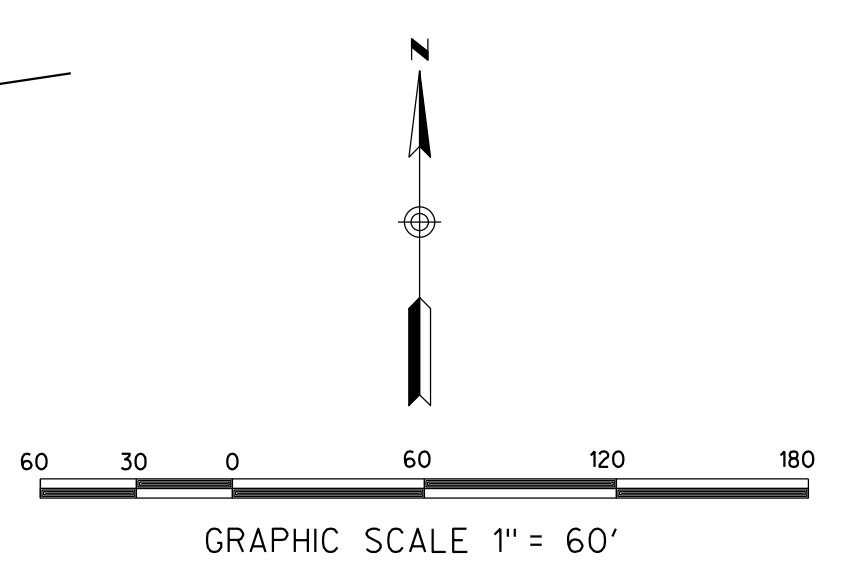
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TITLE:

**PHASE 4  
GRADING**

23 OF 23  
DRAWING NO:

**C-4.3**

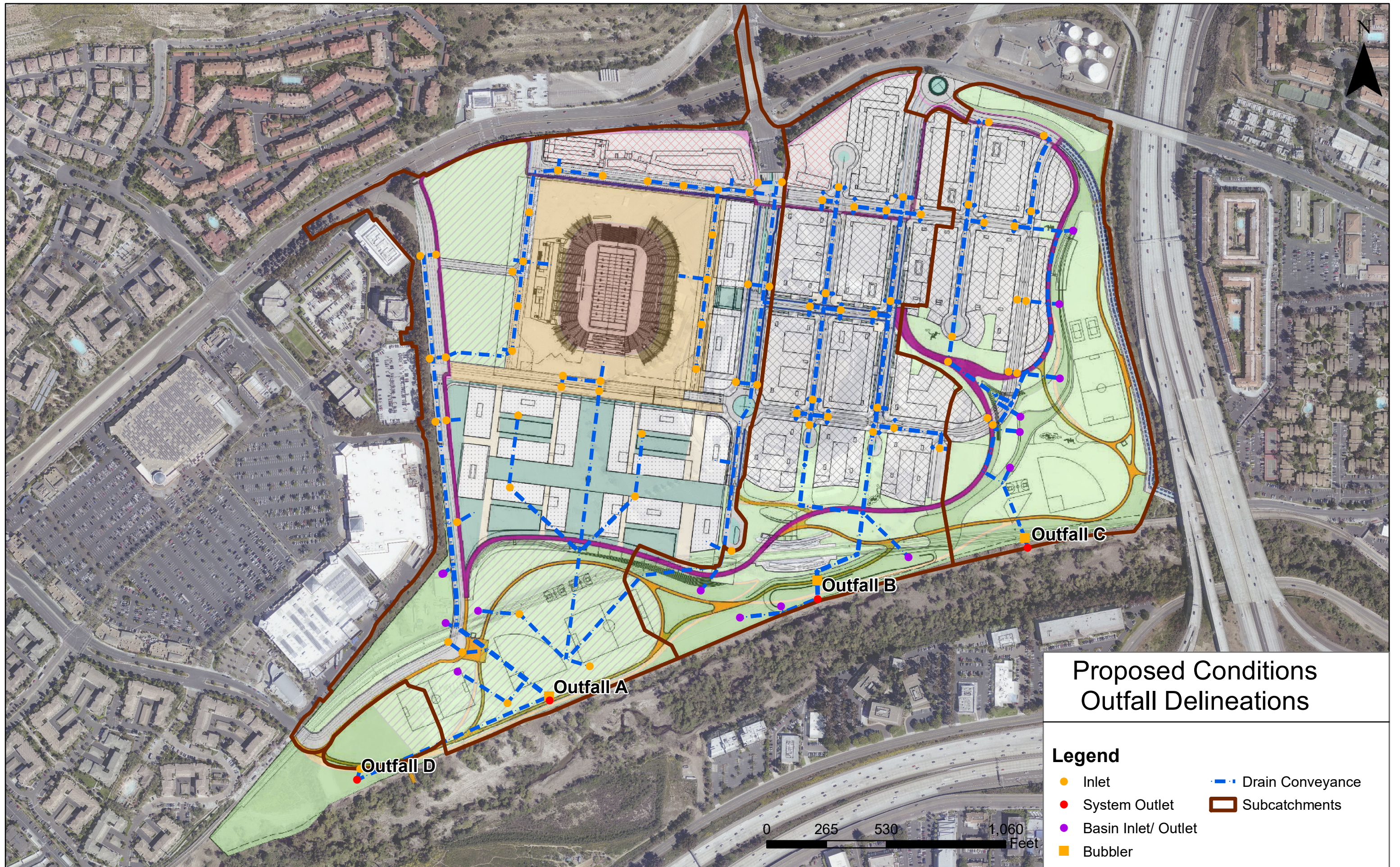




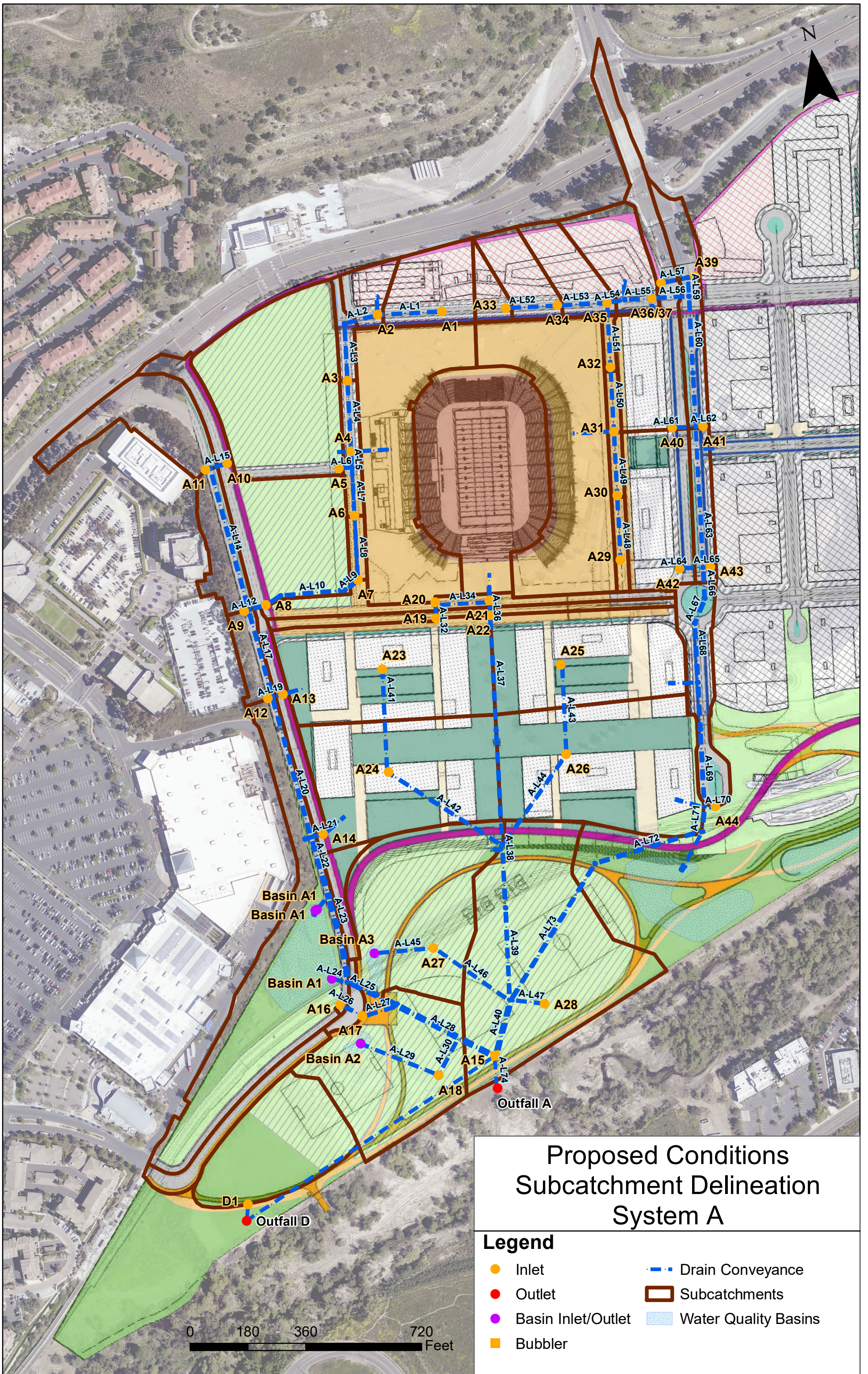
## **APPENDIX B.2**

### Existing Conditions: Drainage Area Exhibit

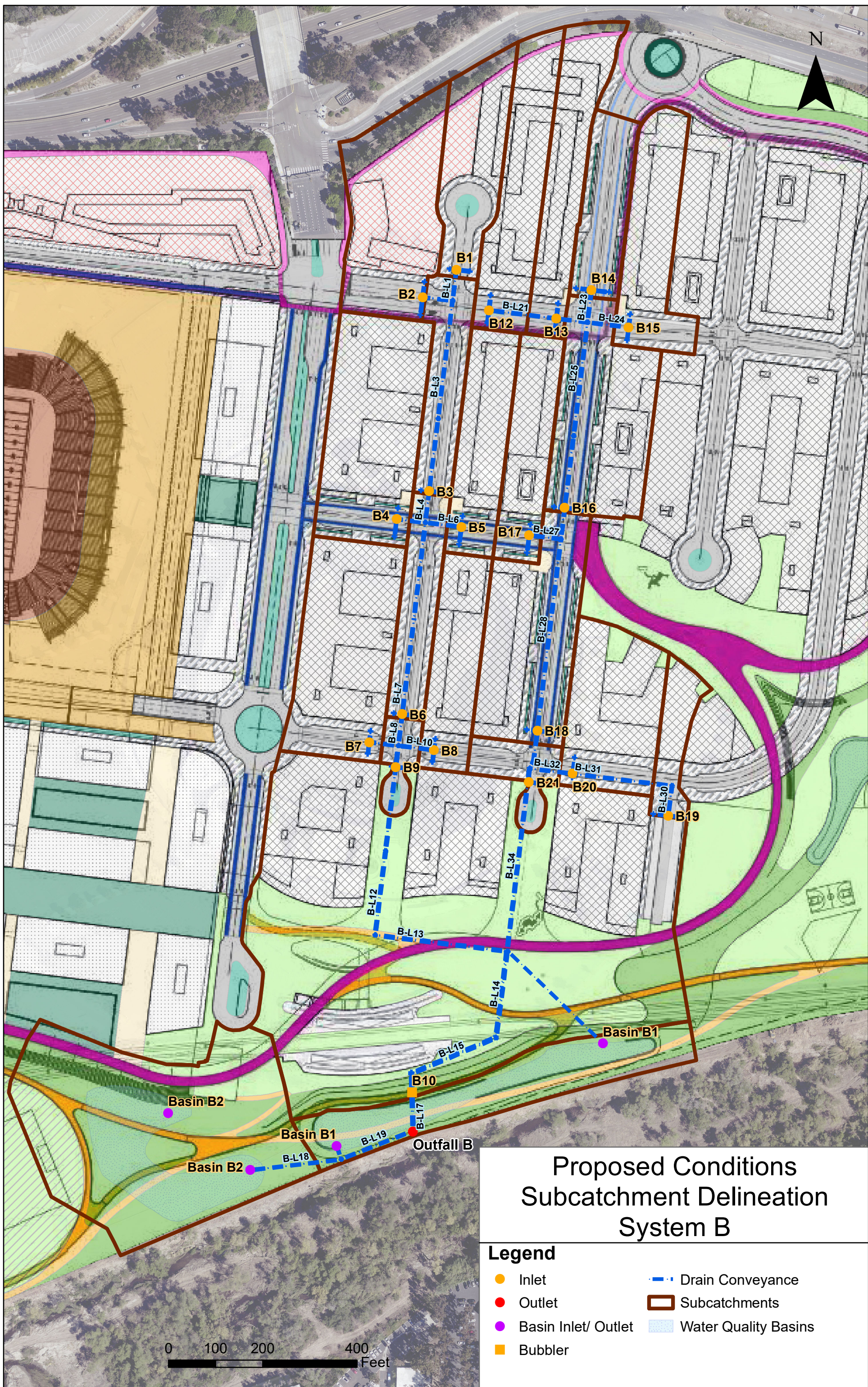












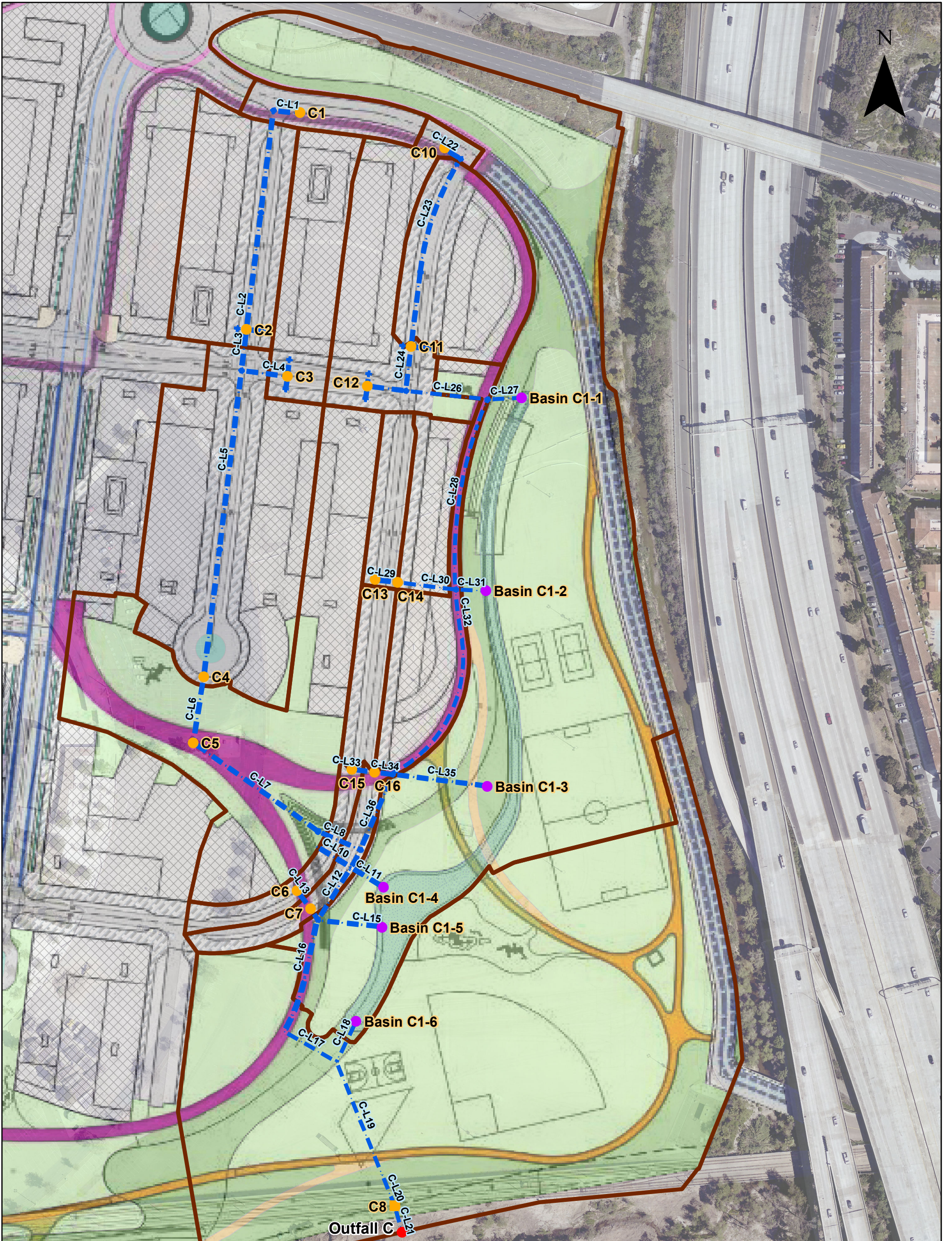
## Proposed Conditions Subcatchment Delineation System B

### Legend

- |   |   |
|---|---|
| <span style="color: yellow;">●</span> Inlet               | <span style="color: blue;">- - -</span> Drain Conveyance  |
| <span style="color: red;">●</span> Outlet                 | <span style="border: 2px solid brown; display: inline-block; width: 15px; height: 10px;"></span> Subcatchments                                    |
| <span style="color: purple;">●</span> Basin Inlet/ Outlet | <span style="background-color: lightblue; border: 1px solid blue; display: inline-block; width: 15px; height: 10px;"></span> Water Quality Basins |
| <span style="color: orange;">■</span> Bubbler             |   |

0 100 200 400 Feet





## Proposed Conditions Subcatchment Delineation System C

### Legend

- |   |  |
|---|--|
| <span style="color: yellow;">●</span> Inlet               | <span style="color: blue;">- - -</span> Drain Conveyance   |
| <span style="color: red;">●</span> System C Outlet        | <span style="border: 1px solid brown; display: inline-block; width: 15px; height: 10px;"></span> Subcatchments   |
| <span style="color: purple;">●</span> Basin Inlet/ Outlet | <span style="background-color: lightblue; border: 1px solid lightblue; display: inline-block; width: 15px; height: 10px;"></span> Water Quality Basins |
| <span style="color: orange;">●</span> Bubbler             |  |

0 100 200 400 Feet



## **APPENDIX B.3**

### Proposed Conditions: Runoff Coefficients



# Runoff Coefficients

Runoff Coefficients (C) for each sub-catchment were determined using the values in Table 1 below. Values were determined based on Hydrologic Soils Group (HSG) Type D soils. The SDDDM provides runoff coefficients for urban land uses in Table A-1 and recommends that for “parks, golf courses, or other types of non-urban land use that are expected to be permanent, the appropriate value should be selected based upon the soil and cover and approved by the City.” The percent of impervious area was determined from the preliminary plans provided at the end of this appendix.

Land Use	Area (ac)	% Impervious	% Pervious	C
Bike lane and path	1.24	90	10	0.95
Community hike and bike trail	4.08	60	40	0.64
Development	34.16	70	30	0.70
Parking	2.54	90	10	0.95
Median and Stormwater	1.63	10	90	0.41
Hike and Bike Loop	4.30	60	40	0.64
Hospitality	5.26	40	60	0.57
Park and Recreation	70.6	30	70	0.52
Paseos	2.21	40	60	0.57
Stadium	5.44	90	10	0.95
Stadium Concourse	13.53	60	40	0.64
Streets and sidewalks	23.64	90	10	0.95
Trolley Plaza	0.74	70	30	0.70



## APPENDIX A: RATIONAL METHOD AND MODIFIED RATIONAL METHOD

Combining the units for the expression CIA yields:

$$\left( \frac{1 \text{ acre} \times \text{inch}}{\text{hour}} \right) \left( \frac{43,560 \text{ ft}^2}{\text{acre}} \right) \left( \frac{1 \text{ foot}}{12 \text{ inches}} \right) \left( \frac{1 \text{ hour}}{3,600 \text{ seconds}} \right) \Rightarrow 1.008 \text{ cfs}$$

For practical purposes, the unit conversion coefficient difference of 0.8% can be ignored.

The RM formula is based on the assumption that for constant rainfall intensity, the peak discharge rate at a point will occur when the raindrop that falls at the most upstream point in the tributary drainage basin arrives at the point of interest.

Unlike the MRM (discussed in Appendix A.2) or the NRCS hydrologic method (discussed in Appendix B), the RM does not create hydrographs and therefore does not add separate subarea hydrographs at collection points. Instead, the RM develops peak discharges in the main line by increasing the  $T_c$  as flow travels downstream.

Characteristics of, or assumptions inherent to, the RM are listed below:

1. The discharge resulting from any I is maximum when the I lasts as long as or longer than the  $T_c$ .
2. The storm frequency of peak discharges is the same as that of I for the given  $T_c$ .
3. The fraction of rainfall that becomes runoff (or the runoff coefficient, C) is independent of I or precipitation zone number (PZN) condition (PZN Condition is discussed in the NRCS method).
4. The peak rate of runoff is the only information produced by using the RM.

### A.1.2. Runoff Coefficient

The runoff coefficients are based on land use (see Table A-1). Soil type "D" is used throughout the City of San Diego for storm drain conveyance design. An appropriate runoff coefficient (C) for each type of land use in the subarea should be selected from this table and multiplied by the percentage of the total area (A) included in that class. The sum of the products for all land uses is the weighted runoff coefficient ( $\Sigma[CA]$ ). Good engineering judgment should be used when applying the values presented in Table A-1, as adjustments to these values may be appropriate based on site-specific characteristics.



Table A-1. Runoff Coefficients for Rational Method

Land Use	Runoff Coefficient (C)
	Soil Type <sup>(1)</sup>
<b>Residential:</b>	
Single Family	0.55
Multi-Units	0.70
Mobile Homes	0.65
Rural (lots greater than 1/2 acre)	0.45
<b>Commercial <sup>(2)</sup></b>	
80% Impervious	0.85
<b>Industrial <sup>(2)</sup></b>	
90% Impervious	0.95

**Note:**

<sup>(1)</sup> Type D soil to be used for all areas.

<sup>(2)</sup> Where actual conditions deviate significantly from the tabulated imperviousness values of 80% or 90%, the values given for coefficient C, may be revised by multiplying 80% or 90% by the ratio of actual imperviousness to the tabulated imperviousness. However, in case shall the final coefficient be less than 0.50. For example: Consider commercial property on D soil.

$$\begin{aligned}
 \text{Actual imperviousness} &= 50\% \\
 \text{Tabulated imperviousness} &= 80\% \\
 \text{Revised C} &= (50/80) \times 0.85 = 0.53
 \end{aligned}$$

The values in Table A-1 are typical for urban areas. However, if the basin contains rural or agricultural land use, parks, golf courses, or other types of nonurban land use that are expected to be permanent, the appropriate value should be selected based upon the soil and cover and approved by the City.

### A.1.3. Rainfall Intensity

The rainfall intensity (I) is the rainfall in inches per hour (in/hr.) for a duration equal to the T<sub>c</sub> for a selected storm frequency. Once a particular storm frequency has been selected for design and a T<sub>c</sub> calculated for the drainage area, the rainfall intensity can be determined from the Intensity-Duration-Frequency Design Chart (Figure A-1).





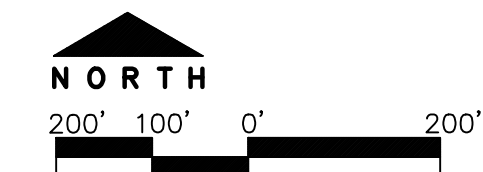


### OPEN SPACE

ACTIVE		
	SHARED SDSU/COMMUNITY ACTIVE PARKS INCLUDING: RECREATION FIELD AND TAIL GATE ZONE (7.22 ACRES) FIELDS AT RIVER PARK (14.5 ACRES)	21.72 ACRES
	ACTIVE PARK AND GREEN SPACE INCLUDING: DOG PARK (1.07 ACRES) COMMUNITY AQUATIC CENTER PAD (0.85)	21.62 ACRES
	HIKE AND BIKE LOOP	4.27 ACRES
	COMMUNITY HIKE AND BIKE TRAIL	4.11 ACRES
<b>SUBTOTAL</b>		<b>51.72 ACRES</b>
PASSIVE		
	CAMPUS PASSIVE PARK AND GREEN SPACE INCLUDING: CAMPUS MALL (2.18 ACRES) CAMPUS GREEN (2.07 ACRES) 50 YARD LINE PLAZA (0.29 ACRES) COURTYARDS (3.91 ACRES)	8.45 ACRES
	COMMUNITY PASSIVE PARK AND GREEN SPACE	18.85 ACRES
<b>SUBTOTAL</b>		<b>27.30 ACRES</b>
PASEOS		
	CAMPUS PASEO	2.01 ACRES
	NEIGHBORHOOD PASEO	0.20 ACRES
<b>SUBTOTAL</b>		<b>2.21 ACRES</b>
<b>TOTAL</b>		<b>81.23 ACRES</b>

### LEGEND

DEVELOPMENT		
	STADIUM	5.46 ACRES
	STADIUM CONCOURSE	13.53 ACRES
	CAMPUS DEVELOPMENT	9.58 ACRES
	RESIDENTIAL DEVELOPMENT	24.56 ACRES
	HOSPITALITY	5.24 ACRES
<b>SUBTOTAL</b>		<b>58.37 ACRES</b>
CIRCULATION		
	STREETS	14.48 ACRES
	SIDEWALKS	9.15 ACRES
	MEDIAN AND STORM WATER	1.64 ACRES
	BIKE LANE AND PATH	1.22 ACRES
	PARKING	2.56 ACRES
<b>SUBTOTAL</b>		<b>29.05 ACRES</b>
<b>TOTAL</b>		<b>87.42 ACRES</b>



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**SDSU MV Area Calculation  
1/10/2019**

Qualcomm Site	Total Acres	South of River (F,G,H)	Floodway (E)	North of River (A,B,C,D,I)
APN 433-250-13 (Parcel A)	81.1	0	0	81.1
APN 433-250-16 (Parcels B,C,D,E,F,G,H,I)	133.5	11.8	31.7	90
				171.1
	214.6		214.6	

Outside Property Line		South of River		North of River
APN 433-250-19 (Parcel J)		0.8		
APN 433-250-14 (Parcel K)				2.6
APN 433-250-05 (Parcel L)		2.8		
			6.2	

	ACRES			
	CAMPUS	NEIGHBORHOOD	COMMUNITY	TOTAL
<b>DEVELOPABLE LAND</b>				
Stadium	5.46			5.46
Stadium Concourse	13.53			13.53
Entry Monument (OMITTED)	0.00			0.00
Development	9.58	24.56		34.15
Hospitality	3.83	1.41		5.24
Pad For Aquatic and Recreational Center			0.85	0.85
Trolley Green Line Station			0.74	0.74
Restrooms (count 2)	0.00		0.02	0.02
San Diego River Park Foundation Shed			0.00	0.00
Subtotal	32.41	25.97	1.61	59.98
<b>OPEN SPACE</b>				
Recreation Field and Tail Gate Zone (Active)	7.22			7.22
Park and Recreation (Active) <sup>1</sup>	15.65		18.55	34.20
Dog Park (Active)			1.07	1.07
Park and Recreation (Passive)			18.85	18.85
Mall (Passive)	2.18			2.18
Green (Passive)	2.07			2.07
50 yard line plaza (Passive)	0.29			0.29
Courtyards (Passive)	3.91			3.91
Paseo (Passive)	2.01	0.20		2.20
Medians and Stormwater (Passive)	1.11	0.53		1.64
Hike and Bike Loop (Active)			4.27	4.27
Community Hike and Bike Trail (Active)			4.11	4.11
Subtotal	34.44	0.73	46.85	82.01
<b>ROADWAYS</b>				
Bike lane and Paths	0.69	0.54		1.22
Streets	6.80	7.67	0.00	14.48
Sidewalks	1.95	7.21	0.00	9.15
Parking	0.66	1.90	0.00	2.56
Subtotal	10.10	17.31	0.00	27.41
<b>RIVERINE</b>				
Floodway (San Diego River)			31.7	31.70
Environmentally Sensitive Land			11.8	11.80
Subtotal			43.5	43.50
<b>TOTALS</b>	76.95	44.01	91.95	212.91

<sup>1</sup> Includes 1.3-acre MTD fee-title for SD Trolley Line. No development proposed within.

Proposed Land Use	Footprint (acres)
Parks (includes 1.3 ac of MTD land)	83.62
Campus (including Stadium)	28.58
Residential	24.56
Hospitality	5.24
Circulation	27.41
Total	169.41

	ACRES	SF
CEQA boundary	169.3043735	7374898.51
Area take offs	169.41	
Off by	0.10	



# **APPENDIX B.4**

## **Proposed Conditions: Time of Concentration**



# Proposed System A Time of Concentration

The Time of Concentration calculations were performed using the following equation:

$$T = \frac{1.8 * (1.1 - C) * \sqrt{D}}{\sqrt[3]{S}}$$

Data in the table below was taken from the Standard Drawings, Grading Plans, and proposed storm drain networks in Appendix B.2. Inlet IDs coincide with the labels on the Proposed Conditions Figure in Appendix B.1-2.

Proposed System A Time of Concentration				
Drainage Area to Pipe	C	LFP		Tc (min)
		Length	Slope (%)	
A01	0.69	100	1	7
A02	0.72	100	1	7
A03	0.95	100	1	5
A04	0.95	100	1	5
A05	0.55	550	1	23
A45	0.65	100	1	8
A06	0.75	100	1	6
A07	0.64	100	1	8
A08	0.58	450	1	20
A09	0.67	100	1	8
A10	0.88	100	1	5
A11	0.94	100	1	5
A12	0.66	100	1	8
A13	0.83	100	1	5
A14	0.77	100	1	6
BasinA1	0.59	650	1	23
A17	0.93	100	1	5
BasinA2	0.84	100	1	5
A18	0.53	300	1	18
A19	0.65	100	1	8
A20	0.61	100	1	9
A21	0.87	100	1	5
A22	0.70	100	1	7
A23	0.63	450	1	18
A24	0.59	600	1	23
A25	0.65	300	1	14



A26	0.59	450	1	19
BasinA3	0.67	100	1	8
A27	0.54	400	1	20
A28	0.53	850	1	30
A29	0.64	100	1	5
A30	0.64	100	1	5
A31	0.64	100	1	5
A32	0.64	100	1	5
A33	0.73	100	1	7
A34	0.68	100	1	8
A35	0.68	100	1	8
A36/37	0.67	100	1	8
A38	0.93	100	1	5
A39	0.91	100	1	5
A40	0.76	100	1	6
A41	0.87	100	1	5
A42	0.74	100	1	6
A43	0.87	100	1	5
A44	0.84	100	1	5
A46	0.66	100	1	5
D1	0.52	350	1	20



# Proposed System B Time of Concentration

The Time of Concentration calculations were performed using the following equation:

$$T = \frac{1.8 * (1.1 - C) * \sqrt{D}}{\sqrt[3]{S}}$$

Data in the table below was taken from the Standard Drawings, Grading Plans, and proposed storm drain networks in Appendix B.2. Inlet IDs coincide with the labels on the Proposed Conditions Figure in Appendix B.1-3.

Proposed System B Time of Concentration				
Drainage Area to Pipe	C	LFP		Tc (min)
		Length	Slope (%)	
B01	0.776126	100	1	6
B02	0.712434	100	1	7
B03	0.949656	100	1	5
B04	0.740567	100	1	6
B05	0.738174	100	1	7
B06	0.930409	100	1	5
B07	0.74655	100	1	6
B08	0.743485	100	1	6
B09	0.911508	100	1	5
B10	0.611745	750	1.3	22
BasinB2	0.539189	550	1	24
BasinB1	0.537563	100	1	10
B12	0.748752	100	1	6
B13	0.737136	100	1	7
B14	0.90873	100	1	5
B15	0.756855	100	1	6
B16	0.813638	100	1	5
B17	0.740209	100	1	6
B18	0.809238	100	1	5
B19	0.779564	100	1	6
B20	0.748089	100	1	6
B21	0.926023	100	1	5



# Proposed System C Time of Concentration

The Time of Concentration calculations were performed using the following equation:

$$T = \frac{1.8 * (1.1 - C) * \sqrt{D}}{\sqrt[3]{S}}$$

Data in the table below was taken from the Standard Drawings, Grading Plans, and proposed storm drain networks in Appendix B.2. Inlet IDs coincide with the labels on the Proposed Conditions Figure in Appendix B.1-4.

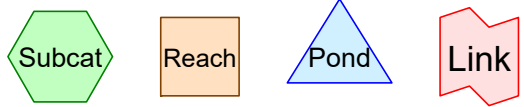
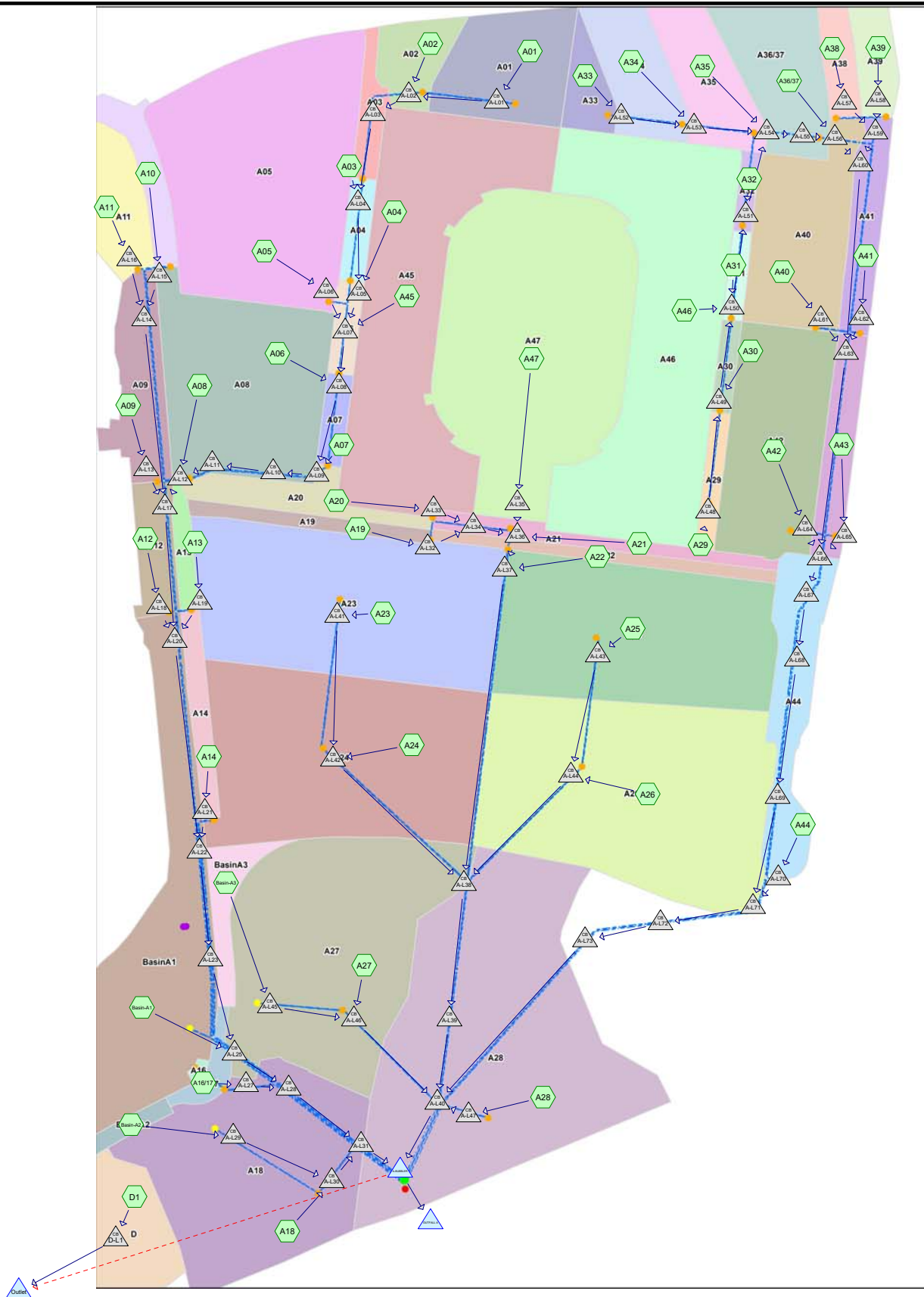
Proposed System C Time of Concentration				
Drainage Area to Pipe	C	LFP		Tc (min)
		Length	Slope (%)	
C01	0.929389	100	1	5
C02	0.810714	100	1	5
C03	0.74289	100	1	6
C04	0.781098	600	1	14
C05	0.6088	350	1.2	16
C06	0.91421	100	1	5
C07	0.922744	100	1	5
BasinC1-6	0.544218	650	1	26
C08	0.552335	850	1	29
C10	0.945514	100	1	5
C11	0.795178	100	1	5
C12	0.763909	100	1	6
C13	0.949999	100	1	5
C14	0.765357	100	1	6
C15	0.95	100	1	5
C16	0.705403	100	1	7



# **APPENDIX B.5**

## **Proposed Conditions: HydroCAD Reports**





**Routing Diagram for Proposed System AD**  
 Prepared by Geosyntec Consultants, Printed 5/24/2019  
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# Proposed System AD

Prepared by Geosyntec Consultants

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## Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	A-BUBBLER	43.60	42.53	108.0	0.0099	0.013	34.0	0.0	0.0
2	A-L01	76.90	74.93	200.0	0.0098	0.013	24.0	0.0	0.0
3	A-L02	74.83	73.51	132.0	0.0100	0.013	24.0	0.0	0.0
4	A-L03	73.41	71.66	175.0	0.0100	0.013	24.0	0.0	0.0
5	A-L04	71.56	69.38	218.0	0.0100	0.013	24.0	0.0	0.0
6	A-L05	69.28	68.79	49.0	0.0100	0.013	24.0	0.0	0.0
7	A-L06	69.26	68.89	37.0	0.0100	0.013	24.0	0.0	0.0
8	A-L07	68.69	67.30	139.0	0.0100	0.013	24.0	0.0	0.0
9	A-L08	67.20	65.24	196.0	0.0100	0.013	24.0	0.0	0.0
10	A-L09	65.14	64.16	98.0	0.0100	0.013	36.0	0.0	0.0
11	A-L10	64.06	60.74	195.0	0.0170	0.013	36.0	0.0	0.0
12	A-L11	60.64	56.47	47.0	0.0887	0.013	36.0	0.0	0.0
13	A-L12	56.37	56.13	48.0	0.0050	0.013	36.0	0.0	0.0
14	A-L13	56.19	56.13	12.0	0.0050	0.013	36.0	0.0	0.0
15	A-L14	80.04	56.13	520.0	0.0460	0.013	36.0	0.0	0.0
16	A-L15	80.51	80.24	54.0	0.0050	0.013	24.0	0.0	0.0
17	A-L16	80.30	80.24	12.0	0.0050	0.013	24.0	0.0	0.0
18	A-L17	55.93	54.06	267.0	0.0070	0.013	42.0	0.0	0.0
19	A-L18	54.70	54.16	12.0	0.0450	0.013	24.0	0.0	0.0
20	A-L19	54.80	54.16	30.0	0.0213	0.013	24.0	0.0	0.0
21	A-L20	53.96	51.65	462.0	0.0050	0.013	42.0	0.0	0.0
22	A-L21	51.80	51.75	30.0	0.0017	0.013	24.0	0.0	0.0
23	A-L22	51.55	50.60	190.0	0.0050	0.013	42.0	0.0	0.0
24	A-L23	50.40	45.83	286.0	0.0160	0.013	42.0	0.0	0.0
25	A-L25	45.73	45.08	163.0	0.0040	0.013	42.0	0.0	0.0
26	A-L27	45.59	45.08	102.0	0.0050	0.013	24.0	0.0	0.0
27	A-L28	44.98	44.13	343.0	0.0025	0.013	42.0	0.0	0.0
28	A-L29	47.25	46.20	233.0	0.0045	0.013	18.0	0.0	0.0
29	A-L30	46.10	45.40	140.0	0.0050	0.013	24.0	0.0	0.0
30	A-L31	44.13	43.60	133.0	0.0040	0.013	42.0	0.0	0.0
31	A-L32	51.46	51.28	36.0	0.0050	0.013	18.0	0.0	0.0
32	A-L33	51.30	51.28	4.0	0.0050	0.013	18.0	0.0	0.0
33	A-L34	51.08	50.58	125.0	0.0040	0.013	18.0	0.0	0.0
34	A-L35	50.40	50.38	4.0	0.0050	0.013	30.0	0.0	0.0
35	A-L36	50.28	50.06	44.0	0.0050	0.013	30.0	0.0	0.0
36	A-L37	49.96	46.50	692.0	0.0050	0.013	30.0	0.0	0.0
37	A-L38	46.40	46.36	8.0	0.0050	0.013	36.0	0.0	0.0
38	A-L39	46.06	44.40	415.0	0.0040	0.013	36.0	0.0	0.0
39	A-L40	44.30	43.60	175.0	0.0040	0.013	42.0	0.0	0.0
40	A-L41	50.35	48.70	330.0	0.0050	0.013	24.0	0.0	0.0
41	A-L42	48.60	46.50	420.0	0.0050	0.013	30.0	0.0	0.0
42	A-L43	49.77	48.32	290.0	0.0050	0.013	24.0	0.0	0.0
43	A-L44	48.22	46.50	344.0	0.0050	0.013	30.0	0.0	0.0



## Proposed System AD

Prepared by Geosyntec Consultants

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### Pipe Listing (all nodes) (continued)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
44	A-L45	48.55	47.10	181.0	0.0080	0.013	18.0	0.0	0.0
45	A-L46	47.00	44.40	289.0	0.0090	0.013	24.0	0.0	0.0
46	A-L47	45.20	44.40	114.0	0.0070	0.013	24.0	0.0	0.0
47	A-L48	77.95	76.96	198.0	0.0050	0.013	24.0	0.0	0.0
48	A-L49	76.86	75.87	198.0	0.0050	0.013	24.0	0.0	0.0
49	A-L50	75.77	74.78	198.0	0.0050	0.013	24.0	0.0	0.0
50	A-L51	74.68	73.69	198.0	0.0050	0.013	24.0	0.0	0.0
51	A-L52	76.90	75.32	160.0	0.0099	0.013	24.0	0.0	0.0
52	A-L53	75.22	73.69	155.0	0.0099	0.013	24.0	0.0	0.0
53	A-L54	73.59	73.07	55.0	0.0095	0.013	24.0	0.0	0.0
54	A-L55	72.97	72.57	80.0	0.0050	0.013	24.0	0.0	0.0
55	A-L56	72.47	69.20	113.0	0.0289	0.013	24.0	0.0	0.0
56	A-L57	70.36	69.73	63.0	0.0100	0.013	24.0	0.0	0.0
57	A-L58	69.93	69.73	20.0	0.0100	0.013	24.0	0.0	0.0
58	A-L59	69.63	69.10	53.0	0.0100	0.013	30.0	0.0	0.0
59	A-L60	69.00	64.92	408.0	0.0100	0.013	30.0	0.0	0.0
60	A-L61	65.73	65.02	71.0	0.0100	0.013	24.0	0.0	0.0
61	A-L62	65.23	65.02	21.0	0.0100	0.013	24.0	0.0	0.0
62	A-L63	64.82	60.42	440.0	0.0100	0.013	36.0	0.0	0.0
63	A-L64	60.88	60.52	72.0	0.0050	0.013	24.0	0.0	0.0
64	A-L65	60.63	60.52	22.0	0.0050	0.013	24.0	0.0	0.0
65	A-L66	60.32	59.73	98.0	0.0060	0.013	36.0	0.0	0.0
66	A-L67	59.63	58.84	88.0	0.0090	0.013	36.0	0.0	0.0
67	A-L68	58.74	56.75	398.0	0.0050	0.013	48.0	0.0	0.0
68	A-L69	56.65	53.56	240.0	0.0129	0.013	48.0	0.0	0.0
69	A-L70	53.80	53.66	28.0	0.0050	0.013	24.0	0.0	0.0
70	A-L71	53.46	53.02	49.0	0.0090	0.013	48.0	0.0	0.0
71	A-L72	52.92	47.50	340.0	0.0159	0.013	48.0	0.0	0.0
72	A-L73	45.30	44.40	492.0	0.0018	0.013	48.0	0.0	0.0
73	D-L1	51.00	46.20	50.0	0.0960	0.013	18.0	0.0	0.0



50-year Rainfall Intensity

ELEV.	FACTOR
0-1500	1.00
1500-3000	1.25
3000-4000	1.42
4000-5000	1.60
5000-6000	1.7
DESERT	1.25

Figure A-1. Intensity-Duration-Frequency Design Chart

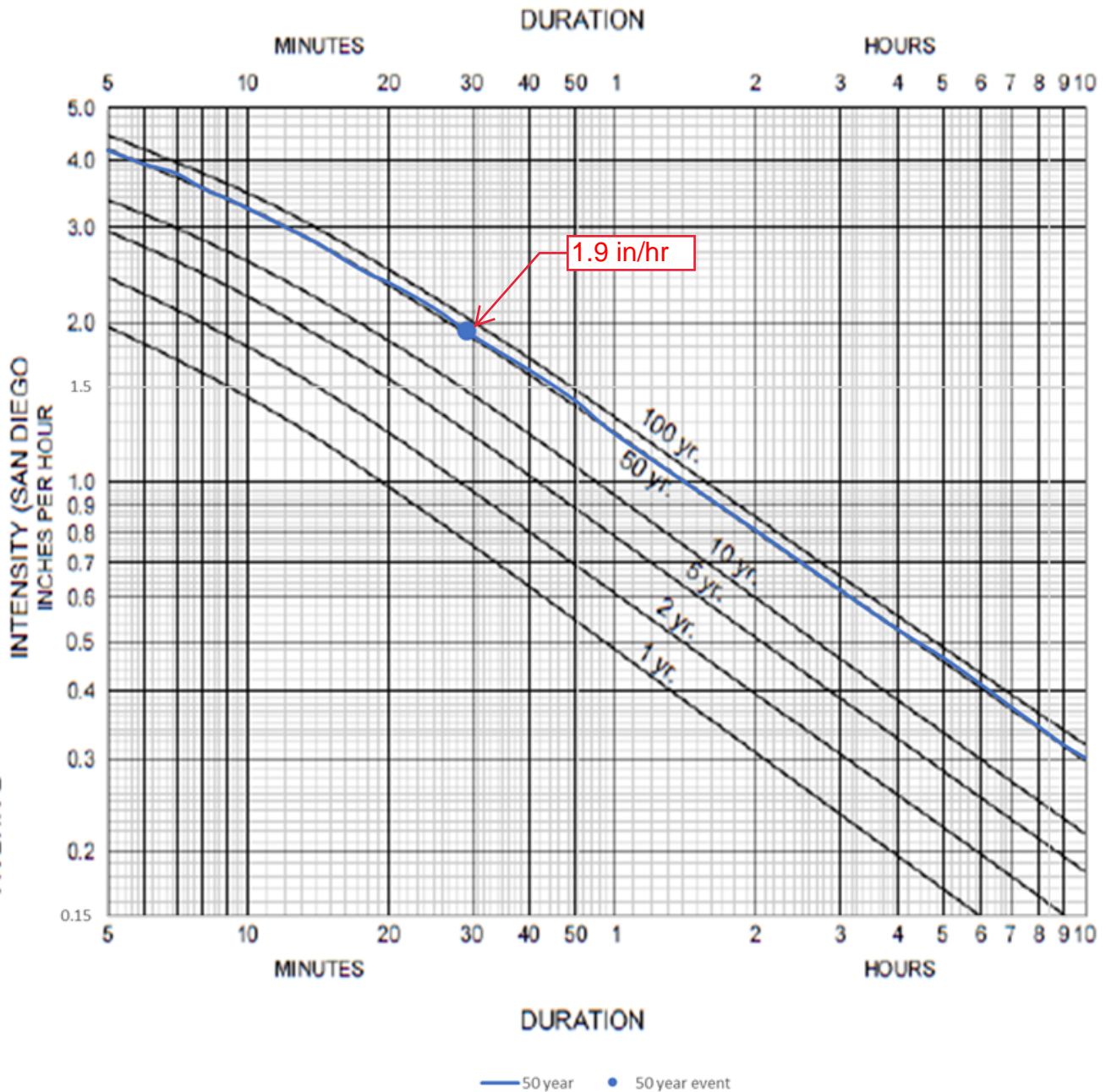
TO OBTAIN CORRECT INTENSITY, MULTIPLY INTENSITY ON CHART BY FACTOR FOR DESIGN ELEVATION.

COUNTY OF SAN DIEGO

FOR

INTENSITY - DURATION - FREQUENCY CURVES

RAINFALL





**Proposed System AD**

City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Prepared by Geosyntec Consultants

Printed 5/24/2019

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points x 3

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**Pond A-BUBBLER: A-BUBBLER** Peak Elev=50.28' Storage=1,890 cf Inflow=108.66 cfs 4.490 af  
Primary=69.62 cfs 3.501 af Secondary=38.81 cfs 0.991 af Outflow=108.42 cfs 4.492 af

**Pond A-L01: A-L01** Peak Elev=77.46' Inflow=1.86 cfs 0.077 af  
24.0" Round Culvert n=0.013 L=200.0' S=0.0098 '/ Outflow=1.86 cfs 0.077 af

**Pond A-L02: A-L02** Peak Elev=75.51' Inflow=2.66 cfs 0.110 af  
24.0" Round Culvert n=0.013 L=132.0' S=0.0100 '/ Outflow=2.66 cfs 0.110 af

**Pond A-L03: A-L03** Peak Elev=74.09' Inflow=2.66 cfs 0.110 af  
24.0" Round Culvert n=0.013 L=175.0' S=0.0100 '/ Outflow=2.66 cfs 0.110 af

**Pond A-L04: A-L04** Peak Elev=72.36' Inflow=3.27 cfs 0.135 af  
24.0" Round Culvert n=0.013 L=218.0' S=0.0100 '/ Outflow=3.27 cfs 0.135 af

**Pond A-L05: A-L05** Peak Elev=70.78' Inflow=3.77 cfs 0.156 af  
24.0" Round Culvert n=0.013 L=49.0' S=0.0100 '/ Outflow=3.77 cfs 0.156 af

**Pond A-L06: A-L06** Peak Elev=70.84' Inflow=5.02 cfs 0.207 af  
24.0" Round Culvert n=0.013 L=37.0' S=0.0100 '/ Outflow=5.02 cfs 0.207 af

**Pond A-L07: A-L07** Peak Elev=70.65' Inflow=14.40 cfs 0.595 af  
24.0" Round Culvert n=0.013 L=139.0' S=0.0100 '/ Outflow=14.40 cfs 0.595 af

**Pond A-L08: A-L08** Peak Elev=69.14' Inflow=14.77 cfs 0.610 af  
24.0" Round Culvert n=0.013 L=196.0' S=0.0100 '/ Outflow=14.77 cfs 0.610 af

**Pond A-L09: A-L09** Peak Elev=66.75' Inflow=15.07 cfs 0.623 af  
36.0" Round Culvert n=0.013 L=98.0' S=0.0100 '/ Outflow=15.07 cfs 0.623 af

**Pond A-L10: A-L10** Peak Elev=65.58' Inflow=15.07 cfs 0.623 af  
36.0" Round Culvert n=0.013 L=195.0' S=0.0170 '/ Outflow=15.07 cfs 0.623 af

**Pond A-L11: A-L11** Peak Elev=62.16' Inflow=15.07 cfs 0.623 af  
36.0" Round Culvert n=0.013 L=47.0' S=0.0887 '/ Outflow=15.07 cfs 0.623 af

**Pond A-L12: A-L12** Peak Elev=58.54' Inflow=18.41 cfs 0.761 af  
36.0" Round Culvert n=0.013 L=48.0' S=0.0050 '/ Outflow=18.41 cfs 0.761 af

**Pond A-L13: A-L13** Peak Elev=57.92' Inflow=1.30 cfs 0.054 af  
36.0" Round Culvert n=0.013 L=12.0' S=0.0050 '/ Outflow=1.30 cfs 0.054 af

**Pond A-L14: A-L14** Peak Elev=80.81' Inflow=4.24 cfs 0.175 af  
36.0" Round Culvert n=0.013 L=520.0' S=0.0460 '/ Outflow=4.24 cfs 0.175 af

**Pond A-L15: A-L15** Peak Elev=81.03' Inflow=0.91 cfs 0.038 af  
24.0" Round Culvert n=0.013 L=54.0' S=0.0050 '/ Outflow=0.91 cfs 0.038 af

**Pond A-L16: A-L16** Peak Elev=81.25' Inflow=3.33 cfs 0.138 af  
24.0" Round Culvert n=0.013 L=12.0' S=0.0050 '/ Outflow=3.33 cfs 0.138 af



**Proposed System AD***City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr*

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<b>Pond A-L17: A-L17</b>	Peak Elev=57.92' Inflow=23.96 cfs 0.990 af 42.0" Round Culvert n=0.013 L=267.0' S=0.0070 '/ Outflow=23.96 cfs 0.990 af
<b>Pond A-L18: A-L18</b>	Peak Elev=55.98' Inflow=0.67 cfs 0.028 af 24.0" Round Culvert n=0.013 L=12.0' S=0.0450 '/ Outflow=0.67 cfs 0.028 af
<b>Pond A-L19: A-L19</b>	Peak Elev=55.98' Inflow=0.38 cfs 0.016 af 24.0" Round Culvert n=0.013 L=30.0' S=0.0213 '/ Outflow=0.38 cfs 0.016 af
<b>Pond A-L20: A-L20</b>	Peak Elev=55.98' Inflow=25.00 cfs 1.033 af 42.0" Round Culvert n=0.013 L=462.0' S=0.0050 '/ Outflow=25.00 cfs 1.033 af
<b>Pond A-L21: A-L21</b>	Peak Elev=53.08' Inflow=0.61 cfs 0.025 af 24.0" Round Culvert n=0.013 L=30.0' S=0.0017 '/ Outflow=0.61 cfs 0.025 af
<b>Pond A-L22: A-L22</b>	Peak Elev=53.07' Inflow=25.61 cfs 1.058 af 42.0" Round Culvert x 2.00 n=0.013 L=190.0' S=0.0050 '/ Outflow=25.61 cfs 1.058 af
<b>Pond A-L23: A-L23</b>	Peak Elev=51.97' Inflow=25.61 cfs 1.058 af 42.0" Round Culvert x 2.00 n=0.013 L=286.0' S=0.0160 '/ Outflow=25.61 cfs 1.058 af
<b>Pond A-L25: A-L25</b>	Peak Elev=50.71' Inflow=31.87 cfs 1.317 af 42.0" Round Culvert x 2.00 n=0.013 L=163.0' S=0.0040 '/ Outflow=31.87 cfs 1.317 af
<b>Pond A-L27: A-L27</b>	Peak Elev=50.59' Inflow=0.47 cfs 0.020 af 24.0" Round Culvert n=0.013 L=102.0' S=0.0050 '/ Outflow=0.47 cfs 0.020 af
<b>Pond A-L28: A-L28</b>	Peak Elev=50.59' Inflow=32.34 cfs 1.336 af 42.0" Round Culvert x 2.00 n=0.013 L=343.0' S=0.0025 '/ Outflow=32.34 cfs 1.336 af
<b>Pond A-L29: A-L29</b>	Peak Elev=50.61' Inflow=1.19 cfs 0.049 af 18.0" Round Culvert n=0.013 L=233.0' S=0.0045 '/ Outflow=1.19 cfs 0.049 af
<b>Pond A-L30: A-L30</b>	Peak Elev=50.57' Inflow=5.14 cfs 0.212 af 24.0" Round Culvert n=0.013 L=140.0' S=0.0050 '/ Outflow=5.14 cfs 0.212 af
<b>Pond A-L31: A-L31</b>	Peak Elev=50.44' Inflow=37.47 cfs 1.549 af 42.0" Round Culvert x 2.00 n=0.013 L=133.0' S=0.0040 '/ Outflow=37.47 cfs 1.549 af
<b>Pond A-L32: A-L32</b>	Peak Elev=54.47' Inflow=0.45 cfs 0.019 af 18.0" Round Culvert n=0.013 L=36.0' S=0.0050 '/ Outflow=0.45 cfs 0.019 af
<b>Pond A-L33: A-L33</b>	Peak Elev=54.48' Inflow=0.75 cfs 0.031 af 18.0" Round Culvert n=0.013 L=4.0' S=0.0050 '/ Outflow=0.75 cfs 0.031 af
<b>Pond A-L34: A-L34</b>	Peak Elev=54.47' Inflow=1.20 cfs 0.050 af 18.0" Round Culvert n=0.013 L=125.0' S=0.0040 '/ Outflow=1.20 cfs 0.050 af
<b>Pond A-L35: A-L35</b>	Peak Elev=54.61' Inflow=9.56 cfs 0.395 af 30.0" Round Culvert n=0.013 L=4.0' S=0.0050 '/ Outflow=9.56 cfs 0.395 af
<b>Pond A-L36: A-L36</b>	Peak Elev=54.45' Inflow=11.65 cfs 0.481 af 30.0" Round Culvert n=0.013 L=44.0' S=0.0050 '/ Outflow=11.65 cfs 0.481 af



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<b>Pond A-L37: A-L37</b>	Peak Elev=54.20' Inflow=12.23 cfs 0.505 af 30.0" Round Culvert n=0.013 L=692.0' S=0.0050 '/ Outflow=12.23 cfs 0.505 af
<b>Pond A-L38: A-L38</b>	Peak Elev=53.45' Inflow=33.75 cfs 1.395 af 36.0" Round Culvert n=0.013 L=8.0' S=0.0050 '/ Outflow=33.75 cfs 1.395 af
<b>Pond A-L39: A-L39</b>	Peak Elev=52.46' Inflow=33.75 cfs 1.395 af 36.0" Round Culvert n=0.013 L=415.0' S=0.0040 '/ Outflow=33.75 cfs 1.395 af
<b>Pond A-L40: A-L40</b>	Peak Elev=50.87' Inflow=71.19 cfs 2.942 af 42.0" Round Culvert x 2.00 n=0.013 L=175.0' S=0.0040 '/ Outflow=71.19 cfs 2.942 af
<b>Pond A-L41: A-L41</b>	Peak Elev=54.03' Inflow=4.68 cfs 0.193 af 24.0" Round Culvert n=0.013 L=330.0' S=0.0050 '/ Outflow=4.68 cfs 0.193 af
<b>Pond A-L42: A-L42</b>	Peak Elev=53.83' Inflow=10.60 cfs 0.438 af 30.0" Round Culvert n=0.013 L=420.0' S=0.0050 '/ Outflow=10.60 cfs 0.438 af
<b>Pond A-L43: A-L43</b>	Peak Elev=53.98' Inflow=4.68 cfs 0.193 af 24.0" Round Culvert n=0.013 L=290.0' S=0.0050 '/ Outflow=4.68 cfs 0.193 af
<b>Pond A-L44: A-L44</b>	Peak Elev=53.80' Inflow=10.92 cfs 0.451 af 30.0" Round Culvert n=0.013 L=344.0' S=0.0050 '/ Outflow=10.92 cfs 0.451 af
<b>Pond A-L45: A-L45</b>	Peak Elev=51.09' Inflow=0.57 cfs 0.023 af 18.0" Round Culvert n=0.013 L=181.0' S=0.0080 '/ Outflow=0.57 cfs 0.023 af
<b>Pond A-L46: A-L46</b>	Peak Elev=51.08' Inflow=5.22 cfs 0.216 af 24.0" Round Culvert n=0.013 L=289.0' S=0.0090 '/ Outflow=5.22 cfs 0.216 af
<b>Pond A-L47: A-L47</b>	Peak Elev=51.09' Inflow=6.82 cfs 0.282 af 24.0" Round Culvert n=0.013 L=114.0' S=0.0070 '/ Outflow=6.82 cfs 0.282 af
<b>Pond A-L48: A-L48</b>	Peak Elev=78.24' Inflow=0.34 cfs 0.014 af 24.0" Round Culvert n=0.013 L=198.0' S=0.0050 '/ Outflow=0.34 cfs 0.014 af
<b>Pond A-L49: A-L49</b>	Peak Elev=77.41' Inflow=0.57 cfs 0.023 af 24.0" Round Culvert n=0.013 L=198.0' S=0.0050 '/ Outflow=0.57 cfs 0.023 af
<b>Pond A-L50: A-L50</b>	Peak Elev=77.22' Inflow=7.07 cfs 0.292 af 24.0" Round Culvert n=0.013 L=198.0' S=0.0050 '/ Outflow=7.07 cfs 0.292 af
<b>Pond A-L51: A-L51</b>	Peak Elev=76.26' Inflow=7.26 cfs 0.300 af 24.0" Round Culvert n=0.013 L=198.0' S=0.0050 '/ Outflow=7.26 cfs 0.300 af
<b>Pond A-L52: A-L52</b>	Peak Elev=77.22' Inflow=0.58 cfs 0.024 af 24.0" Round Culvert n=0.013 L=160.0' S=0.0099 '/ Outflow=0.58 cfs 0.024 af
<b>Pond A-L53: A-L53</b>	Peak Elev=75.96' Inflow=1.83 cfs 0.076 af 24.0" Round Culvert n=0.013 L=155.0' S=0.0099 '/ Outflow=1.83 cfs 0.076 af
<b>Pond A-L54: A-L54</b>	Peak Elev=75.46' Inflow=10.44 cfs 0.432 af 24.0" Round Culvert n=0.013 L=55.0' S=0.0095 '/ Outflow=10.44 cfs 0.432 af



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<b>Pond A-L55: A-L55</b>	Peak Elev=74.84' Inflow=10.44 cfs 0.432 af 24.0" Round Culvert n=0.013 L=80.0' S=0.0050 '/' Outflow=10.44 cfs 0.432 af
<b>Pond A-L56: A-L56</b>	Peak Elev=74.13' Inflow=12.22 cfs 0.505 af 24.0" Round Culvert n=0.013 L=113.0' S=0.0289 '/' Outflow=12.22 cfs 0.505 af
<b>Pond A-L57: A-L57</b>	Peak Elev=71.08' Inflow=1.81 cfs 0.075 af 24.0" Round Culvert n=0.013 L=63.0' S=0.0100 '/' Outflow=1.81 cfs 0.075 af
<b>Pond A-L58: A-L58</b>	Peak Elev=70.79' Inflow=0.89 cfs 0.037 af 24.0" Round Culvert n=0.013 L=20.0' S=0.0100 '/' Outflow=0.89 cfs 0.037 af
<b>Pond A-L59: A-L59</b>	Peak Elev=70.76' Inflow=2.70 cfs 0.112 af 30.0" Round Culvert n=0.013 L=53.0' S=0.0100 '/' Outflow=2.70 cfs 0.112 af
<b>Pond A-L60: A-L60</b>	Peak Elev=70.64' Inflow=14.92 cfs 0.617 af 30.0" Round Culvert n=0.013 L=408.0' S=0.0100 '/' Outflow=14.92 cfs 0.617 af
<b>Pond A-L61: A-L61</b>	Peak Elev=66.77' Inflow=2.68 cfs 0.111 af 24.0" Round Culvert n=0.013 L=71.0' S=0.0100 '/' Outflow=2.68 cfs 0.111 af
<b>Pond A-L62: A-L62</b>	Peak Elev=66.56' Inflow=1.18 cfs 0.049 af 24.0" Round Culvert n=0.013 L=21.0' S=0.0100 '/' Outflow=1.18 cfs 0.049 af
<b>Pond A-L63: A-L63</b>	Peak Elev=66.54' Inflow=18.78 cfs 0.776 af 36.0" Round Culvert n=0.013 L=440.0' S=0.0100 '/' Outflow=18.78 cfs 0.776 af
<b>Pond A-L64: A-L64</b>	Peak Elev=62.77' Inflow=3.27 cfs 0.135 af 24.0" Round Culvert n=0.013 L=72.0' S=0.0050 '/' Outflow=3.27 cfs 0.135 af
<b>Pond A-L65: A-L65</b>	Peak Elev=62.71' Inflow=1.18 cfs 0.049 af 24.0" Round Culvert n=0.013 L=22.0' S=0.0050 '/' Outflow=1.18 cfs 0.049 af
<b>Pond A-L66: A-L66</b>	Peak Elev=62.71' Inflow=23.23 cfs 0.960 af 36.0" Round Culvert n=0.013 L=98.0' S=0.0060 '/' Outflow=23.23 cfs 0.960 af
<b>Pond A-L67: A-L67</b>	Peak Elev=61.79' Inflow=23.23 cfs 0.960 af 36.0" Round Culvert n=0.013 L=88.0' S=0.0090 '/' Outflow=23.23 cfs 0.960 af
<b>Pond A-L68: A-L68</b>	Peak Elev=60.63' Inflow=23.23 cfs 0.960 af 48.0" Round Culvert n=0.013 L=398.0' S=0.0050 '/' Outflow=23.23 cfs 0.960 af
<b>Pond A-L69: A-L69</b>	Peak Elev=58.38' Inflow=23.23 cfs 0.960 af 48.0" Round Culvert n=0.013 L=240.0' S=0.0129 '/' Outflow=23.23 cfs 0.960 af
<b>Pond A-L70: A-L70</b>	Peak Elev=55.60' Inflow=2.17 cfs 0.090 af 24.0" Round Culvert n=0.013 L=28.0' S=0.0050 '/' Outflow=2.17 cfs 0.090 af
<b>Pond A-L71: A-L71</b>	Peak Elev=55.57' Inflow=25.40 cfs 1.050 af 48.0" Round Culvert n=0.013 L=49.0' S=0.0090 '/' Outflow=25.40 cfs 1.050 af
<b>Pond A-L72: A-L72</b>	Peak Elev=54.73' Inflow=25.40 cfs 1.050 af 48.0" Round Culvert n=0.013 L=340.0' S=0.0159 '/' Outflow=25.40 cfs 1.050 af



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**Pond A-L73: A-L73**Peak Elev=51.12' Inflow=25.40 cfs 1.050 af  
48.0" Round Culvert n=0.013 L=492.0' S=0.0018 '/' Outflow=25.40 cfs 1.050 af**Subcatchment A01: A01**Runoff Area=1.430 ac 0.00% Impervious Runoff Depth=0.65"  
Tc=7.0 min C=0.69 Runoff=1.86 cfs 0.077 af**Subcatchment A02: A02**Runoff Area=0.590 ac 0.00% Impervious Runoff Depth=0.67"  
Tc=7.0 min C=0.72 Runoff=0.80 cfs 0.033 af**Subcatchment A03: A03**Runoff Area=0.340 ac 100.00% Impervious Runoff Depth=0.89"  
Tc=5.0 min C=0.95 Runoff=0.61 cfs 0.025 af**Subcatchment A04: A04**Runoff Area=0.280 ac 100.00% Impervious Runoff Depth=0.89"  
Tc=5.0 min C=0.95 Runoff=0.50 cfs 0.021 af**Subcatchment A05: A05**Runoff Area=4.840 ac 0.00% Impervious Runoff Depth=0.51"  
Tc=23.0 min C=0.55 Runoff=5.02 cfs 0.207 af**Subcatchment A06: A06**Runoff Area=0.260 ac 0.00% Impervious Runoff Depth=0.70"  
Tc=6.0 min C=0.75 Runoff=0.37 cfs 0.015 af**Subcatchment A07: A07**Runoff Area=0.250 ac 0.00% Impervious Runoff Depth=0.60"  
Tc=8.0 min C=0.64 Runoff=0.30 cfs 0.012 af**Subcatchment A08: A08**Runoff Area=3.050 ac 0.00% Impervious Runoff Depth=0.54"  
Tc=20.0 min C=0.58 Runoff=3.34 cfs 0.138 af**Subcatchment A09: A09**Runoff Area=1.030 ac 0.00% Impervious Runoff Depth=0.63"  
Tc=8.0 min C=0.67 Runoff=1.30 cfs 0.054 af**Subcatchment A10: A10**Runoff Area=0.550 ac 0.00% Impervious Runoff Depth=0.82"  
Tc=5.0 min C=0.88 Runoff=0.91 cfs 0.038 af**Subcatchment A11: A11**Runoff Area=1.880 ac 0.00% Impervious Runoff Depth=0.88"  
Tc=5.0 min C=0.94 Runoff=3.33 cfs 0.138 af**Subcatchment A12: A12**Runoff Area=0.540 ac 0.00% Impervious Runoff Depth=0.62"  
Tc=5.0 min C=0.66 Runoff=0.67 cfs 0.028 af**Subcatchment A13: A13**Runoff Area=0.240 ac 0.00% Impervious Runoff Depth=0.78"  
Tc=5.0 min C=0.83 Runoff=0.38 cfs 0.016 af**Subcatchment A14: A14**Runoff Area=0.420 ac 0.00% Impervious Runoff Depth=0.72"  
Tc=6.0 min C=0.77 Runoff=0.61 cfs 0.025 af**Subcatchment A16/17: A16/17**Runoff Area=0.270 ac 0.00% Impervious Runoff Depth=0.87"  
Tc=5.0 min C=0.93 Runoff=0.47 cfs 0.020 af**Subcatchment A18: A18**Runoff Area=3.950 ac 0.00% Impervious Runoff Depth=0.50"  
Tc=18.0 min C=0.53 Runoff=3.95 cfs 0.163 af**Subcatchment A19: A19**Runoff Area=0.370 ac 0.00% Impervious Runoff Depth=0.61"  
Tc=8.0 min C=0.65 Runoff=0.45 cfs 0.019 af



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<b>Subcatchment A20: A20</b>	Runoff Area=0.650 ac 0.00% Impervious Runoff Depth=0.57" Tc=9.0 min C=0.61 Runoff=0.75 cfs 0.031 af
<b>Subcatchment A21: A21</b>	Runoff Area=0.540 ac 0.00% Impervious Runoff Depth=0.81" Tc=5.0 min C=0.87 Runoff=0.89 cfs 0.037 af
<b>Subcatchment A22: A22</b>	Runoff Area=0.440 ac 0.00% Impervious Runoff Depth=0.65" Tc=7.0 min C=0.70 Runoff=0.58 cfs 0.024 af
<b>Subcatchment A23: A23</b>	Runoff Area=3.940 ac 0.00% Impervious Runoff Depth=0.59" Tc=18.0 min C=0.63 Runoff=4.68 cfs 0.193 af
<b>Subcatchment A24: A24</b>	Runoff Area=5.320 ac 0.00% Impervious Runoff Depth=0.55" Tc=23.0 min C=0.59 Runoff=5.92 cfs 0.245 af
<b>Subcatchment A25: A25</b>	Runoff Area=3.820 ac 0.00% Impervious Runoff Depth=0.61" Tc=14.0 min C=0.65 Runoff=4.68 cfs 0.193 af
<b>Subcatchment A26: A26</b>	Runoff Area=5.610 ac 0.00% Impervious Runoff Depth=0.55" Tc=19.0 min C=0.59 Runoff=6.24 cfs 0.258 af
<b>Subcatchment A27: A27</b>	Runoff Area=4.570 ac 0.00% Impervious Runoff Depth=0.50" Tc=20.0 min C=0.54 Runoff=4.65 cfs 0.192 af
<b>Subcatchment A28: A28</b>	Runoff Area=6.820 ac 0.00% Impervious Runoff Depth=0.50" Tc=30.0 min C=0.53 Runoff=6.82 cfs 0.282 af
<b>Subcatchment A29: A29</b>	Runoff Area=0.280 ac 0.00% Impervious Runoff Depth=0.60" Tc=5.0 min C=0.64 Runoff=0.34 cfs 0.014 af
<b>Subcatchment A30: A30</b>	Runoff Area=0.190 ac 0.00% Impervious Runoff Depth=0.60" Tc=5.0 min C=0.64 Runoff=0.23 cfs 0.009 af
<b>Subcatchment A31: A31</b>	Runoff Area=0.180 ac 0.00% Impervious Runoff Depth=0.60" Tc=5.0 min C=0.64 Runoff=0.22 cfs 0.009 af
<b>Subcatchment A32: A32</b>	Runoff Area=0.160 ac 0.00% Impervious Runoff Depth=0.60" Tc=5.0 min C=0.64 Runoff=0.19 cfs 0.008 af
<b>Subcatchment A33: A33</b>	Runoff Area=0.420 ac 0.00% Impervious Runoff Depth=0.68" Tc=7.0 min C=0.73 Runoff=0.58 cfs 0.024 af
<b>Subcatchment A34: A34</b>	Runoff Area=0.980 ac 0.00% Impervious Runoff Depth=0.64" Tc=8.0 min C=0.68 Runoff=1.26 cfs 0.052 af
<b>Subcatchment A35: A35</b>	Runoff Area=1.050 ac 0.00% Impervious Runoff Depth=0.64" Tc=8.0 min C=0.68 Runoff=1.35 cfs 0.056 af
<b>Subcatchment A36/37: A36/37</b>	Runoff Area=1.410 ac 0.00% Impervious Runoff Depth=0.63" Tc=8.0 min C=0.67 Runoff=1.78 cfs 0.074 af
<b>Subcatchment A38: A38</b>	Runoff Area=1.030 ac 0.00% Impervious Runoff Depth=0.87" Tc=5.0 min C=0.93 Runoff=1.81 cfs 0.075 af



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<b>Subcatchment A39: A39</b>	Runoff Area=0.520 ac 0.00% Impervious Runoff Depth=0.85" Tc=5.0 min C=0.91 Runoff=0.89 cfs 0.037 af
<b>Subcatchment A40: A40</b>	Runoff Area=1.870 ac 0.00% Impervious Runoff Depth=0.71" Tc=6.0 min C=0.76 Runoff=2.68 cfs 0.111 af
<b>Subcatchment A41: A41</b>	Runoff Area=0.720 ac 0.00% Impervious Runoff Depth=0.81" Tc=5.0 min C=0.87 Runoff=1.18 cfs 0.049 af
<b>Subcatchment A42: A42</b>	Runoff Area=2.340 ac 0.00% Impervious Runoff Depth=0.69" Tc=6.0 min C=0.74 Runoff=3.27 cfs 0.135 af
<b>Subcatchment A43: A43</b>	Runoff Area=0.720 ac 0.00% Impervious Runoff Depth=0.81" Tc=5.0 min C=0.87 Runoff=1.18 cfs 0.049 af
<b>Subcatchment A44: A44</b>	Runoff Area=1.370 ac 0.00% Impervious Runoff Depth=0.79" Tc=5.0 min C=0.84 Runoff=2.17 cfs 0.090 af
<b>Subcatchment A45: A45</b>	Runoff Area=4.580 ac 0.00% Impervious Runoff Depth=0.61" Tc=5.0 min C=0.65 Runoff=5.61 cfs 0.232 af
<b>Subcatchment A46: A46</b>	Runoff Area=5.050 ac 0.00% Impervious Runoff Depth=0.62" Tc=5.0 min C=0.66 Runoff=6.28 cfs 0.260 af
<b>Subcatchment A47: A47</b>	Runoff Area=5.510 ac 0.00% Impervious Runoff Depth=0.86" Tc=5.0 min C=0.92 Runoff=9.56 cfs 0.395 af
<b>Subcatchment Basin-A1: Basin A1</b>	Runoff Area=5.620 ac 0.00% Impervious Runoff Depth=0.55" Tc=23.0 min C=0.59 Runoff=6.25 cfs 0.258 af
<b>Subcatchment Basin-A2: ABASIN2</b>	Runoff Area=0.750 ac 0.00% Impervious Runoff Depth=0.79" Tc=5.0 min C=0.84 Runoff=1.19 cfs 0.049 af
<b>Subcatchment Basin-A3: Basin A3</b>	Runoff Area=0.450 ac 0.00% Impervious Runoff Depth=0.63" Tc=8.0 min C=0.67 Runoff=0.57 cfs 0.023 af
<b>Pond D-L1: D-L1</b>	Peak Elev=51.79' Inflow=2.85 cfs 0.118 af 18.0" Round Culvert n=0.013 L=50.0' S=0.0960 '/' Outflow=2.85 cfs 0.118 af
<b>Subcatchment D1: D1</b>	Runoff Area=2.910 ac 0.00% Impervious Runoff Depth=0.49" Tc=20.0 min C=0.52 Runoff=2.85 cfs 0.118 af
<b>Pond OUTFALL A: OUTFALL A</b>	Inflow=69.62 cfs 3.501 af Primary=69.62 cfs 3.501 af
<b>Pond Outlet: BasinOutlet</b>	Inflow=41.66 cfs 1.109 af Primary=41.66 cfs 1.109 af

**Total Runoff Area = 90.110 ac Runoff Volume = 4.608 af Average Runoff Depth = 0.61"**  
**99.31% Pervious = 89.490 ac 0.69% Impervious = 0.620 ac**



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**Summary for Pond A-BUBBLER: A-BUBBLER**

Inflow Area = 87.200 ac, 0.71% Impervious, Inflow Depth = 0.62" for 50-Year event  
 Inflow = 108.66 cfs @ 0.50 hrs, Volume= 4.490 af  
 Outflow = 108.42 cfs @ 0.50 hrs, Volume= 4.492 af, Atten= 0%, Lag= 0.0 min  
 Primary = 69.62 cfs @ 0.50 hrs, Volume= 3.501 af  
 Secondary = 38.81 cfs @ 0.50 hrs, Volume= 0.991 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 50.28' @ 0.50 hrs Surf.Area= 13,634 sf Storage= 1,890 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 0.2 min ( 21.7 - 21.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	50.00'	24,687 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
50.00	100	0	0
51.00	49,273	24,687	24,687

Device	Routing	Invert	Outlet Devices
#1	Primary	43.60'	<b>34.0" Round Outfall Pipe</b> L= 108.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 43.60' / 42.53' S= 0.0099 '/ Cc= 0.900 n= 0.013, Flow Area= 6.31 sf
#2	Secondary	50.00'	<b>100.0' long x 500.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=69.62 cfs @ 0.50 hrs HW=50.28' TW=0.00' (Dynamic Tailwater)  
 ↖**1=Outfall Pipe** (Inlet Controls 69.62 cfs @ 11.04 fps)

**Secondary OutFlow** Max=38.81 cfs @ 0.50 hrs HW=50.28' TW=0.00' (Dynamic Tailwater)  
 ↖**2=Broad-Crested Rectangular Weir** (Weir Controls 38.81 cfs @ 1.41 fps)

**Summary for Pond A-L01: A-L01**

Inflow Area = 1.430 ac, 0.00% Impervious, Inflow Depth = 0.65" for 50-Year event  
 Inflow = 1.86 cfs @ 0.12 hrs, Volume= 0.077 af  
 Outflow = 1.86 cfs @ 0.13 hrs, Volume= 0.077 af, Atten= 0%, Lag= 0.6 min  
 Primary = 1.86 cfs @ 0.13 hrs, Volume= 0.077 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 77.46' @ 0.12 hrs  
 Flood Elev= 82.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	76.90'	<b>24.0" Round A-L1</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 76.90' / 74.93' S= 0.0098 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf



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**Primary OutFlow** Max=1.86 cfs @ 0.13 hrs HW=77.46' TW=75.51' (Dynamic Tailwater)↑**1=A-L1** (Inlet Controls 1.86 cfs @ 2.56 fps)**Summary for Pond A-L02: A-L02**

Inflow Area = 2.020 ac, 0.00% Impervious, Inflow Depth = 0.65" for 50-Year event  
 Inflow = 2.66 cfs @ 0.13 hrs, Volume= 0.110 af  
 Outflow = 2.66 cfs @ 0.13 hrs, Volume= 0.110 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.66 cfs @ 0.13 hrs, Volume= 0.110 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 75.51' @ 0.12 hrs

Flood Elev= 82.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	74.83'	<b>24.0" Round A-L2</b> L= 132.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 74.83' / 73.51' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.66 cfs @ 0.13 hrs HW=75.51' TW=74.09' (Dynamic Tailwater)↑**1=A-L2** (Inlet Controls 2.66 cfs @ 2.81 fps)**Summary for Pond A-L03: A-L03**

Inflow Area = 2.020 ac, 0.00% Impervious, Inflow Depth = 0.65" for 50-Year event  
 Inflow = 2.66 cfs @ 0.13 hrs, Volume= 0.110 af  
 Outflow = 2.66 cfs @ 0.13 hrs, Volume= 0.110 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.66 cfs @ 0.13 hrs, Volume= 0.110 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 74.09' @ 0.12 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	73.41'	<b>24.0" Round Pipe</b> L= 175.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 73.41' / 71.66' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.66 cfs @ 0.13 hrs HW=74.09' TW=72.33' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 2.66 cfs @ 2.81 fps)**Summary for Pond A-L04: A-L04**

Inflow Area = 2.360 ac, 14.41% Impervious, Inflow Depth = 0.69" for 50-Year event  
 Inflow = 3.27 cfs @ 0.13 hrs, Volume= 0.135 af  
 Outflow = 3.27 cfs @ 0.12 hrs, Volume= 0.135 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.27 cfs @ 0.12 hrs, Volume= 0.135 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 72.36' @ 0.40 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	71.56'	<b>24.0" Round A-L4</b> L= 218.0' RCP, sq.cut end projecting, Ke= 0.500



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Inlet / Outlet Invert= 71.56' / 69.38' S= 0.0100 '/ Cc= 0.900  
n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=3.27 cfs @ 0.12 hrs HW=72.33' TW=70.51' (Dynamic Tailwater)

↑1=A-L4 (Outlet Controls 3.27 cfs @ 4.35 fps)

**Summary for Pond A-L05: A-L05**

Inflow Area = 2.640 ac, 23.48% Impervious, Inflow Depth = 0.71" for 50-Year event  
Inflow = 3.77 cfs @ 0.12 hrs, Volume= 0.156 af  
Outflow = 3.77 cfs @ 0.33 hrs, Volume= 0.156 af, Atten= 0%, Lag= 12.6 min  
Primary = 3.77 cfs @ 0.33 hrs, Volume= 0.156 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 70.78' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	69.28'	<b>24.0" Round A-L5</b> L= 49.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 69.28' / 68.79' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=3.77 cfs @ 0.33 hrs HW=70.72' TW=70.56' (Dynamic Tailwater)

↑1=A-L5 (Outlet Controls 3.77 cfs @ 2.19 fps)

**Summary for Pond A-L06: A-L06**

Inflow Area = 4.840 ac, 0.00% Impervious, Inflow Depth = 0.51" for 50-Year event  
Inflow = 5.02 cfs @ 0.39 hrs, Volume= 0.207 af  
Outflow = 5.02 cfs @ 0.40 hrs, Volume= 0.207 af, Atten= 0%, Lag= 0.6 min  
Primary = 5.02 cfs @ 0.40 hrs, Volume= 0.207 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 70.84' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	69.26'	<b>24.0" Round A-L6</b> L= 37.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 69.26' / 68.89' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=5.02 cfs @ 0.40 hrs HW=70.84' TW=70.65' (Dynamic Tailwater)

↑1=A-L6 (Outlet Controls 5.02 cfs @ 2.58 fps)

**Summary for Pond A-L07: A-L07**

Inflow Area = 12.060 ac, 5.14% Impervious, Inflow Depth = 0.59" for 50-Year event  
Inflow = 14.40 cfs @ 0.39 hrs, Volume= 0.595 af  
Outflow = 14.40 cfs @ 0.39 hrs, Volume= 0.595 af, Atten= 0%, Lag= 0.0 min  
Primary = 14.40 cfs @ 0.39 hrs, Volume= 0.595 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 70.65' @ 0.39 hrs



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Device	Routing	Invert	Outlet Devices
#1	Primary	68.69'	<b>24.0" Round Pipe</b> L= 139.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 68.69' / 67.30' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=14.40 cfs @ 0.39 hrs HW=70.65' TW=69.14' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 14.40 cfs @ 5.83 fps)**Summary for Pond A-L08: A-L08**

Inflow Area = 12.320 ac, 5.03% Impervious, Inflow Depth = 0.59" for 50-Year event  
 Inflow = 14.77 cfs @ 0.39 hrs, Volume= 0.610 af  
 Outflow = 14.77 cfs @ 0.39 hrs, Volume= 0.610 af, Atten= 0%, Lag= 0.0 min  
 Primary = 14.77 cfs @ 0.39 hrs, Volume= 0.610 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 69.14' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	67.20'	<b>24.0" Round A-L8</b> L= 196.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 67.20' / 65.24' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=14.77 cfs @ 0.39 hrs HW=69.14' TW=66.75' (Dynamic Tailwater)↑**1=A-L8** (Inlet Controls 14.77 cfs @ 4.74 fps)**Summary for Pond A-L09: A-L09**

Inflow Area = 12.570 ac, 4.93% Impervious, Inflow Depth = 0.59" for 50-Year event  
 Inflow = 15.07 cfs @ 0.39 hrs, Volume= 0.623 af  
 Outflow = 15.07 cfs @ 0.39 hrs, Volume= 0.623 af, Atten= 0%, Lag= 0.0 min  
 Primary = 15.07 cfs @ 0.39 hrs, Volume= 0.623 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 66.75' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	65.14'	<b>36.0" Round A-L9</b> L= 98.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 65.14' / 64.16' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=15.07 cfs @ 0.39 hrs HW=66.75' TW=65.58' (Dynamic Tailwater)↑**1=A-L9** (Outlet Controls 15.07 cfs @ 5.65 fps)**Summary for Pond A-L10: A-L10**

Inflow Area = 12.570 ac, 4.93% Impervious, Inflow Depth = 0.59" for 50-Year event  
 Inflow = 15.07 cfs @ 0.39 hrs, Volume= 0.623 af  
 Outflow = 15.07 cfs @ 0.39 hrs, Volume= 0.623 af, Atten= 0%, Lag= 0.0 min  
 Primary = 15.07 cfs @ 0.39 hrs, Volume= 0.623 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3



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Peak Elev= 65.58' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	64.06'	<b>36.0" Round Pipe</b> L= 195.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 64.06' / 60.74' S= 0.0170 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=15.07 cfs @ 0.39 hrs HW=65.58' TW=62.16' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 15.07 cfs @ 4.20 fps)**Summary for Pond A-L11: A-L11**

Inflow Area = 12.570 ac, 4.93% Impervious, Inflow Depth = 0.59" for 50-Year event  
 Inflow = 15.07 cfs @ 0.39 hrs, Volume= 0.623 af  
 Outflow = 15.07 cfs @ 0.43 hrs, Volume= 0.623 af, Atten= 0%, Lag= 2.4 min  
 Primary = 15.07 cfs @ 0.43 hrs, Volume= 0.623 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 62.16' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	60.64'	<b>36.0" Round Pipe</b> L= 47.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 60.64' / 56.47' S= 0.0887 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=15.07 cfs @ 0.43 hrs HW=62.16' TW=58.54' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 15.07 cfs @ 4.20 fps)**Summary for Pond A-L12: A-L12**

Inflow Area = 15.620 ac, 3.97% Impervious, Inflow Depth = 0.58" for 50-Year event  
 Inflow = 18.41 cfs @ 0.43 hrs, Volume= 0.761 af  
 Outflow = 18.41 cfs @ 0.43 hrs, Volume= 0.761 af, Atten= 0%, Lag= 0.0 min  
 Primary = 18.41 cfs @ 0.43 hrs, Volume= 0.761 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 58.54' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.37'	<b>36.0" Round A-L12</b> L= 48.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.37' / 56.13' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=18.41 cfs @ 0.43 hrs HW=58.54' TW=57.92' (Dynamic Tailwater)↑**1=A-L12** (Outlet Controls 18.41 cfs @ 4.69 fps)**Summary for Pond A-L13: A-L13**

Inflow Area = 1.030 ac, 0.00% Impervious, Inflow Depth = 0.63" for 50-Year event  
 Inflow = 1.30 cfs @ 0.14 hrs, Volume= 0.054 af  
 Outflow = 1.30 cfs @ 0.33 hrs, Volume= 0.054 af, Atten= 0%, Lag= 11.4 min  
 Primary = 1.30 cfs @ 0.33 hrs, Volume= 0.054 af



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Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 57.92' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.19'	<b>36.0" Round A-L13</b> L= 12.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.19' / 56.13' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=1.30 cfs @ 0.33 hrs HW=57.89' TW=57.88' (Dynamic Tailwater)↑**1=A-L13** (Outlet Controls 1.30 cfs @ 0.46 fps)**Summary for Pond A-L14: A-L14**

Inflow Area = 2.430 ac, 0.00% Impervious, Inflow Depth = 0.87" for 50-Year event  
 Inflow = 4.24 cfs @ 0.09 hrs, Volume= 0.175 af  
 Outflow = 4.24 cfs @ 0.09 hrs, Volume= 0.175 af, Atten= 0%, Lag= 0.0 min  
 Primary = 4.24 cfs @ 0.09 hrs, Volume= 0.175 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 80.81' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	80.04'	<b>36.0" Round Pipe</b> L= 520.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 80.04' / 56.13' S= 0.0460 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=4.24 cfs @ 0.09 hrs HW=80.81' TW=57.52' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 4.24 cfs @ 2.98 fps)**Summary for Pond A-L15: A-L15**

Inflow Area = 0.550 ac, 0.00% Impervious, Inflow Depth = 0.82" for 50-Year event  
 Inflow = 0.91 cfs @ 0.09 hrs, Volume= 0.038 af  
 Outflow = 0.91 cfs @ 0.09 hrs, Volume= 0.038 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.91 cfs @ 0.09 hrs, Volume= 0.038 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 81.03' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	80.51'	<b>24.0" Round A-L15</b> L= 54.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 80.51' / 80.24' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.91 cfs @ 0.09 hrs HW=81.03' TW=80.81' (Dynamic Tailwater)↑**1=A-L15** (Outlet Controls 0.91 cfs @ 2.10 fps)



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**Summary for Pond A-L16: A-L16**

Inflow Area = 1.880 ac, 0.00% Impervious, Inflow Depth = 0.88" for 50-Year event  
 Inflow = 3.33 cfs @ 0.09 hrs, Volume= 0.138 af  
 Outflow = 3.33 cfs @ 0.09 hrs, Volume= 0.138 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.33 cfs @ 0.09 hrs, Volume= 0.138 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 81.25' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	80.30'	<b>24.0" Round A-L16</b> L= 12.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 80.30' / 80.24' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=3.33 cfs @ 0.09 hrs HW=81.25' TW=80.81' (Dynamic Tailwater)  
 ↑**1=A-L16** (Barrel Controls 3.33 cfs @ 3.34 fps)

**Summary for Pond A-L17: A-L17**

Inflow Area = 19.080 ac, 3.25% Impervious, Inflow Depth = 0.62" for 50-Year event  
 Inflow = 23.96 cfs @ 0.43 hrs, Volume= 0.990 af  
 Outflow = 23.96 cfs @ 0.43 hrs, Volume= 0.990 af, Atten= 0%, Lag= 0.0 min  
 Primary = 23.96 cfs @ 0.43 hrs, Volume= 0.990 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 57.92' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	55.93'	<b>42.0" Round Pipe</b> L= 267.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 55.93' / 54.06' S= 0.0070 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=23.96 cfs @ 0.43 hrs HW=57.92' TW=55.98' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 23.96 cfs @ 6.13 fps)

**Summary for Pond A-L18: A-L18**

Inflow Area = 0.540 ac, 0.00% Impervious, Inflow Depth = 0.62" for 50-Year event  
 Inflow = 0.67 cfs @ 0.09 hrs, Volume= 0.028 af  
 Outflow = 0.67 cfs @ 0.37 hrs, Volume= 0.028 af, Atten= 0%, Lag= 16.8 min  
 Primary = 0.67 cfs @ 0.37 hrs, Volume= 0.028 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 55.98' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	54.70'	<b>24.0" Round A-L18</b> L= 12.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 54.70' / 54.16' S= 0.0450 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.67 cfs @ 0.37 hrs HW=55.98' TW=55.97' (Dynamic Tailwater)  
 ↑**1=A-L18** (Outlet Controls 0.67 cfs @ 0.45 fps)



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**Summary for Pond A-L19: A-L19**

Inflow Area = 0.240 ac, 0.00% Impervious, Inflow Depth = 0.78" for 50-Year event  
 Inflow = 0.38 cfs @ 0.09 hrs, Volume= 0.016 af  
 Outflow = 0.38 cfs @ 0.30 hrs, Volume= 0.016 af, Atten= 0%, Lag= 12.6 min  
 Primary = 0.38 cfs @ 0.30 hrs, Volume= 0.016 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 55.98' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	54.80'	<b>24.0" Round A-L19</b> L= 30.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 54.80' / 54.16' S= 0.0213 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.38 cfs @ 0.30 hrs HW=55.91' TW=55.91' (Dynamic Tailwater)↑**1=A-L19** (Outlet Controls 0.38 cfs @ 0.30 fps)**Summary for Pond A-L20: A-L20**

Inflow Area = 19.860 ac, 3.12% Impervious, Inflow Depth = 0.62" for 50-Year event  
 Inflow = 25.00 cfs @ 0.43 hrs, Volume= 1.033 af  
 Outflow = 25.00 cfs @ 0.43 hrs, Volume= 1.033 af, Atten= 0%, Lag= 0.0 min  
 Primary = 25.00 cfs @ 0.43 hrs, Volume= 1.033 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 55.98' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	53.96'	<b>42.0" Round Pipe</b> L= 462.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 53.96' / 51.65' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=25.00 cfs @ 0.43 hrs HW=55.98' TW=53.07' (Dynamic Tailwater)↑**1=Pipe** (Barrel Controls 25.00 cfs @ 6.27 fps)**Summary for Pond A-L21: A-L21**

Inflow Area = 0.420 ac, 0.00% Impervious, Inflow Depth = 0.72" for 50-Year event  
 Inflow = 0.61 cfs @ 0.10 hrs, Volume= 0.025 af  
 Outflow = 0.61 cfs @ 0.11 hrs, Volume= 0.025 af, Atten= 0%, Lag= 0.6 min  
 Primary = 0.61 cfs @ 0.11 hrs, Volume= 0.025 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 53.08' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	51.80'	<b>24.0" Round A-L21</b> L= 30.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 51.80' / 51.75' S= 0.0017 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.57 cfs @ 0.11 hrs HW=52.83' TW=52.82' (Dynamic Tailwater)↑**1=A-L21** (Outlet Controls 0.57 cfs @ 0.51 fps)



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**Summary for Pond A-L22: A-L22**

Inflow Area = 20.280 ac, 3.06% Impervious, Inflow Depth = 0.63" for 50-Year event  
 Inflow = 25.61 cfs @ 0.43 hrs, Volume= 1.058 af  
 Outflow = 25.61 cfs @ 0.43 hrs, Volume= 1.058 af, Atten= 0%, Lag= 0.0 min  
 Primary = 25.61 cfs @ 0.43 hrs, Volume= 1.058 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 53.07' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	51.55'	<b>42.0" Round Pipe X 2.00</b> L= 190.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 51.55' / 50.60' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=25.61 cfs @ 0.43 hrs HW=53.07' TW=51.97' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 25.61 cfs @ 4.71 fps)**Summary for Pond A-L23: A-L23**

Inflow Area = 20.280 ac, 3.06% Impervious, Inflow Depth = 0.63" for 50-Year event  
 Inflow = 25.61 cfs @ 0.43 hrs, Volume= 1.058 af  
 Outflow = 25.61 cfs @ 0.47 hrs, Volume= 1.058 af, Atten= 0%, Lag= 2.4 min  
 Primary = 25.61 cfs @ 0.47 hrs, Volume= 1.058 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 51.97' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	50.40'	<b>42.0" Round Pipe X 2.00</b> L= 286.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 50.40' / 45.83' S= 0.0160 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=25.61 cfs @ 0.47 hrs HW=51.97' TW=50.71' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 25.61 cfs @ 4.51 fps)**Summary for Pond A-L25: A-L25**

Inflow Area = 25.900 ac, 2.39% Impervious, Inflow Depth = 0.61" for 50-Year event  
 Inflow = 31.87 cfs @ 0.47 hrs, Volume= 1.317 af  
 Outflow = 31.87 cfs @ 0.43 hrs, Volume= 1.317 af, Atten= 0%, Lag= 0.0 min  
 Primary = 31.87 cfs @ 0.43 hrs, Volume= 1.317 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 50.71' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	45.73'	<b>42.0" Round Pipe X 2.00</b> L= 163.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 45.73' / 45.08' S= 0.0040 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf



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**Primary OutFlow** Max=31.76 cfs @ 0.43 hrs HW=50.71' TW=50.59' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 31.76 cfs @ 1.65 fps)**Summary for Pond A-L27: A-L27**

Inflow Area = 0.270 ac, 0.00% Impervious, Inflow Depth = 0.87" for 50-Year event  
 Inflow = 0.47 cfs @ 0.09 hrs, Volume= 0.020 af  
 Outflow = 0.47 cfs @ 0.09 hrs, Volume= 0.020 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.47 cfs @ 0.09 hrs, Volume= 0.020 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 50.59' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	45.59'	<b>24.0" Round A-L27</b> L= 102.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 45.59' / 45.08' S= 0.0050 ' /' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.00 cfs @ 0.09 hrs HW=50.12' TW=50.12' (Dynamic Tailwater)↑**1=A-L27** ( Controls 0.00 cfs)**Summary for Pond A-L28: A-L28**

Inflow Area = 26.170 ac, 2.37% Impervious, Inflow Depth = 0.61" for 50-Year event  
 Inflow = 32.34 cfs @ 0.43 hrs, Volume= 1.336 af  
 Outflow = 32.34 cfs @ 0.41 hrs, Volume= 1.336 af, Atten= 0%, Lag= 0.0 min  
 Primary = 32.34 cfs @ 0.41 hrs, Volume= 1.336 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 50.59' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	44.98'	<b>42.0" Round Pipe X 2.00</b> L= 343.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 44.98' / 44.13' S= 0.0025 ' /' Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=32.34 cfs @ 0.41 hrs HW=50.59' TW=50.43' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 32.34 cfs @ 1.68 fps)**Summary for Pond A-L29: A-L29**

Inflow Area = 0.750 ac, 0.00% Impervious, Inflow Depth = 0.79" for 50-Year event  
 Inflow = 1.19 cfs @ 0.09 hrs, Volume= 0.049 af  
 Outflow = 1.19 cfs @ 0.47 hrs, Volume= 0.049 af, Atten= 0%, Lag= 22.8 min  
 Primary = 1.19 cfs @ 0.47 hrs, Volume= 0.049 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 50.61' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	47.25'	<b>18.0" Round A-L29</b> L= 233.0' RCP, sq.cut end projecting, Ke= 0.500



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Inlet / Outlet Invert= 47.25' / 46.20' S= 0.0045 '/' Cc= 0.900  
 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=1.18 cfs @ 0.47 hrs HW=50.61' TW=50.57' (Dynamic Tailwater)

↑**1=A-L29** (Outlet Controls 1.18 cfs @ 0.67 fps)

**Summary for Pond A-L30: A-L30**

Inflow Area = 4.700 ac, 0.00% Impervious, Inflow Depth = 0.54" for 50-Year event  
 Inflow = 5.14 cfs @ 0.30 hrs, Volume= 0.212 af  
 Outflow = 5.14 cfs @ 0.39 hrs, Volume= 0.212 af, Atten= 0%, Lag= 5.4 min  
 Primary = 5.14 cfs @ 0.39 hrs, Volume= 0.212 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 50.57' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	46.10'	<b>24.0" Round A-L30</b> L= 140.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 46.10' / 45.40' S= 0.0050 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=5.14 cfs @ 0.39 hrs HW=50.56' TW=50.43' (Dynamic Tailwater)

↑**1=A-L30** (Outlet Controls 5.14 cfs @ 1.63 fps)

**Summary for Pond A-L31: A-L31**

Inflow Area = 30.870 ac, 2.01% Impervious, Inflow Depth = 0.60" for 50-Year event  
 Inflow = 37.47 cfs @ 0.41 hrs, Volume= 1.549 af  
 Outflow = 37.47 cfs @ 0.41 hrs, Volume= 1.549 af, Atten= 0%, Lag= 0.0 min  
 Primary = 37.47 cfs @ 0.41 hrs, Volume= 1.549 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 50.44' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	44.13'	<b>42.0" Round Pipe X 2.00</b> L= 133.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 44.13' / 43.60' S= 0.0040 '/' Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=37.47 cfs @ 0.41 hrs HW=50.43' TW=50.27' (Dynamic Tailwater)

↑**1=Pipe** (Inlet Controls 37.47 cfs @ 1.95 fps)

**Summary for Pond A-L32: A-L32**

Inflow Area = 0.370 ac, 0.00% Impervious, Inflow Depth = 0.61" for 50-Year event  
 Inflow = 0.45 cfs @ 0.14 hrs, Volume= 0.019 af  
 Outflow = 0.45 cfs @ 0.42 hrs, Volume= 0.019 af, Atten= 0%, Lag= 16.8 min  
 Primary = 0.45 cfs @ 0.42 hrs, Volume= 0.019 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

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Peak Elev= 54.47' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	51.46'	<b>18.0" Round A-L32</b> L= 36.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 51.46' / 51.28' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=0.21 cfs @ 0.42 hrs HW=54.45' TW=54.45' (Dynamic Tailwater)↑**1=A-L32** (Inlet Controls 0.21 cfs @ 0.12 fps)**Summary for Pond A-L33: A-L33**

Inflow Area = 0.650 ac, 0.00% Impervious, Inflow Depth = 0.57" for 50-Year event  
 Inflow = 0.75 cfs @ 0.15 hrs, Volume= 0.031 af  
 Outflow = 0.75 cfs @ 0.25 hrs, Volume= 0.031 af, Atten= 0%, Lag= 6.0 min  
 Primary = 0.75 cfs @ 0.25 hrs, Volume= 0.031 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 54.48' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	51.30'	<b>18.0" Round A-L33</b> L= 4.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 51.30' / 51.28' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=0.00 cfs @ 0.25 hrs HW=53.51' TW=53.52' (Dynamic Tailwater)↑**1=A-L33** ( Controls 0.00 cfs)**Summary for Pond A-L34: A-L34**

Inflow Area = 1.020 ac, 0.00% Impervious, Inflow Depth = 0.58" for 50-Year event  
 Inflow = 1.20 cfs @ 0.23 hrs, Volume= 0.050 af  
 Outflow = 1.20 cfs @ 0.31 hrs, Volume= 0.050 af, Atten= 0%, Lag= 4.8 min  
 Primary = 1.20 cfs @ 0.31 hrs, Volume= 0.050 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 54.47' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	51.08'	<b>18.0" Round Pipe</b> L= 125.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 51.08' / 50.58' S= 0.0040 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=0.24 cfs @ 0.31 hrs HW=54.11' TW=54.11' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 0.24 cfs @ 0.14 fps)**Summary for Pond A-L35: A-L35**

Inflow Area = 5.510 ac, 0.00% Impervious, Inflow Depth = 0.86" for 50-Year event  
 Inflow = 9.56 cfs @ 0.09 hrs, Volume= 0.395 af  
 Outflow = 9.56 cfs @ 0.42 hrs, Volume= 0.395 af, Atten= 0%, Lag= 19.8 min  
 Primary = 9.56 cfs @ 0.42 hrs, Volume= 0.395 af



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Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 54.61' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	50.40'	<b>30.0" Round A-L35</b> L= 4.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 50.40' / 50.38' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=9.52 cfs @ 0.42 hrs HW=54.59' TW=54.42' (Dynamic Tailwater)↑**1=A-L35** (Inlet Controls 9.52 cfs @ 1.94 fps)**Summary for Pond A-L36: A-L36**

Inflow Area = 7.070 ac, 0.00% Impervious, Inflow Depth = 0.82" for 50-Year event  
 Inflow = 11.65 cfs @ 0.25 hrs, Volume= 0.481 af  
 Outflow = 11.65 cfs @ 0.46 hrs, Volume= 0.481 af, Atten= 0%, Lag= 12.6 min  
 Primary = 11.65 cfs @ 0.46 hrs, Volume= 0.481 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 54.45' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	50.28'	<b>30.0" Round Pipe</b> L= 44.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 50.28' / 50.06' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=11.65 cfs @ 0.46 hrs HW=54.44' TW=54.19' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 11.65 cfs @ 2.37 fps)**Summary for Pond A-L37: A-L37**

Inflow Area = 7.510 ac, 0.00% Impervious, Inflow Depth = 0.81" for 50-Year event  
 Inflow = 12.23 cfs @ 0.46 hrs, Volume= 0.505 af  
 Outflow = 12.23 cfs @ 0.46 hrs, Volume= 0.505 af, Atten= 0%, Lag= 0.0 min  
 Primary = 12.23 cfs @ 0.46 hrs, Volume= 0.505 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 54.20' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	49.96'	<b>30.0" Round A-L37</b> L= 692.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 49.96' / 46.50' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=12.21 cfs @ 0.46 hrs HW=54.19' TW=53.43' (Dynamic Tailwater)↑**1=A-L37** (Outlet Controls 12.21 cfs @ 2.49 fps)

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**Summary for Pond A-L38: A-L38**

Inflow Area = 26.200 ac, 0.00% Impervious, Inflow Depth = 0.64" for 50-Year event  
 Inflow = 33.75 cfs @ 0.39 hrs, Volume= 1.395 af  
 Outflow = 33.75 cfs @ 0.39 hrs, Volume= 1.395 af, Atten= 0%, Lag= 0.0 min  
 Primary = 33.75 cfs @ 0.39 hrs, Volume= 1.395 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 53.45' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	46.40'	<b>36.0" Round Pipe</b> L= 8.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 46.40' / 46.36' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=33.71 cfs @ 0.39 hrs HW=53.41' TW=52.43' (Dynamic Tailwater)  
 ↑**1=Pipe** (Inlet Controls 33.71 cfs @ 4.77 fps)

**Summary for Pond A-L39: A-L39**

Inflow Area = 26.200 ac, 0.00% Impervious, Inflow Depth = 0.64" for 50-Year event  
 Inflow = 33.75 cfs @ 0.39 hrs, Volume= 1.395 af  
 Outflow = 33.75 cfs @ 0.43 hrs, Volume= 1.395 af, Atten= 0%, Lag= 2.4 min  
 Primary = 33.75 cfs @ 0.43 hrs, Volume= 1.395 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 52.46' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	46.06'	<b>36.0" Round Pipe</b> L= 415.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 46.06' / 44.40' S= 0.0040 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=33.75 cfs @ 0.43 hrs HW=52.44' TW=50.85' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 33.75 cfs @ 4.77 fps)

**Summary for Pond A-L40: A-L40**

Inflow Area = 56.330 ac, 0.00% Impervious, Inflow Depth = 0.63" for 50-Year event  
 Inflow = 71.19 cfs @ 0.50 hrs, Volume= 2.942 af  
 Outflow = 71.19 cfs @ 0.50 hrs, Volume= 2.942 af, Atten= 0%, Lag= 0.0 min  
 Primary = 71.19 cfs @ 0.50 hrs, Volume= 2.942 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 50.87' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	44.30'	<b>42.0" Round Pipe X 2.00</b> L= 175.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 44.30' / 43.60' S= 0.0040 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf



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**Primary OutFlow** Max=71.19 cfs @ 0.50 hrs HW=50.87' TW=50.28' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 71.19 cfs @ 3.70 fps)**Summary for Pond A-L41: A-L41**

Inflow Area = 3.940 ac, 0.00% Impervious, Inflow Depth = 0.59" for 50-Year event  
 Inflow = 4.68 cfs @ 0.30 hrs, Volume= 0.193 af  
 Outflow = 4.68 cfs @ 0.31 hrs, Volume= 0.193 af, Atten= 0%, Lag= 0.6 min  
 Primary = 4.68 cfs @ 0.31 hrs, Volume= 0.193 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 54.03' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	50.35'	<b>24.0" Round A-L41</b> L= 330.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 50.35' / 48.70' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=4.28 cfs @ 0.31 hrs HW=53.61' TW=53.45' (Dynamic Tailwater)↑**1=A-L41** (Outlet Controls 4.28 cfs @ 1.36 fps)**Summary for Pond A-L42: A-L42**

Inflow Area = 9.260 ac, 0.00% Impervious, Inflow Depth = 0.57" for 50-Year event  
 Inflow = 10.60 cfs @ 0.39 hrs, Volume= 0.438 af  
 Outflow = 10.60 cfs @ 0.39 hrs, Volume= 0.438 af, Atten= 0%, Lag= 0.0 min  
 Primary = 10.60 cfs @ 0.39 hrs, Volume= 0.438 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 53.83' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	48.60'	<b>30.0" Round A-L42</b> L= 420.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 48.60' / 46.50' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=10.56 cfs @ 0.39 hrs HW=53.80' TW=53.41' (Dynamic Tailwater)↑**1=A-L42** (Outlet Controls 10.56 cfs @ 2.15 fps)**Summary for Pond A-L43: A-L43**

Inflow Area = 3.820 ac, 0.00% Impervious, Inflow Depth = 0.61" for 50-Year event  
 Inflow = 4.68 cfs @ 0.24 hrs, Volume= 0.193 af  
 Outflow = 4.68 cfs @ 0.45 hrs, Volume= 0.193 af, Atten= 0%, Lag= 12.6 min  
 Primary = 4.68 cfs @ 0.45 hrs, Volume= 0.193 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 53.98' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	49.77'	<b>24.0" Round A-L43</b> L= 290.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 49.77' / 48.32' S= 0.0050 '/ Cc= 0.900

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n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=4.68 cfs @ 0.45 hrs HW=53.97' TW=53.79' (Dynamic Tailwater)↑**1=A-L43** (Outlet Controls 4.68 cfs @ 1.49 fps)**Summary for Pond A-L44: A-L44**

Inflow Area = 9.430 ac, 0.00% Impervious, Inflow Depth = 0.57" for 50-Year event  
 Inflow = 10.92 cfs @ 0.45 hrs, Volume= 0.451 af  
 Outflow = 10.92 cfs @ 0.33 hrs, Volume= 0.451 af, Atten= 0%, Lag= 0.0 min  
 Primary = 10.92 cfs @ 0.33 hrs, Volume= 0.451 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 53.80' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	48.22'	<b>30.0" Round A-L44</b> L= 344.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 48.22' / 46.50' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=10.82 cfs @ 0.33 hrs HW=53.59' TW=53.24' (Dynamic Tailwater)↑**1=A-L44** (Outlet Controls 10.82 cfs @ 2.20 fps)**Summary for Pond A-L45: A-L45**

Inflow Area = 0.450 ac, 0.00% Impervious, Inflow Depth = 0.63" for 50-Year event  
 Inflow = 0.57 cfs @ 0.14 hrs, Volume= 0.023 af  
 Outflow = 0.57 cfs @ 0.15 hrs, Volume= 0.023 af, Atten= 0%, Lag= 0.6 min  
 Primary = 0.57 cfs @ 0.15 hrs, Volume= 0.023 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 51.09' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	48.55'	<b>18.0" Round A-L45</b> L= 181.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 48.55' / 47.10' S= 0.0080 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=0.00 cfs @ 0.15 hrs HW=50.49' TW=50.49' (Dynamic Tailwater)↑**1=A-L45** ( Controls 0.00 cfs)**Summary for Pond A-L46: A-L46**

Inflow Area = 5.020 ac, 0.00% Impervious, Inflow Depth = 0.52" for 50-Year event  
 Inflow = 5.22 cfs @ 0.34 hrs, Volume= 0.216 af  
 Outflow = 5.22 cfs @ 0.35 hrs, Volume= 0.216 af, Atten= 0%, Lag= 0.6 min  
 Primary = 5.22 cfs @ 0.35 hrs, Volume= 0.216 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 51.08' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	47.00'	<b>24.0" Round A-L46</b> L= 289.0' RCP, sq.cut end projecting, Ke= 0.500



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Inlet / Outlet Invert= 47.00' / 44.40' S= 0.0090 '/ Cc= 0.900  
n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=5.22 cfs @ 0.35 hrs HW=51.02' TW=50.80' (Dynamic Tailwater)

↑1=A-L46 (Outlet Controls 5.22 cfs @ 1.66 fps)

### Summary for Pond A-L47: A-L47

Inflow Area = 6.820 ac, 0.00% Impervious, Inflow Depth = 0.50" for 50-Year event  
Inflow = 6.82 cfs @ 0.50 hrs, Volume= 0.282 af  
Outflow = 6.82 cfs @ 0.50 hrs, Volume= 0.282 af, Atten= 0%, Lag= 0.0 min  
Primary = 6.82 cfs @ 0.50 hrs, Volume= 0.282 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 51.09' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	45.20'	<b>24.0" Round A-L47</b> L= 114.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 45.20' / 44.40' S= 0.0070 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=6.82 cfs @ 0.50 hrs HW=51.08' TW=50.87' (Dynamic Tailwater)

↑1=A-L47 (Outlet Controls 6.82 cfs @ 2.17 fps)

### Summary for Pond A-L48: A-L48

Inflow Area = 0.280 ac, 0.00% Impervious, Inflow Depth = 0.60" for 50-Year event  
Inflow = 0.34 cfs @ 0.09 hrs, Volume= 0.014 af  
Outflow = 0.34 cfs @ 0.10 hrs, Volume= 0.014 af, Atten= 0%, Lag= 0.6 min  
Primary = 0.34 cfs @ 0.10 hrs, Volume= 0.014 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 78.24' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	77.95'	<b>24.0" Round A-L48</b> L= 198.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 77.95' / 76.96' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.34 cfs @ 0.10 hrs HW=78.24' TW=77.40' (Dynamic Tailwater)

↑1=A-L48 (Outlet Controls 0.34 cfs @ 1.86 fps)

### Summary for Pond A-L49: A-L49

Inflow Area = 0.470 ac, 0.00% Impervious, Inflow Depth = 0.60" for 50-Year event  
Inflow = 0.57 cfs @ 0.10 hrs, Volume= 0.023 af  
Outflow = 0.57 cfs @ 0.15 hrs, Volume= 0.023 af, Atten= 0%, Lag= 3.0 min  
Primary = 0.57 cfs @ 0.15 hrs, Volume= 0.023 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 77.41' @ 0.15 hrs

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Device	Routing	Invert	Outlet Devices
#1	Primary	76.86'	<b>24.0" Round A-L49</b> L= 198.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 76.86' / 75.87' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.57 cfs @ 0.15 hrs HW=77.41' TW=77.22' (Dynamic Tailwater)↑**1=A-L49** (Outlet Controls 0.57 cfs @ 1.23 fps)**Summary for Pond A-L50: A-L50**

Inflow Area = 5.700 ac, 0.00% Impervious, Inflow Depth = 0.61" for 50-Year event  
 Inflow = 7.07 cfs @ 0.09 hrs, Volume= 0.292 af  
 Outflow = 7.07 cfs @ 0.10 hrs, Volume= 0.292 af, Atten= 0%, Lag= 0.6 min  
 Primary = 7.07 cfs @ 0.10 hrs, Volume= 0.292 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 77.22' @ 0.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	75.77'	<b>24.0" Round A-L50</b> L= 198.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 75.77' / 74.78' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=7.06 cfs @ 0.10 hrs HW=77.21' TW=76.21' (Dynamic Tailwater)↑**1=A-L50** (Outlet Controls 7.06 cfs @ 4.09 fps)**Summary for Pond A-L51: A-L51**

Inflow Area = 5.860 ac, 0.00% Impervious, Inflow Depth = 0.61" for 50-Year event  
 Inflow = 7.26 cfs @ 0.10 hrs, Volume= 0.300 af  
 Outflow = 7.26 cfs @ 0.10 hrs, Volume= 0.300 af, Atten= 0%, Lag= 0.0 min  
 Primary = 7.26 cfs @ 0.10 hrs, Volume= 0.300 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 76.26' @ 0.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	74.68'	<b>24.0" Round A-L51</b> L= 198.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 74.68' / 73.69' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=7.22 cfs @ 0.10 hrs HW=76.21' TW=75.35' (Dynamic Tailwater)↑**1=A-L51** (Outlet Controls 7.22 cfs @ 3.87 fps)**Summary for Pond A-L52: A-L52**

Inflow Area = 0.420 ac, 0.00% Impervious, Inflow Depth = 0.68" for 50-Year event  
 Inflow = 0.58 cfs @ 0.12 hrs, Volume= 0.024 af  
 Outflow = 0.58 cfs @ 0.18 hrs, Volume= 0.024 af, Atten= 0%, Lag= 3.6 min  
 Primary = 0.58 cfs @ 0.18 hrs, Volume= 0.024 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3



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Peak Elev= 77.22' @ 0.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	76.90'	<b>24.0" Round A-L52</b> L= 160.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 76.90' / 75.32' S= 0.0099 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.58 cfs @ 0.18 hrs HW=77.22' TW=75.96' (Dynamic Tailwater)↑**1=A-L52** (Outlet Controls 0.58 cfs @ 2.68 fps)**Summary for Pond A-L53: A-L53**

Inflow Area = 1.400 ac, 0.00% Impervious, Inflow Depth = 0.65" for 50-Year event  
 Inflow = 1.83 cfs @ 0.16 hrs, Volume= 0.076 af  
 Outflow = 1.83 cfs @ 0.15 hrs, Volume= 0.076 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.83 cfs @ 0.15 hrs, Volume= 0.076 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 75.96' @ 0.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	75.22'	<b>24.0" Round A-L53</b> L= 155.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 75.22' / 73.69' S= 0.0099 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.83 cfs @ 0.15 hrs HW=75.96' TW=75.46' (Dynamic Tailwater)↑**1=A-L53** (Outlet Controls 1.83 cfs @ 2.56 fps)**Summary for Pond A-L54: A-L54**

Inflow Area = 8.310 ac, 0.00% Impervious, Inflow Depth = 0.62" for 50-Year event  
 Inflow = 10.44 cfs @ 0.15 hrs, Volume= 0.432 af  
 Outflow = 10.44 cfs @ 0.15 hrs, Volume= 0.432 af, Atten= 0%, Lag= 0.0 min  
 Primary = 10.44 cfs @ 0.15 hrs, Volume= 0.432 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 75.46' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	73.59'	<b>24.0" Round A-L54</b> L= 55.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 73.59' / 73.07' S= 0.0095 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=10.44 cfs @ 0.15 hrs HW=75.46' TW=74.84' (Dynamic Tailwater)↑**1=A-L54** (Outlet Controls 10.44 cfs @ 4.44 fps)**Summary for Pond A-L55: A-L55**

Inflow Area = 8.310 ac, 0.00% Impervious, Inflow Depth = 0.62" for 50-Year event  
 Inflow = 10.44 cfs @ 0.15 hrs, Volume= 0.432 af  
 Outflow = 10.44 cfs @ 0.15 hrs, Volume= 0.432 af, Atten= 0%, Lag= 0.0 min  
 Primary = 10.44 cfs @ 0.15 hrs, Volume= 0.432 af

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Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 74.84' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	72.97'	<b>24.0" Round Pipe</b> L= 80.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 72.97' / 72.57' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=10.44 cfs @ 0.15 hrs HW=74.84' TW=74.13' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 10.44 cfs @ 4.45 fps)**Summary for Pond A-L56: A-L56**

Inflow Area = 9.720 ac, 0.00% Impervious, Inflow Depth = 0.62" for 50-Year event  
 Inflow = 12.22 cfs @ 0.15 hrs, Volume= 0.505 af  
 Outflow = 12.22 cfs @ 0.15 hrs, Volume= 0.505 af, Atten= 0%, Lag= 0.0 min  
 Primary = 12.22 cfs @ 0.15 hrs, Volume= 0.505 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 74.13' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	72.47'	<b>24.0" Round A-L56</b> L= 113.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 72.47' / 69.20' S= 0.0289 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=12.22 cfs @ 0.15 hrs HW=74.13' TW=70.64' (Dynamic Tailwater)↑**1=A-L56** (Inlet Controls 12.22 cfs @ 4.39 fps)**Summary for Pond A-L57: A-L57**

Inflow Area = 1.030 ac, 0.00% Impervious, Inflow Depth = 0.87" for 50-Year event  
 Inflow = 1.81 cfs @ 0.09 hrs, Volume= 0.075 af  
 Outflow = 1.81 cfs @ 0.09 hrs, Volume= 0.075 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.81 cfs @ 0.09 hrs, Volume= 0.075 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 71.08' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	70.36'	<b>24.0" Round A-L57</b> L= 63.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 70.36' / 69.73' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.81 cfs @ 0.09 hrs HW=71.05' TW=70.68' (Dynamic Tailwater)↑**1=A-L57** (Outlet Controls 1.81 cfs @ 2.80 fps)



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**Summary for Pond A-L58: A-L58**

Inflow Area = 0.520 ac, 0.00% Impervious, Inflow Depth = 0.85" for 50-Year event  
 Inflow = 0.89 cfs @ 0.09 hrs, Volume= 0.037 af  
 Outflow = 0.89 cfs @ 0.12 hrs, Volume= 0.037 af, Atten= 0%, Lag= 1.8 min  
 Primary = 0.89 cfs @ 0.12 hrs, Volume= 0.037 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 70.79' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	69.93'	<b>24.0" Round A-L58</b> L= 20.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 69.93' / 69.73' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.89 cfs @ 0.12 hrs HW=70.77' TW=70.74' (Dynamic Tailwater)↑**1=A-L58** (Outlet Controls 0.89 cfs @ 1.05 fps)**Summary for Pond A-L59: A-L59**

Inflow Area = 1.550 ac, 0.00% Impervious, Inflow Depth = 0.86" for 50-Year event  
 Inflow = 2.70 cfs @ 0.09 hrs, Volume= 0.112 af  
 Outflow = 2.70 cfs @ 0.10 hrs, Volume= 0.112 af, Atten= 0%, Lag= 0.6 min  
 Primary = 2.70 cfs @ 0.10 hrs, Volume= 0.112 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 70.76' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	69.63'	<b>30.0" Round Pipe</b> L= 53.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 69.63' / 69.10' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=2.70 cfs @ 0.10 hrs HW=70.70' TW=70.56' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 2.70 cfs @ 1.98 fps)**Summary for Pond A-L60: A-L60**

Inflow Area = 11.270 ac, 0.00% Impervious, Inflow Depth = 0.66" for 50-Year event  
 Inflow = 14.92 cfs @ 0.15 hrs, Volume= 0.617 af  
 Outflow = 14.92 cfs @ 0.15 hrs, Volume= 0.617 af, Atten= 0%, Lag= 0.0 min  
 Primary = 14.92 cfs @ 0.15 hrs, Volume= 0.617 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 70.64' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	69.00'	<b>30.0" Round Pipe</b> L= 408.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 69.00' / 64.92' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=14.92 cfs @ 0.15 hrs HW=70.64' TW=66.54' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 14.92 cfs @ 4.36 fps)

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**Summary for Pond A-L61: A-L61**

Inflow Area = 1.870 ac, 0.00% Impervious, Inflow Depth = 0.71" for 50-Year event  
 Inflow = 2.68 cfs @ 0.10 hrs, Volume= 0.111 af  
 Outflow = 2.68 cfs @ 0.10 hrs, Volume= 0.111 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.68 cfs @ 0.10 hrs, Volume= 0.111 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 66.77' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	65.73'	<b>24.0" Round A-L61</b> L= 71.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 65.73' / 65.02' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.68 cfs @ 0.10 hrs HW=66.73' TW=66.48' (Dynamic Tailwater)↑**1=A-L61** (Outlet Controls 2.68 cfs @ 2.48 fps)**Summary for Pond A-L62: A-L62**

Inflow Area = 0.720 ac, 0.00% Impervious, Inflow Depth = 0.81" for 50-Year event  
 Inflow = 1.18 cfs @ 0.09 hrs, Volume= 0.049 af  
 Outflow = 1.18 cfs @ 0.09 hrs, Volume= 0.049 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.18 cfs @ 0.09 hrs, Volume= 0.049 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 66.56' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	65.23'	<b>24.0" Round A-L62</b> L= 21.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 65.23' / 65.02' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.18 cfs @ 0.09 hrs HW=66.46' TW=66.44' (Dynamic Tailwater)↑**1=A-L62** (Outlet Controls 1.18 cfs @ 0.83 fps)**Summary for Pond A-L63: A-L63**

Inflow Area = 13.860 ac, 0.00% Impervious, Inflow Depth = 0.67" for 50-Year event  
 Inflow = 18.78 cfs @ 0.15 hrs, Volume= 0.776 af  
 Outflow = 18.78 cfs @ 0.15 hrs, Volume= 0.776 af, Atten= 0%, Lag= 0.0 min  
 Primary = 18.78 cfs @ 0.15 hrs, Volume= 0.776 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 66.54' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	64.82'	<b>36.0" Round Pipe</b> L= 440.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 64.82' / 60.42' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=18.78 cfs @ 0.15 hrs HW=66.54' TW=62.71' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 18.78 cfs @ 4.47 fps)



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**Summary for Pond A-L64: A-L64**

Inflow Area = 2.340 ac, 0.00% Impervious, Inflow Depth = 0.69" for 50-Year event  
 Inflow = 3.27 cfs @ 0.10 hrs, Volume= 0.135 af  
 Outflow = 3.27 cfs @ 0.12 hrs, Volume= 0.135 af, Atten= 0%, Lag= 1.2 min  
 Primary = 3.27 cfs @ 0.12 hrs, Volume= 0.135 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 62.77' @ 0.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	60.88'	<b>24.0" Round A-L64</b> L= 72.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 60.88' / 60.52' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=3.20 cfs @ 0.12 hrs HW=62.74' TW=62.68' (Dynamic Tailwater)↑**1=A-L64** (Outlet Controls 3.20 cfs @ 1.37 fps)**Summary for Pond A-L65: A-L65**

Inflow Area = 0.720 ac, 0.00% Impervious, Inflow Depth = 0.81" for 50-Year event  
 Inflow = 1.18 cfs @ 0.09 hrs, Volume= 0.049 af  
 Outflow = 1.18 cfs @ 0.10 hrs, Volume= 0.049 af, Atten= 0%, Lag= 0.6 min  
 Primary = 1.18 cfs @ 0.10 hrs, Volume= 0.049 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 62.71' @ 0.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	60.63'	<b>24.0" Round A-L65</b> L= 22.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 60.63' / 60.52' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.00 cfs @ 0.10 hrs HW=62.62' TW=62.62' (Dynamic Tailwater)↑**1=A-L65** ( Controls 0.00 cfs)**Summary for Pond A-L66: A-L66**

Inflow Area = 16.920 ac, 0.00% Impervious, Inflow Depth = 0.68" for 50-Year event  
 Inflow = 23.23 cfs @ 0.15 hrs, Volume= 0.960 af  
 Outflow = 23.23 cfs @ 0.15 hrs, Volume= 0.960 af, Atten= 0%, Lag= 0.0 min  
 Primary = 23.23 cfs @ 0.15 hrs, Volume= 0.960 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 62.71' @ 0.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	60.32'	<b>36.0" Round Pipe</b> L= 98.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 60.32' / 59.73' S= 0.0060 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=23.23 cfs @ 0.15 hrs HW=62.71' TW=61.79' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 23.23 cfs @ 5.28 fps)

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**Summary for Pond A-L67: A-L67**

Inflow Area = 16.920 ac, 0.00% Impervious, Inflow Depth = 0.68" for 50-Year event  
 Inflow = 23.23 cfs @ 0.15 hrs, Volume= 0.960 af  
 Outflow = 23.23 cfs @ 0.15 hrs, Volume= 0.960 af, Atten= 0%, Lag= 0.0 min  
 Primary = 23.23 cfs @ 0.15 hrs, Volume= 0.960 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 61.79' @ 0.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	59.63'	<b>36.0" Round Pipe</b> L= 88.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 59.63' / 58.84' S= 0.0090 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=23.23 cfs @ 0.15 hrs HW=61.79' TW=60.63' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 23.23 cfs @ 5.97 fps)

**Summary for Pond A-L68: A-L68**

Inflow Area = 16.920 ac, 0.00% Impervious, Inflow Depth = 0.68" for 50-Year event  
 Inflow = 23.23 cfs @ 0.15 hrs, Volume= 0.960 af  
 Outflow = 23.23 cfs @ 0.15 hrs, Volume= 0.960 af, Atten= 0%, Lag= 0.0 min  
 Primary = 23.23 cfs @ 0.15 hrs, Volume= 0.960 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 60.63' @ 0.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	58.74'	<b>48.0" Round Pipe</b> L= 398.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 58.74' / 56.75' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 12.57 sf

**Primary OutFlow** Max=23.23 cfs @ 0.15 hrs HW=60.63' TW=58.38' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 23.23 cfs @ 5.84 fps)

**Summary for Pond A-L69: A-L69**

Inflow Area = 16.920 ac, 0.00% Impervious, Inflow Depth = 0.68" for 50-Year event  
 Inflow = 23.23 cfs @ 0.15 hrs, Volume= 0.960 af  
 Outflow = 23.23 cfs @ 0.15 hrs, Volume= 0.960 af, Atten= 0%, Lag= 0.0 min  
 Primary = 23.23 cfs @ 0.15 hrs, Volume= 0.960 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 58.38' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.65'	<b>48.0" Round Pipe</b> L= 240.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.65' / 53.56' S= 0.0129 '/ Cc= 0.900 n= 0.013, Flow Area= 12.57 sf

**Primary OutFlow** Max=23.23 cfs @ 0.15 hrs HW=58.38' TW=55.57' (Dynamic Tailwater)  
 ↑**1=Pipe** (Inlet Controls 23.23 cfs @ 4.47 fps)



**Proposed System AD**

City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

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**Summary for Pond A-L70: A-L70**

Inflow Area = 1.370 ac, 0.00% Impervious, Inflow Depth = 0.79" for 50-Year event  
 Inflow = 2.17 cfs @ 0.09 hrs, Volume= 0.090 af  
 Outflow = 2.17 cfs @ 0.10 hrs, Volume= 0.090 af, Atten= 0%, Lag= 0.6 min  
 Primary = 2.17 cfs @ 0.10 hrs, Volume= 0.090 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 55.60' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	53.80'	<b>24.0" Round A-L77</b> L= 28.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 53.80' / 53.66' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.17 cfs @ 0.10 hrs HW=55.54' TW=55.51' (Dynamic Tailwater)↑**1=A-L77** (Outlet Controls 2.17 cfs @ 1.00 fps)**Summary for Pond A-L71: A-L71**

Inflow Area = 18.290 ac, 0.00% Impervious, Inflow Depth = 0.69" for 50-Year event  
 Inflow = 25.40 cfs @ 0.15 hrs, Volume= 1.050 af  
 Outflow = 25.40 cfs @ 0.15 hrs, Volume= 1.050 af, Atten= 0%, Lag= 0.0 min  
 Primary = 25.40 cfs @ 0.15 hrs, Volume= 1.050 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 55.57' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	53.46'	<b>48.0" Round Pipe</b> L= 49.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 53.46' / 53.02' S= 0.0090 '/ Cc= 0.900 n= 0.013, Flow Area= 12.57 sf

**Primary OutFlow** Max=25.40 cfs @ 0.15 hrs HW=55.57' TW=54.73' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 25.40 cfs @ 5.48 fps)**Summary for Pond A-L72: A-L72**

Inflow Area = 18.290 ac, 0.00% Impervious, Inflow Depth = 0.69" for 50-Year event  
 Inflow = 25.40 cfs @ 0.15 hrs, Volume= 1.050 af  
 Outflow = 25.40 cfs @ 0.15 hrs, Volume= 1.050 af, Atten= 0%, Lag= 0.0 min  
 Primary = 25.40 cfs @ 0.15 hrs, Volume= 1.050 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 54.73' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	52.92'	<b>48.0" Round Pipe</b> L= 340.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 52.92' / 47.50' S= 0.0159 '/ Cc= 0.900 n= 0.013, Flow Area= 12.57 sf

**Primary OutFlow** Max=25.40 cfs @ 0.15 hrs HW=54.73' TW=50.68' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 25.40 cfs @ 4.59 fps)

**Proposed System AD**

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**Summary for Pond A-L73: A-L73**

Inflow Area = 18.290 ac, 0.00% Impervious, Inflow Depth = 0.69" for 50-Year event  
 Inflow = 25.40 cfs @ 0.15 hrs, Volume= 1.050 af  
 Outflow = 25.40 cfs @ 0.15 hrs, Volume= 1.050 af, Atten= 0%, Lag= 0.0 min  
 Primary = 25.40 cfs @ 0.15 hrs, Volume= 1.050 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 51.12' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	45.30'	<b>48.0" Round Pipe</b> L= 492.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 45.30' / 44.40' S= 0.0018 '/ Cc= 0.900 n= 0.013, Flow Area= 12.57 sf

**Primary OutFlow** Max=25.40 cfs @ 0.15 hrs HW=50.68' TW=50.44' (Dynamic Tailwater)  
 ←**1=Pipe** (Outlet Controls 25.40 cfs @ 2.02 fps)

**Summary for Subcatchment A01: A01**

Runoff = 1.86 cfs @ 0.12 hrs, Volume= 0.077 af, Depth= 0.65"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
 City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
1.430	0.69	Mixed Use, HSG D
1.430		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment A02: A02**

Runoff = 0.80 cfs @ 0.12 hrs, Volume= 0.033 af, Depth= 0.67"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
 City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.590	0.72	Mixed Use, HSG D
0.590		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.0					<b>Direct Entry, P-A02</b>



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**Summary for Subcatchment A03: A03**

Runoff = 0.61 cfs @ 0.09 hrs, Volume= 0.025 af, Depth= 0.89"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.340	0.95	Mixed Use, HSG D
0.340		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, P-A02

**Summary for Subcatchment A04: A04**

Runoff = 0.50 cfs @ 0.09 hrs, Volume= 0.021 af, Depth= 0.89"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.280	0.95	Mixed Use, HSG D
0.280		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, P-A02

**Summary for Subcatchment A05: A05**

Runoff = 5.02 cfs @ 0.39 hrs, Volume= 0.207 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
4.840	0.55	Mixed Use, HSG D
4.840		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0					Direct Entry, P-A02

**Summary for Subcatchment A06: A06**

Runoff = 0.37 cfs @ 0.10 hrs, Volume= 0.015 af, Depth= 0.70"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

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Area (ac)	C	Description
0.260	0.75	Mixed Use, HSG D
0.260		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, P-A02

**Summary for Subcatchment A07: A07**

Runoff = 0.30 cfs @ 0.14 hrs, Volume= 0.012 af, Depth= 0.60"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.250	0.64	Mixed Use, HSG D
0.250		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					Direct Entry, P-A02

**Summary for Subcatchment A08: A08**

Runoff = 3.34 cfs @ 0.34 hrs, Volume= 0.138 af, Depth= 0.54"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
3.050	0.58	Mixed Use, HSG D
3.050		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.0					Direct Entry, P-A02

**Summary for Subcatchment A09: A09**

Runoff = 1.30 cfs @ 0.14 hrs, Volume= 0.054 af, Depth= 0.63"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
1.030	0.67	Mixed Use, HSG D
1.030		100.00% Pervious Area



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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					<b>Direct Entry, P-A02</b>

**Summary for Subcatchment A10: A10**

Runoff = 0.91 cfs @ 0.09 hrs, Volume= 0.038 af, Depth= 0.82"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.550	0.88	Mixed Use, HSG D
0.550		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, P-A02</b>

**Summary for Subcatchment A11: A11**

Runoff = 3.33 cfs @ 0.09 hrs, Volume= 0.138 af, Depth= 0.88"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
1.880	0.94	Mixed Use, HSG D
1.880		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, P-A02</b>

**Summary for Subcatchment A12: A12**

Runoff = 0.67 cfs @ 0.09 hrs, Volume= 0.028 af, Depth= 0.62"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.540	0.66	Mixed Use, HSG D
0.540		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, P-A02</b>

**Proposed System AD**

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**Summary for Subcatchment A13: A13**

Runoff = 0.38 cfs @ 0.09 hrs, Volume= 0.016 af, Depth= 0.78"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.240	0.83	Mixed Use, HSG D
0.240		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, P-A02

**Summary for Subcatchment A14: A14**

Runoff = 0.61 cfs @ 0.10 hrs, Volume= 0.025 af, Depth= 0.72"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.420	0.77	Mixed Use, HSG D
0.420		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, P-A02

**Summary for Subcatchment A16/17: A16/17**

Runoff = 0.47 cfs @ 0.09 hrs, Volume= 0.020 af, Depth= 0.87"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.270	0.93	Mixed Use, HSG D
0.270		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, P-A02

**Summary for Subcatchment A18: A18**

Runoff = 3.95 cfs @ 0.30 hrs, Volume= 0.163 af, Depth= 0.50"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr



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City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

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Area (ac)	C	Description
3.950	0.53	Mixed Use, HSG D
3.950		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.0					Direct Entry, P-A02

**Summary for Subcatchment A19: A19**

Runoff = 0.45 cfs @ 0.14 hrs, Volume= 0.019 af, Depth= 0.61"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.370	0.65	Mixed Use, HSG D
0.370		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					Direct Entry, P-A02

**Summary for Subcatchment A20: A20**

Runoff = 0.75 cfs @ 0.15 hrs, Volume= 0.031 af, Depth= 0.57"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.650	0.61	Mixed Use, HSG D
0.650		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.0					Direct Entry, P-A02

**Summary for Subcatchment A21: A21**

Runoff = 0.89 cfs @ 0.09 hrs, Volume= 0.037 af, Depth= 0.81"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.540	0.87	Mixed Use, HSG D
0.540		100.00% Pervious Area

**Proposed System AD**

City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, P-A02</b>

**Summary for Subcatchment A22: A22**

Runoff = 0.58 cfs @ 0.12 hrs, Volume= 0.024 af, Depth= 0.65"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.440	0.70	Mixed Use, HSG D
0.440		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.0					<b>Direct Entry, P-A02</b>

**Summary for Subcatchment A23: A23**

Runoff = 4.68 cfs @ 0.30 hrs, Volume= 0.193 af, Depth= 0.59"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
3.940	0.63	Mixed Use, HSG D
3.940		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.0					<b>Direct Entry, P-A02</b>

**Summary for Subcatchment A24: A24**

Runoff = 5.92 cfs @ 0.39 hrs, Volume= 0.245 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
5.320	0.59	Mixed Use, HSG D
5.320		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0					<b>Direct Entry, P-A02</b>



**Proposed System AD**

City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

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**Summary for Subcatchment A25: A25**

Runoff = 4.68 cfs @ 0.24 hrs, Volume= 0.193 af, Depth= 0.61"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
3.820	0.65	Mixed Use, HSG D
3.820		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.0					Direct Entry, P-A02

**Summary for Subcatchment A26: A26**

Runoff = 6.24 cfs @ 0.32 hrs, Volume= 0.258 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
5.610	0.59	Mixed Use, HSG D
5.610		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.0					Direct Entry, P-A02

**Summary for Subcatchment A27: A27**

Runoff = 4.65 cfs @ 0.34 hrs, Volume= 0.192 af, Depth= 0.50"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
4.570	0.54	Mixed Use, HSG D
4.570		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.0					Direct Entry, P-A02

**Summary for Subcatchment A28: A28**

Runoff = 6.82 cfs @ 0.50 hrs, Volume= 0.282 af, Depth= 0.50"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

**Proposed System AD**

City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

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Area (ac)	C	Description
6.820	0.53	Mixed Use, HSG D
6.820		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.0					Direct Entry, P-A02

**Summary for Subcatchment A29: A29**

Runoff = 0.34 cfs @ 0.09 hrs, Volume= 0.014 af, Depth= 0.60"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.280	0.64	Mixed Use, HSG D
0.280		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment A30: A30**

Runoff = 0.23 cfs @ 0.09 hrs, Volume= 0.009 af, Depth= 0.60"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.190	0.64	Mixed Use, HSG D
0.190		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment A31: A31**

Runoff = 0.22 cfs @ 0.09 hrs, Volume= 0.009 af, Depth= 0.60"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.180	0.64	Mixed Use, HSG D
0.180		100.00% Pervious Area



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City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment A32: A32**

Runoff = 0.19 cfs @ 0.09 hrs, Volume= 0.008 af, Depth= 0.60"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.160	0.64	Mixed Use, HSG D
0.160		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment A33: A33**

Runoff = 0.58 cfs @ 0.12 hrs, Volume= 0.024 af, Depth= 0.68"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.420	0.73	Mixed Use, HSG D
0.420		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment A34: A34**

Runoff = 1.26 cfs @ 0.14 hrs, Volume= 0.052 af, Depth= 0.64"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.980	0.68	Mixed Use, HSG D
0.980		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					<b>Direct Entry, PA01</b>

**Proposed System AD**

City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

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**Summary for Subcatchment A35: A35**

Runoff = 1.35 cfs @ 0.14 hrs, Volume= 0.056 af, Depth= 0.64"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
1.050	0.68	Mixed Use, HSG D
1.050		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					Direct Entry, PA01

**Summary for Subcatchment A36/37: A36/37**

Runoff = 1.78 cfs @ 0.14 hrs, Volume= 0.074 af, Depth= 0.63"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
1.410	0.67	Mixed Use, HSG D
1.410		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					Direct Entry, PA01

**Summary for Subcatchment A38: A38**

Runoff = 1.81 cfs @ 0.09 hrs, Volume= 0.075 af, Depth= 0.87"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
1.030	0.93	Mixed Use, HSG D
1.030		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment A39: A39**

Runoff = 0.89 cfs @ 0.09 hrs, Volume= 0.037 af, Depth= 0.85"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr



**Proposed System AD**

City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

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Area (ac)	C	Description
0.520	0.91	Mixed Use, HSG D
0.520		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment A40: A40**

Runoff = 2.68 cfs @ 0.10 hrs, Volume= 0.111 af, Depth= 0.71"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
1.870	0.76	Mixed Use, HSG D
1.870		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, PA01

**Summary for Subcatchment A41: A41**

Runoff = 1.18 cfs @ 0.09 hrs, Volume= 0.049 af, Depth= 0.81"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.720	0.87	Mixed Use, HSG D
0.720		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment A42: A42**

Runoff = 3.27 cfs @ 0.10 hrs, Volume= 0.135 af, Depth= 0.69"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
2.340	0.74	Mixed Use, HSG D
2.340		100.00% Pervious Area

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City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment A43: A43**

Runoff = 1.18 cfs @ 0.09 hrs, Volume= 0.049 af, Depth= 0.81"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.720	0.87	Mixed Use, HSG D
0.720		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment A44: A44**

Runoff = 2.17 cfs @ 0.09 hrs, Volume= 0.090 af, Depth= 0.79"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
1.370	0.84	Mixed Use, HSG D
1.370		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment A45: A45**

Runoff = 5.61 cfs @ 0.09 hrs, Volume= 0.232 af, Depth= 0.61"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
4.580	0.65	Mixed Use, HSG D
4.580		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, P-A02</b>



**Proposed System AD**

City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

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**Summary for Subcatchment A46: A46**

Runoff = 6.28 cfs @ 0.09 hrs, Volume= 0.260 af, Depth= 0.62"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
5.050	0.66	Mixed Use, HSG D
5.050		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, P-A02

**Summary for Subcatchment A47: A47**

Runoff = 9.56 cfs @ 0.09 hrs, Volume= 0.395 af, Depth= 0.86"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
5.510	0.92	Mixed Use, HSG D
5.510		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, P-A02

**Summary for Subcatchment Basin-A1: Basin A1**

Runoff = 6.25 cfs @ 0.39 hrs, Volume= 0.258 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
5.620	0.59	Mixed Use, HSG D
5.620		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0					Direct Entry, P-A02

**Summary for Subcatchment Basin-A2: ABASIN2**

Runoff = 1.19 cfs @ 0.09 hrs, Volume= 0.049 af, Depth= 0.79"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

**Proposed System AD**

City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

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Area (ac)	C	Description
0.750	0.84	Mixed Use, HSG D
0.750		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, P-A02

**Summary for Subcatchment Basin-A3: Basin A3**

Runoff = 0.57 cfs @ 0.14 hrs, Volume= 0.023 af, Depth= 0.63"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

Area (ac)	C	Description
0.450	0.67	Mixed Use, HSG D
0.450		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					Direct Entry, P-A02

**Summary for Pond D-L1: D-L1**

Inflow Area = 2.910 ac, 0.00% Impervious, Inflow Depth = 0.49" for 50-Year event  
 Inflow = 2.85 cfs @ 0.34 hrs, Volume= 0.118 af  
 Outflow = 2.85 cfs @ 0.34 hrs, Volume= 0.118 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.85 cfs @ 0.34 hrs, Volume= 0.118 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
Peak Elev= 51.79' @ 0.34 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	51.00'	<b>18.0" Round D-L1</b> L= 50.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 51.00' / 46.20' S= 0.0960 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=2.85 cfs @ 0.34 hrs HW=51.79' TW=0.00' (Dynamic Tailwater)  
 ←1=D-L1 (Inlet Controls 2.85 cfs @ 3.03 fps)

**Summary for Subcatchment D1: D1**

Runoff = 2.85 cfs @ 0.34 hrs, Volume= 0.118 af, Depth= 0.49"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr



**Proposed System AD**

City of San Diego 50-Year Duration=30 min, Inten=1.87 in/hr

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Area (ac)	C	Description
2.910	0.52	Mixed Use, HSG D
2.910		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.0					Direct Entry, P-A02

**Summary for Pond OUTFALL A: OUTFALL A**

Inflow Area = 87.200 ac, 0.71% Impervious, Inflow Depth = 0.48" for 50-Year event  
 Inflow = 69.62 cfs @ 0.50 hrs, Volume= 3.501 af  
 Primary = 69.62 cfs @ 0.50 hrs, Volume= 3.501 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

**Summary for Pond Outlet: BasinOutlet**

Inflow Area = 2.910 ac, 0.00% Impervious, Inflow Depth = 4.57" for 50-Year event  
 Inflow = 41.66 cfs @ 0.50 hrs, Volume= 1.109 af  
 Primary = 41.66 cfs @ 0.50 hrs, Volume= 1.109 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

100-year Rainfall Intensity

Figure A-1. Intensity-Duration-Frequency Design Chart

ELEV.	FACTOR
0-1500	1.00
1500-3000	1.25
3000-4000	1.42
4000-5000	1.60
5000-6000	1.7
DESERT	1.25

TO OBTAIN CORRECT INTENSITY, MULTIPLY INTENSITY ON CHART BY FACTOR FOR DESIGN ELEVATION.

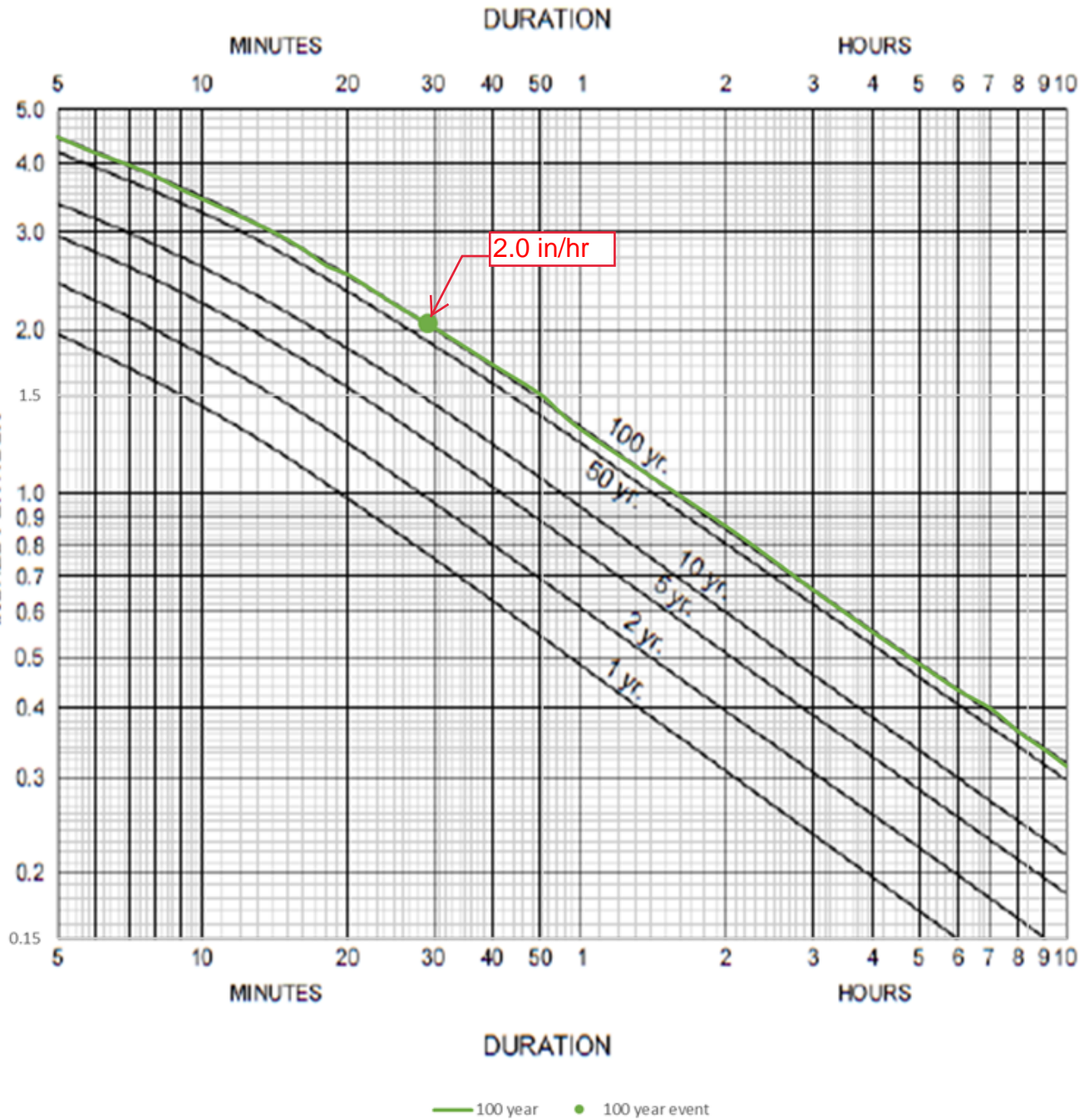
COUNTY OF SAN DIEGO

FOR

INTENSITY - DURATION - FREQUENCY CURVES

RAINFALL

INTENSITY (SAN DIEGO) INCHES PER HOUR





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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points x 3

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Pond A-BUBBLER: A-BUBBLER</b>	Peak Elev=50.31' Storage=2,371 cf Inflow=116.21 cfs 4.802 af Primary=69.84 cfs 3.568 af Secondary=46.11 cfs 1.236 af Outflow=115.95 cfs 4.804 af
<b>Pond A-L01: A-L01</b>	Peak Elev=77.48' Inflow=1.99 cfs 0.082 af 24.0" Round Culvert n=0.013 L=200.0' S=0.0098 '/' Outflow=1.99 cfs 0.082 af
<b>Pond A-L02: A-L02</b>	Peak Elev=75.54' Inflow=2.85 cfs 0.118 af 24.0" Round Culvert n=0.013 L=132.0' S=0.0100 '/' Outflow=2.85 cfs 0.118 af
<b>Pond A-L03: A-L03</b>	Peak Elev=74.12' Inflow=2.85 cfs 0.118 af 24.0" Round Culvert n=0.013 L=175.0' S=0.0100 '/' Outflow=2.85 cfs 0.118 af
<b>Pond A-L04: A-L04</b>	Peak Elev=72.40' Inflow=3.50 cfs 0.145 af 24.0" Round Culvert n=0.013 L=218.0' S=0.0100 '/' Outflow=3.50 cfs 0.145 af
<b>Pond A-L05: A-L05</b>	Peak Elev=70.91' Inflow=4.03 cfs 0.167 af 24.0" Round Culvert n=0.013 L=49.0' S=0.0100 '/' Outflow=4.03 cfs 0.167 af
<b>Pond A-L06: A-L06</b>	Peak Elev=70.97' Inflow=5.37 cfs 0.222 af 24.0" Round Culvert n=0.013 L=37.0' S=0.0100 '/' Outflow=5.37 cfs 0.222 af
<b>Pond A-L07: A-L07</b>	Peak Elev=70.78' Inflow=15.41 cfs 0.637 af 24.0" Round Culvert n=0.013 L=139.0' S=0.0100 '/' Outflow=15.41 cfs 0.637 af
<b>Pond A-L08: A-L08</b>	Peak Elev=69.29' Inflow=15.80 cfs 0.653 af 24.0" Round Culvert n=0.013 L=196.0' S=0.0100 '/' Outflow=15.80 cfs 0.653 af
<b>Pond A-L09: A-L09</b>	Peak Elev=66.82' Inflow=16.12 cfs 0.666 af 36.0" Round Culvert n=0.013 L=98.0' S=0.0100 '/' Outflow=16.12 cfs 0.666 af
<b>Pond A-L10: A-L10</b>	Peak Elev=65.64' Inflow=16.12 cfs 0.666 af 36.0" Round Culvert n=0.013 L=195.0' S=0.0170 '/' Outflow=16.12 cfs 0.666 af
<b>Pond A-L11: A-L11</b>	Peak Elev=62.22' Inflow=16.12 cfs 0.666 af 36.0" Round Culvert n=0.013 L=47.0' S=0.0887 '/' Outflow=16.12 cfs 0.666 af
<b>Pond A-L12: A-L12</b>	Peak Elev=58.64' Inflow=19.69 cfs 0.814 af 36.0" Round Culvert n=0.013 L=48.0' S=0.0050 '/' Outflow=19.69 cfs 0.814 af
<b>Pond A-L13: A-L13</b>	Peak Elev=58.01' Inflow=1.39 cfs 0.058 af 36.0" Round Culvert n=0.013 L=12.0' S=0.0050 '/' Outflow=1.39 cfs 0.058 af
<b>Pond A-L14: A-L14</b>	Peak Elev=80.83' Inflow=4.54 cfs 0.188 af 36.0" Round Culvert n=0.013 L=520.0' S=0.0460 '/' Outflow=4.54 cfs 0.188 af
<b>Pond A-L15: A-L15</b>	Peak Elev=81.06' Inflow=0.98 cfs 0.040 af 24.0" Round Culvert n=0.013 L=54.0' S=0.0050 '/' Outflow=0.98 cfs 0.040 af
<b>Pond A-L16: A-L16</b>	Peak Elev=81.28' Inflow=3.56 cfs 0.147 af 24.0" Round Culvert n=0.013 L=12.0' S=0.0050 '/' Outflow=3.56 cfs 0.147 af

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<b>Pond A-L17: A-L17</b>	Peak Elev=58.00' Inflow=25.62 cfs 1.059 af 42.0" Round Culvert n=0.013 L=267.0' S=0.0070 '/ Outflow=25.62 cfs 1.059 af
<b>Pond A-L18: A-L18</b>	Peak Elev=56.06' Inflow=0.72 cfs 0.030 af 24.0" Round Culvert n=0.013 L=12.0' S=0.0450 '/ Outflow=0.72 cfs 0.030 af
<b>Pond A-L19: A-L19</b>	Peak Elev=56.06' Inflow=0.40 cfs 0.017 af 24.0" Round Culvert n=0.013 L=30.0' S=0.0213 '/ Outflow=0.40 cfs 0.017 af
<b>Pond A-L20: A-L20</b>	Peak Elev=56.06' Inflow=26.74 cfs 1.105 af 42.0" Round Culvert n=0.013 L=462.0' S=0.0050 '/ Outflow=26.74 cfs 1.105 af
<b>Pond A-L21: A-L21</b>	Peak Elev=53.14' Inflow=0.65 cfs 0.027 af 24.0" Round Culvert n=0.013 L=30.0' S=0.0017 '/ Outflow=0.65 cfs 0.027 af
<b>Pond A-L22: A-L22</b>	Peak Elev=53.14' Inflow=27.39 cfs 1.132 af 42.0" Round Culvert x 2.00 n=0.013 L=190.0' S=0.0050 '/ Outflow=27.39 cfs 1.132 af
<b>Pond A-L23: A-L23</b>	Peak Elev=52.04' Inflow=27.39 cfs 1.132 af 42.0" Round Culvert x 2.00 n=0.013 L=286.0' S=0.0160 '/ Outflow=27.39 cfs 1.132 af
<b>Pond A-L25: A-L25</b>	Peak Elev=50.81' Inflow=34.08 cfs 1.408 af 42.0" Round Culvert x 2.00 n=0.013 L=163.0' S=0.0040 '/ Outflow=34.08 cfs 1.408 af
<b>Pond A-L27: A-L27</b>	Peak Elev=50.67' Inflow=0.51 cfs 0.021 af 24.0" Round Culvert n=0.013 L=102.0' S=0.0050 '/ Outflow=0.51 cfs 0.021 af
<b>Pond A-L28: A-L28</b>	Peak Elev=50.67' Inflow=34.59 cfs 1.429 af 42.0" Round Culvert x 2.00 n=0.013 L=343.0' S=0.0025 '/ Outflow=34.59 cfs 1.429 af
<b>Pond A-L29: A-L29</b>	Peak Elev=50.70' Inflow=1.27 cfs 0.052 af 18.0" Round Culvert n=0.013 L=233.0' S=0.0045 '/ Outflow=1.27 cfs 0.052 af
<b>Pond A-L30: A-L30</b>	Peak Elev=50.65' Inflow=5.49 cfs 0.227 af 24.0" Round Culvert n=0.013 L=140.0' S=0.0050 '/ Outflow=5.49 cfs 0.227 af
<b>Pond A-L31: A-L31</b>	Peak Elev=50.50' Inflow=40.08 cfs 1.656 af 42.0" Round Culvert x 2.00 n=0.013 L=133.0' S=0.0040 '/ Outflow=40.08 cfs 1.656 af
<b>Pond A-L32: A-L32</b>	Peak Elev=55.11' Inflow=0.49 cfs 0.020 af 18.0" Round Culvert n=0.013 L=36.0' S=0.0050 '/ Outflow=0.49 cfs 0.020 af
<b>Pond A-L33: A-L33</b>	Peak Elev=55.12' Inflow=0.80 cfs 0.033 af 18.0" Round Culvert n=0.013 L=4.0' S=0.0050 '/ Outflow=0.80 cfs 0.033 af
<b>Pond A-L34: A-L34</b>	Peak Elev=55.11' Inflow=1.28 cfs 0.053 af 18.0" Round Culvert n=0.013 L=125.0' S=0.0040 '/ Outflow=1.28 cfs 0.053 af
<b>Pond A-L35: A-L35</b>	Peak Elev=55.27' Inflow=10.22 cfs 0.422 af 30.0" Round Culvert n=0.013 L=4.0' S=0.0050 '/ Outflow=10.22 cfs 0.422 af
<b>Pond A-L36: A-L36</b>	Peak Elev=55.08' Inflow=12.45 cfs 0.515 af 30.0" Round Culvert n=0.013 L=44.0' S=0.0050 '/ Outflow=12.45 cfs 0.515 af



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<b>Pond A-L37: A-L37</b>	Peak Elev=54.80' Inflow=13.08 cfs 0.540 af 30.0" Round Culvert n=0.013 L=692.0' S=0.0050 '/ Outflow=13.08 cfs 0.540 af
<b>Pond A-L38: A-L38</b>	Peak Elev=53.93' Inflow=36.09 cfs 1.491 af 36.0" Round Culvert n=0.013 L=8.0' S=0.0050 '/ Outflow=36.09 cfs 1.491 af
<b>Pond A-L39: A-L39</b>	Peak Elev=52.81' Inflow=36.09 cfs 1.491 af 36.0" Round Culvert n=0.013 L=415.0' S=0.0040 '/ Outflow=36.09 cfs 1.491 af
<b>Pond A-L40: A-L40</b>	Peak Elev=50.98' Inflow=76.13 cfs 3.146 af 42.0" Round Culvert x 2.00 n=0.013 L=175.0' S=0.0040 '/ Outflow=76.13 cfs 3.146 af
<b>Pond A-L41: A-L41</b>	Peak Elev=54.60' Inflow=5.01 cfs 0.207 af 24.0" Round Culvert n=0.013 L=330.0' S=0.0050 '/ Outflow=5.01 cfs 0.207 af
<b>Pond A-L42: A-L42</b>	Peak Elev=54.38' Inflow=11.34 cfs 0.468 af 30.0" Round Culvert n=0.013 L=420.0' S=0.0050 '/ Outflow=11.34 cfs 0.468 af
<b>Pond A-L43: A-L43</b>	Peak Elev=54.55' Inflow=5.01 cfs 0.207 af 24.0" Round Culvert n=0.013 L=290.0' S=0.0050 '/ Outflow=5.01 cfs 0.207 af
<b>Pond A-L44: A-L44</b>	Peak Elev=54.34' Inflow=11.68 cfs 0.483 af 30.0" Round Culvert n=0.013 L=344.0' S=0.0050 '/ Outflow=11.68 cfs 0.483 af
<b>Pond A-L45: A-L45</b>	Peak Elev=51.24' Inflow=0.61 cfs 0.025 af 18.0" Round Culvert n=0.013 L=181.0' S=0.0080 '/ Outflow=0.61 cfs 0.025 af
<b>Pond A-L46: A-L46</b>	Peak Elev=51.23' Inflow=5.58 cfs 0.231 af 24.0" Round Culvert n=0.013 L=289.0' S=0.0090 '/ Outflow=5.58 cfs 0.231 af
<b>Pond A-L47: A-L47</b>	Peak Elev=51.24' Inflow=7.29 cfs 0.301 af 24.0" Round Culvert n=0.013 L=114.0' S=0.0070 '/ Outflow=7.29 cfs 0.301 af
<b>Pond A-L48: A-L48</b>	Peak Elev=78.25' Inflow=0.36 cfs 0.015 af 24.0" Round Culvert n=0.013 L=198.0' S=0.0050 '/ Outflow=0.36 cfs 0.015 af
<b>Pond A-L49: A-L49</b>	Peak Elev=77.45' Inflow=0.61 cfs 0.025 af 24.0" Round Culvert n=0.013 L=198.0' S=0.0050 '/ Outflow=0.61 cfs 0.025 af
<b>Pond A-L50: A-L50</b>	Peak Elev=77.30' Inflow=7.56 cfs 0.312 af 24.0" Round Culvert n=0.013 L=198.0' S=0.0050 '/ Outflow=7.56 cfs 0.312 af
<b>Pond A-L51: A-L51</b>	Peak Elev=76.35' Inflow=7.77 cfs 0.321 af 24.0" Round Culvert n=0.013 L=198.0' S=0.0050 '/ Outflow=7.77 cfs 0.321 af
<b>Pond A-L52: A-L52</b>	Peak Elev=77.24' Inflow=0.62 cfs 0.026 af 24.0" Round Culvert n=0.013 L=160.0' S=0.0099 '/ Outflow=0.62 cfs 0.026 af
<b>Pond A-L53: A-L53</b>	Peak Elev=76.02' Inflow=1.96 cfs 0.081 af 24.0" Round Culvert n=0.013 L=155.0' S=0.0099 '/ Outflow=1.96 cfs 0.081 af
<b>Pond A-L54: A-L54</b>	Peak Elev=75.57' Inflow=11.17 cfs 0.462 af 24.0" Round Culvert n=0.013 L=55.0' S=0.0095 '/ Outflow=11.17 cfs 0.462 af

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<b>Pond A-L55: A-L55</b>	Peak Elev=74.93' Inflow=11.17 cfs 0.462 af 24.0" Round Culvert n=0.013 L=80.0' S=0.0050 '/ Outflow=11.17 cfs 0.462 af
<b>Pond A-L56: A-L56</b>	Peak Elev=74.21' Inflow=13.07 cfs 0.540 af 24.0" Round Culvert n=0.013 L=113.0' S=0.0289 '/ Outflow=13.07 cfs 0.540 af
<b>Pond A-L57: A-L57</b>	Peak Elev=71.12' Inflow=1.93 cfs 0.080 af 24.0" Round Culvert n=0.013 L=63.0' S=0.0100 '/ Outflow=1.93 cfs 0.080 af
<b>Pond A-L58: A-L58</b>	Peak Elev=70.85' Inflow=0.95 cfs 0.039 af 24.0" Round Culvert n=0.013 L=20.0' S=0.0100 '/ Outflow=0.95 cfs 0.039 af
<b>Pond A-L59: A-L59</b>	Peak Elev=70.82' Inflow=2.89 cfs 0.119 af 30.0" Round Culvert n=0.013 L=53.0' S=0.0100 '/ Outflow=2.89 cfs 0.119 af
<b>Pond A-L60: A-L60</b>	Peak Elev=70.71' Inflow=15.96 cfs 0.660 af 30.0" Round Culvert n=0.013 L=408.0' S=0.0100 '/ Outflow=15.96 cfs 0.660 af
<b>Pond A-L61: A-L61</b>	Peak Elev=66.83' Inflow=2.87 cfs 0.118 af 24.0" Round Culvert n=0.013 L=71.0' S=0.0100 '/ Outflow=2.87 cfs 0.118 af
<b>Pond A-L62: A-L62</b>	Peak Elev=66.63' Inflow=1.26 cfs 0.052 af 24.0" Round Culvert n=0.013 L=21.0' S=0.0100 '/ Outflow=1.26 cfs 0.052 af
<b>Pond A-L63: A-L63</b>	Peak Elev=66.61' Inflow=20.09 cfs 0.830 af 36.0" Round Culvert n=0.013 L=440.0' S=0.0100 '/ Outflow=20.09 cfs 0.830 af
<b>Pond A-L64: A-L64</b>	Peak Elev=62.88' Inflow=3.49 cfs 0.144 af 24.0" Round Culvert n=0.013 L=72.0' S=0.0050 '/ Outflow=3.49 cfs 0.144 af
<b>Pond A-L65: A-L65</b>	Peak Elev=62.83' Inflow=1.26 cfs 0.052 af 24.0" Round Culvert n=0.013 L=22.0' S=0.0050 '/ Outflow=1.26 cfs 0.052 af
<b>Pond A-L66: A-L66</b>	Peak Elev=62.82' Inflow=24.84 cfs 1.027 af 36.0" Round Culvert n=0.013 L=98.0' S=0.0060 '/ Outflow=24.84 cfs 1.027 af
<b>Pond A-L67: A-L67</b>	Peak Elev=61.88' Inflow=24.84 cfs 1.027 af 36.0" Round Culvert n=0.013 L=88.0' S=0.0090 '/ Outflow=24.84 cfs 1.027 af
<b>Pond A-L68: A-L68</b>	Peak Elev=60.70' Inflow=24.84 cfs 1.027 af 48.0" Round Culvert n=0.013 L=398.0' S=0.0050 '/ Outflow=24.84 cfs 1.027 af
<b>Pond A-L69: A-L69</b>	Peak Elev=58.44' Inflow=24.84 cfs 1.027 af 48.0" Round Culvert n=0.013 L=240.0' S=0.0129 '/ Outflow=24.84 cfs 1.027 af
<b>Pond A-L70: A-L70</b>	Peak Elev=55.69' Inflow=2.32 cfs 0.096 af 24.0" Round Culvert n=0.013 L=28.0' S=0.0050 '/ Outflow=2.32 cfs 0.096 af
<b>Pond A-L71: A-L71</b>	Peak Elev=55.66' Inflow=27.17 cfs 1.123 af 48.0" Round Culvert n=0.013 L=49.0' S=0.0090 '/ Outflow=27.17 cfs 1.123 af
<b>Pond A-L72: A-L72</b>	Peak Elev=54.80' Inflow=27.17 cfs 1.123 af 48.0" Round Culvert n=0.013 L=340.0' S=0.0159 '/ Outflow=27.17 cfs 1.123 af



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**Pond A-L73: A-L73**Peak Elev=51.27' Inflow=27.17 cfs 1.123 af  
48.0" Round Culvert n=0.013 L=492.0' S=0.0018 '/' Outflow=27.17 cfs 1.123 af**Subcatchment A01: A01**Runoff Area=1.430 ac 0.00% Impervious Runoff Depth=0.69"  
Tc=7.0 min C=0.69 Runoff=1.99 cfs 0.082 af**Subcatchment A02: A02**Runoff Area=0.590 ac 0.00% Impervious Runoff Depth=0.72"  
Tc=7.0 min C=0.72 Runoff=0.86 cfs 0.035 af**Subcatchment A03: A03**Runoff Area=0.340 ac 100.00% Impervious Runoff Depth=0.95"  
Tc=5.0 min C=0.95 Runoff=0.65 cfs 0.027 af**Subcatchment A04: A04**Runoff Area=0.280 ac 100.00% Impervious Runoff Depth=0.95"  
Tc=5.0 min C=0.95 Runoff=0.54 cfs 0.022 af**Subcatchment A05: A05**Runoff Area=4.840 ac 0.00% Impervious Runoff Depth=0.55"  
Tc=23.0 min C=0.55 Runoff=5.37 cfs 0.222 af**Subcatchment A06: A06**Runoff Area=0.260 ac 0.00% Impervious Runoff Depth=0.75"  
Tc=6.0 min C=0.75 Runoff=0.39 cfs 0.016 af**Subcatchment A07: A07**Runoff Area=0.250 ac 0.00% Impervious Runoff Depth=0.64"  
Tc=8.0 min C=0.64 Runoff=0.32 cfs 0.013 af**Subcatchment A08: A08**Runoff Area=3.050 ac 0.00% Impervious Runoff Depth=0.58"  
Tc=20.0 min C=0.58 Runoff=3.57 cfs 0.147 af**Subcatchment A09: A09**Runoff Area=1.030 ac 0.00% Impervious Runoff Depth=0.67"  
Tc=8.0 min C=0.67 Runoff=1.39 cfs 0.058 af**Subcatchment A10: A10**Runoff Area=0.550 ac 0.00% Impervious Runoff Depth=0.88"  
Tc=5.0 min C=0.88 Runoff=0.98 cfs 0.040 af**Subcatchment A11: A11**Runoff Area=1.880 ac 0.00% Impervious Runoff Depth=0.94"  
Tc=5.0 min C=0.94 Runoff=3.56 cfs 0.147 af**Subcatchment A12: A12**Runoff Area=0.540 ac 0.00% Impervious Runoff Depth=0.66"  
Tc=5.0 min C=0.66 Runoff=0.72 cfs 0.030 af**Subcatchment A13: A13**Runoff Area=0.240 ac 0.00% Impervious Runoff Depth=0.83"  
Tc=5.0 min C=0.83 Runoff=0.40 cfs 0.017 af**Subcatchment A14: A14**Runoff Area=0.420 ac 0.00% Impervious Runoff Depth=0.77"  
Tc=6.0 min C=0.77 Runoff=0.65 cfs 0.027 af**Subcatchment A16/17: A16/17**Runoff Area=0.270 ac 0.00% Impervious Runoff Depth=0.93"  
Tc=5.0 min C=0.93 Runoff=0.51 cfs 0.021 af**Subcatchment A18: A18**Runoff Area=3.950 ac 0.00% Impervious Runoff Depth=0.53"  
Tc=18.0 min C=0.53 Runoff=4.22 cfs 0.174 af**Subcatchment A19: A19**Runoff Area=0.370 ac 0.00% Impervious Runoff Depth=0.65"  
Tc=8.0 min C=0.65 Runoff=0.49 cfs 0.020 af

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<b>Subcatchment A20: A20</b>	Runoff Area=0.650 ac 0.00% Impervious Runoff Depth=0.61" Tc=9.0 min C=0.61 Runoff=0.80 cfs 0.033 af
<b>Subcatchment A21: A21</b>	Runoff Area=0.540 ac 0.00% Impervious Runoff Depth=0.87" Tc=5.0 min C=0.87 Runoff=0.95 cfs 0.039 af
<b>Subcatchment A22: A22</b>	Runoff Area=0.440 ac 0.00% Impervious Runoff Depth=0.70" Tc=7.0 min C=0.70 Runoff=0.62 cfs 0.026 af
<b>Subcatchment A23: A23</b>	Runoff Area=3.940 ac 0.00% Impervious Runoff Depth=0.63" Tc=18.0 min C=0.63 Runoff=5.01 cfs 0.207 af
<b>Subcatchment A24: A24</b>	Runoff Area=5.320 ac 0.00% Impervious Runoff Depth=0.59" Tc=23.0 min C=0.59 Runoff=6.33 cfs 0.262 af
<b>Subcatchment A25: A25</b>	Runoff Area=3.820 ac 0.00% Impervious Runoff Depth=0.65" Tc=14.0 min C=0.65 Runoff=5.01 cfs 0.207 af
<b>Subcatchment A26: A26</b>	Runoff Area=5.610 ac 0.00% Impervious Runoff Depth=0.59" Tc=19.0 min C=0.59 Runoff=6.67 cfs 0.276 af
<b>Subcatchment A27: A27</b>	Runoff Area=4.570 ac 0.00% Impervious Runoff Depth=0.54" Tc=20.0 min C=0.54 Runoff=4.98 cfs 0.206 af
<b>Subcatchment A28: A28</b>	Runoff Area=6.820 ac 0.00% Impervious Runoff Depth=0.53" Tc=30.0 min C=0.53 Runoff=7.29 cfs 0.301 af
<b>Subcatchment A29: A29</b>	Runoff Area=0.280 ac 0.00% Impervious Runoff Depth=0.64" Tc=5.0 min C=0.64 Runoff=0.36 cfs 0.015 af
<b>Subcatchment A30: A30</b>	Runoff Area=0.190 ac 0.00% Impervious Runoff Depth=0.64" Tc=5.0 min C=0.64 Runoff=0.25 cfs 0.010 af
<b>Subcatchment A31: A31</b>	Runoff Area=0.180 ac 0.00% Impervious Runoff Depth=0.64" Tc=5.0 min C=0.64 Runoff=0.23 cfs 0.010 af
<b>Subcatchment A32: A32</b>	Runoff Area=0.160 ac 0.00% Impervious Runoff Depth=0.64" Tc=5.0 min C=0.64 Runoff=0.21 cfs 0.009 af
<b>Subcatchment A33: A33</b>	Runoff Area=0.420 ac 0.00% Impervious Runoff Depth=0.73" Tc=7.0 min C=0.73 Runoff=0.62 cfs 0.026 af
<b>Subcatchment A34: A34</b>	Runoff Area=0.980 ac 0.00% Impervious Runoff Depth=0.68" Tc=8.0 min C=0.68 Runoff=1.34 cfs 0.056 af
<b>Subcatchment A35: A35</b>	Runoff Area=1.050 ac 0.00% Impervious Runoff Depth=0.68" Tc=8.0 min C=0.68 Runoff=1.44 cfs 0.059 af
<b>Subcatchment A36/37: A36/37</b>	Runoff Area=1.410 ac 0.00% Impervious Runoff Depth=0.67" Tc=8.0 min C=0.67 Runoff=1.91 cfs 0.079 af
<b>Subcatchment A38: A38</b>	Runoff Area=1.030 ac 0.00% Impervious Runoff Depth=0.93" Tc=5.0 min C=0.93 Runoff=1.93 cfs 0.080 af



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<b>Subcatchment A39: A39</b>	Runoff Area=0.520 ac 0.00% Impervious Runoff Depth=0.91" Tc=5.0 min C=0.91 Runoff=0.95 cfs 0.039 af
<b>Subcatchment A40: A40</b>	Runoff Area=1.870 ac 0.00% Impervious Runoff Depth=0.76" Tc=6.0 min C=0.76 Runoff=2.87 cfs 0.118 af
<b>Subcatchment A41: A41</b>	Runoff Area=0.720 ac 0.00% Impervious Runoff Depth=0.87" Tc=5.0 min C=0.87 Runoff=1.26 cfs 0.052 af
<b>Subcatchment A42: A42</b>	Runoff Area=2.340 ac 0.00% Impervious Runoff Depth=0.74" Tc=6.0 min C=0.74 Runoff=3.49 cfs 0.144 af
<b>Subcatchment A43: A43</b>	Runoff Area=0.720 ac 0.00% Impervious Runoff Depth=0.87" Tc=5.0 min C=0.87 Runoff=1.26 cfs 0.052 af
<b>Subcatchment A44: A44</b>	Runoff Area=1.370 ac 0.00% Impervious Runoff Depth=0.84" Tc=5.0 min C=0.84 Runoff=2.32 cfs 0.096 af
<b>Subcatchment A45: A45</b>	Runoff Area=4.580 ac 0.00% Impervious Runoff Depth=0.65" Tc=5.0 min C=0.65 Runoff=6.00 cfs 0.248 af
<b>Subcatchment A46: A46</b>	Runoff Area=5.050 ac 0.00% Impervious Runoff Depth=0.66" Tc=5.0 min C=0.66 Runoff=6.72 cfs 0.278 af
<b>Subcatchment A47: A47</b>	Runoff Area=5.510 ac 0.00% Impervious Runoff Depth=0.92" Tc=5.0 min C=0.92 Runoff=10.22 cfs 0.422 af
<b>Subcatchment Basin-A1: Basin A1</b>	Runoff Area=5.620 ac 0.00% Impervious Runoff Depth=0.59" Tc=23.0 min C=0.59 Runoff=6.69 cfs 0.276 af
<b>Subcatchment Basin-A2: ABASIN2</b>	Runoff Area=0.750 ac 0.00% Impervious Runoff Depth=0.84" Tc=5.0 min C=0.84 Runoff=1.27 cfs 0.052 af
<b>Subcatchment Basin-A3: Basin A3</b>	Runoff Area=0.450 ac 0.00% Impervious Runoff Depth=0.67" Tc=8.0 min C=0.67 Runoff=0.61 cfs 0.025 af
<b>Pond D-L1: D-L1</b>	Peak Elev=51.82' Inflow=3.05 cfs 0.126 af 18.0" Round Culvert n=0.013 L=50.0' S=0.0960 '/' Outflow=3.05 cfs 0.126 af
<b>Subcatchment D1: D1</b>	Runoff Area=2.910 ac 0.00% Impervious Runoff Depth=0.52" Tc=20.0 min C=0.52 Runoff=3.05 cfs 0.126 af
<b>Pond OUTFALL A: OUTFALL A</b>	Inflow=69.84 cfs 3.568 af Primary=69.84 cfs 3.568 af
<b>Pond Outlet: BasinOutlet</b>	Inflow=49.16 cfs 1.362 af Primary=49.16 cfs 1.362 af

**Total Runoff Area = 90.110 ac Runoff Volume = 4.928 af Average Runoff Depth = 0.66"**  
**99.31% Pervious = 89.490 ac 0.69% Impervious = 0.620 ac**

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**Summary for Pond A-BUBBLER: A-BUBBLER**

Inflow Area = 87.200 ac, 0.71% Impervious, Inflow Depth = 0.66" for 100-Year event  
 Inflow = 116.21 cfs @ 0.50 hrs, Volume= 4.802 af  
 Outflow = 115.95 cfs @ 0.50 hrs, Volume= 4.804 af, Atten= 0%, Lag= 0.0 min  
 Primary = 69.84 cfs @ 0.50 hrs, Volume= 3.568 af  
 Secondary = 46.11 cfs @ 0.50 hrs, Volume= 1.236 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 50.31' @ 0.50 hrs Surf.Area= 15,270 sf Storage= 2,371 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 0.2 min ( 21.8 - 21.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	50.00'	24,687 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
50.00	100	0	0
51.00	49,273	24,687	24,687

Device	Routing	Invert	Outlet Devices
#1	Primary	43.60'	<b>34.0" Round Outfall Pipe</b> L= 108.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 43.60' / 42.53' S= 0.0099 '/ Cc= 0.900 n= 0.013, Flow Area= 6.31 sf
#2	Secondary	50.00'	<b>100.0' long x 500.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=69.84 cfs @ 0.50 hrs HW=50.31' TW=0.00' (Dynamic Tailwater)  
 ↖**1=Outfall Pipe** (Inlet Controls 69.84 cfs @ 11.08 fps)

**Secondary OutFlow** Max=46.11 cfs @ 0.50 hrs HW=50.31' TW=0.00' (Dynamic Tailwater)  
 ↖**2=Broad-Crested Rectangular Weir** (Weir Controls 46.11 cfs @ 1.49 fps)

**Summary for Pond A-L01: A-L01**

Inflow Area = 1.430 ac, 0.00% Impervious, Inflow Depth = 0.69" for 100-Year event  
 Inflow = 1.99 cfs @ 0.12 hrs, Volume= 0.082 af  
 Outflow = 1.99 cfs @ 0.14 hrs, Volume= 0.082 af, Atten= 0%, Lag= 1.2 min  
 Primary = 1.99 cfs @ 0.14 hrs, Volume= 0.082 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 77.48' @ 0.12 hrs  
 Flood Elev= 82.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	76.90'	<b>24.0" Round A-L1</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 76.90' / 74.93' S= 0.0098 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf



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**Primary OutFlow** Max=1.99 cfs @ 0.14 hrs HW=77.48' TW=75.54' (Dynamic Tailwater)↑**1=A-L1** (Inlet Controls 1.99 cfs @ 2.60 fps)**Summary for Pond A-L02: A-L02**

Inflow Area = 2.020 ac, 0.00% Impervious, Inflow Depth = 0.70" for 100-Year event  
 Inflow = 2.85 cfs @ 0.14 hrs, Volume= 0.118 af  
 Outflow = 2.85 cfs @ 0.16 hrs, Volume= 0.118 af, Atten= 0%, Lag= 1.2 min  
 Primary = 2.85 cfs @ 0.16 hrs, Volume= 0.118 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 75.54' @ 0.12 hrs

Flood Elev= 82.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	74.83'	<b>24.0" Round A-L2</b> L= 132.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 74.83' / 73.51' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.85 cfs @ 0.16 hrs HW=75.54' TW=74.12' (Dynamic Tailwater)↑**1=A-L2** (Inlet Controls 2.85 cfs @ 2.86 fps)**Summary for Pond A-L03: A-L03**

Inflow Area = 2.020 ac, 0.00% Impervious, Inflow Depth = 0.70" for 100-Year event  
 Inflow = 2.85 cfs @ 0.16 hrs, Volume= 0.118 af  
 Outflow = 2.85 cfs @ 0.16 hrs, Volume= 0.118 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.85 cfs @ 0.16 hrs, Volume= 0.118 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 74.12' @ 0.12 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	73.41'	<b>24.0" Round Pipe</b> L= 175.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 73.41' / 71.66' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.85 cfs @ 0.16 hrs HW=74.12' TW=72.37' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 2.85 cfs @ 2.86 fps)**Summary for Pond A-L04: A-L04**

Inflow Area = 2.360 ac, 14.41% Impervious, Inflow Depth = 0.73" for 100-Year event  
 Inflow = 3.50 cfs @ 0.16 hrs, Volume= 0.145 af  
 Outflow = 3.50 cfs @ 0.16 hrs, Volume= 0.145 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.50 cfs @ 0.16 hrs, Volume= 0.145 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 72.40' @ 0.40 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	71.56'	<b>24.0" Round A-L4</b> L= 218.0' RCP, sq.cut end projecting, Ke= 0.500

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Inlet / Outlet Invert= 71.56' / 69.38' S= 0.0100 '/ Cc= 0.900  
 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=3.49 cfs @ 0.16 hrs HW=72.37' TW=70.62' (Dynamic Tailwater)

↑**1=A-L4** (Outlet Controls 3.49 cfs @ 4.35 fps)

**Summary for Pond A-L05: A-L05**

Inflow Area = 2.640 ac, 23.48% Impervious, Inflow Depth = 0.76" for 100-Year event  
 Inflow = 4.03 cfs @ 0.14 hrs, Volume= 0.167 af  
 Outflow = 4.03 cfs @ 0.14 hrs, Volume= 0.167 af, Atten= 0%, Lag= 0.0 min  
 Primary = 4.03 cfs @ 0.14 hrs, Volume= 0.167 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 70.91' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	69.28'	<b>24.0" Round A-L5</b> L= 49.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 69.28' / 68.79' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=4.03 cfs @ 0.14 hrs HW=70.60' TW=70.38' (Dynamic Tailwater)

↑**1=A-L5** (Outlet Controls 4.03 cfs @ 2.60 fps)

**Summary for Pond A-L06: A-L06**

Inflow Area = 4.840 ac, 0.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 5.37 cfs @ 0.39 hrs, Volume= 0.222 af  
 Outflow = 5.37 cfs @ 0.43 hrs, Volume= 0.222 af, Atten= 0%, Lag= 2.4 min  
 Primary = 5.37 cfs @ 0.43 hrs, Volume= 0.222 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 70.97' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	69.26'	<b>24.0" Round A-L6</b> L= 37.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 69.26' / 68.89' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=5.37 cfs @ 0.43 hrs HW=70.97' TW=70.78' (Dynamic Tailwater)

↑**1=A-L6** (Outlet Controls 5.37 cfs @ 2.52 fps)

**Summary for Pond A-L07: A-L07**

Inflow Area = 12.060 ac, 5.14% Impervious, Inflow Depth = 0.63" for 100-Year event  
 Inflow = 15.41 cfs @ 0.43 hrs, Volume= 0.637 af  
 Outflow = 15.41 cfs @ 0.39 hrs, Volume= 0.637 af, Atten= 0%, Lag= 0.0 min  
 Primary = 15.41 cfs @ 0.39 hrs, Volume= 0.637 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 70.78' @ 0.39 hrs



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Device	Routing	Invert	Outlet Devices
#1	Primary	68.69'	<b>24.0" Round Pipe</b> L= 139.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 68.69' / 67.30' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=15.41 cfs @ 0.39 hrs HW=70.78' TW=69.29' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 15.41 cfs @ 5.82 fps)

**Summary for Pond A-L08: A-L08**

Inflow Area = 12.320 ac, 5.03% Impervious, Inflow Depth = 0.64" for 100-Year event  
 Inflow = 15.80 cfs @ 0.39 hrs, Volume= 0.653 af  
 Outflow = 15.80 cfs @ 0.43 hrs, Volume= 0.653 af, Atten= 0%, Lag= 2.4 min  
 Primary = 15.80 cfs @ 0.43 hrs, Volume= 0.653 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 69.29' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	67.20'	<b>24.0" Round A-L8</b> L= 196.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 67.20' / 65.24' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=15.80 cfs @ 0.43 hrs HW=69.29' TW=66.82' (Dynamic Tailwater)  
 ↑**1=A-L8** (Inlet Controls 15.80 cfs @ 5.03 fps)

**Summary for Pond A-L09: A-L09**

Inflow Area = 12.570 ac, 4.93% Impervious, Inflow Depth = 0.64" for 100-Year event  
 Inflow = 16.12 cfs @ 0.43 hrs, Volume= 0.666 af  
 Outflow = 16.12 cfs @ 0.43 hrs, Volume= 0.666 af, Atten= 0%, Lag= 0.0 min  
 Primary = 16.12 cfs @ 0.43 hrs, Volume= 0.666 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 66.82' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	65.14'	<b>36.0" Round A-L9</b> L= 98.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 65.14' / 64.16' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=16.12 cfs @ 0.43 hrs HW=66.82' TW=65.64' (Dynamic Tailwater)  
 ↑**1=A-L9** (Outlet Controls 16.12 cfs @ 5.72 fps)

**Summary for Pond A-L10: A-L10**

Inflow Area = 12.570 ac, 4.93% Impervious, Inflow Depth = 0.64" for 100-Year event  
 Inflow = 16.12 cfs @ 0.43 hrs, Volume= 0.666 af  
 Outflow = 16.12 cfs @ 0.43 hrs, Volume= 0.666 af, Atten= 0%, Lag= 0.0 min  
 Primary = 16.12 cfs @ 0.43 hrs, Volume= 0.666 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

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Peak Elev= 65.64' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	64.06'	<b>36.0" Round Pipe</b> L= 195.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 64.06' / 60.74' S= 0.0170 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=16.12 cfs @ 0.43 hrs HW=65.64' TW=62.22' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 16.12 cfs @ 4.28 fps)**Summary for Pond A-L11: A-L11**

Inflow Area = 12.570 ac, 4.93% Impervious, Inflow Depth = 0.64" for 100-Year event  
 Inflow = 16.12 cfs @ 0.43 hrs, Volume= 0.666 af  
 Outflow = 16.12 cfs @ 0.43 hrs, Volume= 0.666 af, Atten= 0%, Lag= 0.0 min  
 Primary = 16.12 cfs @ 0.43 hrs, Volume= 0.666 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 62.22' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	60.64'	<b>36.0" Round Pipe</b> L= 47.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 60.64' / 56.47' S= 0.0887 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=16.12 cfs @ 0.43 hrs HW=62.22' TW=58.64' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 16.12 cfs @ 4.28 fps)**Summary for Pond A-L12: A-L12**

Inflow Area = 15.620 ac, 3.97% Impervious, Inflow Depth = 0.63" for 100-Year event  
 Inflow = 19.69 cfs @ 0.43 hrs, Volume= 0.814 af  
 Outflow = 19.69 cfs @ 0.43 hrs, Volume= 0.814 af, Atten= 0%, Lag= 0.0 min  
 Primary = 19.69 cfs @ 0.43 hrs, Volume= 0.814 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 58.64' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.37'	<b>36.0" Round A-L12</b> L= 48.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.37' / 56.13' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=19.69 cfs @ 0.43 hrs HW=58.64' TW=58.00' (Dynamic Tailwater)↑**1=A-L12** (Outlet Controls 19.69 cfs @ 4.75 fps)**Summary for Pond A-L13: A-L13**

Inflow Area = 1.030 ac, 0.00% Impervious, Inflow Depth = 0.67" for 100-Year event  
 Inflow = 1.39 cfs @ 0.14 hrs, Volume= 0.058 af  
 Outflow = 1.39 cfs @ 0.14 hrs, Volume= 0.058 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.39 cfs @ 0.14 hrs, Volume= 0.058 af



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Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 58.01' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.19'	<b>36.0" Round A-L13</b> L= 12.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.19' / 56.13' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=1.39 cfs @ 0.14 hrs HW=57.73' TW=57.72' (Dynamic Tailwater)↑**1=A-L13** (Outlet Controls 1.39 cfs @ 0.56 fps)**Summary for Pond A-L14: A-L14**

Inflow Area = 2.430 ac, 0.00% Impervious, Inflow Depth = 0.93" for 100-Year event  
 Inflow = 4.54 cfs @ 0.09 hrs, Volume= 0.188 af  
 Outflow = 4.54 cfs @ 0.09 hrs, Volume= 0.188 af, Atten= 0%, Lag= 0.0 min  
 Primary = 4.54 cfs @ 0.09 hrs, Volume= 0.188 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 80.83' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	80.04'	<b>36.0" Round Pipe</b> L= 520.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 80.04' / 56.13' S= 0.0460 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=4.54 cfs @ 0.09 hrs HW=80.83' TW=57.59' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 4.54 cfs @ 3.03 fps)**Summary for Pond A-L15: A-L15**

Inflow Area = 0.550 ac, 0.00% Impervious, Inflow Depth = 0.88" for 100-Year event  
 Inflow = 0.98 cfs @ 0.09 hrs, Volume= 0.040 af  
 Outflow = 0.98 cfs @ 0.09 hrs, Volume= 0.040 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.98 cfs @ 0.09 hrs, Volume= 0.040 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 81.06' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	80.51'	<b>24.0" Round A-L15</b> L= 54.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 80.51' / 80.24' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.98 cfs @ 0.09 hrs HW=81.06' TW=80.83' (Dynamic Tailwater)↑**1=A-L15** (Outlet Controls 0.98 cfs @ 2.11 fps)

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**Summary for Pond A-L16: A-L16**

Inflow Area = 1.880 ac, 0.00% Impervious, Inflow Depth = 0.94" for 100-Year event  
 Inflow = 3.56 cfs @ 0.09 hrs, Volume= 0.147 af  
 Outflow = 3.56 cfs @ 0.09 hrs, Volume= 0.147 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.56 cfs @ 0.09 hrs, Volume= 0.147 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 81.28' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	80.30'	<b>24.0" Round A-L16</b> L= 12.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 80.30' / 80.24' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=3.56 cfs @ 0.09 hrs HW=81.28' TW=80.83' (Dynamic Tailwater)  
 ↑**1=A-L16** (Barrel Controls 3.56 cfs @ 3.40 fps)

**Summary for Pond A-L17: A-L17**

Inflow Area = 19.080 ac, 3.25% Impervious, Inflow Depth = 0.67" for 100-Year event  
 Inflow = 25.62 cfs @ 0.43 hrs, Volume= 1.059 af  
 Outflow = 25.62 cfs @ 0.39 hrs, Volume= 1.059 af, Atten= 0%, Lag= 0.0 min  
 Primary = 25.62 cfs @ 0.39 hrs, Volume= 1.059 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 58.00' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	55.93'	<b>42.0" Round Pipe</b> L= 267.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 55.93' / 54.06' S= 0.0070 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=25.62 cfs @ 0.39 hrs HW=58.00' TW=56.06' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 25.62 cfs @ 6.20 fps)

**Summary for Pond A-L18: A-L18**

Inflow Area = 0.540 ac, 0.00% Impervious, Inflow Depth = 0.66" for 100-Year event  
 Inflow = 0.72 cfs @ 0.09 hrs, Volume= 0.030 af  
 Outflow = 0.72 cfs @ 0.40 hrs, Volume= 0.030 af, Atten= 0%, Lag= 18.6 min  
 Primary = 0.72 cfs @ 0.40 hrs, Volume= 0.030 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 56.06' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	54.70'	<b>24.0" Round A-L18</b> L= 12.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 54.70' / 54.16' S= 0.0450 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.72 cfs @ 0.40 hrs HW=56.06' TW=56.06' (Dynamic Tailwater)  
 ↑**1=A-L18** (Outlet Controls 0.72 cfs @ 0.44 fps)



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**Summary for Pond A-L19: A-L19**

Inflow Area = 0.240 ac, 0.00% Impervious, Inflow Depth = 0.83" for 100-Year event  
 Inflow = 0.40 cfs @ 0.09 hrs, Volume= 0.017 af  
 Outflow = 0.40 cfs @ 0.22 hrs, Volume= 0.017 af, Atten= 0%, Lag= 7.8 min  
 Primary = 0.40 cfs @ 0.22 hrs, Volume= 0.017 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 56.06' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	54.80'	<b>24.0" Round A-L19</b> L= 30.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 54.80' / 54.16' S= 0.0213 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.40 cfs @ 0.22 hrs HW=55.90' TW=55.90' (Dynamic Tailwater)↑**1=A-L19** (Outlet Controls 0.40 cfs @ 0.33 fps)**Summary for Pond A-L20: A-L20**

Inflow Area = 19.860 ac, 3.12% Impervious, Inflow Depth = 0.67" for 100-Year event  
 Inflow = 26.74 cfs @ 0.39 hrs, Volume= 1.105 af  
 Outflow = 26.74 cfs @ 0.39 hrs, Volume= 1.105 af, Atten= 0%, Lag= 0.0 min  
 Primary = 26.74 cfs @ 0.39 hrs, Volume= 1.105 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 56.06' @ 0.39 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	53.96'	<b>42.0" Round Pipe</b> L= 462.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 53.96' / 51.65' S= 0.0050 '/' Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=26.74 cfs @ 0.39 hrs HW=56.06' TW=53.14' (Dynamic Tailwater)↑**1=Pipe** (Barrel Controls 26.74 cfs @ 6.37 fps)**Summary for Pond A-L21: A-L21**

Inflow Area = 0.420 ac, 0.00% Impervious, Inflow Depth = 0.77" for 100-Year event  
 Inflow = 0.65 cfs @ 0.10 hrs, Volume= 0.027 af  
 Outflow = 0.65 cfs @ 0.43 hrs, Volume= 0.027 af, Atten= 0%, Lag= 19.8 min  
 Primary = 0.65 cfs @ 0.43 hrs, Volume= 0.027 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 53.14' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	51.80'	<b>24.0" Round A-L21</b> L= 30.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 51.80' / 51.75' S= 0.0017 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.65 cfs @ 0.43 hrs HW=53.14' TW=53.14' (Dynamic Tailwater)↑**1=A-L21** (Outlet Controls 0.65 cfs @ 0.41 fps)

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**Summary for Pond A-L22: A-L22**

Inflow Area = 20.280 ac, 3.06% Impervious, Inflow Depth = 0.67" for 100-Year event  
 Inflow = 27.39 cfs @ 0.39 hrs, Volume= 1.132 af  
 Outflow = 27.39 cfs @ 0.39 hrs, Volume= 1.132 af, Atten= 0%, Lag= 0.0 min  
 Primary = 27.39 cfs @ 0.39 hrs, Volume= 1.132 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 53.14' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	51.55'	<b>42.0" Round Pipe X 2.00</b> L= 190.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 51.55' / 50.60' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=27.39 cfs @ 0.39 hrs HW=53.14' TW=52.04' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 27.39 cfs @ 4.75 fps)**Summary for Pond A-L23: A-L23**

Inflow Area = 20.280 ac, 3.06% Impervious, Inflow Depth = 0.67" for 100-Year event  
 Inflow = 27.39 cfs @ 0.39 hrs, Volume= 1.132 af  
 Outflow = 27.39 cfs @ 0.39 hrs, Volume= 1.132 af, Atten= 0%, Lag= 0.0 min  
 Primary = 27.39 cfs @ 0.39 hrs, Volume= 1.132 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 52.04' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	50.40'	<b>42.0" Round Pipe X 2.00</b> L= 286.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 50.40' / 45.83' S= 0.0160 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=27.38 cfs @ 0.39 hrs HW=52.04' TW=50.80' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 27.38 cfs @ 4.55 fps)**Summary for Pond A-L25: A-L25**

Inflow Area = 25.900 ac, 2.39% Impervious, Inflow Depth = 0.65" for 100-Year event  
 Inflow = 34.08 cfs @ 0.39 hrs, Volume= 1.408 af  
 Outflow = 34.08 cfs @ 0.43 hrs, Volume= 1.408 af, Atten= 0%, Lag= 2.4 min  
 Primary = 34.08 cfs @ 0.43 hrs, Volume= 1.408 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 50.81' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	45.73'	<b>42.0" Round Pipe X 2.00</b> L= 163.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 45.73' / 45.08' S= 0.0040 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf



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**Primary OutFlow** Max=33.97 cfs @ 0.43 hrs HW=50.80' TW=50.67' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 33.97 cfs @ 1.77 fps)**Summary for Pond A-L27: A-L27**

Inflow Area = 0.270 ac, 0.00% Impervious, Inflow Depth = 0.93" for 100-Year event  
 Inflow = 0.51 cfs @ 0.09 hrs, Volume= 0.021 af  
 Outflow = 0.51 cfs @ 0.10 hrs, Volume= 0.021 af, Atten= 0%, Lag= 0.6 min  
 Primary = 0.51 cfs @ 0.10 hrs, Volume= 0.021 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 50.67' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	45.59'	<b>24.0" Round A-L27</b> L= 102.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 45.59' / 45.08' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.00 cfs @ 0.10 hrs HW=50.18' TW=50.21' (Dynamic Tailwater)↑**1=A-L27** ( Controls 0.00 cfs)**Summary for Pond A-L28: A-L28**

Inflow Area = 26.170 ac, 2.37% Impervious, Inflow Depth = 0.66" for 100-Year event  
 Inflow = 34.59 cfs @ 0.43 hrs, Volume= 1.429 af  
 Outflow = 34.59 cfs @ 0.43 hrs, Volume= 1.429 af, Atten= 0%, Lag= 0.0 min  
 Primary = 34.59 cfs @ 0.43 hrs, Volume= 1.429 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 50.67' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	44.98'	<b>42.0" Round Pipe X 2.00</b> L= 343.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 44.98' / 44.13' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=34.59 cfs @ 0.43 hrs HW=50.67' TW=50.49' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 34.59 cfs @ 1.80 fps)**Summary for Pond A-L29: A-L29**

Inflow Area = 0.750 ac, 0.00% Impervious, Inflow Depth = 0.84" for 100-Year event  
 Inflow = 1.27 cfs @ 0.09 hrs, Volume= 0.052 af  
 Outflow = 1.27 cfs @ 0.12 hrs, Volume= 0.052 af, Atten= 0%, Lag= 1.8 min  
 Primary = 1.27 cfs @ 0.12 hrs, Volume= 0.052 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 50.70' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	47.25'	<b>18.0" Round A-L29</b> L= 233.0' RCP, sq.cut end projecting, Ke= 0.500

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Inlet / Outlet Invert= 47.25' / 46.20' S= 0.0045 '/' Cc= 0.900  
 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=0.99 cfs @ 0.12 hrs HW=50.26' TW=50.23' (Dynamic Tailwater)

↑**1=A-L29** (Outlet Controls 0.99 cfs @ 0.56 fps)

**Summary for Pond A-L30: A-L30**

Inflow Area = 4.700 ac, 0.00% Impervious, Inflow Depth = 0.58" for 100-Year event  
 Inflow = 5.49 cfs @ 0.30 hrs, Volume= 0.227 af  
 Outflow = 5.49 cfs @ 0.31 hrs, Volume= 0.227 af, Atten= 0%, Lag= 0.6 min  
 Primary = 5.49 cfs @ 0.31 hrs, Volume= 0.227 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 50.65' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	46.10'	<b>24.0" Round A-L30</b> L= 140.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 46.10' / 45.40' S= 0.0050 '/' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=5.49 cfs @ 0.31 hrs HW=50.59' TW=50.43' (Dynamic Tailwater)

↑**1=A-L30** (Outlet Controls 5.49 cfs @ 1.75 fps)

**Summary for Pond A-L31: A-L31**

Inflow Area = 30.870 ac, 2.01% Impervious, Inflow Depth = 0.64" for 100-Year event  
 Inflow = 40.08 cfs @ 0.43 hrs, Volume= 1.656 af  
 Outflow = 40.08 cfs @ 0.43 hrs, Volume= 1.656 af, Atten= 0%, Lag= 0.0 min  
 Primary = 40.08 cfs @ 0.43 hrs, Volume= 1.656 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 50.50' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	44.13'	<b>42.0" Round Pipe X 2.00</b> L= 133.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 44.13' / 43.60' S= 0.0040 '/' Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=40.08 cfs @ 0.43 hrs HW=50.49' TW=50.30' (Dynamic Tailwater)

↑**1=Pipe** (Inlet Controls 40.08 cfs @ 2.08 fps)

**Summary for Pond A-L32: A-L32**

Inflow Area = 0.370 ac, 0.00% Impervious, Inflow Depth = 0.65" for 100-Year event  
 Inflow = 0.49 cfs @ 0.14 hrs, Volume= 0.020 af  
 Outflow = 0.49 cfs @ 0.45 hrs, Volume= 0.020 af, Atten= 0%, Lag= 18.6 min  
 Primary = 0.49 cfs @ 0.45 hrs, Volume= 0.020 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3



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Peak Elev= 55.11' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	51.46'	<b>18.0" Round A-L32</b> L= 36.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 51.46' / 51.28' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=0.22 cfs @ 0.45 hrs HW=55.10' TW=55.10' (Dynamic Tailwater)↑**1=A-L32** (Inlet Controls 0.22 cfs @ 0.13 fps)**Summary for Pond A-L33: A-L33**

Inflow Area = 0.650 ac, 0.00% Impervious, Inflow Depth = 0.61" for 100-Year event  
 Inflow = 0.80 cfs @ 0.15 hrs, Volume= 0.033 af  
 Outflow = 0.80 cfs @ 0.21 hrs, Volume= 0.033 af, Atten= 0%, Lag= 3.6 min  
 Primary = 0.80 cfs @ 0.21 hrs, Volume= 0.033 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 55.12' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	51.30'	<b>18.0" Round A-L33</b> L= 4.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 51.30' / 51.28' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=0.00 cfs @ 0.21 hrs HW=53.53' TW=53.54' (Dynamic Tailwater)↑**1=A-L33** ( Controls 0.00 cfs)**Summary for Pond A-L34: A-L34**

Inflow Area = 1.020 ac, 0.00% Impervious, Inflow Depth = 0.62" for 100-Year event  
 Inflow = 1.28 cfs @ 0.21 hrs, Volume= 0.053 af  
 Outflow = 1.28 cfs @ 0.28 hrs, Volume= 0.053 af, Atten= 0%, Lag= 4.2 min  
 Primary = 1.28 cfs @ 0.28 hrs, Volume= 0.053 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 55.11' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	51.08'	<b>18.0" Round Pipe</b> L= 125.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 51.08' / 50.58' S= 0.0040 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=0.00 cfs @ 0.28 hrs HW=54.39' TW=54.40' (Dynamic Tailwater)↑**1=Pipe** ( Controls 0.00 cfs)**Summary for Pond A-L35: A-L35**

Inflow Area = 5.510 ac, 0.00% Impervious, Inflow Depth = 0.92" for 100-Year event  
 Inflow = 10.22 cfs @ 0.09 hrs, Volume= 0.422 af  
 Outflow = 10.22 cfs @ 0.41 hrs, Volume= 0.422 af, Atten= 0%, Lag= 19.2 min  
 Primary = 10.22 cfs @ 0.41 hrs, Volume= 0.422 af

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Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 55.27' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	50.40'	<b>30.0" Round A-L35</b> L= 4.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 50.40' / 50.38' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=10.17 cfs @ 0.41 hrs HW=55.24' TW=55.05' (Dynamic Tailwater)↑**1=A-L35** (Inlet Controls 10.17 cfs @ 2.07 fps)**Summary for Pond A-L36: A-L36**

Inflow Area = 7.070 ac, 0.00% Impervious, Inflow Depth = 0.87" for 100-Year event  
 Inflow = 12.45 cfs @ 0.41 hrs, Volume= 0.515 af  
 Outflow = 12.45 cfs @ 0.50 hrs, Volume= 0.515 af, Atten= 0%, Lag= 5.4 min  
 Primary = 12.45 cfs @ 0.50 hrs, Volume= 0.515 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 55.08' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	50.28'	<b>30.0" Round Pipe</b> L= 44.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 50.28' / 50.06' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=12.45 cfs @ 0.50 hrs HW=55.08' TW=54.80' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 12.45 cfs @ 2.54 fps)**Summary for Pond A-L37: A-L37**

Inflow Area = 7.510 ac, 0.00% Impervious, Inflow Depth = 0.86" for 100-Year event  
 Inflow = 13.08 cfs @ 0.50 hrs, Volume= 0.540 af  
 Outflow = 13.08 cfs @ 0.50 hrs, Volume= 0.540 af, Atten= 0%, Lag= 0.0 min  
 Primary = 13.08 cfs @ 0.50 hrs, Volume= 0.540 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 54.80' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	49.96'	<b>30.0" Round A-L37</b> L= 692.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 49.96' / 46.50' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=13.06 cfs @ 0.50 hrs HW=54.80' TW=53.93' (Dynamic Tailwater)↑**1=A-L37** (Outlet Controls 13.06 cfs @ 2.66 fps)



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**Summary for Pond A-L38: A-L38**

Inflow Area = 26.200 ac, 0.00% Impervious, Inflow Depth = 0.68" for 100-Year event  
 Inflow = 36.09 cfs @ 0.40 hrs, Volume= 1.491 af  
 Outflow = 36.09 cfs @ 0.45 hrs, Volume= 1.491 af, Atten= 0%, Lag= 3.0 min  
 Primary = 36.09 cfs @ 0.45 hrs, Volume= 1.491 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 53.93' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	46.40'	<b>36.0" Round Pipe</b> L= 8.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 46.40' / 46.36' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=36.08 cfs @ 0.45 hrs HW=53.92' TW=52.79' (Dynamic Tailwater)  
 ↑**1=Pipe** (Inlet Controls 36.08 cfs @ 5.10 fps)

**Summary for Pond A-L39: A-L39**

Inflow Area = 26.200 ac, 0.00% Impervious, Inflow Depth = 0.68" for 100-Year event  
 Inflow = 36.09 cfs @ 0.45 hrs, Volume= 1.491 af  
 Outflow = 36.09 cfs @ 0.45 hrs, Volume= 1.491 af, Atten= 0%, Lag= 0.0 min  
 Primary = 36.09 cfs @ 0.45 hrs, Volume= 1.491 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 52.81' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	46.06'	<b>36.0" Round Pipe</b> L= 415.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 46.06' / 44.40' S= 0.0040 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=36.09 cfs @ 0.45 hrs HW=52.79' TW=50.97' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 36.09 cfs @ 5.11 fps)

**Summary for Pond A-L40: A-L40**

Inflow Area = 56.330 ac, 0.00% Impervious, Inflow Depth = 0.67" for 100-Year event  
 Inflow = 76.13 cfs @ 0.50 hrs, Volume= 3.146 af  
 Outflow = 76.13 cfs @ 0.50 hrs, Volume= 3.146 af, Atten= 0%, Lag= 0.0 min  
 Primary = 76.13 cfs @ 0.50 hrs, Volume= 3.146 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 50.98' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	44.30'	<b>42.0" Round Pipe X 2.00</b> L= 175.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 44.30' / 43.60' S= 0.0040 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

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**Primary OutFlow** Max=76.13 cfs @ 0.50 hrs HW=50.98' TW=50.31' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 76.13 cfs @ 3.96 fps)**Summary for Pond A-L41: A-L41**

Inflow Area = 3.940 ac, 0.00% Impervious, Inflow Depth = 0.63" for 100-Year event  
 Inflow = 5.01 cfs @ 0.30 hrs, Volume= 0.207 af  
 Outflow = 5.01 cfs @ 0.37 hrs, Volume= 0.207 af, Atten= 0%, Lag= 4.2 min  
 Primary = 5.01 cfs @ 0.37 hrs, Volume= 0.207 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 54.60' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	50.35'	<b>24.0" Round A-L41</b> L= 330.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 50.35' / 48.70' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=4.81 cfs @ 0.37 hrs HW=54.47' TW=54.27' (Dynamic Tailwater)↑**1=A-L41** (Outlet Controls 4.81 cfs @ 1.53 fps)**Summary for Pond A-L42: A-L42**

Inflow Area = 9.260 ac, 0.00% Impervious, Inflow Depth = 0.61" for 100-Year event  
 Inflow = 11.34 cfs @ 0.39 hrs, Volume= 0.468 af  
 Outflow = 11.34 cfs @ 0.39 hrs, Volume= 0.468 af, Atten= 0%, Lag= 0.0 min  
 Primary = 11.34 cfs @ 0.39 hrs, Volume= 0.468 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 54.38' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	48.60'	<b>30.0" Round A-L42</b> L= 420.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 48.60' / 46.50' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=11.29 cfs @ 0.39 hrs HW=54.34' TW=53.90' (Dynamic Tailwater)↑**1=A-L42** (Outlet Controls 11.29 cfs @ 2.30 fps)**Summary for Pond A-L43: A-L43**

Inflow Area = 3.820 ac, 0.00% Impervious, Inflow Depth = 0.65" for 100-Year event  
 Inflow = 5.01 cfs @ 0.24 hrs, Volume= 0.207 af  
 Outflow = 5.01 cfs @ 0.24 hrs, Volume= 0.207 af, Atten= 0%, Lag= 0.0 min  
 Primary = 5.01 cfs @ 0.24 hrs, Volume= 0.207 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 54.55' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	49.77'	<b>24.0" Round A-L43</b> L= 290.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 49.77' / 48.32' S= 0.0050 '/ Cc= 0.900



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n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=4.30 cfs @ 0.24 hrs HW=53.35' TW=53.21' (Dynamic Tailwater)↑**1=A-L43** (Outlet Controls 4.30 cfs @ 1.37 fps)**Summary for Pond A-L44: A-L44**

Inflow Area = 9.430 ac, 0.00% Impervious, Inflow Depth = 0.61" for 100-Year event  
 Inflow = 11.68 cfs @ 0.32 hrs, Volume= 0.483 af  
 Outflow = 11.68 cfs @ 0.32 hrs, Volume= 0.483 af, Atten= 0%, Lag= 0.0 min  
 Primary = 11.68 cfs @ 0.32 hrs, Volume= 0.483 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 54.34' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	48.22'	<b>30.0" Round A-L44</b> L= 344.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 48.22' / 46.50' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=11.54 cfs @ 0.32 hrs HW=54.06' TW=53.66' (Dynamic Tailwater)↑**1=A-L44** (Outlet Controls 11.54 cfs @ 2.35 fps)**Summary for Pond A-L45: A-L45**

Inflow Area = 0.450 ac, 0.00% Impervious, Inflow Depth = 0.67" for 100-Year event  
 Inflow = 0.61 cfs @ 0.14 hrs, Volume= 0.025 af  
 Outflow = 0.61 cfs @ 0.44 hrs, Volume= 0.025 af, Atten= 0%, Lag= 18.0 min  
 Primary = 0.61 cfs @ 0.44 hrs, Volume= 0.025 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 51.24' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	48.55'	<b>18.0" Round A-L45</b> L= 181.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 48.55' / 47.10' S= 0.0080 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=0.58 cfs @ 0.44 hrs HW=51.22' TW=51.21' (Dynamic Tailwater)↑**1=A-L45** (Outlet Controls 0.58 cfs @ 0.33 fps)**Summary for Pond A-L46: A-L46**

Inflow Area = 5.020 ac, 0.00% Impervious, Inflow Depth = 0.55" for 100-Year event  
 Inflow = 5.58 cfs @ 0.34 hrs, Volume= 0.231 af  
 Outflow = 5.58 cfs @ 0.44 hrs, Volume= 0.231 af, Atten= 0%, Lag= 6.0 min  
 Primary = 5.58 cfs @ 0.44 hrs, Volume= 0.231 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 51.23' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	47.00'	<b>24.0" Round A-L46</b> L= 289.0' RCP, sq.cut end projecting, Ke= 0.500

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Inlet / Outlet Invert= 47.00' / 44.40' S= 0.0090 ' /' Cc= 0.900  
 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=5.58 cfs @ 0.44 hrs HW=51.21' TW=50.96' (Dynamic Tailwater)

↑1=A-L46 (Outlet Controls 5.58 cfs @ 1.78 fps)

**Summary for Pond A-L47: A-L47**

Inflow Area = 6.820 ac, 0.00% Impervious, Inflow Depth = 0.53" for 100-Year event  
 Inflow = 7.29 cfs @ 0.50 hrs, Volume= 0.301 af  
 Outflow = 7.29 cfs @ 0.50 hrs, Volume= 0.301 af, Atten= 0%, Lag= 0.0 min  
 Primary = 7.29 cfs @ 0.50 hrs, Volume= 0.301 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 51.24' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	45.20'	<b>24.0" Round A-L47</b> L= 114.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 45.20' / 44.40' S= 0.0070 ' /' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=7.29 cfs @ 0.50 hrs HW=51.23' TW=50.98' (Dynamic Tailwater)

↑1=A-L47 (Outlet Controls 7.29 cfs @ 2.32 fps)

**Summary for Pond A-L48: A-L48**

Inflow Area = 0.280 ac, 0.00% Impervious, Inflow Depth = 0.64" for 100-Year event  
 Inflow = 0.36 cfs @ 0.09 hrs, Volume= 0.015 af  
 Outflow = 0.36 cfs @ 0.15 hrs, Volume= 0.015 af, Atten= 0%, Lag= 3.6 min  
 Primary = 0.36 cfs @ 0.15 hrs, Volume= 0.015 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 78.25' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	77.95'	<b>24.0" Round A-L48</b> L= 198.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 77.95' / 76.96' S= 0.0050 ' /' Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.36 cfs @ 0.15 hrs HW=78.25' TW=77.45' (Dynamic Tailwater)

↑1=A-L48 (Outlet Controls 0.36 cfs @ 1.87 fps)

**Summary for Pond A-L49: A-L49**

Inflow Area = 0.470 ac, 0.00% Impervious, Inflow Depth = 0.64" for 100-Year event  
 Inflow = 0.61 cfs @ 0.15 hrs, Volume= 0.025 af  
 Outflow = 0.61 cfs @ 0.15 hrs, Volume= 0.025 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.61 cfs @ 0.15 hrs, Volume= 0.025 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 77.45' @ 0.15 hrs



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Device	Routing	Invert	Outlet Devices
#1	Primary	76.86'	<b>24.0" Round A-L49</b> L= 198.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 76.86' / 75.87' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.61 cfs @ 0.15 hrs HW=77.45' TW=77.30' (Dynamic Tailwater)↑**1=A-L49** (Outlet Controls 0.61 cfs @ 1.17 fps)**Summary for Pond A-L50: A-L50**

Inflow Area = 5.700 ac, 0.00% Impervious, Inflow Depth = 0.66" for 100-Year event  
 Inflow = 7.56 cfs @ 0.15 hrs, Volume= 0.312 af  
 Outflow = 7.56 cfs @ 0.15 hrs, Volume= 0.312 af, Atten= 0%, Lag= 0.0 min  
 Primary = 7.56 cfs @ 0.15 hrs, Volume= 0.312 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 77.30' @ 0.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	75.77'	<b>24.0" Round A-L50</b> L= 198.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 75.77' / 74.78' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=7.56 cfs @ 0.15 hrs HW=77.30' TW=76.35' (Dynamic Tailwater)↑**1=A-L50** (Outlet Controls 7.56 cfs @ 4.06 fps)**Summary for Pond A-L51: A-L51**

Inflow Area = 5.860 ac, 0.00% Impervious, Inflow Depth = 0.66" for 100-Year event  
 Inflow = 7.77 cfs @ 0.15 hrs, Volume= 0.321 af  
 Outflow = 7.77 cfs @ 0.12 hrs, Volume= 0.321 af, Atten= 0%, Lag= 0.0 min  
 Primary = 7.77 cfs @ 0.12 hrs, Volume= 0.321 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 76.35' @ 0.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	74.68'	<b>24.0" Round A-L51</b> L= 198.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 74.68' / 73.69' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=7.71 cfs @ 0.12 hrs HW=76.32' TW=75.51' (Dynamic Tailwater)↑**1=A-L51** (Outlet Controls 7.71 cfs @ 3.80 fps)**Summary for Pond A-L52: A-L52**

Inflow Area = 0.420 ac, 0.00% Impervious, Inflow Depth = 0.73" for 100-Year event  
 Inflow = 0.62 cfs @ 0.12 hrs, Volume= 0.026 af  
 Outflow = 0.62 cfs @ 0.12 hrs, Volume= 0.026 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.62 cfs @ 0.12 hrs, Volume= 0.026 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

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Peak Elev= 77.24' @ 0.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	76.90'	<b>24.0" Round A-L52</b> L= 160.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 76.90' / 75.32' S= 0.0099 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.62 cfs @ 0.12 hrs HW=77.23' TW=75.98' (Dynamic Tailwater)↑**1=A-L52** (Outlet Controls 0.62 cfs @ 2.73 fps)**Summary for Pond A-L53: A-L53**

Inflow Area = 1.400 ac, 0.00% Impervious, Inflow Depth = 0.69" for 100-Year event  
 Inflow = 1.96 cfs @ 0.14 hrs, Volume= 0.081 af  
 Outflow = 1.96 cfs @ 0.16 hrs, Volume= 0.081 af, Atten= 0%, Lag= 1.2 min  
 Primary = 1.96 cfs @ 0.16 hrs, Volume= 0.081 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 76.02' @ 0.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	75.22'	<b>24.0" Round A-L53</b> L= 155.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 75.22' / 73.69' S= 0.0099 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.96 cfs @ 0.16 hrs HW=76.02' TW=75.57' (Dynamic Tailwater)↑**1=A-L53** (Outlet Controls 1.96 cfs @ 2.48 fps)**Summary for Pond A-L54: A-L54**

Inflow Area = 8.310 ac, 0.00% Impervious, Inflow Depth = 0.67" for 100-Year event  
 Inflow = 11.17 cfs @ 0.14 hrs, Volume= 0.462 af  
 Outflow = 11.17 cfs @ 0.14 hrs, Volume= 0.462 af, Atten= 0%, Lag= 0.0 min  
 Primary = 11.17 cfs @ 0.14 hrs, Volume= 0.462 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 75.57' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	73.59'	<b>24.0" Round A-L54</b> L= 55.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 73.59' / 73.07' S= 0.0095 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=11.17 cfs @ 0.14 hrs HW=75.57' TW=74.93' (Dynamic Tailwater)↑**1=A-L54** (Outlet Controls 11.17 cfs @ 4.47 fps)**Summary for Pond A-L55: A-L55**

Inflow Area = 8.310 ac, 0.00% Impervious, Inflow Depth = 0.67" for 100-Year event  
 Inflow = 11.17 cfs @ 0.14 hrs, Volume= 0.462 af  
 Outflow = 11.17 cfs @ 0.14 hrs, Volume= 0.462 af, Atten= 0%, Lag= 0.0 min  
 Primary = 11.17 cfs @ 0.14 hrs, Volume= 0.462 af



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Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 74.93' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	72.97'	<b>24.0" Round Pipe</b> L= 80.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 72.97' / 72.57' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=11.17 cfs @ 0.14 hrs HW=74.93' TW=74.21' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 11.17 cfs @ 4.51 fps)**Summary for Pond A-L56: A-L56**

Inflow Area = 9.720 ac, 0.00% Impervious, Inflow Depth = 0.67" for 100-Year event  
 Inflow = 13.07 cfs @ 0.14 hrs, Volume= 0.540 af  
 Outflow = 13.07 cfs @ 0.15 hrs, Volume= 0.540 af, Atten= 0%, Lag= 0.6 min  
 Primary = 13.07 cfs @ 0.15 hrs, Volume= 0.540 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 74.21' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	72.47'	<b>24.0" Round A-L56</b> L= 113.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 72.47' / 69.20' S= 0.0289 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=13.07 cfs @ 0.15 hrs HW=74.21' TW=70.71' (Dynamic Tailwater)↑**1=A-L56** (Inlet Controls 13.07 cfs @ 4.50 fps)**Summary for Pond A-L57: A-L57**

Inflow Area = 1.030 ac, 0.00% Impervious, Inflow Depth = 0.93" for 100-Year event  
 Inflow = 1.93 cfs @ 0.09 hrs, Volume= 0.080 af  
 Outflow = 1.93 cfs @ 0.13 hrs, Volume= 0.080 af, Atten= 0%, Lag= 2.4 min  
 Primary = 1.93 cfs @ 0.13 hrs, Volume= 0.080 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 71.12' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	70.36'	<b>24.0" Round A-L57</b> L= 63.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 70.36' / 69.73' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.93 cfs @ 0.13 hrs HW=71.12' TW=70.82' (Dynamic Tailwater)↑**1=A-L57** (Outlet Controls 1.93 cfs @ 2.61 fps)

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**Summary for Pond A-L58: A-L58**

Inflow Area = 0.520 ac, 0.00% Impervious, Inflow Depth = 0.91" for 100-Year event  
 Inflow = 0.95 cfs @ 0.09 hrs, Volume= 0.039 af  
 Outflow = 0.95 cfs @ 0.12 hrs, Volume= 0.039 af, Atten= 0%, Lag= 1.8 min  
 Primary = 0.95 cfs @ 0.12 hrs, Volume= 0.039 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 70.85' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	69.93'	<b>24.0" Round A-L58</b> L= 20.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 69.93' / 69.73' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.95 cfs @ 0.12 hrs HW=70.83' TW=70.80' (Dynamic Tailwater)↑**1=A-L58** (Outlet Controls 0.95 cfs @ 1.02 fps)**Summary for Pond A-L59: A-L59**

Inflow Area = 1.550 ac, 0.00% Impervious, Inflow Depth = 0.92" for 100-Year event  
 Inflow = 2.89 cfs @ 0.09 hrs, Volume= 0.119 af  
 Outflow = 2.89 cfs @ 0.11 hrs, Volume= 0.119 af, Atten= 0%, Lag= 1.2 min  
 Primary = 2.89 cfs @ 0.11 hrs, Volume= 0.119 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 70.82' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	69.63'	<b>30.0" Round Pipe</b> L= 53.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 69.63' / 69.10' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=2.89 cfs @ 0.11 hrs HW=70.78' TW=70.65' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 2.89 cfs @ 1.92 fps)**Summary for Pond A-L60: A-L60**

Inflow Area = 11.270 ac, 0.00% Impervious, Inflow Depth = 0.70" for 100-Year event  
 Inflow = 15.96 cfs @ 0.15 hrs, Volume= 0.660 af  
 Outflow = 15.96 cfs @ 0.15 hrs, Volume= 0.660 af, Atten= 0%, Lag= 0.0 min  
 Primary = 15.96 cfs @ 0.15 hrs, Volume= 0.660 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 70.71' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	69.00'	<b>30.0" Round Pipe</b> L= 408.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 69.00' / 64.92' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=15.96 cfs @ 0.15 hrs HW=70.71' TW=66.61' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 15.96 cfs @ 4.45 fps)



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**Summary for Pond A-L61: A-L61**

Inflow Area = 1.870 ac, 0.00% Impervious, Inflow Depth = 0.76" for 100-Year event  
 Inflow = 2.87 cfs @ 0.10 hrs, Volume= 0.118 af  
 Outflow = 2.87 cfs @ 0.10 hrs, Volume= 0.118 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.87 cfs @ 0.10 hrs, Volume= 0.118 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 66.83' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	65.73'	<b>24.0" Round A-L61</b> L= 71.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 65.73' / 65.02' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.87 cfs @ 0.10 hrs HW=66.79' TW=66.55' (Dynamic Tailwater)  
 ↑**1=A-L61** (Outlet Controls 2.87 cfs @ 2.46 fps)

**Summary for Pond A-L62: A-L62**

Inflow Area = 0.720 ac, 0.00% Impervious, Inflow Depth = 0.87" for 100-Year event  
 Inflow = 1.26 cfs @ 0.09 hrs, Volume= 0.052 af  
 Outflow = 1.26 cfs @ 0.09 hrs, Volume= 0.052 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.26 cfs @ 0.09 hrs, Volume= 0.052 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 66.63' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	65.23'	<b>24.0" Round A-L62</b> L= 21.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 65.23' / 65.02' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.26 cfs @ 0.09 hrs HW=66.53' TW=66.51' (Dynamic Tailwater)  
 ↑**1=A-L62** (Outlet Controls 1.26 cfs @ 0.83 fps)

**Summary for Pond A-L63: A-L63**

Inflow Area = 13.860 ac, 0.00% Impervious, Inflow Depth = 0.72" for 100-Year event  
 Inflow = 20.09 cfs @ 0.15 hrs, Volume= 0.830 af  
 Outflow = 20.09 cfs @ 0.15 hrs, Volume= 0.830 af, Atten= 0%, Lag= 0.0 min  
 Primary = 20.09 cfs @ 0.15 hrs, Volume= 0.830 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 66.61' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	64.82'	<b>36.0" Round Pipe</b> L= 440.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 64.82' / 60.42' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=20.09 cfs @ 0.15 hrs HW=66.61' TW=62.82' (Dynamic Tailwater)  
 ↑**1=Pipe** (Inlet Controls 20.09 cfs @ 4.56 fps)

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**Summary for Pond A-L64: A-L64**

Inflow Area = 2.340 ac, 0.00% Impervious, Inflow Depth = 0.74" for 100-Year event  
 Inflow = 3.49 cfs @ 0.10 hrs, Volume= 0.144 af  
 Outflow = 3.49 cfs @ 0.13 hrs, Volume= 0.144 af, Atten= 0%, Lag= 1.8 min  
 Primary = 3.49 cfs @ 0.13 hrs, Volume= 0.144 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 62.88' @ 0.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	60.88'	<b>24.0" Round A-L64</b> L= 72.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 60.88' / 60.52' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=3.42 cfs @ 0.13 hrs HW=62.87' TW=62.81' (Dynamic Tailwater)↑**1=A-L64** (Outlet Controls 3.42 cfs @ 1.36 fps)**Summary for Pond A-L65: A-L65**

Inflow Area = 0.720 ac, 0.00% Impervious, Inflow Depth = 0.87" for 100-Year event  
 Inflow = 1.26 cfs @ 0.09 hrs, Volume= 0.052 af  
 Outflow = 1.26 cfs @ 0.09 hrs, Volume= 0.052 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.26 cfs @ 0.09 hrs, Volume= 0.052 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 62.83' @ 0.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	60.63'	<b>24.0" Round A-L65</b> L= 22.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 60.63' / 60.52' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.00 cfs @ 0.09 hrs HW=62.65' TW=62.66' (Dynamic Tailwater)↑**1=A-L65** ( Controls 0.00 cfs)**Summary for Pond A-L66: A-L66**

Inflow Area = 16.920 ac, 0.00% Impervious, Inflow Depth = 0.73" for 100-Year event  
 Inflow = 24.84 cfs @ 0.15 hrs, Volume= 1.027 af  
 Outflow = 24.84 cfs @ 0.15 hrs, Volume= 1.027 af, Atten= 0%, Lag= 0.0 min  
 Primary = 24.84 cfs @ 0.15 hrs, Volume= 1.027 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 62.82' @ 0.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	60.32'	<b>36.0" Round Pipe</b> L= 98.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 60.32' / 59.73' S= 0.0060 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=24.84 cfs @ 0.15 hrs HW=62.82' TW=61.88' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 24.84 cfs @ 5.35 fps)



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**Summary for Pond A-L67: A-L67**

Inflow Area = 16.920 ac, 0.00% Impervious, Inflow Depth = 0.73" for 100-Year event  
 Inflow = 24.84 cfs @ 0.15 hrs, Volume= 1.027 af  
 Outflow = 24.84 cfs @ 0.15 hrs, Volume= 1.027 af, Atten= 0%, Lag= 0.0 min  
 Primary = 24.84 cfs @ 0.15 hrs, Volume= 1.027 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 61.88' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	59.63'	<b>36.0" Round Pipe</b> L= 88.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 59.63' / 58.84' S= 0.0090 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=24.84 cfs @ 0.15 hrs HW=61.88' TW=60.70' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 24.84 cfs @ 6.05 fps)**Summary for Pond A-L68: A-L68**

Inflow Area = 16.920 ac, 0.00% Impervious, Inflow Depth = 0.73" for 100-Year event  
 Inflow = 24.84 cfs @ 0.15 hrs, Volume= 1.027 af  
 Outflow = 24.84 cfs @ 0.15 hrs, Volume= 1.027 af, Atten= 0%, Lag= 0.0 min  
 Primary = 24.84 cfs @ 0.15 hrs, Volume= 1.027 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 60.70' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	58.74'	<b>48.0" Round Pipe</b> L= 398.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 58.74' / 56.75' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 12.57 sf

**Primary OutFlow** Max=24.84 cfs @ 0.15 hrs HW=60.70' TW=58.44' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 24.84 cfs @ 5.93 fps)**Summary for Pond A-L69: A-L69**

Inflow Area = 16.920 ac, 0.00% Impervious, Inflow Depth = 0.73" for 100-Year event  
 Inflow = 24.84 cfs @ 0.15 hrs, Volume= 1.027 af  
 Outflow = 24.84 cfs @ 0.15 hrs, Volume= 1.027 af, Atten= 0%, Lag= 0.0 min  
 Primary = 24.84 cfs @ 0.15 hrs, Volume= 1.027 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 58.44' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.65'	<b>48.0" Round Pipe</b> L= 240.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.65' / 53.56' S= 0.0129 '/ Cc= 0.900 n= 0.013, Flow Area= 12.57 sf

**Primary OutFlow** Max=24.84 cfs @ 0.15 hrs HW=58.44' TW=55.66' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 24.84 cfs @ 4.56 fps)

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**Summary for Pond A-L70: A-L70**

Inflow Area = 1.370 ac, 0.00% Impervious, Inflow Depth = 0.84" for 100-Year event  
 Inflow = 2.32 cfs @ 0.09 hrs, Volume= 0.096 af  
 Outflow = 2.32 cfs @ 0.12 hrs, Volume= 0.096 af, Atten= 0%, Lag= 1.8 min  
 Primary = 2.32 cfs @ 0.12 hrs, Volume= 0.096 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 55.69' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	53.80'	<b>24.0" Round A-L77</b> L= 28.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 53.80' / 53.66' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.32 cfs @ 0.12 hrs HW=55.66' TW=55.64' (Dynamic Tailwater)↑**1=A-L77** (Outlet Controls 2.32 cfs @ 0.99 fps)**Summary for Pond A-L71: A-L71**

Inflow Area = 18.290 ac, 0.00% Impervious, Inflow Depth = 0.74" for 100-Year event  
 Inflow = 27.17 cfs @ 0.15 hrs, Volume= 1.123 af  
 Outflow = 27.17 cfs @ 0.15 hrs, Volume= 1.123 af, Atten= 0%, Lag= 0.0 min  
 Primary = 27.17 cfs @ 0.15 hrs, Volume= 1.123 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 55.66' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	53.46'	<b>48.0" Round Pipe</b> L= 49.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 53.46' / 53.02' S= 0.0090 '/ Cc= 0.900 n= 0.013, Flow Area= 12.57 sf

**Primary OutFlow** Max=27.17 cfs @ 0.15 hrs HW=55.66' TW=54.80' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 27.17 cfs @ 5.55 fps)**Summary for Pond A-L72: A-L72**

Inflow Area = 18.290 ac, 0.00% Impervious, Inflow Depth = 0.74" for 100-Year event  
 Inflow = 27.17 cfs @ 0.15 hrs, Volume= 1.123 af  
 Outflow = 27.17 cfs @ 0.15 hrs, Volume= 1.123 af, Atten= 0%, Lag= 0.0 min  
 Primary = 27.17 cfs @ 0.15 hrs, Volume= 1.123 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 54.80' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	52.92'	<b>48.0" Round Pipe</b> L= 340.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 52.92' / 47.50' S= 0.0159 '/ Cc= 0.900 n= 0.013, Flow Area= 12.57 sf

**Primary OutFlow** Max=27.17 cfs @ 0.15 hrs HW=54.80' TW=50.80' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 27.17 cfs @ 4.67 fps)



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**Summary for Pond A-L73: A-L73**

Inflow Area = 18.290 ac, 0.00% Impervious, Inflow Depth = 0.74" for 100-Year event  
 Inflow = 27.17 cfs @ 0.15 hrs, Volume= 1.123 af  
 Outflow = 27.17 cfs @ 0.15 hrs, Volume= 1.123 af, Atten= 0%, Lag= 0.0 min  
 Primary = 27.17 cfs @ 0.15 hrs, Volume= 1.123 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 51.27' @ 0.50 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	45.30'	<b>48.0" Round Pipe</b> L= 492.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 45.30' / 44.40' S= 0.0018 ' / S= 0.0018 ' Cc= 0.900 n= 0.013, Flow Area= 12.57 sf

**Primary OutFlow** Max=27.17 cfs @ 0.15 hrs HW=50.80' TW=50.52' (Dynamic Tailwater)

1=Pipe (Outlet Controls 27.17 cfs @ 2.16 fps)

**Summary for Subcatchment A01: A01**

Runoff = 1.99 cfs @ 0.12 hrs, Volume= 0.082 af, Depth= 0.69"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
1.430	0.69	Mixed Use, HSG D
1.430		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment A02: A02**

Runoff = 0.86 cfs @ 0.12 hrs, Volume= 0.035 af, Depth= 0.72"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.590	0.72	Mixed Use, HSG D
0.590		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.0					<b>Direct Entry, P-A02</b>

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**Summary for Subcatchment A03: A03**

Runoff = 0.65 cfs @ 0.09 hrs, Volume= 0.027 af, Depth= 0.95"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.340	0.95	Mixed Use, HSG D
0.340		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, P-A02

**Summary for Subcatchment A04: A04**

Runoff = 0.54 cfs @ 0.09 hrs, Volume= 0.022 af, Depth= 0.95"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.280	0.95	Mixed Use, HSG D
0.280		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, P-A02

**Summary for Subcatchment A05: A05**

Runoff = 5.37 cfs @ 0.39 hrs, Volume= 0.222 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
4.840	0.55	Mixed Use, HSG D
4.840		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0					Direct Entry, P-A02

**Summary for Subcatchment A06: A06**

Runoff = 0.39 cfs @ 0.10 hrs, Volume= 0.016 af, Depth= 0.75"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr



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Area (ac)	C	Description
0.260	0.75	Mixed Use, HSG D
0.260		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, P-A02

**Summary for Subcatchment A07: A07**

Runoff = 0.32 cfs @ 0.14 hrs, Volume= 0.013 af, Depth= 0.64"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.250	0.64	Mixed Use, HSG D
0.250		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					Direct Entry, P-A02

**Summary for Subcatchment A08: A08**

Runoff = 3.57 cfs @ 0.34 hrs, Volume= 0.147 af, Depth= 0.58"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
3.050	0.58	Mixed Use, HSG D
3.050		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.0					Direct Entry, P-A02

**Summary for Subcatchment A09: A09**

Runoff = 1.39 cfs @ 0.14 hrs, Volume= 0.058 af, Depth= 0.67"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
1.030	0.67	Mixed Use, HSG D
1.030		100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					<b>Direct Entry, P-A02</b>

**Summary for Subcatchment A10: A10**

Runoff = 0.98 cfs @ 0.09 hrs, Volume= 0.040 af, Depth= 0.88"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.550	0.88	Mixed Use, HSG D
0.550		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, P-A02</b>

**Summary for Subcatchment A11: A11**

Runoff = 3.56 cfs @ 0.09 hrs, Volume= 0.147 af, Depth= 0.94"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
1.880	0.94	Mixed Use, HSG D
1.880		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, P-A02</b>

**Summary for Subcatchment A12: A12**

Runoff = 0.72 cfs @ 0.09 hrs, Volume= 0.030 af, Depth= 0.66"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.540	0.66	Mixed Use, HSG D
0.540		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, P-A02</b>



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**Summary for Subcatchment A13: A13**

Runoff = 0.40 cfs @ 0.09 hrs, Volume= 0.017 af, Depth= 0.83"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.240	0.83	Mixed Use, HSG D
0.240		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, P-A02

**Summary for Subcatchment A14: A14**

Runoff = 0.65 cfs @ 0.10 hrs, Volume= 0.027 af, Depth= 0.77"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.420	0.77	Mixed Use, HSG D
0.420		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, P-A02

**Summary for Subcatchment A16/17: A16/17**

Runoff = 0.51 cfs @ 0.09 hrs, Volume= 0.021 af, Depth= 0.93"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.270	0.93	Mixed Use, HSG D
0.270		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, P-A02

**Summary for Subcatchment A18: A18**

Runoff = 4.22 cfs @ 0.30 hrs, Volume= 0.174 af, Depth= 0.53"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

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Area (ac)	C	Description
3.950	0.53	Mixed Use, HSG D
3.950		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.0					Direct Entry, P-A02

**Summary for Subcatchment A19: A19**

Runoff = 0.49 cfs @ 0.14 hrs, Volume= 0.020 af, Depth= 0.65"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.370	0.65	Mixed Use, HSG D
0.370		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					Direct Entry, P-A02

**Summary for Subcatchment A20: A20**

Runoff = 0.80 cfs @ 0.15 hrs, Volume= 0.033 af, Depth= 0.61"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.650	0.61	Mixed Use, HSG D
0.650		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.0					Direct Entry, P-A02

**Summary for Subcatchment A21: A21**

Runoff = 0.95 cfs @ 0.09 hrs, Volume= 0.039 af, Depth= 0.87"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.540	0.87	Mixed Use, HSG D
0.540		100.00% Pervious Area



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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, P-A02</b>

**Summary for Subcatchment A22: A22**

Runoff = 0.62 cfs @ 0.12 hrs, Volume= 0.026 af, Depth= 0.70"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.440	0.70	Mixed Use, HSG D
0.440		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.0					<b>Direct Entry, P-A02</b>

**Summary for Subcatchment A23: A23**

Runoff = 5.01 cfs @ 0.30 hrs, Volume= 0.207 af, Depth= 0.63"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
3.940	0.63	Mixed Use, HSG D
3.940		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.0					<b>Direct Entry, P-A02</b>

**Summary for Subcatchment A24: A24**

Runoff = 6.33 cfs @ 0.39 hrs, Volume= 0.262 af, Depth= 0.59"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
5.320	0.59	Mixed Use, HSG D
5.320		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0					<b>Direct Entry, P-A02</b>

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**Summary for Subcatchment A25: A25**

Runoff = 5.01 cfs @ 0.24 hrs, Volume= 0.207 af, Depth= 0.65"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
3.820	0.65	Mixed Use, HSG D
3.820		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.0					Direct Entry, P-A02

**Summary for Subcatchment A26: A26**

Runoff = 6.67 cfs @ 0.32 hrs, Volume= 0.276 af, Depth= 0.59"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
5.610	0.59	Mixed Use, HSG D
5.610		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.0					Direct Entry, P-A02

**Summary for Subcatchment A27: A27**

Runoff = 4.98 cfs @ 0.34 hrs, Volume= 0.206 af, Depth= 0.54"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
4.570	0.54	Mixed Use, HSG D
4.570		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.0					Direct Entry, P-A02

**Summary for Subcatchment A28: A28**

Runoff = 7.29 cfs @ 0.50 hrs, Volume= 0.301 af, Depth= 0.53"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr



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Area (ac)	C	Description
6.820	0.53	Mixed Use, HSG D
6.820		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
30.0					Direct Entry, P-A02

**Summary for Subcatchment A29: A29**

Runoff = 0.36 cfs @ 0.09 hrs, Volume= 0.015 af, Depth= 0.64"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.280	0.64	Mixed Use, HSG D
0.280		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment A30: A30**

Runoff = 0.25 cfs @ 0.09 hrs, Volume= 0.010 af, Depth= 0.64"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.190	0.64	Mixed Use, HSG D
0.190		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment A31: A31**

Runoff = 0.23 cfs @ 0.09 hrs, Volume= 0.010 af, Depth= 0.64"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.180	0.64	Mixed Use, HSG D
0.180		100.00% Pervious Area

**Proposed System AD**

City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment A32: A32**

Runoff = 0.21 cfs @ 0.09 hrs, Volume= 0.009 af, Depth= 0.64"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.160	0.64	Mixed Use, HSG D
0.160		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment A33: A33**

Runoff = 0.62 cfs @ 0.12 hrs, Volume= 0.026 af, Depth= 0.73"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.420	0.73	Mixed Use, HSG D
0.420		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment A34: A34**

Runoff = 1.34 cfs @ 0.14 hrs, Volume= 0.056 af, Depth= 0.68"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.980	0.68	Mixed Use, HSG D
0.980		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					<b>Direct Entry, PA01</b>



**Proposed System AD**

City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

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**Summary for Subcatchment A35: A35**

Runoff = 1.44 cfs @ 0.14 hrs, Volume= 0.059 af, Depth= 0.68"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
1.050	0.68	Mixed Use, HSG D
1.050		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					Direct Entry, PA01

**Summary for Subcatchment A36/37: A36/37**

Runoff = 1.91 cfs @ 0.14 hrs, Volume= 0.079 af, Depth= 0.67"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
1.410	0.67	Mixed Use, HSG D
1.410		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					Direct Entry, PA01

**Summary for Subcatchment A38: A38**

Runoff = 1.93 cfs @ 0.09 hrs, Volume= 0.080 af, Depth= 0.93"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
1.030	0.93	Mixed Use, HSG D
1.030		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment A39: A39**

Runoff = 0.95 cfs @ 0.09 hrs, Volume= 0.039 af, Depth= 0.91"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

**Proposed System AD**

City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

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Area (ac)	C	Description
0.520	0.91	Mixed Use, HSG D
0.520		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment A40: A40**

Runoff = 2.87 cfs @ 0.10 hrs, Volume= 0.118 af, Depth= 0.76"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
1.870	0.76	Mixed Use, HSG D
1.870		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, PA01

**Summary for Subcatchment A41: A41**

Runoff = 1.26 cfs @ 0.09 hrs, Volume= 0.052 af, Depth= 0.87"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.720	0.87	Mixed Use, HSG D
0.720		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment A42: A42**

Runoff = 3.49 cfs @ 0.10 hrs, Volume= 0.144 af, Depth= 0.74"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
2.340	0.74	Mixed Use, HSG D
2.340		100.00% Pervious Area



**Proposed System AD**

City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment A43: A43**

Runoff = 1.26 cfs @ 0.09 hrs, Volume= 0.052 af, Depth= 0.87"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.720	0.87	Mixed Use, HSG D
0.720		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment A44: A44**

Runoff = 2.32 cfs @ 0.09 hrs, Volume= 0.096 af, Depth= 0.84"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
1.370	0.84	Mixed Use, HSG D
1.370		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment A45: A45**

Runoff = 6.00 cfs @ 0.09 hrs, Volume= 0.248 af, Depth= 0.65"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
4.580	0.65	Mixed Use, HSG D
4.580		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, P-A02</b>

**Proposed System AD**

City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

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**Summary for Subcatchment A46: A46**

Runoff = 6.72 cfs @ 0.09 hrs, Volume= 0.278 af, Depth= 0.66"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
5.050	0.66	Mixed Use, HSG D
5.050		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, P-A02

**Summary for Subcatchment A47: A47**

Runoff = 10.22 cfs @ 0.09 hrs, Volume= 0.422 af, Depth= 0.92"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
5.510	0.92	Mixed Use, HSG D
5.510		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, P-A02

**Summary for Subcatchment Basin-A1: Basin A1**

Runoff = 6.69 cfs @ 0.39 hrs, Volume= 0.276 af, Depth= 0.59"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
5.620	0.59	Mixed Use, HSG D
5.620		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
23.0					Direct Entry, P-A02

**Summary for Subcatchment Basin-A2: ABASIN2**

Runoff = 1.27 cfs @ 0.09 hrs, Volume= 0.052 af, Depth= 0.84"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr



**Proposed System AD**

City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

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Area (ac)	C	Description
0.750	0.84	Mixed Use, HSG D
0.750		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, P-A02

**Summary for Subcatchment Basin-A3: Basin A3**

Runoff = 0.61 cfs @ 0.14 hrs, Volume= 0.025 af, Depth= 0.67"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

Area (ac)	C	Description
0.450	0.67	Mixed Use, HSG D
0.450		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					Direct Entry, P-A02

**Summary for Pond D-L1: D-L1**

Inflow Area = 2.910 ac, 0.00% Impervious, Inflow Depth = 0.52" for 100-Year event  
 Inflow = 3.05 cfs @ 0.34 hrs, Volume= 0.126 af  
 Outflow = 3.05 cfs @ 0.34 hrs, Volume= 0.126 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.05 cfs @ 0.34 hrs, Volume= 0.126 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3  
Peak Elev= 51.82' @ 0.34 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	51.00'	<b>18.0" Round D-L1</b> L= 50.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 51.00' / 46.20' S= 0.0960 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=3.05 cfs @ 0.34 hrs HW=51.82' TW=0.00' (Dynamic Tailwater)  
 ←**1=D-L1** (Inlet Controls 3.05 cfs @ 3.08 fps)

**Summary for Subcatchment D1: D1**

Runoff = 3.05 cfs @ 0.34 hrs, Volume= 0.126 af, Depth= 0.52"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

**Proposed System AD**

City of San Diego 100-Year Duration=30 min, Inten=2.00 in/hr

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Area (ac)	C	Description
2.910	0.52	Mixed Use, HSG D
2.910		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.0					Direct Entry, P-A02

**Summary for Pond OUTFALL A: OUTFALL A**

Inflow Area = 87.200 ac, 0.71% Impervious, Inflow Depth = 0.49" for 100-Year event  
 Inflow = 69.84 cfs @ 0.50 hrs, Volume= 3.568 af  
 Primary = 69.84 cfs @ 0.50 hrs, Volume= 3.568 af, Atten= 0%, Lag= 0.0 min

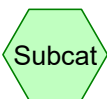
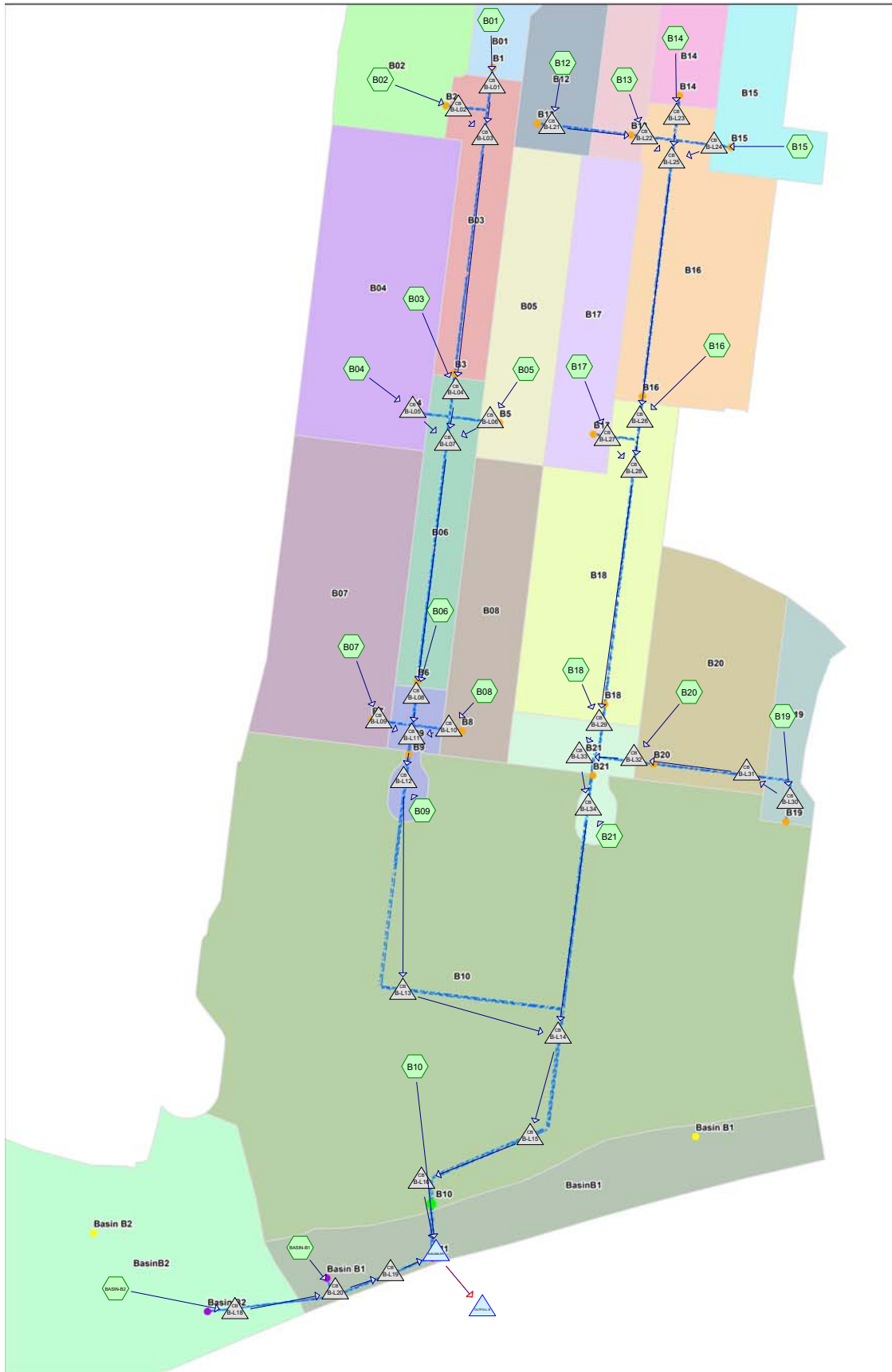
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3

**Summary for Pond Outlet: BasinOutlet**

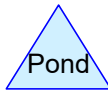
Inflow Area = 2.910 ac, 0.00% Impervious, Inflow Depth = 5.62" for 100-Year event  
 Inflow = 49.16 cfs @ 0.50 hrs, Volume= 1.362 af  
 Primary = 49.16 cfs @ 0.50 hrs, Volume= 1.362 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3





Reach



**Routing Diagram for Proposed System B**  
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## Proposed System B

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### Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	B-BUBBLER	44.31	43.59	100.0	0.0072	0.013	34.0	0.0	0.0
2	B-L01	63.87	63.64	62.0	0.0037	0.013	24.0	0.0	0.0
3	B-L02	63.93	63.64	60.0	0.0048	0.013	24.0	0.0	0.0
4	B-L03	63.54	61.36	399.0	0.0055	0.013	24.0	0.0	0.0
5	B-L04	61.26	60.94	63.0	0.0051	0.013	24.0	0.0	0.0
6	B-L05	61.33	61.04	57.0	0.0051	0.013	24.0	0.0	0.0
7	B-L06	61.41	61.04	73.0	0.0051	0.013	24.0	0.0	0.0
8	B-L07	60.84	58.72	424.0	0.0050	0.013	30.0	0.0	0.0
9	B-L08	58.62	58.33	64.0	0.0045	0.013	36.0	0.0	0.0
10	B-L09	58.62	58.33	58.0	0.0050	0.013	24.0	0.0	0.0
11	B-L10	58.83	58.33	72.0	0.0069	0.013	24.0	0.0	0.0
12	B-L11	58.23	57.99	48.0	0.0050	0.013	36.0	0.0	0.0
13	B-L12	57.89	56.03	372.0	0.0050	0.013	36.0	0.0	0.0
14	B-L13	55.93	54.54	278.0	0.0050	0.013	36.0	0.0	0.0
15	B-L14	54.44	47.90	180.0	0.0363	0.013	48.0	0.0	0.0
16	B-L15	47.80	44.60	188.0	0.0170	0.013	48.0	0.0	0.0
17	B-L16	44.50	44.41	38.0	0.0024	0.013	48.0	0.0	0.0
18	B-L18	45.90	44.70	200.0	0.0060	0.013	18.0	0.0	0.0
19	B-L19	44.70	43.73	162.0	0.0060	0.013	24.0	0.0	0.0
20	B-L20	46.80	44.70	26.0	0.0808	0.013	24.0	0.0	0.0
21	B-L21	64.68	63.88	160.0	0.0050	0.013	24.0	0.0	0.0
22	B-L22	63.78	63.41	74.0	0.0050	0.013	24.0	0.0	0.0
23	B-L23	63.80	63.41	78.0	0.0050	0.013	24.0	0.0	0.0
24	B-L24	64.25	63.41	84.0	0.0100	0.013	24.0	0.0	0.0
25	B-L25	63.31	60.51	401.0	0.0070	0.013	30.0	0.0	0.0
26	B-L26	60.40	60.08	64.0	0.0050	0.013	30.0	0.0	0.0
27	B-L27	60.50	60.18	46.0	0.0070	0.013	24.0	0.0	0.0
28	B-L28	59.98	57.15	404.0	0.0070	0.013	30.0	0.0	0.0
29	B-L29	57.05	56.66	78.0	0.0050	0.013	30.0	0.0	0.0
30	B-L30	60.15	59.46	69.0	0.0100	0.013	24.0	0.0	0.0
31	B-L31	59.36	57.38	198.0	0.0100	0.013	24.0	0.0	0.0
32	B-L32	57.28	56.66	89.0	0.0070	0.013	24.0	0.0	0.0
33	B-L33	56.56	56.43	26.0	0.0050	0.013	36.0	0.0	0.0
34	B-L34	56.33	54.54	358.0	0.0050	0.013	36.0	0.0	0.0



50-year Rainfall Intensity

ELEV.	FACTOR
0-1500	1.00
1500-3000	1.25
3000-4000	1.42
4000-5000	1.60
5000-6000	1.7
DESERT	1.25

TO OBTAIN CORRECT INTENSITY, MULTIPLY INTENSITY ON CHART BY FACTOR FOR DESIGN ELEVATION.

INTENSITY - DURATION - FREQUENCY CURVES  
FOR  
COUNTY OF SAN DIEGO  
RAINFALL

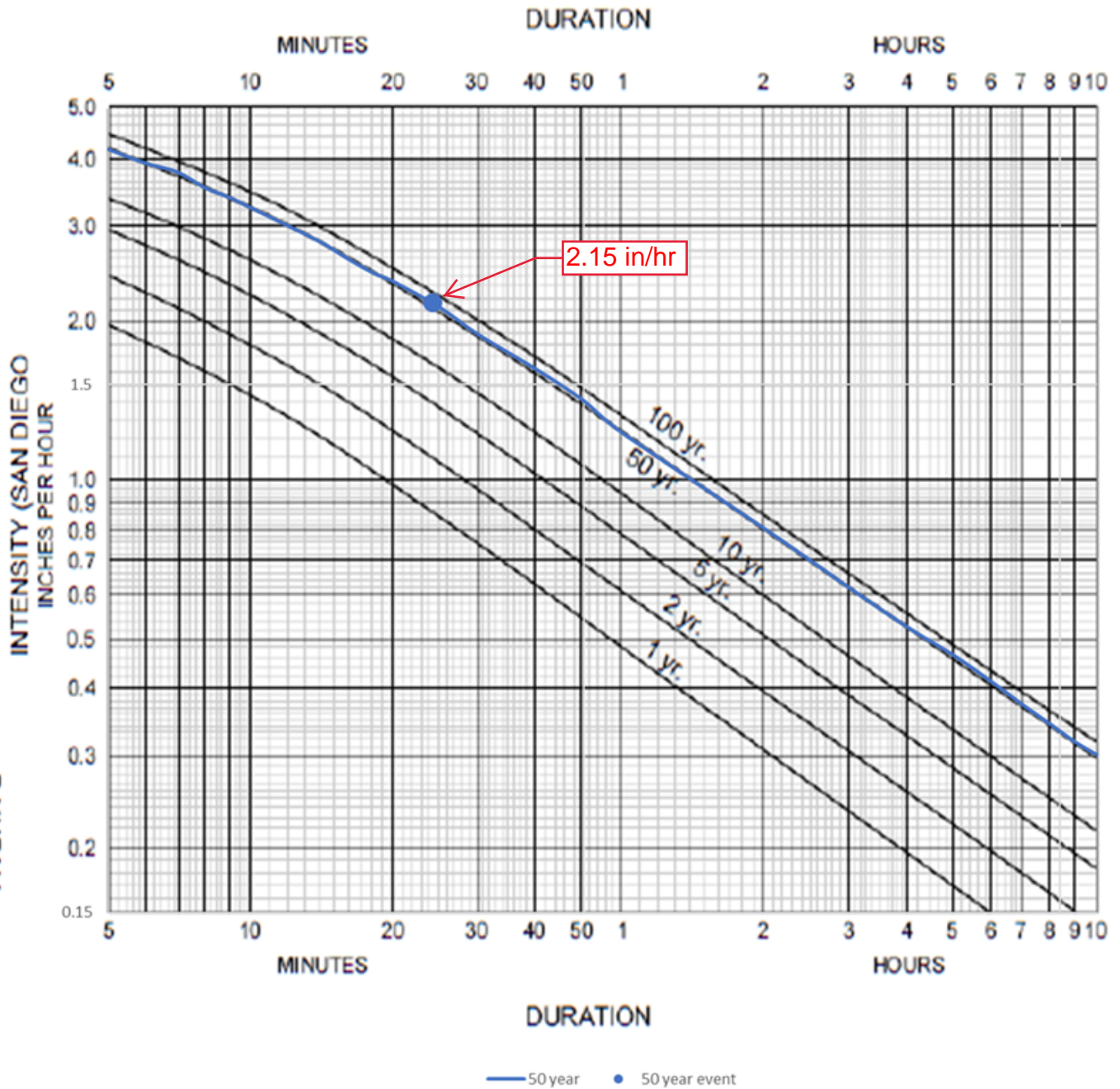


Figure A-1. Intensity-Duration-Frequency Design Chart

**Proposed System B**

City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

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Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points x 3

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Pond B-BUBBLER: B-BUBBLER</b>	Peak Elev=50.05' Storage=582 cf Inflow=65.67 cfs 2.171 af Primary=63.11 cfs 2.174 af Secondary=0.00 cfs 0.000 af Outflow=63.11 cfs 2.174 af
<b>Pond B-L01: B-L01</b>	Peak Elev=64.86' Inflow=2.14 cfs 0.071 af 24.0" Round Culvert n=0.013 L=62.0' S=0.0037 '/' Outflow=2.14 cfs 0.071 af
<b>Pond B-L02: B-L02</b>	Peak Elev=64.96' Inflow=2.99 cfs 0.099 af 24.0" Round Culvert n=0.013 L=60.0' S=0.0048 '/' Outflow=2.99 cfs 0.099 af
<b>Pond B-L03: B-L03</b>	Peak Elev=64.70' Inflow=5.13 cfs 0.170 af 24.0" Round Culvert n=0.013 L=399.0' S=0.0055 '/' Outflow=5.13 cfs 0.170 af
<b>Pond B-L04: B-L04</b>	Peak Elev=62.98' Inflow=7.00 cfs 0.231 af 24.0" Round Culvert n=0.013 L=63.0' S=0.0051 '/' Outflow=7.00 cfs 0.231 af
<b>Pond B-L05: B-L05</b>	Peak Elev=62.77' Inflow=3.52 cfs 0.116 af 24.0" Round Culvert n=0.013 L=57.0' S=0.0051 '/' Outflow=3.52 cfs 0.116 af
<b>Pond B-L06: B-L06</b>	Peak Elev=62.69' Inflow=1.86 cfs 0.061 af 24.0" Round Culvert n=0.013 L=73.0' S=0.0051 '/' Outflow=1.86 cfs 0.061 af
<b>Pond B-L07: B-L07</b>	Peak Elev=62.63' Inflow=12.38 cfs 0.409 af 30.0" Round Culvert n=0.013 L=424.0' S=0.0050 '/' Outflow=12.38 cfs 0.409 af
<b>Pond B-L08: B-L08</b>	Peak Elev=60.90' Inflow=14.15 cfs 0.468 af 36.0" Round Culvert n=0.013 L=64.0' S=0.0045 '/' Outflow=14.15 cfs 0.468 af
<b>Pond B-L09: B-L09</b>	Peak Elev=60.61' Inflow=3.45 cfs 0.114 af 24.0" Round Culvert n=0.013 L=58.0' S=0.0050 '/' Outflow=3.45 cfs 0.114 af
<b>Pond B-L10: B-L8</b>	Peak Elev=60.58' Inflow=1.78 cfs 0.059 af 24.0" Round Culvert n=0.013 L=72.0' S=0.0069 '/' Outflow=1.78 cfs 0.059 af
<b>Pond B-L11: B-L11</b>	Peak Elev=60.55' Inflow=19.37 cfs 0.640 af 36.0" Round Culvert n=0.013 L=48.0' S=0.0050 '/' Outflow=19.37 cfs 0.640 af
<b>Pond B-L12: B-L21</b>	Peak Elev=59.97' Inflow=19.96 cfs 0.660 af 36.0" Round Culvert n=0.013 L=372.0' S=0.0050 '/' Outflow=19.96 cfs 0.660 af
<b>Pond B-L13: B-L13</b>	Peak Elev=58.14' Inflow=19.96 cfs 0.660 af 36.0" Round Culvert n=0.013 L=278.0' S=0.0050 '/' Outflow=19.96 cfs 0.660 af
<b>Pond B-L14: B-L14</b>	Peak Elev=56.83' Inflow=41.32 cfs 1.366 af 48.0" Round Culvert n=0.013 L=180.0' S=0.0363 '/' Outflow=41.32 cfs 1.366 af
<b>Pond B-L15: B-L15</b>	Peak Elev=51.30' Inflow=41.32 cfs 1.366 af 48.0" Round Culvert n=0.013 L=188.0' S=0.0170 '/' Outflow=41.32 cfs 1.366 af
<b>Pond B-L16: B-L16</b>	Peak Elev=50.52' Inflow=41.32 cfs 1.366 af 48.0" Round Culvert n=0.013 L=38.0' S=0.0024 '/' Outflow=41.32 cfs 1.366 af



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<b>Pond B-L18: B-L18</b>	Peak Elev=51.45' Inflow=5.60 cfs 0.185 af 18.0" Round Culvert n=0.013 L=200.0' S=0.0060 '/ Outflow=5.60 cfs 0.185 af
<b>Pond B-L19: B-L19</b>	Peak Elev=50.38' Inflow=7.79 cfs 0.257 af 24.0" Round Culvert n=0.013 L=162.0' S=0.0060 '/ Outflow=7.79 cfs 0.257 af
<b>Pond B-L20: B-L20</b>	Peak Elev=50.65' Inflow=7.79 cfs 0.257 af 24.0" Round Culvert n=0.013 L=26.0' S=0.0808 '/ Outflow=7.79 cfs 0.257 af
<b>Pond B-L21: B-L21</b>	Peak Elev=65.54' Inflow=2.40 cfs 0.079 af 24.0" Round Culvert n=0.013 L=160.0' S=0.0050 '/ Outflow=2.40 cfs 0.079 af
<b>Pond B-L22: B-L22</b>	Peak Elev=65.00' Inflow=4.57 cfs 0.151 af 24.0" Round Culvert n=0.013 L=74.0' S=0.0050 '/ Outflow=4.57 cfs 0.151 af
<b>Pond B-L23: B-L23</b>	Peak Elev=64.74' Inflow=1.77 cfs 0.059 af 24.0" Round Culvert n=0.013 L=78.0' S=0.0050 '/ Outflow=1.77 cfs 0.059 af
<b>Pond B-L24: B-L24</b>	Peak Elev=65.02' Inflow=2.18 cfs 0.072 af 24.0" Round Culvert n=0.013 L=84.0' S=0.0100 '/ Outflow=2.18 cfs 0.072 af
<b>Pond B-L25: B-L25</b>	Peak Elev=64.60' Inflow=8.53 cfs 0.282 af 30.0" Round Culvert n=0.013 L=401.0' S=0.0070 '/ Outflow=8.53 cfs 0.282 af
<b>Pond B-L26: B-L26</b>	Peak Elev=62.29' Inflow=11.93 cfs 0.394 af 30.0" Round Culvert n=0.013 L=64.0' S=0.0050 '/ Outflow=11.93 cfs 0.394 af
<b>Pond B-L27: B-L27</b>	Peak Elev=61.78' Inflow=1.68 cfs 0.056 af 24.0" Round Culvert n=0.013 L=46.0' S=0.0070 '/ Outflow=1.68 cfs 0.056 af
<b>Pond B-L28: B-L28</b>	Peak Elev=61.74' Inflow=13.61 cfs 0.450 af 30.0" Round Culvert n=0.013 L=404.0' S=0.0070 '/ Outflow=13.61 cfs 0.450 af
<b>Pond B-L29: B-L29</b>	Peak Elev=59.61' Inflow=17.04 cfs 0.563 af 30.0" Round Culvert n=0.013 L=78.0' S=0.0050 '/ Outflow=17.04 cfs 0.563 af
<b>Pond B-L30: B-L30</b>	Peak Elev=60.54' Inflow=0.86 cfs 0.028 af 24.0" Round Culvert n=0.013 L=69.0' S=0.0100 '/ Outflow=0.86 cfs 0.028 af
<b>Pond B-L31: B-L31</b>	Peak Elev=59.84' Inflow=0.86 cfs 0.028 af 24.0" Round Culvert n=0.013 L=198.0' S=0.0100 '/ Outflow=0.86 cfs 0.028 af
<b>Pond B-L32: B-L32</b>	Peak Elev=59.13' Inflow=3.37 cfs 0.111 af 24.0" Round Culvert n=0.013 L=89.0' S=0.0070 '/ Outflow=3.37 cfs 0.111 af
<b>Pond B-L33: B-L33</b>	Peak Elev=59.05' Inflow=20.41 cfs 0.675 af 36.0" Round Culvert n=0.013 L=26.0' S=0.0050 '/ Outflow=20.41 cfs 0.675 af
<b>Pond B-L34: B-L34</b>	Peak Elev=58.54' Inflow=21.35 cfs 0.706 af 36.0" Round Culvert n=0.013 L=358.0' S=0.0050 '/ Outflow=21.35 cfs 0.706 af
<b>Subcatchment B01: B01</b>	Runoff Area=1.270 ac 0.00% Impervious Runoff Depth=0.67" Tc=6.0 min C=0.78 Runoff=2.14 cfs 0.071 af

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<b>Subcatchment B02: B02</b>	Runoff Area=1.950 ac 0.00% Impervious Runoff Depth=0.61" Tc=7.0 min C=0.71 Runoff=2.99 cfs 0.099 af
<b>Subcatchment B03: B03</b>	Runoff Area=0.910 ac 100.00% Impervious Runoff Depth=0.81" Tc=5.0 min C=0.95 Runoff=1.87 cfs 0.062 af
<b>Subcatchment B04: B04</b>	Runoff Area=2.200 ac 0.00% Impervious Runoff Depth=0.63" Tc=6.0 min C=0.74 Runoff=3.52 cfs 0.116 af
<b>Subcatchment B05: B05</b>	Runoff Area=1.160 ac 0.00% Impervious Runoff Depth=0.63" Tc=7.0 min C=0.74 Runoff=1.86 cfs 0.061 af
<b>Subcatchment B06: B06</b>	Runoff Area=0.880 ac 0.00% Impervious Runoff Depth=0.80" Tc=5.0 min C=0.93 Runoff=1.77 cfs 0.058 af
<b>Subcatchment B07: B07</b>	Runoff Area=2.130 ac 0.00% Impervious Runoff Depth=0.64" Tc=6.0 min C=0.75 Runoff=3.45 cfs 0.114 af
<b>Subcatchment B08: B08</b>	Runoff Area=1.110 ac 0.00% Impervious Runoff Depth=0.63" Tc=6.0 min C=0.74 Runoff=1.78 cfs 0.059 af
<b>Subcatchment B09: B09</b>	Runoff Area=0.300 ac 0.00% Impervious Runoff Depth=0.78" Tc=5.0 min C=0.91 Runoff=0.59 cfs 0.020 af
<b>Subcatchment B10: B17</b>	Runoff Area=12.560 ac 0.00% Impervious Runoff Depth=0.52" Tc=22.0 min C=0.61 Runoff=16.56 cfs 0.547 af
<b>Subcatchment B12: B12</b>	Runoff Area=1.480 ac 0.00% Impervious Runoff Depth=0.64" Tc=6.0 min C=0.75 Runoff=2.40 cfs 0.079 af
<b>Subcatchment B13: B13</b>	Runoff Area=1.360 ac 0.00% Impervious Runoff Depth=0.63" Tc=7.0 min C=0.74 Runoff=2.18 cfs 0.072 af
<b>Subcatchment B14: B14</b>	Runoff Area=0.900 ac 0.00% Impervious Runoff Depth=0.78" Tc=5.0 min C=0.91 Runoff=1.77 cfs 0.059 af
<b>Subcatchment B15: B15</b>	Runoff Area=1.330 ac 0.00% Impervious Runoff Depth=0.65" Tc=6.0 min C=0.76 Runoff=2.18 cfs 0.072 af
<b>Subcatchment B16: B16</b>	Runoff Area=1.940 ac 0.00% Impervious Runoff Depth=0.69" Tc=5.0 min C=0.81 Runoff=3.40 cfs 0.112 af
<b>Subcatchment B17: B17</b>	Runoff Area=1.050 ac 0.00% Impervious Runoff Depth=0.63" Tc=6.0 min C=0.74 Runoff=1.68 cfs 0.056 af
<b>Subcatchment B18: B18</b>	Runoff Area=1.960 ac 0.00% Impervious Runoff Depth=0.69" Tc=5.0 min C=0.81 Runoff=3.43 cfs 0.113 af
<b>Subcatchment B19: B19</b>	Runoff Area=0.510 ac 0.00% Impervious Runoff Depth=0.67" Tc=6.0 min C=0.78 Runoff=0.86 cfs 0.028 af
<b>Subcatchment B20: B20</b>	Runoff Area=1.550 ac 0.00% Impervious Runoff Depth=0.64" Tc=6.0 min C=0.75 Runoff=2.51 cfs 0.083 af



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**Subcatchment B21: B21**

Runoff Area=0.470 ac 0.00% Impervious Runoff Depth=0.80"  
Tc=5.0 min C=0.93 Runoff=0.94 cfs 0.031 af

**Subcatchment BASIN-B1: BASIN-B1**

Runoff Area=1.870 ac 0.00% Impervious Runoff Depth=0.46"  
Tc=10.0 min C=0.54 Runoff=2.18 cfs 0.072 af

**Subcatchment BASIN-B2: BASIN-B2**

Runoff Area=4.800 ac 0.00% Impervious Runoff Depth=0.46"  
Tc=24.0 min C=0.54 Runoff=5.60 cfs 0.185 af

**Pond OUTFALL B: OUTFALL B**

Inflow=63.11 cfs 2.174 af  
Primary=63.11 cfs 2.174 af

**Total Runoff Area = 43.690 ac Runoff Volume = 2.171 af Average Runoff Depth = 0.60"**  
**97.92% Pervious = 42.780 ac 2.08% Impervious = 0.910 ac**

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**Summary for Pond B-BUBBLER: B-BUBBLER**

Inflow Area = 43.690 ac, 2.08% Impervious, Inflow Depth = 0.60" for 50-Year event  
 Inflow = 65.67 cfs @ 0.40 hrs, Volume= 2.171 af  
 Outflow = 63.11 cfs @ 0.41 hrs, Volume= 2.174 af, Atten= 4%, Lag= 0.6 min  
 Primary = 63.11 cfs @ 0.41 hrs, Volume= 2.174 af  
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 50.05' @ 0.41 hrs Surf.Area= 11,830 sf Storage= 582 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 0.1 min ( 17.9 - 17.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	50.00'	212,572 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
50.00	11,326	0	0
52.00	31,363	42,689	42,689
54.00	43,124	74,487	117,176
56.00	52,272	95,396	212,572

Device	Routing	Invert	Outlet Devices
#1	Primary	44.31'	<b>34.0" Round Outlet</b> L= 100.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 44.31' / 43.59' S= 0.0072 '/ Cc= 0.900 n= 0.013, Flow Area= 6.31 sf
#2	Secondary	50.25'	<b>80.0' long x 20.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=63.11 cfs @ 0.41 hrs HW=50.05' TW=0.00' (Dynamic Tailwater)  
 ↖1=Outlet (Inlet Controls 63.11 cfs @ 10.01 fps)

**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=50.00' TW=0.00' (Dynamic Tailwater)  
 ↖2=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)

**Summary for Pond B-L01: B-L01**

Inflow Area = 1.270 ac, 0.00% Impervious, Inflow Depth = 0.67" for 50-Year event  
 Inflow = 2.14 cfs @ 0.10 hrs, Volume= 0.071 af  
 Outflow = 2.14 cfs @ 0.14 hrs, Volume= 0.071 af, Atten= 0%, Lag= 2.4 min  
 Primary = 2.14 cfs @ 0.14 hrs, Volume= 0.071 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 64.86' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	63.87'	<b>24.0" Round B-L1</b> L= 62.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 63.87' / 63.64' S= 0.0037 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf



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**Primary OutFlow** Max=2.14 cfs @ 0.14 hrs HW=64.86' TW=64.70' (Dynamic Tailwater)↑**1=B-L1** (Outlet Controls 2.14 cfs @ 2.02 fps)**Summary for Pond B-L02: B-L02**

Inflow Area = 1.950 ac, 0.00% Impervious, Inflow Depth = 0.61" for 50-Year event  
 Inflow = 2.99 cfs @ 0.12 hrs, Volume= 0.099 af  
 Outflow = 2.99 cfs @ 0.12 hrs, Volume= 0.099 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.99 cfs @ 0.12 hrs, Volume= 0.099 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 64.96' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	63.93'	<b>24.0" Round B-L2</b> L= 60.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 63.93' / 63.64' S= 0.0048 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.98 cfs @ 0.12 hrs HW=64.96' TW=64.70' (Dynamic Tailwater)↑**1=B-L2** (Outlet Controls 2.98 cfs @ 2.65 fps)**Summary for Pond B-L03: B-L03**

Inflow Area = 3.220 ac, 0.00% Impervious, Inflow Depth = 0.63" for 50-Year event  
 Inflow = 5.13 cfs @ 0.14 hrs, Volume= 0.170 af  
 Outflow = 5.13 cfs @ 0.18 hrs, Volume= 0.170 af, Atten= 0%, Lag= 2.4 min  
 Primary = 5.13 cfs @ 0.18 hrs, Volume= 0.170 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 64.70' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	63.54'	<b>24.0" Round Pipe</b> L= 399.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 63.54' / 61.36' S= 0.0055 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=5.13 cfs @ 0.18 hrs HW=64.70' TW=62.98' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 5.13 cfs @ 3.93 fps)**Summary for Pond B-L04: B-L04**

Inflow Area = 4.130 ac, 22.03% Impervious, Inflow Depth = 0.67" for 50-Year event  
 Inflow = 7.00 cfs @ 0.18 hrs, Volume= 0.231 af  
 Outflow = 7.00 cfs @ 0.18 hrs, Volume= 0.231 af, Atten= 0%, Lag= 0.0 min  
 Primary = 7.00 cfs @ 0.18 hrs, Volume= 0.231 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 62.98' @ 0.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	61.26'	<b>24.0" Round B-L4</b> L= 63.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 61.26' / 60.94' S= 0.0051 '/ Cc= 0.900

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n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=7.00 cfs @ 0.18 hrs HW=62.98' TW=62.63' (Dynamic Tailwater)↑**1=B-L4** (Outlet Controls 7.00 cfs @ 3.26 fps)**Summary for Pond B-L05: B-L05**

Inflow Area = 2.200 ac, 0.00% Impervious, Inflow Depth = 0.63" for 50-Year event  
 Inflow = 3.52 cfs @ 0.10 hrs, Volume= 0.116 af  
 Outflow = 3.52 cfs @ 0.13 hrs, Volume= 0.116 af, Atten= 0%, Lag= 1.8 min  
 Primary = 3.52 cfs @ 0.13 hrs, Volume= 0.116 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 62.77' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	61.33'	<b>24.0" Round B-L5</b> L= 57.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 61.33' / 61.04' S= 0.0051 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=3.52 cfs @ 0.13 hrs HW=62.77' TW=62.63' (Dynamic Tailwater)↑**1=B-L5** (Outlet Controls 3.52 cfs @ 2.04 fps)**Summary for Pond B-L06: B-L06**

Inflow Area = 1.160 ac, 0.00% Impervious, Inflow Depth = 0.63" for 50-Year event  
 Inflow = 1.86 cfs @ 0.12 hrs, Volume= 0.061 af  
 Outflow = 1.86 cfs @ 0.19 hrs, Volume= 0.061 af, Atten= 0%, Lag= 4.2 min  
 Primary = 1.86 cfs @ 0.19 hrs, Volume= 0.061 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 62.69' @ 0.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	61.41'	<b>24.0" Round B-L6</b> L= 73.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 61.41' / 61.04' S= 0.0051 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.86 cfs @ 0.19 hrs HW=62.69' TW=62.63' (Dynamic Tailwater)↑**1=B-L6** (Outlet Controls 1.86 cfs @ 1.25 fps)**Summary for Pond B-L07: B-L07**

Inflow Area = 7.490 ac, 12.15% Impervious, Inflow Depth = 0.66" for 50-Year event  
 Inflow = 12.38 cfs @ 0.16 hrs, Volume= 0.409 af  
 Outflow = 12.38 cfs @ 0.16 hrs, Volume= 0.409 af, Atten= 0%, Lag= 0.0 min  
 Primary = 12.38 cfs @ 0.16 hrs, Volume= 0.409 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 62.63' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	60.84'	<b>30.0" Round Pipe</b> L= 424.0' RCP, sq.cut end projecting, Ke= 0.500



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Inlet / Outlet Invert= 60.84' / 58.72' S= 0.0050 '/ Cc= 0.900  
n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=12.38 cfs @ 0.16 hrs HW=62.63' TW=60.90' (Dynamic Tailwater)

↑**1=Pipe** (Outlet Controls 12.38 cfs @ 4.62 fps)

**Summary for Pond B-L08: B-L08**

Inflow Area = 8.370 ac, 10.87% Impervious, Inflow Depth = 0.67" for 50-Year event  
Inflow = 14.15 cfs @ 0.16 hrs, Volume= 0.468 af  
Outflow = 14.15 cfs @ 0.16 hrs, Volume= 0.468 af, Atten= 0%, Lag= 0.0 min  
Primary = 14.15 cfs @ 0.16 hrs, Volume= 0.468 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 60.90' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	58.62'	<b>36.0" Round B-L8</b> L= 64.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 58.62' / 58.33' S= 0.0045 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=14.15 cfs @ 0.16 hrs HW=60.90' TW=60.55' (Dynamic Tailwater)

↑**1=B-L8** (Outlet Controls 14.15 cfs @ 3.40 fps)

**Summary for Pond B-L09: B-L09**

Inflow Area = 2.130 ac, 0.00% Impervious, Inflow Depth = 0.64" for 50-Year event  
Inflow = 3.45 cfs @ 0.10 hrs, Volume= 0.114 af  
Outflow = 3.45 cfs @ 0.10 hrs, Volume= 0.114 af, Atten= 0%, Lag= 0.0 min  
Primary = 3.45 cfs @ 0.10 hrs, Volume= 0.114 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 60.61' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	58.62'	<b>24.0" Round B-L9</b> L= 58.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 58.62' / 58.33' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=3.13 cfs @ 0.10 hrs HW=60.55' TW=60.49' (Dynamic Tailwater)

↑**1=B-L9** (Outlet Controls 3.13 cfs @ 1.29 fps)

**Summary for Pond B-L10: B-L8**

Inflow Area = 1.110 ac, 0.00% Impervious, Inflow Depth = 0.63" for 50-Year event  
Inflow = 1.78 cfs @ 0.10 hrs, Volume= 0.059 af  
Outflow = 1.78 cfs @ 0.13 hrs, Volume= 0.059 af, Atten= 0%, Lag= 1.8 min  
Primary = 1.78 cfs @ 0.13 hrs, Volume= 0.059 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 60.58' @ 0.13 hrs

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Device	Routing	Invert	Outlet Devices
#1	Primary	58.83'	<b>24.0" Round B-L10</b> L= 72.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 58.83' / 58.33' S= 0.0069 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.78 cfs @ 0.13 hrs HW=60.58' TW=60.55' (Dynamic Tailwater)↑**1=B-L10** (Outlet Controls 1.78 cfs @ 0.81 fps)**Summary for Pond B-L11: B-L11**

Inflow Area = 11.610 ac, 7.84% Impervious, Inflow Depth = 0.66" for 50-Year event  
 Inflow = 19.37 cfs @ 0.16 hrs, Volume= 0.640 af  
 Outflow = 19.37 cfs @ 0.16 hrs, Volume= 0.640 af, Atten= 0%, Lag= 0.0 min  
 Primary = 19.37 cfs @ 0.16 hrs, Volume= 0.640 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 60.55' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	58.23'	<b>36.0" Round Pipe</b> L= 48.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 58.23' / 57.99' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=19.37 cfs @ 0.16 hrs HW=60.55' TW=59.97' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 19.37 cfs @ 4.55 fps)**Summary for Pond B-L12: B-L21**

Inflow Area = 11.910 ac, 7.64% Impervious, Inflow Depth = 0.66" for 50-Year event  
 Inflow = 19.96 cfs @ 0.16 hrs, Volume= 0.660 af  
 Outflow = 19.96 cfs @ 0.13 hrs, Volume= 0.660 af, Atten= 0%, Lag= 0.0 min  
 Primary = 19.96 cfs @ 0.13 hrs, Volume= 0.660 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 59.97' @ 0.12 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	57.89'	<b>36.0" Round B-L12</b> L= 372.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 57.89' / 56.03' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=19.96 cfs @ 0.13 hrs HW=59.97' TW=58.14' (Dynamic Tailwater)↑**1=B-L12** (Outlet Controls 19.96 cfs @ 5.36 fps)**Summary for Pond B-L13: B-L13**

Inflow Area = 11.910 ac, 7.64% Impervious, Inflow Depth = 0.66" for 50-Year event  
 Inflow = 19.96 cfs @ 0.13 hrs, Volume= 0.660 af  
 Outflow = 19.96 cfs @ 0.16 hrs, Volume= 0.660 af, Atten= 0%, Lag= 1.8 min  
 Primary = 19.96 cfs @ 0.16 hrs, Volume= 0.660 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3



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Peak Elev= 58.14' @ 0.12 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	55.93'	<b>36.0" Round Pipe</b> L= 278.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 55.93' / 54.54' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=19.96 cfs @ 0.16 hrs HW=58.14' TW=56.83' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 19.96 cfs @ 4.99 fps)**Summary for Pond B-L14: B-L14**

Inflow Area = 24.460 ac, 3.72% Impervious, Inflow Depth = 0.67" for 50-Year event  
 Inflow = 41.32 cfs @ 0.16 hrs, Volume= 1.366 af  
 Outflow = 41.32 cfs @ 0.18 hrs, Volume= 1.366 af, Atten= 0%, Lag= 1.2 min  
 Primary = 41.32 cfs @ 0.18 hrs, Volume= 1.366 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 56.83' @ 0.12 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	54.44'	<b>48.0" Round Pipe</b> L= 180.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 54.44' / 47.90' S= 0.0363 '/ Cc= 0.900 n= 0.013, Flow Area= 12.57 sf

**Primary OutFlow** Max=41.32 cfs @ 0.18 hrs HW=56.83' TW=51.27' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 41.32 cfs @ 5.27 fps)**Summary for Pond B-L15: B-L15**

Inflow Area = 24.460 ac, 3.72% Impervious, Inflow Depth = 0.67" for 50-Year event  
 Inflow = 41.32 cfs @ 0.18 hrs, Volume= 1.366 af  
 Outflow = 41.32 cfs @ 0.18 hrs, Volume= 1.366 af, Atten= 0%, Lag= 0.0 min  
 Primary = 41.32 cfs @ 0.18 hrs, Volume= 1.366 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 51.30' @ 0.40 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	47.80'	<b>48.0" Round Pipe</b> L= 188.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 47.80' / 44.60' S= 0.0170 '/ Cc= 0.900 n= 0.013, Flow Area= 12.57 sf

**Primary OutFlow** Max=41.32 cfs @ 0.18 hrs HW=51.27' TW=50.47' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 41.32 cfs @ 4.77 fps)**Summary for Pond B-L16: B-L16**

Inflow Area = 24.460 ac, 3.72% Impervious, Inflow Depth = 0.67" for 50-Year event  
 Inflow = 41.32 cfs @ 0.18 hrs, Volume= 1.366 af  
 Outflow = 41.32 cfs @ 0.16 hrs, Volume= 1.366 af, Atten= 0%, Lag= 0.0 min  
 Primary = 41.32 cfs @ 0.16 hrs, Volume= 1.366 af

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Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 50.52' @ 0.40 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	44.50'	<b>48.0" Round Pipe</b> L= 38.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 44.50' / 44.41' S= 0.0024 '/ Cc= 0.900 n= 0.013, Flow Area= 12.57 sf

**Primary OutFlow** Max=41.32 cfs @ 0.16 hrs HW=50.47' TW=50.00' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 41.32 cfs @ 3.29 fps)**Summary for Pond B-L18: B-L18**

Inflow Area = 4.800 ac, 0.00% Impervious, Inflow Depth = 0.46" for 50-Year event  
 Inflow = 5.60 cfs @ 0.40 hrs, Volume= 0.185 af  
 Outflow = 5.60 cfs @ 0.40 hrs, Volume= 0.185 af, Atten= 0%, Lag= 0.0 min  
 Primary = 5.60 cfs @ 0.40 hrs, Volume= 0.185 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 51.45' @ 0.40 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	45.90'	<b>18.0" Round B-L18</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 45.90' / 44.70' S= 0.0060 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=5.58 cfs @ 0.40 hrs HW=51.45' TW=50.65' (Dynamic Tailwater)↑**1=B-L18** (Outlet Controls 5.58 cfs @ 3.16 fps)**Summary for Pond B-L19: B-L19**

Inflow Area = 6.670 ac, 0.00% Impervious, Inflow Depth = 0.46" for 50-Year event  
 Inflow = 7.79 cfs @ 0.40 hrs, Volume= 0.257 af  
 Outflow = 7.79 cfs @ 0.40 hrs, Volume= 0.257 af, Atten= 0%, Lag= 0.0 min  
 Primary = 7.79 cfs @ 0.40 hrs, Volume= 0.257 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 50.38' @ 0.40 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	44.70'	<b>24.0" Round Pipe</b> L= 162.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 44.70' / 43.73' S= 0.0060 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=7.76 cfs @ 0.40 hrs HW=50.38' TW=50.05' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 7.76 cfs @ 2.47 fps)



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**Summary for Pond B-L20: B-L20**

Inflow Area = 6.670 ac, 0.00% Impervious, Inflow Depth = 0.46" for 50-Year event  
 Inflow = 7.79 cfs @ 0.40 hrs, Volume= 0.257 af  
 Outflow = 7.79 cfs @ 0.40 hrs, Volume= 0.257 af, Atten= 0%, Lag= 0.0 min  
 Primary = 7.79 cfs @ 0.40 hrs, Volume= 0.257 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 50.65' @ 0.40 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	46.80'	<b>24.0" Round B-L20</b> L= 26.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 46.80' / 44.70' S= 0.0808 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=7.76 cfs @ 0.40 hrs HW=50.64' TW=50.38' (Dynamic Tailwater)  
 ↑**1=B-L20** (Inlet Controls 7.76 cfs @ 2.47 fps)

**Summary for Pond B-L21: B-L21**

Inflow Area = 1.480 ac, 0.00% Impervious, Inflow Depth = 0.64" for 50-Year event  
 Inflow = 2.40 cfs @ 0.10 hrs, Volume= 0.079 af  
 Outflow = 2.40 cfs @ 0.14 hrs, Volume= 0.079 af, Atten= 0%, Lag= 2.4 min  
 Primary = 2.40 cfs @ 0.14 hrs, Volume= 0.079 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 65.54' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	64.68'	<b>24.0" Round B-L21</b> L= 160.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 64.68' / 63.88' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.40 cfs @ 0.14 hrs HW=65.54' TW=65.00' (Dynamic Tailwater)  
 ↑**1=B-L21** (Outlet Controls 2.40 cfs @ 2.75 fps)

**Summary for Pond B-L22: B-L22**

Inflow Area = 2.840 ac, 0.00% Impervious, Inflow Depth = 0.64" for 50-Year event  
 Inflow = 4.57 cfs @ 0.12 hrs, Volume= 0.151 af  
 Outflow = 4.57 cfs @ 0.13 hrs, Volume= 0.151 af, Atten= 0%, Lag= 0.6 min  
 Primary = 4.57 cfs @ 0.13 hrs, Volume= 0.151 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 65.00' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	63.78'	<b>24.0" Round B-L22</b> L= 74.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 63.78' / 63.41' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=4.57 cfs @ 0.13 hrs HW=65.00' TW=64.60' (Dynamic Tailwater)  
 ↑**1=B-L22** (Outlet Controls 4.57 cfs @ 3.25 fps)

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**Summary for Pond B-L23: B-L23**

Inflow Area = 0.900 ac, 0.00% Impervious, Inflow Depth = 0.78" for 50-Year event  
 Inflow = 1.77 cfs @ 0.09 hrs, Volume= 0.059 af  
 Outflow = 1.77 cfs @ 0.10 hrs, Volume= 0.059 af, Atten= 0%, Lag= 0.6 min  
 Primary = 1.77 cfs @ 0.10 hrs, Volume= 0.059 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 64.74' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	63.80'	<b>24.0" Round B-L23</b> L= 78.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 63.80' / 63.41' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.74 cfs @ 0.10 hrs HW=64.72' TW=64.57' (Dynamic Tailwater)↑**1=B-L23** (Outlet Controls 1.74 cfs @ 1.83 fps)**Summary for Pond B-L24: B-L24**

Inflow Area = 1.330 ac, 0.00% Impervious, Inflow Depth = 0.65" for 50-Year event  
 Inflow = 2.18 cfs @ 0.10 hrs, Volume= 0.072 af  
 Outflow = 2.18 cfs @ 0.10 hrs, Volume= 0.072 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.18 cfs @ 0.10 hrs, Volume= 0.072 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 65.02' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	64.25'	<b>24.0" Round B-L24</b> L= 84.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 64.25' / 63.41' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.17 cfs @ 0.10 hrs HW=65.01' TW=64.57' (Dynamic Tailwater)↑**1=B-L24** (Outlet Controls 2.17 cfs @ 2.94 fps)**Summary for Pond B-L25: B-L25**

Inflow Area = 5.070 ac, 0.00% Impervious, Inflow Depth = 0.67" for 50-Year event  
 Inflow = 8.53 cfs @ 0.13 hrs, Volume= 0.282 af  
 Outflow = 8.53 cfs @ 0.13 hrs, Volume= 0.282 af, Atten= 0%, Lag= 0.0 min  
 Primary = 8.53 cfs @ 0.13 hrs, Volume= 0.282 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 64.60' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	63.31'	<b>30.0" Round Pipe</b> L= 401.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 63.31' / 60.51' S= 0.0070 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=8.53 cfs @ 0.13 hrs HW=64.60' TW=62.29' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 8.53 cfs @ 4.89 fps)



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**Summary for Pond B-L26: B-L26**

Inflow Area = 7.010 ac, 0.00% Impervious, Inflow Depth = 0.67" for 50-Year event  
 Inflow = 11.93 cfs @ 0.13 hrs, Volume= 0.394 af  
 Outflow = 11.93 cfs @ 0.16 hrs, Volume= 0.394 af, Atten= 0%, Lag= 1.9 min  
 Primary = 11.93 cfs @ 0.16 hrs, Volume= 0.394 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 62.29' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	60.40'	<b>30.0" Round B-L26</b> L= 64.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 60.40' / 60.08' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=11.93 cfs @ 0.16 hrs HW=62.29' TW=61.74' (Dynamic Tailwater)  
 ↑**1=B-L26** (Outlet Controls 11.93 cfs @ 4.17 fps)

**Summary for Pond B-L27: B-L27**

Inflow Area = 1.050 ac, 0.00% Impervious, Inflow Depth = 0.63" for 50-Year event  
 Inflow = 1.68 cfs @ 0.10 hrs, Volume= 0.056 af  
 Outflow = 1.68 cfs @ 0.10 hrs, Volume= 0.056 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.68 cfs @ 0.10 hrs, Volume= 0.056 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 61.78' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	60.50'	<b>24.0" Round B-L27</b> L= 46.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 60.50' / 60.18' S= 0.0070 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.58 cfs @ 0.10 hrs HW=61.74' TW=61.70' (Dynamic Tailwater)  
 ↑**1=B-L27** (Outlet Controls 1.58 cfs @ 1.10 fps)

**Summary for Pond B-L28: B-L28**

Inflow Area = 8.060 ac, 0.00% Impervious, Inflow Depth = 0.67" for 50-Year event  
 Inflow = 13.61 cfs @ 0.16 hrs, Volume= 0.450 af  
 Outflow = 13.61 cfs @ 0.16 hrs, Volume= 0.450 af, Atten= 0%, Lag= 0.0 min  
 Primary = 13.61 cfs @ 0.16 hrs, Volume= 0.450 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 61.74' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	59.98'	<b>30.0" Round Pipe</b> L= 404.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 59.98' / 57.15' S= 0.0070 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=13.61 cfs @ 0.16 hrs HW=61.74' TW=59.61' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 13.61 cfs @ 5.19 fps)

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**Summary for Pond B-L29: B-L29**

Inflow Area = 10.020 ac, 0.00% Impervious, Inflow Depth = 0.67" for 50-Year event  
 Inflow = 17.04 cfs @ 0.16 hrs, Volume= 0.563 af  
 Outflow = 17.04 cfs @ 0.16 hrs, Volume= 0.563 af, Atten= 0%, Lag= 0.0 min  
 Primary = 17.04 cfs @ 0.16 hrs, Volume= 0.563 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 59.61' @ 0.16 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	57.05'	<b>30.0" Round B-L29</b> L= 78.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 57.05' / 56.66' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=17.04 cfs @ 0.16 hrs HW=59.61' TW=59.05' (Dynamic Tailwater)  
 ↑**1=B-L29** (Outlet Controls 17.04 cfs @ 4.21 fps)

**Summary for Pond B-L30: B-L30**

Inflow Area = 0.510 ac, 0.00% Impervious, Inflow Depth = 0.67" for 50-Year event  
 Inflow = 0.86 cfs @ 0.10 hrs, Volume= 0.028 af  
 Outflow = 0.86 cfs @ 0.13 hrs, Volume= 0.028 af, Atten= 0%, Lag= 1.8 min  
 Primary = 0.86 cfs @ 0.13 hrs, Volume= 0.028 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 60.54' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	60.15'	<b>24.0" Round B-L30</b> L= 69.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 60.15' / 59.46' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.86 cfs @ 0.13 hrs HW=60.54' TW=59.84' (Dynamic Tailwater)  
 ↑**1=B-L30** (Outlet Controls 0.86 cfs @ 3.05 fps)

**Summary for Pond B-L31: B-L31**

Inflow Area = 0.510 ac, 0.00% Impervious, Inflow Depth = 0.67" for 50-Year event  
 Inflow = 0.86 cfs @ 0.13 hrs, Volume= 0.028 af  
 Outflow = 0.86 cfs @ 0.13 hrs, Volume= 0.028 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.86 cfs @ 0.13 hrs, Volume= 0.028 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 59.84' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	59.36'	<b>24.0" Round Pipe</b> L= 198.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 59.36' / 57.38' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.86 cfs @ 0.13 hrs HW=59.84' TW=59.13' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 0.86 cfs @ 2.26 fps)



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**Summary for Pond B-L32: B-L32**

Inflow Area = 2.060 ac, 0.00% Impervious, Inflow Depth = 0.65" for 50-Year event  
 Inflow = 3.37 cfs @ 0.11 hrs, Volume= 0.111 af  
 Outflow = 3.37 cfs @ 0.10 hrs, Volume= 0.111 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.37 cfs @ 0.10 hrs, Volume= 0.111 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 59.13' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	57.28'	<b>24.0" Round B-L32</b> L= 89.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 57.28' / 56.66' S= 0.0070 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.98 cfs @ 0.10 hrs HW=59.08' TW=59.02' (Dynamic Tailwater)↑**1=B-L32** (Outlet Controls 2.98 cfs @ 1.32 fps)**Summary for Pond B-L33: B-L33**

Inflow Area = 12.080 ac, 0.00% Impervious, Inflow Depth = 0.67" for 50-Year event  
 Inflow = 20.41 cfs @ 0.16 hrs, Volume= 0.675 af  
 Outflow = 20.41 cfs @ 0.16 hrs, Volume= 0.675 af, Atten= 0%, Lag= 0.1 min  
 Primary = 20.41 cfs @ 0.16 hrs, Volume= 0.675 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 59.05' @ 0.12 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.56'	<b>36.0" Round Pipe</b> L= 26.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.56' / 56.43' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=20.41 cfs @ 0.16 hrs HW=59.05' TW=58.54' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 20.41 cfs @ 4.42 fps)**Summary for Pond B-L34: B-L34**

Inflow Area = 12.550 ac, 0.00% Impervious, Inflow Depth = 0.68" for 50-Year event  
 Inflow = 21.35 cfs @ 0.16 hrs, Volume= 0.706 af  
 Outflow = 21.35 cfs @ 0.16 hrs, Volume= 0.706 af, Atten= 0%, Lag= 0.0 min  
 Primary = 21.35 cfs @ 0.16 hrs, Volume= 0.706 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 58.54' @ 0.12 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.33'	<b>36.0" Round B-L34</b> L= 358.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.33' / 54.54' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=21.35 cfs @ 0.16 hrs HW=58.54' TW=56.83' (Dynamic Tailwater)↑**1=B-L34** (Outlet Controls 21.35 cfs @ 5.32 fps)

**Proposed System B**

City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

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**Summary for Subcatchment B01: B01**

Runoff = 2.14 cfs @ 0.10 hrs, Volume= 0.071 af, Depth= 0.67"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
1.270	0.78	Mixed Use, HSG D
1.270		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, PA01

**Summary for Subcatchment B02: B02**

Runoff = 2.99 cfs @ 0.12 hrs, Volume= 0.099 af, Depth= 0.61"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
1.950	0.71	Mixed Use, HSG D
1.950		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.0					Direct Entry, PA01

**Summary for Subcatchment B03: B03**

Runoff = 1.87 cfs @ 0.09 hrs, Volume= 0.062 af, Depth= 0.81"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
0.910	0.95	Mixed Use, HSG D
0.910		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment B04: B04**

Runoff = 3.52 cfs @ 0.10 hrs, Volume= 0.116 af, Depth= 0.63"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr



**Proposed System B**

City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

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Area (ac)	C	Description
2.200	0.74	Mixed Use, HSG D
2.200		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, PA01

**Summary for Subcatchment B05: B05**

Runoff = 1.86 cfs @ 0.12 hrs, Volume= 0.061 af, Depth= 0.63"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
1.160	0.74	Mixed Use, HSG D
1.160		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.0					Direct Entry, PA01

**Summary for Subcatchment B06: B06**

Runoff = 1.77 cfs @ 0.09 hrs, Volume= 0.058 af, Depth= 0.80"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
0.880	0.93	Mixed Use, HSG D
0.880		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment B07: B07**

Runoff = 3.45 cfs @ 0.10 hrs, Volume= 0.114 af, Depth= 0.64"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
2.130	0.75	Mixed Use, HSG D
2.130		100.00% Pervious Area

**Proposed System B**

City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment B08: B08**

Runoff = 1.78 cfs @ 0.10 hrs, Volume= 0.059 af, Depth= 0.63"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
1.110	0.74	Mixed Use, HSG D
1.110		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment B09: B09**

Runoff = 0.59 cfs @ 0.09 hrs, Volume= 0.020 af, Depth= 0.78"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
0.300	0.91	Mixed Use, HSG D
0.300		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment B10: B17**

Runoff = 16.56 cfs @ 0.37 hrs, Volume= 0.547 af, Depth= 0.52"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
12.560	0.61	Mixed Use, HSG D
12.560		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
22.0					<b>Direct Entry, PA01</b>



**Proposed System B**

City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

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**Summary for Subcatchment B12: B12**

Runoff = 2.40 cfs @ 0.10 hrs, Volume= 0.079 af, Depth= 0.64"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
1.480	0.75	Mixed Use, HSG D
1.480		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, PA01

**Summary for Subcatchment B13: B13**

Runoff = 2.18 cfs @ 0.12 hrs, Volume= 0.072 af, Depth= 0.63"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
1.360	0.74	Mixed Use, HSG D
1.360		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.0					Direct Entry, PA01

**Summary for Subcatchment B14: B14**

Runoff = 1.77 cfs @ 0.09 hrs, Volume= 0.059 af, Depth= 0.78"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
0.900	0.91	Mixed Use, HSG D
0.900		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment B15: B15**

Runoff = 2.18 cfs @ 0.10 hrs, Volume= 0.072 af, Depth= 0.65"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

**Proposed System B**

City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

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Area (ac)	C	Description
1.330	0.76	Mixed Use, HSG D
1.330		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, PA01

**Summary for Subcatchment B16: B16**

Runoff = 3.40 cfs @ 0.09 hrs, Volume= 0.112 af, Depth= 0.69"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
1.940	0.81	Mixed Use, HSG D
1.940		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment B17: B17**

Runoff = 1.68 cfs @ 0.10 hrs, Volume= 0.056 af, Depth= 0.63"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
1.050	0.74	Mixed Use, HSG D
1.050		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, PA01

**Summary for Subcatchment B18: B18**

Runoff = 3.43 cfs @ 0.09 hrs, Volume= 0.113 af, Depth= 0.69"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
1.960	0.81	Mixed Use, HSG D
1.960		100.00% Pervious Area



**Proposed System B**

City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment B19: B19**

Runoff = 0.86 cfs @ 0.10 hrs, Volume= 0.028 af, Depth= 0.67"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
0.510	0.78	Mixed Use, HSG D
0.510		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment B20: B20**

Runoff = 2.51 cfs @ 0.10 hrs, Volume= 0.083 af, Depth= 0.64"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
1.550	0.75	Mixed Use, HSG D
1.550		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment B21: B21**

Runoff = 0.94 cfs @ 0.09 hrs, Volume= 0.031 af, Depth= 0.80"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
0.470	0.93	Mixed Use, HSG D
0.470		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Proposed System B**

City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

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**Summary for Subcatchment BASIN-B1: BASIN-B1**

Runoff = 2.18 cfs @ 0.17 hrs, Volume= 0.072 af, Depth= 0.46"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
1.870	0.54	Mixed Use, HSG D
1.870		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					Direct Entry, PA01

**Summary for Subcatchment BASIN-B2: BASIN-B2**

Runoff = 5.60 cfs @ 0.40 hrs, Volume= 0.185 af, Depth= 0.46"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=24 min, Inten=2.14 in/hr

Area (ac)	C	Description
4.800	0.54	Mixed Use, HSG D
4.800		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.0					Direct Entry, PA01

**Summary for Pond OUTFALL B: OUTFALL B**

Inflow Area = 43.690 ac, 2.08% Impervious, Inflow Depth = 0.60" for 50-Year event

Inflow = 63.11 cfs @ 0.41 hrs, Volume= 2.174 af

Primary = 63.11 cfs @ 0.41 hrs, Volume= 2.174 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3



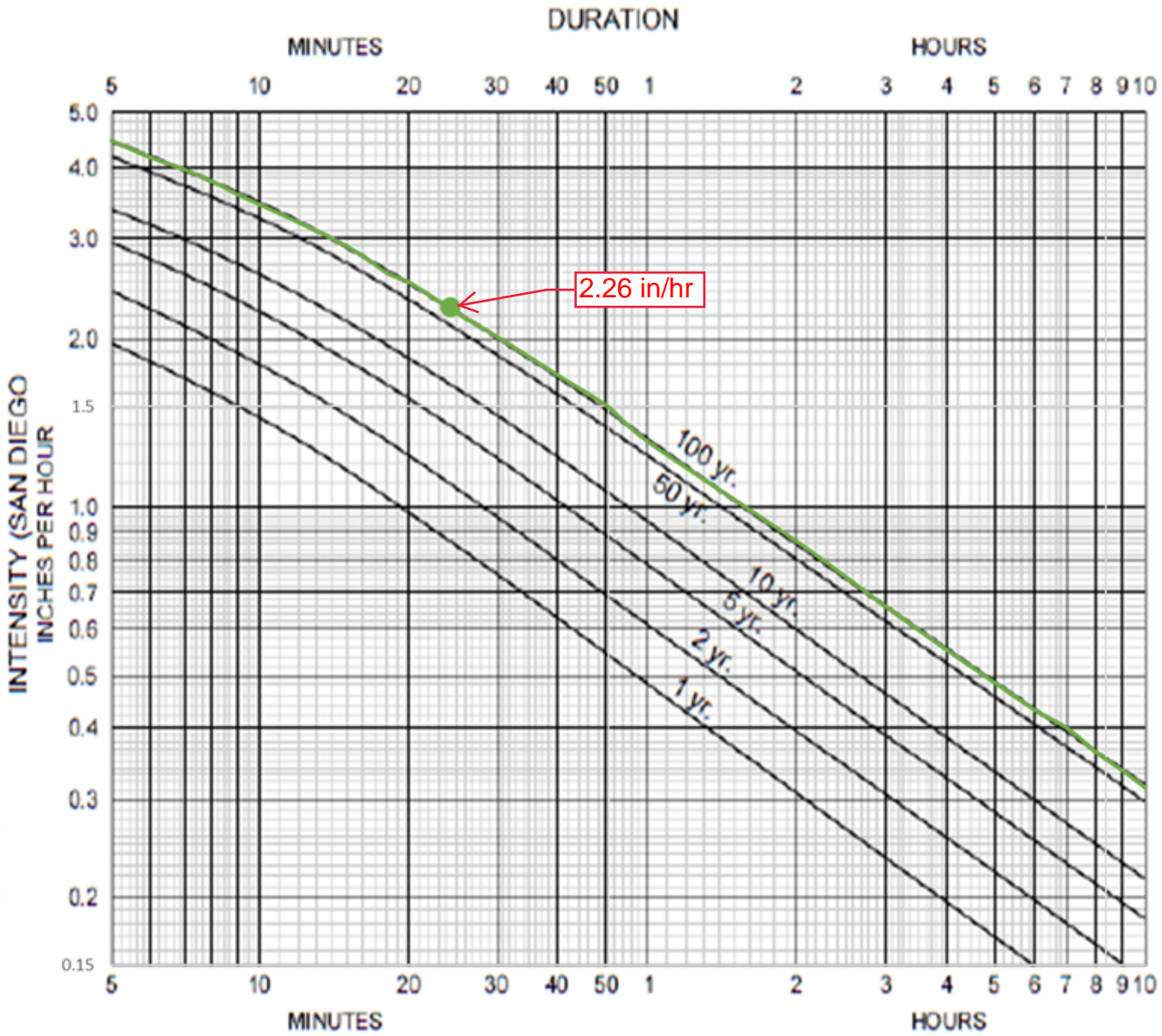
100-year Rainfall Intensity

ELEV.	FACTOR
0-1500	1.00
1500-3000	1.25
3000-4000	1.42
4000-5000	1.60
5000-6000	1.7
DESERT	1.25

Figure A-1. Intensity-Duration-Frequency Design Chart

TO OBTAIN CORRECT INTENSITY, MULTIPLY INTENSITY ON CHART BY FACTOR FOR DESIGN ELEVATION.

INTENSITY - DURATION - FREQUENCY RAINFALL CURVES FOR COUNTY OF SAN DIEGO



— 100 year    ● 100 year event

**Proposed System B**

City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

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Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points x 3

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Pond B-BUBBLER: B-BUBBLER</b>	Peak Elev=50.14' Storage=1,689 cf Inflow=68.99 cfs 2.281 af Primary=63.78 cfs 2.284 af Secondary=0.00 cfs 0.000 af Outflow=63.78 cfs 2.284 af
<b>Pond B-L01: B-L01</b>	Peak Elev=64.89' Inflow=2.25 cfs 0.074 af 24.0" Round Culvert n=0.013 L=62.0' S=0.0037 '/' Outflow=2.25 cfs 0.074 af
<b>Pond B-L02: B-L02</b>	Peak Elev=65.00' Inflow=3.14 cfs 0.104 af 24.0" Round Culvert n=0.013 L=60.0' S=0.0048 '/' Outflow=3.14 cfs 0.104 af
<b>Pond B-L03: B-L03</b>	Peak Elev=64.74' Inflow=5.39 cfs 0.178 af 24.0" Round Culvert n=0.013 L=399.0' S=0.0055 '/' Outflow=5.39 cfs 0.178 af
<b>Pond B-L04: B-L04</b>	Peak Elev=63.05' Inflow=7.36 cfs 0.243 af 24.0" Round Culvert n=0.013 L=63.0' S=0.0051 '/' Outflow=7.36 cfs 0.243 af
<b>Pond B-L05: B-L05</b>	Peak Elev=62.83' Inflow=3.70 cfs 0.122 af 24.0" Round Culvert n=0.013 L=57.0' S=0.0051 '/' Outflow=3.70 cfs 0.122 af
<b>Pond B-L06: B-L06</b>	Peak Elev=62.75' Inflow=1.95 cfs 0.064 af 24.0" Round Culvert n=0.013 L=73.0' S=0.0051 '/' Outflow=1.95 cfs 0.064 af
<b>Pond B-L07: B-L07</b>	Peak Elev=62.69' Inflow=13.00 cfs 0.430 af 30.0" Round Culvert n=0.013 L=424.0' S=0.0050 '/' Outflow=13.00 cfs 0.430 af
<b>Pond B-L08: B-L08</b>	Peak Elev=60.98' Inflow=14.86 cfs 0.491 af 36.0" Round Culvert n=0.013 L=64.0' S=0.0045 '/' Outflow=14.86 cfs 0.491 af
<b>Pond B-L09: B-L09</b>	Peak Elev=60.69' Inflow=3.63 cfs 0.120 af 24.0" Round Culvert n=0.013 L=58.0' S=0.0050 '/' Outflow=3.63 cfs 0.120 af
<b>Pond B-L10: B-L8</b>	Peak Elev=60.66' Inflow=1.87 cfs 0.062 af 24.0" Round Culvert n=0.013 L=72.0' S=0.0069 '/' Outflow=1.87 cfs 0.062 af
<b>Pond B-L11: B-L11</b>	Peak Elev=60.63' Inflow=20.35 cfs 0.673 af 36.0" Round Culvert n=0.013 L=48.0' S=0.0050 '/' Outflow=20.35 cfs 0.673 af
<b>Pond B-L12: B-L21</b>	Peak Elev=60.04' Inflow=20.97 cfs 0.693 af 36.0" Round Culvert n=0.013 L=372.0' S=0.0050 '/' Outflow=20.97 cfs 0.693 af
<b>Pond B-L13: B-L13</b>	Peak Elev=58.21' Inflow=20.97 cfs 0.693 af 36.0" Round Culvert n=0.013 L=278.0' S=0.0050 '/' Outflow=20.97 cfs 0.693 af
<b>Pond B-L14: B-L14</b>	Peak Elev=56.90' Inflow=43.41 cfs 1.435 af 48.0" Round Culvert n=0.013 L=180.0' S=0.0363 '/' Outflow=43.41 cfs 1.435 af
<b>Pond B-L15: B-L15</b>	Peak Elev=51.46' Inflow=43.41 cfs 1.435 af 48.0" Round Culvert n=0.013 L=188.0' S=0.0170 '/' Outflow=43.41 cfs 1.435 af
<b>Pond B-L16: B-L16</b>	Peak Elev=50.65' Inflow=43.41 cfs 1.435 af 48.0" Round Culvert n=0.013 L=38.0' S=0.0024 '/' Outflow=43.41 cfs 1.435 af



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<b>Pond B-L18: B-L18</b>	Peak Elev=51.67' Inflow=5.89 cfs 0.195 af 18.0" Round Culvert n=0.013 L=200.0' S=0.0060 '/ Outflow=5.89 cfs 0.195 af
<b>Pond B-L19: B-L19</b>	Peak Elev=50.50' Inflow=8.18 cfs 0.270 af 24.0" Round Culvert n=0.013 L=162.0' S=0.0060 '/ Outflow=8.18 cfs 0.270 af
<b>Pond B-L20: B-L20</b>	Peak Elev=50.80' Inflow=8.18 cfs 0.270 af 24.0" Round Culvert n=0.013 L=26.0' S=0.0808 '/ Outflow=8.18 cfs 0.270 af
<b>Pond B-L21: B-L21</b>	Peak Elev=65.57' Inflow=2.52 cfs 0.083 af 24.0" Round Culvert n=0.013 L=160.0' S=0.0050 '/ Outflow=2.52 cfs 0.083 af
<b>Pond B-L22: B-L22</b>	Peak Elev=65.05' Inflow=4.81 cfs 0.159 af 24.0" Round Culvert n=0.013 L=74.0' S=0.0050 '/ Outflow=4.81 cfs 0.159 af
<b>Pond B-L23: B-L23</b>	Peak Elev=64.78' Inflow=1.86 cfs 0.061 af 24.0" Round Culvert n=0.013 L=78.0' S=0.0050 '/ Outflow=1.86 cfs 0.061 af
<b>Pond B-L24: B-L24</b>	Peak Elev=65.05' Inflow=2.30 cfs 0.076 af 24.0" Round Culvert n=0.013 L=84.0' S=0.0100 '/ Outflow=2.30 cfs 0.076 af
<b>Pond B-L25: B-L25</b>	Peak Elev=64.64' Inflow=8.96 cfs 0.296 af 30.0" Round Culvert n=0.013 L=401.0' S=0.0070 '/ Outflow=8.96 cfs 0.296 af
<b>Pond B-L26: B-L26</b>	Peak Elev=62.35' Inflow=12.53 cfs 0.414 af 30.0" Round Culvert n=0.013 L=64.0' S=0.0050 '/ Outflow=12.53 cfs 0.414 af
<b>Pond B-L27: B-L27</b>	Peak Elev=61.84' Inflow=1.76 cfs 0.058 af 24.0" Round Culvert n=0.013 L=46.0' S=0.0070 '/ Outflow=1.76 cfs 0.058 af
<b>Pond B-L28: B-L28</b>	Peak Elev=61.80' Inflow=14.29 cfs 0.473 af 30.0" Round Culvert n=0.013 L=404.0' S=0.0070 '/ Outflow=14.29 cfs 0.473 af
<b>Pond B-L29: B-L29</b>	Peak Elev=59.71' Inflow=17.90 cfs 0.592 af 30.0" Round Culvert n=0.013 L=78.0' S=0.0050 '/ Outflow=17.90 cfs 0.592 af
<b>Pond B-L30: B-L30</b>	Peak Elev=60.55' Inflow=0.90 cfs 0.030 af 24.0" Round Culvert n=0.013 L=69.0' S=0.0100 '/ Outflow=0.90 cfs 0.030 af
<b>Pond B-L31: B-L31</b>	Peak Elev=59.86' Inflow=0.90 cfs 0.030 af 24.0" Round Culvert n=0.013 L=198.0' S=0.0100 '/ Outflow=0.90 cfs 0.030 af
<b>Pond B-L32: B-L32</b>	Peak Elev=59.21' Inflow=3.54 cfs 0.117 af 24.0" Round Culvert n=0.013 L=89.0' S=0.0070 '/ Outflow=3.54 cfs 0.117 af
<b>Pond B-L33: B-L33</b>	Peak Elev=59.13' Inflow=21.44 cfs 0.709 af 36.0" Round Culvert n=0.013 L=26.0' S=0.0050 '/ Outflow=21.44 cfs 0.709 af
<b>Pond B-L34: B-L34</b>	Peak Elev=58.62' Inflow=22.43 cfs 0.742 af 36.0" Round Culvert n=0.013 L=358.0' S=0.0050 '/ Outflow=22.43 cfs 0.742 af
<b>Subcatchment B01: B01</b>	Runoff Area=1.270 ac 0.00% Impervious Runoff Depth=0.70" Tc=6.0 min C=0.78 Runoff=2.25 cfs 0.074 af

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<b>Subcatchment B02: B02</b>	Runoff Area=1.950 ac 0.00% Impervious Runoff Depth=0.64" Tc=7.0 min C=0.71 Runoff=3.14 cfs 0.104 af
<b>Subcatchment B03: B03</b>	Runoff Area=0.910 ac 100.00% Impervious Runoff Depth=0.86" Tc=5.0 min C=0.95 Runoff=1.96 cfs 0.065 af
<b>Subcatchment B04: B04</b>	Runoff Area=2.200 ac 0.00% Impervious Runoff Depth=0.67" Tc=6.0 min C=0.74 Runoff=3.70 cfs 0.122 af
<b>Subcatchment B05: B05</b>	Runoff Area=1.160 ac 0.00% Impervious Runoff Depth=0.67" Tc=7.0 min C=0.74 Runoff=1.95 cfs 0.064 af
<b>Subcatchment B06: B06</b>	Runoff Area=0.880 ac 0.00% Impervious Runoff Depth=0.84" Tc=5.0 min C=0.93 Runoff=1.86 cfs 0.061 af
<b>Subcatchment B07: B07</b>	Runoff Area=2.130 ac 0.00% Impervious Runoff Depth=0.68" Tc=6.0 min C=0.75 Runoff=3.63 cfs 0.120 af
<b>Subcatchment B08: B08</b>	Runoff Area=1.110 ac 0.00% Impervious Runoff Depth=0.67" Tc=6.0 min C=0.74 Runoff=1.87 cfs 0.062 af
<b>Subcatchment B09: B09</b>	Runoff Area=0.300 ac 0.00% Impervious Runoff Depth=0.82" Tc=5.0 min C=0.91 Runoff=0.62 cfs 0.020 af
<b>Subcatchment B10: B17</b>	Runoff Area=12.560 ac 0.00% Impervious Runoff Depth=0.55" Tc=22.0 min C=0.61 Runoff=17.40 cfs 0.575 af
<b>Subcatchment B12: B12</b>	Runoff Area=1.480 ac 0.00% Impervious Runoff Depth=0.68" Tc=6.0 min C=0.75 Runoff=2.52 cfs 0.083 af
<b>Subcatchment B13: B13</b>	Runoff Area=1.360 ac 0.00% Impervious Runoff Depth=0.67" Tc=7.0 min C=0.74 Runoff=2.29 cfs 0.076 af
<b>Subcatchment B14: B14</b>	Runoff Area=0.900 ac 0.00% Impervious Runoff Depth=0.82" Tc=5.0 min C=0.91 Runoff=1.86 cfs 0.061 af
<b>Subcatchment B15: B15</b>	Runoff Area=1.330 ac 0.00% Impervious Runoff Depth=0.68" Tc=6.0 min C=0.76 Runoff=2.30 cfs 0.076 af
<b>Subcatchment B16: B16</b>	Runoff Area=1.940 ac 0.00% Impervious Runoff Depth=0.73" Tc=5.0 min C=0.81 Runoff=3.57 cfs 0.118 af
<b>Subcatchment B17: B17</b>	Runoff Area=1.050 ac 0.00% Impervious Runoff Depth=0.67" Tc=6.0 min C=0.74 Runoff=1.76 cfs 0.058 af
<b>Subcatchment B18: B18</b>	Runoff Area=1.960 ac 0.00% Impervious Runoff Depth=0.73" Tc=5.0 min C=0.81 Runoff=3.61 cfs 0.119 af
<b>Subcatchment B19: B19</b>	Runoff Area=0.510 ac 0.00% Impervious Runoff Depth=0.70" Tc=6.0 min C=0.78 Runoff=0.90 cfs 0.030 af
<b>Subcatchment B20: B20</b>	Runoff Area=1.550 ac 0.00% Impervious Runoff Depth=0.68" Tc=6.0 min C=0.75 Runoff=2.64 cfs 0.087 af



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**Subcatchment B21: B21**Runoff Area=0.470 ac 0.00% Impervious Runoff Depth=0.84"  
Tc=5.0 min C=0.93 Runoff=0.99 cfs 0.033 af**Subcatchment BASIN-B1: BASIN-B1**Runoff Area=1.870 ac 0.00% Impervious Runoff Depth=0.49"  
Tc=10.0 min C=0.54 Runoff=2.29 cfs 0.076 af**Subcatchment BASIN-B2: BASIN-B2**Runoff Area=4.800 ac 0.00% Impervious Runoff Depth=0.49"  
Tc=24.0 min C=0.54 Runoff=5.89 cfs 0.195 af**Pond OUTFALL B: OUTFALL B**Inflow=63.78 cfs 2.284 af  
Primary=63.78 cfs 2.284 af**Total Runoff Area = 43.690 ac Runoff Volume = 2.281 af Average Runoff Depth = 0.63"**  
**97.92% Pervious = 42.780 ac 2.08% Impervious = 0.910 ac**

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**Summary for Pond B-BUBBLER: B-BUBBLER**

Inflow Area = 43.690 ac, 2.08% Impervious, Inflow Depth = 0.63" for 100-Year event  
 Inflow = 68.99 cfs @ 0.40 hrs, Volume= 2.281 af  
 Outflow = 63.78 cfs @ 0.41 hrs, Volume= 2.284 af, Atten= 8%, Lag= 0.6 min  
 Primary = 63.78 cfs @ 0.41 hrs, Volume= 2.284 af  
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 50.14' @ 0.41 hrs Surf.Area= 12,732 sf Storage= 1,689 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 0.1 min ( 17.9 - 17.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	50.00'	212,572 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
50.00	11,326	0	0
52.00	31,363	42,689	42,689
54.00	43,124	74,487	117,176
56.00	52,272	95,396	212,572

Device	Routing	Invert	Outlet Devices
#1	Primary	44.31'	<b>34.0" Round Outlet</b> L= 100.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 44.31' / 43.59' S= 0.0072 '/ Cc= 0.900 n= 0.013, Flow Area= 6.31 sf
#2	Secondary	50.25'	<b>80.0' long x 20.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Primary OutFlow** Max=63.78 cfs @ 0.41 hrs HW=50.14' TW=0.00' (Dynamic Tailwater)  
 ↖1=Outlet (Inlet Controls 63.78 cfs @ 10.12 fps)

**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=50.00' TW=0.00' (Dynamic Tailwater)  
 ↖2=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)

**Summary for Pond B-L01: B-L01**

Inflow Area = 1.270 ac, 0.00% Impervious, Inflow Depth = 0.70" for 100-Year event  
 Inflow = 2.25 cfs @ 0.10 hrs, Volume= 0.074 af  
 Outflow = 2.25 cfs @ 0.10 hrs, Volume= 0.074 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.25 cfs @ 0.10 hrs, Volume= 0.074 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 64.89' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	63.87'	<b>24.0" Round B-L1</b> L= 62.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 63.87' / 63.64' S= 0.0037 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf



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**Primary OutFlow** Max=2.20 cfs @ 0.10 hrs HW=64.85' TW=64.67' (Dynamic Tailwater)↑**1=B-L1** (Outlet Controls 2.20 cfs @ 2.11 fps)**Summary for Pond B-L02: B-L02**

Inflow Area = 1.950 ac, 0.00% Impervious, Inflow Depth = 0.64" for 100-Year event  
 Inflow = 3.14 cfs @ 0.12 hrs, Volume= 0.104 af  
 Outflow = 3.14 cfs @ 0.13 hrs, Volume= 0.104 af, Atten= 0%, Lag= 0.6 min  
 Primary = 3.14 cfs @ 0.13 hrs, Volume= 0.104 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 65.00' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	63.93'	<b>24.0" Round B-L2</b> L= 60.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 63.93' / 63.64' S= 0.0048 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=3.14 cfs @ 0.13 hrs HW=65.00' TW=64.74' (Dynamic Tailwater)↑**1=B-L2** (Outlet Controls 3.14 cfs @ 2.66 fps)**Summary for Pond B-L03: B-L03**

Inflow Area = 3.220 ac, 0.00% Impervious, Inflow Depth = 0.66" for 100-Year event  
 Inflow = 5.39 cfs @ 0.13 hrs, Volume= 0.178 af  
 Outflow = 5.39 cfs @ 0.13 hrs, Volume= 0.178 af, Atten= 0%, Lag= 0.0 min  
 Primary = 5.39 cfs @ 0.13 hrs, Volume= 0.178 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 64.74' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	63.54'	<b>24.0" Round Pipe</b> L= 399.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 63.54' / 61.36' S= 0.0055 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=5.39 cfs @ 0.13 hrs HW=64.74' TW=63.05' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 5.39 cfs @ 3.95 fps)**Summary for Pond B-L04: B-L04**

Inflow Area = 4.130 ac, 22.03% Impervious, Inflow Depth = 0.71" for 100-Year event  
 Inflow = 7.36 cfs @ 0.13 hrs, Volume= 0.243 af  
 Outflow = 7.36 cfs @ 0.12 hrs, Volume= 0.243 af, Atten= 0%, Lag= 0.0 min  
 Primary = 7.36 cfs @ 0.12 hrs, Volume= 0.243 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 63.05' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	61.26'	<b>24.0" Round B-L4</b> L= 63.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 61.26' / 60.94' S= 0.0051 '/ Cc= 0.900

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n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=7.33 cfs @ 0.12 hrs HW=63.05' TW=62.69' (Dynamic Tailwater)↑**1=B-L4** (Outlet Controls 7.33 cfs @ 3.28 fps)**Summary for Pond B-L05: B-L05**

Inflow Area = 2.200 ac, 0.00% Impervious, Inflow Depth = 0.67" for 100-Year event  
 Inflow = 3.70 cfs @ 0.10 hrs, Volume= 0.122 af  
 Outflow = 3.70 cfs @ 0.12 hrs, Volume= 0.122 af, Atten= 0%, Lag= 1.2 min  
 Primary = 3.70 cfs @ 0.12 hrs, Volume= 0.122 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 62.83' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	61.33'	<b>24.0" Round B-L5</b> L= 57.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 61.33' / 61.04' S= 0.0051 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=3.67 cfs @ 0.12 hrs HW=62.83' TW=62.69' (Dynamic Tailwater)↑**1=B-L5** (Outlet Controls 3.67 cfs @ 2.02 fps)**Summary for Pond B-L06: B-L06**

Inflow Area = 1.160 ac, 0.00% Impervious, Inflow Depth = 0.67" for 100-Year event  
 Inflow = 1.95 cfs @ 0.12 hrs, Volume= 0.064 af  
 Outflow = 1.95 cfs @ 0.12 hrs, Volume= 0.064 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.95 cfs @ 0.12 hrs, Volume= 0.064 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 62.75' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	61.41'	<b>24.0" Round B-L6</b> L= 73.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 61.41' / 61.04' S= 0.0051 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.91 cfs @ 0.12 hrs HW=62.74' TW=62.69' (Dynamic Tailwater)↑**1=B-L6** (Outlet Controls 1.91 cfs @ 1.22 fps)**Summary for Pond B-L07: B-L07**

Inflow Area = 7.490 ac, 12.15% Impervious, Inflow Depth = 0.69" for 100-Year event  
 Inflow = 13.00 cfs @ 0.12 hrs, Volume= 0.430 af  
 Outflow = 13.00 cfs @ 0.12 hrs, Volume= 0.430 af, Atten= 0%, Lag= 0.0 min  
 Primary = 13.00 cfs @ 0.12 hrs, Volume= 0.430 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 62.69' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	60.84'	<b>30.0" Round Pipe</b> L= 424.0' RCP, sq.cut end projecting, Ke= 0.500



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Inlet / Outlet Invert= 60.84' / 58.72' S= 0.0050 '/ Cc= 0.900  
 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=12.98 cfs @ 0.12 hrs HW=62.69' TW=60.98' (Dynamic Tailwater)

↑**1=Pipe** (Outlet Controls 12.98 cfs @ 4.64 fps)

**Summary for Pond B-L08: B-L08**

Inflow Area = 8.370 ac, 10.87% Impervious, Inflow Depth = 0.70" for 100-Year event  
 Inflow = 14.86 cfs @ 0.12 hrs, Volume= 0.491 af  
 Outflow = 14.86 cfs @ 0.12 hrs, Volume= 0.491 af, Atten= 0%, Lag= 0.0 min  
 Primary = 14.86 cfs @ 0.12 hrs, Volume= 0.491 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 60.98' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	58.62'	<b>36.0" Round B-L8</b> L= 64.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 58.62' / 58.33' S= 0.0045 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=14.81 cfs @ 0.12 hrs HW=60.98' TW=60.63' (Dynamic Tailwater)

↑**1=B-L8** (Outlet Controls 14.81 cfs @ 3.42 fps)

**Summary for Pond B-L09: B-L09**

Inflow Area = 2.130 ac, 0.00% Impervious, Inflow Depth = 0.68" for 100-Year event  
 Inflow = 3.63 cfs @ 0.10 hrs, Volume= 0.120 af  
 Outflow = 3.63 cfs @ 0.13 hrs, Volume= 0.120 af, Atten= 0%, Lag= 1.8 min  
 Primary = 3.63 cfs @ 0.13 hrs, Volume= 0.120 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 60.69' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	58.62'	<b>24.0" Round B-L9</b> L= 58.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 58.62' / 58.33' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=3.63 cfs @ 0.13 hrs HW=60.69' TW=60.63' (Dynamic Tailwater)

↑**1=B-L9** (Outlet Controls 3.63 cfs @ 1.38 fps)

**Summary for Pond B-L10: B-L8**

Inflow Area = 1.110 ac, 0.00% Impervious, Inflow Depth = 0.67" for 100-Year event  
 Inflow = 1.87 cfs @ 0.10 hrs, Volume= 0.062 af  
 Outflow = 1.87 cfs @ 0.11 hrs, Volume= 0.062 af, Atten= 0%, Lag= 0.6 min  
 Primary = 1.87 cfs @ 0.11 hrs, Volume= 0.062 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 60.66' @ 0.13 hrs

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Device	Routing	Invert	Outlet Devices
#1	Primary	58.83'	<b>24.0" Round B-L10</b> L= 72.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 58.83' / 58.33' S= 0.0069 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.72 cfs @ 0.11 hrs HW=60.63' TW=60.61' (Dynamic Tailwater)↑**1=B-L10** (Outlet Controls 1.72 cfs @ 0.76 fps)**Summary for Pond B-L11: B-L11**

Inflow Area = 11.610 ac, 7.84% Impervious, Inflow Depth = 0.70" for 100-Year event  
 Inflow = 20.35 cfs @ 0.13 hrs, Volume= 0.673 af  
 Outflow = 20.35 cfs @ 0.12 hrs, Volume= 0.673 af, Atten= 0%, Lag= 0.0 min  
 Primary = 20.35 cfs @ 0.12 hrs, Volume= 0.673 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 60.63' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	58.23'	<b>36.0" Round Pipe</b> L= 48.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 58.23' / 57.99' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=20.33 cfs @ 0.12 hrs HW=60.63' TW=60.04' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 20.33 cfs @ 4.58 fps)**Summary for Pond B-L12: B-L21**

Inflow Area = 11.910 ac, 7.64% Impervious, Inflow Depth = 0.70" for 100-Year event  
 Inflow = 20.97 cfs @ 0.12 hrs, Volume= 0.693 af  
 Outflow = 20.97 cfs @ 0.13 hrs, Volume= 0.693 af, Atten= 0%, Lag= 0.6 min  
 Primary = 20.97 cfs @ 0.13 hrs, Volume= 0.693 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 60.04' @ 0.12 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	57.89'	<b>36.0" Round B-L12</b> L= 372.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 57.89' / 56.03' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=20.97 cfs @ 0.13 hrs HW=60.04' TW=58.21' (Dynamic Tailwater)↑**1=B-L12** (Outlet Controls 20.97 cfs @ 5.40 fps)**Summary for Pond B-L13: B-L13**

Inflow Area = 11.910 ac, 7.64% Impervious, Inflow Depth = 0.70" for 100-Year event  
 Inflow = 20.97 cfs @ 0.13 hrs, Volume= 0.693 af  
 Outflow = 20.97 cfs @ 0.13 hrs, Volume= 0.693 af, Atten= 0%, Lag= 0.0 min  
 Primary = 20.97 cfs @ 0.13 hrs, Volume= 0.693 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3



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Peak Elev= 58.21' @ 0.12 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	55.93'	<b>36.0" Round Pipe</b> L= 278.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 55.93' / 54.54' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=20.97 cfs @ 0.13 hrs HW=58.21' TW=56.90' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 20.97 cfs @ 5.04 fps)**Summary for Pond B-L14: B-L14**

Inflow Area = 24.460 ac, 3.72% Impervious, Inflow Depth = 0.70" for 100-Year event  
 Inflow = 43.41 cfs @ 0.13 hrs, Volume= 1.435 af  
 Outflow = 43.41 cfs @ 0.13 hrs, Volume= 1.435 af, Atten= 0%, Lag= 0.0 min  
 Primary = 43.41 cfs @ 0.13 hrs, Volume= 1.435 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 56.90' @ 0.12 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	54.44'	<b>48.0" Round Pipe</b> L= 180.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 54.44' / 47.90' S= 0.0363 '/ Cc= 0.900 n= 0.013, Flow Area= 12.57 sf

**Primary OutFlow** Max=43.41 cfs @ 0.13 hrs HW=56.90' TW=51.35' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 43.41 cfs @ 5.34 fps)**Summary for Pond B-L15: B-L15**

Inflow Area = 24.460 ac, 3.72% Impervious, Inflow Depth = 0.70" for 100-Year event  
 Inflow = 43.41 cfs @ 0.13 hrs, Volume= 1.435 af  
 Outflow = 43.41 cfs @ 0.13 hrs, Volume= 1.435 af, Atten= 0%, Lag= 0.0 min  
 Primary = 43.41 cfs @ 0.13 hrs, Volume= 1.435 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 51.46' @ 0.40 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	47.80'	<b>48.0" Round Pipe</b> L= 188.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 47.80' / 44.60' S= 0.0170 '/ Cc= 0.900 n= 0.013, Flow Area= 12.57 sf

**Primary OutFlow** Max=43.41 cfs @ 0.13 hrs HW=51.35' TW=50.51' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 43.41 cfs @ 4.88 fps)**Summary for Pond B-L16: B-L16**

Inflow Area = 24.460 ac, 3.72% Impervious, Inflow Depth = 0.70" for 100-Year event  
 Inflow = 43.41 cfs @ 0.13 hrs, Volume= 1.435 af  
 Outflow = 43.41 cfs @ 0.13 hrs, Volume= 1.435 af, Atten= 0%, Lag= 0.0 min  
 Primary = 43.41 cfs @ 0.13 hrs, Volume= 1.435 af

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Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 50.65' @ 0.40 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	44.50'	<b>48.0" Round Pipe</b> L= 38.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 44.50' / 44.41' S= 0.0024 '/ Cc= 0.900 n= 0.013, Flow Area= 12.57 sf

**Primary OutFlow** Max=43.41 cfs @ 0.13 hrs HW=50.51' TW=50.00' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 43.41 cfs @ 3.45 fps)**Summary for Pond B-L18: B-L18**

Inflow Area = 4.800 ac, 0.00% Impervious, Inflow Depth = 0.49" for 100-Year event  
 Inflow = 5.89 cfs @ 0.40 hrs, Volume= 0.195 af  
 Outflow = 5.89 cfs @ 0.40 hrs, Volume= 0.195 af, Atten= 0%, Lag= 0.0 min  
 Primary = 5.89 cfs @ 0.40 hrs, Volume= 0.195 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 51.67' @ 0.40 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	45.90'	<b>18.0" Round B-L18</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 45.90' / 44.70' S= 0.0060 '/ Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

**Primary OutFlow** Max=5.84 cfs @ 0.40 hrs HW=51.67' TW=50.80' (Dynamic Tailwater)↑**1=B-L18** (Outlet Controls 5.84 cfs @ 3.30 fps)**Summary for Pond B-L19: B-L19**

Inflow Area = 6.670 ac, 0.00% Impervious, Inflow Depth = 0.49" for 100-Year event  
 Inflow = 8.18 cfs @ 0.40 hrs, Volume= 0.270 af  
 Outflow = 8.18 cfs @ 0.40 hrs, Volume= 0.270 af, Atten= 0%, Lag= 0.0 min  
 Primary = 8.18 cfs @ 0.40 hrs, Volume= 0.270 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 50.50' @ 0.40 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	44.70'	<b>24.0" Round Pipe</b> L= 162.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 44.70' / 43.73' S= 0.0060 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=8.16 cfs @ 0.40 hrs HW=50.50' TW=50.13' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 8.16 cfs @ 2.60 fps)

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**Summary for Pond B-L20: B-L20**

Inflow Area = 6.670 ac, 0.00% Impervious, Inflow Depth = 0.49" for 100-Year event  
 Inflow = 8.18 cfs @ 0.40 hrs, Volume= 0.270 af  
 Outflow = 8.18 cfs @ 0.40 hrs, Volume= 0.270 af, Atten= 0%, Lag= 0.0 min  
 Primary = 8.18 cfs @ 0.40 hrs, Volume= 0.270 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 50.80' @ 0.40 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	46.80'	<b>24.0" Round B-L20</b> L= 26.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 46.80' / 44.70' S= 0.0808 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=8.16 cfs @ 0.40 hrs HW=50.79' TW=50.50' (Dynamic Tailwater)  
 ↑**1=B-L20** (Inlet Controls 8.16 cfs @ 2.60 fps)

**Summary for Pond B-L21: B-L21**

Inflow Area = 1.480 ac, 0.00% Impervious, Inflow Depth = 0.68" for 100-Year event  
 Inflow = 2.52 cfs @ 0.10 hrs, Volume= 0.083 af  
 Outflow = 2.52 cfs @ 0.10 hrs, Volume= 0.083 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.52 cfs @ 0.10 hrs, Volume= 0.083 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 65.57' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	64.68'	<b>24.0" Round B-L21</b> L= 160.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 64.68' / 63.88' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.51 cfs @ 0.10 hrs HW=65.55' TW=65.00' (Dynamic Tailwater)  
 ↑**1=B-L21** (Outlet Controls 2.51 cfs @ 2.81 fps)

**Summary for Pond B-L22: B-L22**

Inflow Area = 2.840 ac, 0.00% Impervious, Inflow Depth = 0.67" for 100-Year event  
 Inflow = 4.81 cfs @ 0.12 hrs, Volume= 0.159 af  
 Outflow = 4.81 cfs @ 0.12 hrs, Volume= 0.159 af, Atten= 0%, Lag= 0.0 min  
 Primary = 4.81 cfs @ 0.12 hrs, Volume= 0.159 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 65.05' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	63.78'	<b>24.0" Round B-L22</b> L= 74.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 63.78' / 63.41' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=4.80 cfs @ 0.12 hrs HW=65.05' TW=64.64' (Dynamic Tailwater)  
 ↑**1=B-L22** (Outlet Controls 4.80 cfs @ 3.27 fps)



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**Summary for Pond B-L23: B-L23**

Inflow Area = 0.900 ac, 0.00% Impervious, Inflow Depth = 0.82" for 100-Year event  
 Inflow = 1.86 cfs @ 0.09 hrs, Volume= 0.061 af  
 Outflow = 1.86 cfs @ 0.13 hrs, Volume= 0.061 af, Atten= 0%, Lag= 2.4 min  
 Primary = 1.86 cfs @ 0.13 hrs, Volume= 0.061 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 64.78' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	63.80'	<b>24.0" Round B-L23</b> L= 78.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 63.80' / 63.41' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.86 cfs @ 0.13 hrs HW=64.78' TW=64.64' (Dynamic Tailwater)  
 ↑**1=B-L23** (Outlet Controls 1.86 cfs @ 1.79 fps)

**Summary for Pond B-L24: B-L24**

Inflow Area = 1.330 ac, 0.00% Impervious, Inflow Depth = 0.68" for 100-Year event  
 Inflow = 2.30 cfs @ 0.10 hrs, Volume= 0.076 af  
 Outflow = 2.30 cfs @ 0.10 hrs, Volume= 0.076 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.30 cfs @ 0.10 hrs, Volume= 0.076 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 65.05' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	64.25'	<b>24.0" Round B-L24</b> L= 84.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 64.25' / 63.41' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.28 cfs @ 0.10 hrs HW=65.04' TW=64.61' (Dynamic Tailwater)  
 ↑**1=B-L24** (Outlet Controls 2.28 cfs @ 2.93 fps)

**Summary for Pond B-L25: B-L25**

Inflow Area = 5.070 ac, 0.00% Impervious, Inflow Depth = 0.70" for 100-Year event  
 Inflow = 8.96 cfs @ 0.12 hrs, Volume= 0.296 af  
 Outflow = 8.96 cfs @ 0.13 hrs, Volume= 0.296 af, Atten= 0%, Lag= 0.6 min  
 Primary = 8.96 cfs @ 0.13 hrs, Volume= 0.296 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 64.64' @ 0.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	63.31'	<b>30.0" Round Pipe</b> L= 401.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 63.31' / 60.51' S= 0.0070 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=8.96 cfs @ 0.13 hrs HW=64.64' TW=62.35' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 8.96 cfs @ 4.92 fps)

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**Summary for Pond B-L26: B-L26**

Inflow Area = 7.010 ac, 0.00% Impervious, Inflow Depth = 0.71" for 100-Year event  
 Inflow = 12.53 cfs @ 0.13 hrs, Volume= 0.414 af  
 Outflow = 12.53 cfs @ 0.13 hrs, Volume= 0.414 af, Atten= 0%, Lag= 0.0 min  
 Primary = 12.53 cfs @ 0.13 hrs, Volume= 0.414 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 62.35' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	60.40'	<b>30.0" Round B-L26</b> L= 64.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 60.40' / 60.08' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=12.53 cfs @ 0.13 hrs HW=62.35' TW=61.80' (Dynamic Tailwater)  
 ↑**1=B-L26** (Outlet Controls 12.53 cfs @ 4.19 fps)

**Summary for Pond B-L27: B-L27**

Inflow Area = 1.050 ac, 0.00% Impervious, Inflow Depth = 0.67" for 100-Year event  
 Inflow = 1.76 cfs @ 0.10 hrs, Volume= 0.058 af  
 Outflow = 1.76 cfs @ 0.10 hrs, Volume= 0.058 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.76 cfs @ 0.10 hrs, Volume= 0.058 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 61.84' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	60.50'	<b>24.0" Round B-L27</b> L= 46.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 60.50' / 60.18' S= 0.0070 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.64 cfs @ 0.10 hrs HW=61.80' TW=61.77' (Dynamic Tailwater)  
 ↑**1=B-L27** (Outlet Controls 1.64 cfs @ 1.08 fps)

**Summary for Pond B-L28: B-L28**

Inflow Area = 8.060 ac, 0.00% Impervious, Inflow Depth = 0.70" for 100-Year event  
 Inflow = 14.29 cfs @ 0.13 hrs, Volume= 0.473 af  
 Outflow = 14.29 cfs @ 0.13 hrs, Volume= 0.473 af, Atten= 0%, Lag= 0.0 min  
 Primary = 14.29 cfs @ 0.13 hrs, Volume= 0.473 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 61.80' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	59.98'	<b>30.0" Round Pipe</b> L= 404.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 59.98' / 57.15' S= 0.0070 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=14.29 cfs @ 0.13 hrs HW=61.80' TW=59.71' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 14.29 cfs @ 5.20 fps)

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**Summary for Pond B-L29: B-L29**

Inflow Area = 10.020 ac, 0.00% Impervious, Inflow Depth = 0.71" for 100-Year event  
 Inflow = 17.90 cfs @ 0.13 hrs, Volume= 0.592 af  
 Outflow = 17.90 cfs @ 0.13 hrs, Volume= 0.592 af, Atten= 0%, Lag= 0.0 min  
 Primary = 17.90 cfs @ 0.13 hrs, Volume= 0.592 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 59.71' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	57.05'	<b>30.0" Round B-L29</b> L= 78.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 57.05' / 56.66' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=17.90 cfs @ 0.13 hrs HW=59.71' TW=59.13' (Dynamic Tailwater)  
 ↑**1=B-L29** (Outlet Controls 17.90 cfs @ 4.26 fps)

**Summary for Pond B-L30: B-L30**

Inflow Area = 0.510 ac, 0.00% Impervious, Inflow Depth = 0.70" for 100-Year event  
 Inflow = 0.90 cfs @ 0.10 hrs, Volume= 0.030 af  
 Outflow = 0.90 cfs @ 0.13 hrs, Volume= 0.030 af, Atten= 0%, Lag= 1.8 min  
 Primary = 0.90 cfs @ 0.13 hrs, Volume= 0.030 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 60.55' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	60.15'	<b>24.0" Round B-L30</b> L= 69.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 60.15' / 59.46' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.90 cfs @ 0.13 hrs HW=60.55' TW=59.86' (Dynamic Tailwater)  
 ↑**1=B-L30** (Outlet Controls 0.90 cfs @ 3.06 fps)

**Summary for Pond B-L31: B-L31**

Inflow Area = 0.510 ac, 0.00% Impervious, Inflow Depth = 0.70" for 100-Year event  
 Inflow = 0.90 cfs @ 0.13 hrs, Volume= 0.030 af  
 Outflow = 0.90 cfs @ 0.11 hrs, Volume= 0.030 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.90 cfs @ 0.11 hrs, Volume= 0.030 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 59.86' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	59.36'	<b>24.0" Round Pipe</b> L= 198.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 59.36' / 57.38' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.90 cfs @ 0.11 hrs HW=59.86' TW=59.19' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 0.90 cfs @ 2.23 fps)



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**Summary for Pond B-L32: B-L32**

Inflow Area = 2.060 ac, 0.00% Impervious, Inflow Depth = 0.68" for 100-Year event  
 Inflow = 3.54 cfs @ 0.11 hrs, Volume= 0.117 af  
 Outflow = 3.54 cfs @ 0.11 hrs, Volume= 0.117 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.54 cfs @ 0.11 hrs, Volume= 0.117 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 59.21' @ 0.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	57.28'	<b>24.0" Round B-L32</b> L= 89.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 57.28' / 56.66' S= 0.0070 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=3.44 cfs @ 0.11 hrs HW=59.19' TW=59.12' (Dynamic Tailwater)↑**1=B-L32** (Outlet Controls 3.44 cfs @ 1.42 fps)**Summary for Pond B-L33: B-L33**

Inflow Area = 12.080 ac, 0.00% Impervious, Inflow Depth = 0.70" for 100-Year event  
 Inflow = 21.44 cfs @ 0.13 hrs, Volume= 0.709 af  
 Outflow = 21.44 cfs @ 0.13 hrs, Volume= 0.709 af, Atten= 0%, Lag= 0.0 min  
 Primary = 21.44 cfs @ 0.13 hrs, Volume= 0.709 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 59.13' @ 0.12 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.56'	<b>36.0" Round Pipe</b> L= 26.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.56' / 56.43' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=21.44 cfs @ 0.13 hrs HW=59.13' TW=58.62' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 21.44 cfs @ 4.46 fps)**Summary for Pond B-L34: B-L34**

Inflow Area = 12.550 ac, 0.00% Impervious, Inflow Depth = 0.71" for 100-Year event  
 Inflow = 22.43 cfs @ 0.13 hrs, Volume= 0.742 af  
 Outflow = 22.43 cfs @ 0.13 hrs, Volume= 0.742 af, Atten= 0%, Lag= 0.0 min  
 Primary = 22.43 cfs @ 0.13 hrs, Volume= 0.742 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 58.62' @ 0.12 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.33'	<b>36.0" Round B-L34</b> L= 358.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.33' / 54.54' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=22.43 cfs @ 0.13 hrs HW=58.62' TW=56.90' (Dynamic Tailwater)↑**1=B-L34** (Outlet Controls 22.43 cfs @ 5.37 fps)

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**Summary for Subcatchment B01: B01**

Runoff = 2.25 cfs @ 0.10 hrs, Volume= 0.074 af, Depth= 0.70"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
1.270	0.78	Mixed Use, HSG D
1.270		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, PA01

**Summary for Subcatchment B02: B02**

Runoff = 3.14 cfs @ 0.12 hrs, Volume= 0.104 af, Depth= 0.64"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
1.950	0.71	Mixed Use, HSG D
1.950		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.0					Direct Entry, PA01

**Summary for Subcatchment B03: B03**

Runoff = 1.96 cfs @ 0.09 hrs, Volume= 0.065 af, Depth= 0.86"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
0.910	0.95	Mixed Use, HSG D
0.910		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment B04: B04**

Runoff = 3.70 cfs @ 0.10 hrs, Volume= 0.122 af, Depth= 0.67"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

**Proposed System B**

City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

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Area (ac)	C	Description
2.200	0.74	Mixed Use, HSG D
2.200		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, PA01

**Summary for Subcatchment B05: B05**

Runoff = 1.95 cfs @ 0.12 hrs, Volume= 0.064 af, Depth= 0.67"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
1.160	0.74	Mixed Use, HSG D
1.160		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.0					Direct Entry, PA01

**Summary for Subcatchment B06: B06**

Runoff = 1.86 cfs @ 0.09 hrs, Volume= 0.061 af, Depth= 0.84"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
0.880	0.93	Mixed Use, HSG D
0.880		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment B07: B07**

Runoff = 3.63 cfs @ 0.10 hrs, Volume= 0.120 af, Depth= 0.68"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
2.130	0.75	Mixed Use, HSG D
2.130		100.00% Pervious Area



**Proposed System B**

City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment B08: B08**

Runoff = 1.87 cfs @ 0.10 hrs, Volume= 0.062 af, Depth= 0.67"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
1.110	0.74	Mixed Use, HSG D
1.110		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment B09: B09**

Runoff = 0.62 cfs @ 0.09 hrs, Volume= 0.020 af, Depth= 0.82"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
0.300	0.91	Mixed Use, HSG D
0.300		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment B10: B17**

Runoff = 17.40 cfs @ 0.37 hrs, Volume= 0.575 af, Depth= 0.55"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
12.560	0.61	Mixed Use, HSG D
12.560		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
22.0					<b>Direct Entry, PA01</b>

**Proposed System B**

City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

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**Summary for Subcatchment B12: B12**

Runoff = 2.52 cfs @ 0.10 hrs, Volume= 0.083 af, Depth= 0.68"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
1.480	0.75	Mixed Use, HSG D
1.480		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, PA01

**Summary for Subcatchment B13: B13**

Runoff = 2.29 cfs @ 0.12 hrs, Volume= 0.076 af, Depth= 0.67"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
1.360	0.74	Mixed Use, HSG D
1.360		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.0					Direct Entry, PA01

**Summary for Subcatchment B14: B14**

Runoff = 1.86 cfs @ 0.09 hrs, Volume= 0.061 af, Depth= 0.82"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
0.900	0.91	Mixed Use, HSG D
0.900		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment B15: B15**

Runoff = 2.30 cfs @ 0.10 hrs, Volume= 0.076 af, Depth= 0.68"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

**Proposed System B**

City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

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Area (ac)	C	Description
1.330	0.76	Mixed Use, HSG D
1.330		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, PA01

**Summary for Subcatchment B16: B16**

Runoff = 3.57 cfs @ 0.09 hrs, Volume= 0.118 af, Depth= 0.73"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
1.940	0.81	Mixed Use, HSG D
1.940		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment B17: B17**

Runoff = 1.76 cfs @ 0.10 hrs, Volume= 0.058 af, Depth= 0.67"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
1.050	0.74	Mixed Use, HSG D
1.050		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, PA01

**Summary for Subcatchment B18: B18**

Runoff = 3.61 cfs @ 0.09 hrs, Volume= 0.119 af, Depth= 0.73"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
1.960	0.81	Mixed Use, HSG D
1.960		100.00% Pervious Area



**Proposed System B**

City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment B19: B19**

Runoff = 0.90 cfs @ 0.10 hrs, Volume= 0.030 af, Depth= 0.70"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
0.510	0.78	Mixed Use, HSG D
0.510		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment B20: B20**

Runoff = 2.64 cfs @ 0.10 hrs, Volume= 0.087 af, Depth= 0.68"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
1.550	0.75	Mixed Use, HSG D
1.550		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment B21: B21**

Runoff = 0.99 cfs @ 0.09 hrs, Volume= 0.033 af, Depth= 0.84"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
0.470	0.93	Mixed Use, HSG D
0.470		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Proposed System B**

City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

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**Summary for Subcatchment BASIN-B1: BASIN-B1**

Runoff = 2.29 cfs @ 0.17 hrs, Volume= 0.076 af, Depth= 0.49"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
1.870	0.54	Mixed Use, HSG D
1.870		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					Direct Entry, PA01

**Summary for Subcatchment BASIN-B2: BASIN-B2**

Runoff = 5.89 cfs @ 0.40 hrs, Volume= 0.195 af, Depth= 0.49"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=24 min, Inten=2.25 in/hr

Area (ac)	C	Description
4.800	0.54	Mixed Use, HSG D
4.800		100.00% Pervious Area

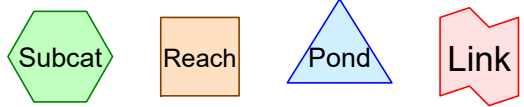
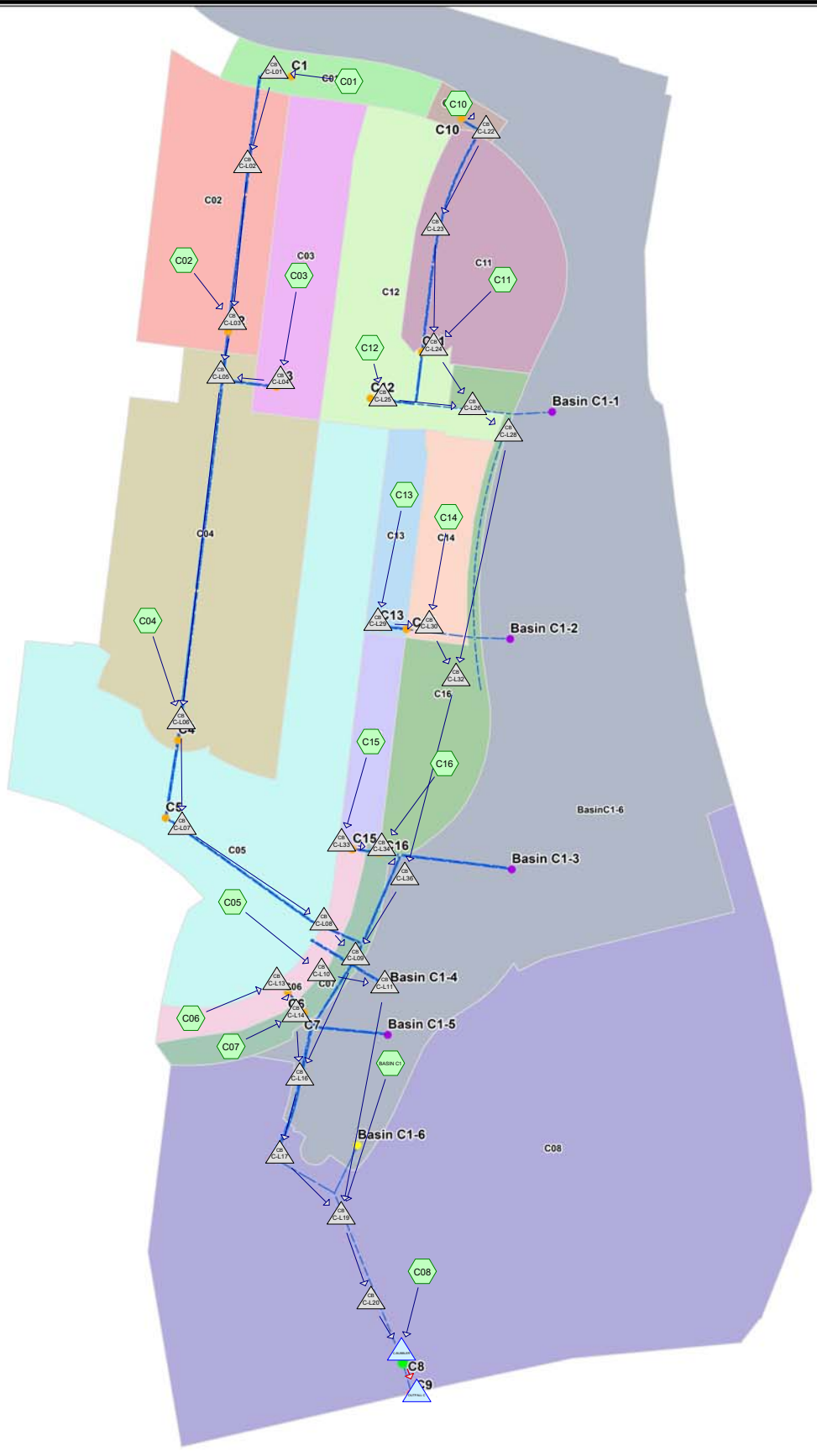
  

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.0					Direct Entry, PA01

**Summary for Pond OUTFALL B: OUTFALL B**

Inflow Area = 43.690 ac, 2.08% Impervious, Inflow Depth = 0.63" for 100-Year event  
 Inflow = 63.78 cfs @ 0.41 hrs, Volume= 2.284 af  
 Primary = 63.78 cfs @ 0.41 hrs, Volume= 2.284 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3



**Routing Diagram for Proposed System C**  
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## Proposed System C

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### Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	C-BUBBLER	46.50	46.20	110.0	0.0027	0.013	34.0	0.0	0.0
2	C-L01	67.78	66.19	41.0	0.0388	0.013	24.0	0.0	0.0
3	C-L02	65.99	63.35	377.0	0.0070	0.013	30.0	0.0	0.0
4	C-L03	63.25	62.78	66.0	0.0071	0.013	30.0	0.0	0.0
5	C-L04	62.94	62.88	74.0	0.0008	0.013	24.0	0.0	0.0
6	C-L05	62.68	59.50	514.0	0.0062	0.013	30.0	0.0	0.0
7	C-L06	59.40	58.64	109.0	0.0070	0.013	30.0	0.0	0.0
8	C-L07	58.54	56.58	261.0	0.0075	0.013	30.0	0.0	0.0
9	C-L08	56.48	56.10	76.0	0.0050	0.013	30.0	0.0	0.0
10	C-L09	55.80	53.23	273.0	0.0094	0.013	36.0	0.0	0.0
11	C-L10	59.62	59.26	80.0	0.0045	0.013	30.0	0.0	0.0
12	C-L11	59.06	47.80	102.8	0.1095	0.013	36.0	0.0	0.0
13	C-L13	53.55	53.31	38.0	0.0063	0.013	24.0	0.0	0.0
14	C-L14	53.21	53.11	16.0	0.0063	0.013	24.0	0.0	0.0
15	C-L16	53.03	50.40	98.0	0.0268	0.013	42.0	0.0	0.0
16	C-L17	50.30	47.80	102.0	0.0245	0.013	42.0	0.0	0.0
17	C-L19	47.70	47.20	200.0	0.0025	0.013	42.0	0.0	0.0
18	C-L20	47.10	46.60	50.0	0.0100	0.013	42.0	0.0	0.0
19	C-L22	67.03	66.89	45.0	0.0031	0.013	24.0	0.0	0.0
20	C-L23	66.69	64.98	311.0	0.0055	0.013	24.0	0.0	0.0
21	C-L24	64.88	64.52	71.0	0.0051	0.013	24.0	0.0	0.0
22	C-L25	64.83	64.52	61.0	0.0051	0.013	24.0	0.0	0.0
23	C-L26	64.32	63.65	135.0	0.0050	0.013	24.0	0.0	0.0
24	C-L28	63.55	59.99	324.0	0.0110	0.013	24.0	0.0	0.0
25	C-L29	61.50	61.10	36.0	0.0111	0.013	24.0	0.0	0.0
26	C-L30	61.00	59.99	92.0	0.0110	0.013	24.0	0.0	0.0
27	C-L32	59.89	56.73	74.0	0.0427	0.013	30.0	0.0	0.0
28	C-L33	57.14	56.95	38.0	0.0050	0.013	24.0	0.0	0.0
29	C-L34	56.85	56.73	24.0	0.0050	0.013	24.0	0.0	0.0
30	C-L36	56.63	56.00	126.0	0.0050	0.013	36.0	0.0	0.0

50-year Rainfall Intensity

ELEV.	FACTOR
0-1500	1.00
1500-3000	1.25
3000-4000	1.42
4000-5000	1.60
5000-6000	1.7
DESERT	1.25

TO OBTAIN CORRECT INTENSITY, MULTIPLY INTENSITY ON CHART BY FACTOR FOR DESIGN ELEVATION.

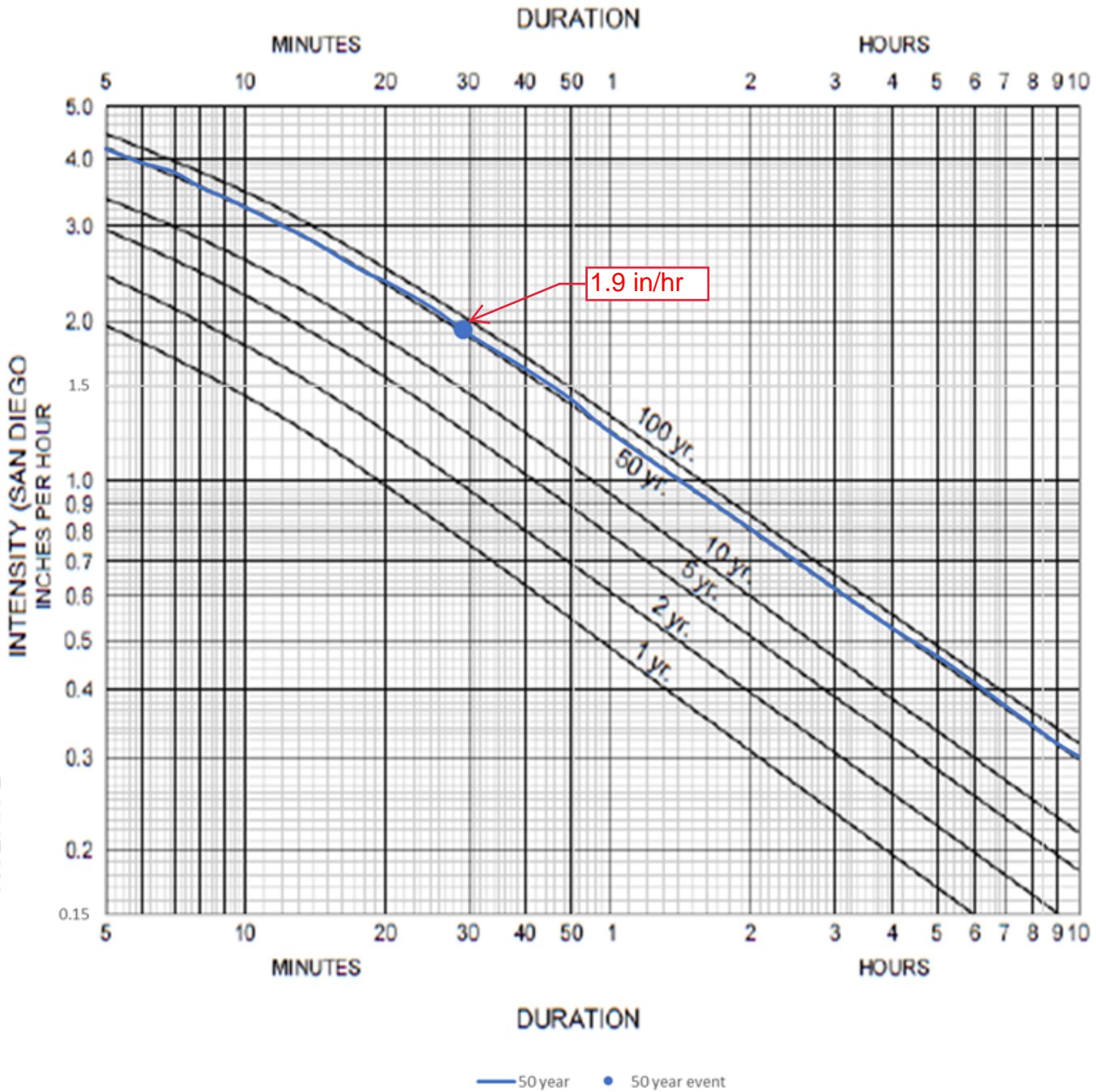
Figure A-1. Intensity-Duration-Frequency Design Chart

COUNTY OF SAN DIEGO

FOR

INTENSITY - DURATION - FREQUENCY CURVES

RAINFALL



**Proposed System C**

City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

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Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points x 3  
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Subcatchment BASIN C1: BASIN C1</b>	Runoff Area=10.380 ac 0.00% Impervious Runoff Depth=0.50" Tc=26.0 min C=0.54 Runoff=10.80 cfs 0.431 af
<b>Pond C-BUBBLER: C-BUBBLER</b>	Peak Elev=51.01' Storage=216 cf Inflow=46.29 cfs 1.848 af 34.0" Round Culvert n=0.013 L=110.0' S=0.0027 '/' Outflow=46.29 cfs 1.848 af
<b>Pond C-L01: C-L01</b>	Peak Elev=68.14' Inflow=0.77 cfs 0.031 af 24.0" Round Culvert n=0.013 L=41.0' S=0.0388 '/' Outflow=0.77 cfs 0.031 af
<b>Pond C-L02: C-L02</b>	Peak Elev=66.35' Inflow=0.77 cfs 0.031 af 30.0" Round Culvert n=0.013 L=377.0' S=0.0070 '/' Outflow=0.77 cfs 0.031 af
<b>Pond C-L03: C-L03</b>	Peak Elev=64.09' Inflow=3.24 cfs 0.129 af 30.0" Round Culvert n=0.013 L=66.0' S=0.0071 '/' Outflow=3.24 cfs 0.129 af
<b>Pond C-L04: C-L04</b>	Peak Elev=63.77' Inflow=1.53 cfs 0.061 af 24.0" Round Culvert n=0.013 L=74.0' S=0.0008 '/' Outflow=1.53 cfs 0.061 af
<b>Pond C-L05: C-L05</b>	Peak Elev=63.61' Inflow=4.76 cfs 0.190 af 30.0" Round Culvert n=0.013 L=514.0' S=0.0062 '/' Outflow=4.76 cfs 0.190 af
<b>Pond C-L06: C-L06</b>	Peak Elev=60.83' Inflow=9.57 cfs 0.382 af 30.0" Round Culvert n=0.013 L=109.0' S=0.0070 '/' Outflow=9.57 cfs 0.382 af
<b>Pond C-L07: C-L07</b>	Peak Elev=59.89' Inflow=9.57 cfs 0.382 af 30.0" Round Culvert n=0.013 L=261.0' S=0.0075 '/' Outflow=9.57 cfs 0.382 af
<b>Pond C-L08: C-L08</b>	Peak Elev=58.07' Inflow=9.57 cfs 0.382 af 30.0" Round Culvert n=0.013 L=76.0' S=0.0050 '/' Outflow=9.57 cfs 0.382 af
<b>Pond C-L09: C-L09</b>	Peak Elev=57.47' Inflow=17.72 cfs 0.708 af 36.0" Round Culvert n=0.013 L=273.0' S=0.0094 '/' Outflow=17.72 cfs 0.708 af
<b>Pond C-L10: C-L10</b>	Peak Elev=60.60' Inflow=4.56 cfs 0.182 af 30.0" Round Culvert n=0.013 L=80.0' S=0.0045 '/' Outflow=4.56 cfs 0.182 af
<b>Pond C-L11: C-L11</b>	Peak Elev=59.86' Inflow=4.56 cfs 0.182 af 36.0" Round Culvert n=0.013 L=102.8' S=0.1095 '/' Outflow=4.56 cfs 0.182 af
<b>Pond C-L13: C-L13</b>	Peak Elev=54.69' Inflow=0.67 cfs 0.027 af 24.0" Round Culvert n=0.013 L=38.0' S=0.0063 '/' Outflow=0.67 cfs 0.027 af
<b>Pond C-L14: C-L14</b>	Peak Elev=54.68' Inflow=1.43 cfs 0.057 af 24.0" Round Culvert n=0.013 L=16.0' S=0.0063 '/' Outflow=1.43 cfs 0.057 af
<b>Pond C-L16: B-L16</b>	Peak Elev=54.66' Inflow=19.15 cfs 0.765 af 42.0" Round Culvert n=0.013 L=98.0' S=0.0268 '/' Outflow=19.15 cfs 0.765 af
<b>Pond C-L17: C-L17</b>	Peak Elev=52.64' Inflow=19.15 cfs 0.765 af 42.0" Round Culvert n=0.013 L=102.0' S=0.0245 '/' Outflow=19.15 cfs 0.765 af



**Proposed System C**

City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

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<b>Pond C-L19: C-L19</b>	Peak Elev=52.12' Inflow=34.51 cfs 1.378 af 42.0" Round Culvert n=0.013 L=200.0' S=0.0025 '/' Outflow=34.51 cfs 1.378 af
<b>Pond C-L20: C-L20</b>	Peak Elev=51.56' Inflow=34.51 cfs 1.378 af 42.0" Round Culvert n=0.013 L=50.0' S=0.0100 '/' Outflow=34.51 cfs 1.378 af
<b>Pond C-L22: C-L22</b>	Peak Elev=67.26' Inflow=0.20 cfs 0.008 af 24.0" Round Culvert n=0.013 L=45.0' S=0.0031 '/' Outflow=0.20 cfs 0.008 af
<b>Pond C-L23: C-L23</b>	Peak Elev=66.92' Inflow=0.20 cfs 0.008 af 24.0" Round Culvert n=0.013 L=311.0' S=0.0055 '/' Outflow=0.20 cfs 0.008 af
<b>Pond C-L24: C-L24</b>	Peak Elev=65.70' Inflow=2.33 cfs 0.093 af 24.0" Round Culvert n=0.013 L=71.0' S=0.0051 '/' Outflow=2.33 cfs 0.093 af
<b>Pond C-L25: C-L25</b>	Peak Elev=65.62' Inflow=2.02 cfs 0.081 af 24.0" Round Culvert n=0.013 L=61.0' S=0.0051 '/' Outflow=2.02 cfs 0.081 af
<b>Pond C-L26: C-L26</b>	Peak Elev=65.34' Inflow=4.35 cfs 0.174 af 24.0" Round Culvert n=0.013 L=135.0' S=0.0050 '/' Outflow=4.35 cfs 0.174 af
<b>Pond C-L28: C-L28</b>	Peak Elev=64.44' Inflow=4.35 cfs 0.174 af 24.0" Round Culvert n=0.013 L=324.0' S=0.0110 '/' Outflow=4.35 cfs 0.174 af
<b>Pond C-L29: C-L29</b>	Peak Elev=61.89' Inflow=0.73 cfs 0.029 af 24.0" Round Culvert n=0.013 L=36.0' S=0.0111 '/' Outflow=0.73 cfs 0.029 af
<b>Pond C-L30: C-L30</b>	Peak Elev=61.56' Inflow=1.56 cfs 0.062 af 24.0" Round Culvert n=0.013 L=92.0' S=0.0110 '/' Outflow=1.56 cfs 0.062 af
<b>Pond C-L32: C-L32</b>	Peak Elev=60.86' Inflow=5.91 cfs 0.236 af 30.0" Round Culvert n=0.013 L=74.0' S=0.0427 '/' Outflow=5.91 cfs 0.236 af
<b>Pond C-L33: C-L33</b>	Peak Elev=58.13' Inflow=0.75 cfs 0.030 af 24.0" Round Culvert n=0.013 L=38.0' S=0.0050 '/' Outflow=0.75 cfs 0.030 af
<b>Pond C-L34: C-L34</b>	Peak Elev=58.11' Inflow=2.24 cfs 0.090 af 24.0" Round Culvert n=0.013 L=24.0' S=0.0050 '/' Outflow=2.24 cfs 0.090 af
<b>Pond C-L36: C-L36</b>	Peak Elev=58.04' Inflow=8.15 cfs 0.326 af 36.0" Round Culvert n=0.013 L=126.0' S=0.0050 '/' Outflow=8.15 cfs 0.326 af
<b>Subcatchment C01: C01</b>	Runoff Area=0.430 ac 0.00% Impervious Runoff Depth=0.86" Tc=5.0 min C=0.93 Runoff=0.77 cfs 0.031 af
<b>Subcatchment C02: C02</b>	Runoff Area=1.580 ac 0.00% Impervious Runoff Depth=0.75" Tc=5.0 min C=0.81 Runoff=2.47 cfs 0.098 af
<b>Subcatchment C03: C03</b>	Runoff Area=1.070 ac 0.00% Impervious Runoff Depth=0.68" Tc=5.0 min C=0.74 Runoff=1.53 cfs 0.061 af
<b>Subcatchment C04: C04</b>	Runoff Area=3.200 ac 0.00% Impervious Runoff Depth=0.72" Tc=5.0 min C=0.78 Runoff=4.81 cfs 0.192 af

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<b>Subcatchment C05: C05</b>	Runoff Area=3.880 ac 0.00% Impervious Runoff Depth=0.56" Tc=5.0 min C=0.61 Runoff=4.56 cfs 0.182 af
<b>Subcatchment C06: C06</b>	Runoff Area=0.380 ac 0.00% Impervious Runoff Depth=0.84" Tc=5.0 min C=0.91 Runoff=0.67 cfs 0.027 af
<b>Subcatchment C07: C07</b>	Runoff Area=0.430 ac 0.00% Impervious Runoff Depth=0.85" Tc=5.0 min C=0.92 Runoff=0.76 cfs 0.030 af
<b>Subcatchment C08: C08</b>	Runoff Area=11.110 ac 0.00% Impervious Runoff Depth=0.51" Tc=5.0 min C=0.55 Runoff=11.77 cfs 0.470 af
<b>Subcatchment C10: C10</b>	Runoff Area=0.110 ac 100.00% Impervious Runoff Depth=0.88" Tc=5.0 min C=0.95 Runoff=0.20 cfs 0.008 af
<b>Subcatchment C11: C11</b>	Runoff Area=1.380 ac 0.00% Impervious Runoff Depth=0.74" Tc=5.0 min C=0.80 Runoff=2.13 cfs 0.085 af
<b>Subcatchment C12: C12</b>	Runoff Area=1.380 ac 0.00% Impervious Runoff Depth=0.70" Tc=5.0 min C=0.76 Runoff=2.02 cfs 0.081 af
<b>Subcatchment C13: C13</b>	Runoff Area=0.400 ac 100.00% Impervious Runoff Depth=0.88" Tc=5.0 min C=0.95 Runoff=0.73 cfs 0.029 af
<b>Subcatchment C14: C14</b>	Runoff Area=0.560 ac 0.00% Impervious Runoff Depth=0.71" Tc=5.0 min C=0.77 Runoff=0.83 cfs 0.033 af
<b>Subcatchment C15: C15</b>	Runoff Area=0.410 ac 100.00% Impervious Runoff Depth=0.88" Tc=5.0 min C=0.95 Runoff=0.75 cfs 0.030 af
<b>Subcatchment C16: C16</b>	Runoff Area=1.090 ac 0.00% Impervious Runoff Depth=0.66" Tc=5.0 min C=0.71 Runoff=1.49 cfs 0.060 af
<b>Pond OUTFALL C: OUTFALL C</b>	Inflow=46.29 cfs 1.848 af Primary=46.29 cfs 1.848 af

**Total Runoff Area = 37.790 ac Runoff Volume = 1.848 af Average Runoff Depth = 0.59"**  
**97.57% Pervious = 36.870 ac 2.43% Impervious = 0.920 ac**

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**Summary for Subcatchment BASIN C1: BASIN C1**

Runoff = 10.80 cfs @ 0.44 hrs, Volume= 0.431 af, Depth= 0.50"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
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Area (ac)	C	Description
10.380	0.54	Mixed Use, HSG D
10.380		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
26.0					<b>Direct Entry, BASIN C1</b>

**Summary for Pond C-BUBBLER: C-BUBBLER**

Inflow Area = 37.790 ac, 2.43% Impervious, Inflow Depth = 0.59" for 50-Year event  
 Inflow = 46.29 cfs @ 0.44 hrs, Volume= 1.848 af  
 Outflow = 46.29 cfs @ 0.45 hrs, Volume= 1.848 af, Atten= 0%, Lag= 0.6 min  
 Primary = 46.29 cfs @ 0.45 hrs, Volume= 1.848 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 51.01' @ 0.45 hrs Surf.Area= 86 sf Storage= 216 cf

Plug-Flow detention time= 0.3 min calculated for 1.848 af (100% of inflow)  
 Center-of-Mass det. time= 0.1 min ( 19.5 - 19.5 )

Volume	Invert	Avail.Storage	Storage Description
#1	46.00'	113,562 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
46.00	0	0	0
51.80	100	290	290
53.00	45,302	27,241	27,531
54.00	126,760	86,031	113,562

Device	Routing	Invert	Outlet Devices
#1	Primary	46.50'	<b>34.0" Round Bubbler</b> L= 110.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 46.50' / 46.20' S= 0.0027 '/' Cc= 0.900 n= 0.013, Flow Area= 6.31 sf

**Primary OutFlow** Max=46.29 cfs @ 0.45 hrs HW=51.01' TW=0.00' (Dynamic Tailwater)  
 ↑**1=Bubbler** (Barrel Controls 46.29 cfs @ 7.34 fps)



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### Summary for Pond C-L01: C-L01

Inflow Area = 0.430 ac, 0.00% Impervious, Inflow Depth = 0.86" for 50-Year event  
Inflow = 0.77 cfs @ 0.09 hrs, Volume= 0.031 af  
Outflow = 0.77 cfs @ 0.10 hrs, Volume= 0.031 af, Atten= 0%, Lag= 0.6 min  
Primary = 0.77 cfs @ 0.10 hrs, Volume= 0.031 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
Peak Elev= 68.14' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	67.78'	<b>24.0" Round C-L1</b> L= 41.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 67.78' / 66.19' S= 0.0388 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.77 cfs @ 0.10 hrs HW=68.14' TW=66.35' (Dynamic Tailwater)  
↑**1=C-L1** (Inlet Controls 0.77 cfs @ 2.03 fps)

### Summary for Pond C-L02: C-L02

Inflow Area = 0.430 ac, 0.00% Impervious, Inflow Depth = 0.86" for 50-Year event  
Inflow = 0.77 cfs @ 0.10 hrs, Volume= 0.031 af  
Outflow = 0.77 cfs @ 0.10 hrs, Volume= 0.031 af, Atten= 0%, Lag= 0.0 min  
Primary = 0.77 cfs @ 0.10 hrs, Volume= 0.031 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
Peak Elev= 66.35' @ 0.11 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	65.99'	<b>30.0" Round Pipe</b> L= 377.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 65.99' / 63.35' S= 0.0070 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=0.77 cfs @ 0.10 hrs HW=66.35' TW=64.09' (Dynamic Tailwater)  
↑**1=Pipe** (Outlet Controls 0.77 cfs @ 2.65 fps)

### Summary for Pond C-L03: C-L03

Inflow Area = 2.010 ac, 0.00% Impervious, Inflow Depth = 0.77" for 50-Year event  
Inflow = 3.24 cfs @ 0.10 hrs, Volume= 0.129 af  
Outflow = 3.24 cfs @ 0.14 hrs, Volume= 0.129 af, Atten= 0%, Lag= 2.4 min  
Primary = 3.24 cfs @ 0.14 hrs, Volume= 0.129 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
Peak Elev= 64.09' @ 0.10 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	63.25'	<b>30.0" Round C-L3</b> L= 66.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 63.25' / 62.78' S= 0.0071 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=3.24 cfs @ 0.14 hrs HW=64.09' TW=63.61' (Dynamic Tailwater)  
↑**1=C-L3** (Outlet Controls 3.24 cfs @ 3.34 fps)

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**Summary for Pond C-L04: C-L04**

Inflow Area = 1.070 ac, 0.00% Impervious, Inflow Depth = 0.68" for 50-Year event  
 Inflow = 1.53 cfs @ 0.09 hrs, Volume= 0.061 af  
 Outflow = 1.53 cfs @ 0.09 hrs, Volume= 0.061 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.53 cfs @ 0.09 hrs, Volume= 0.061 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 63.77' @ 0.10 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	62.94'	<b>24.0" Round C-L4</b> L= 74.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 62.94' / 62.88' S= 0.0008 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.52 cfs @ 0.09 hrs HW=63.77' TW=63.61' (Dynamic Tailwater)  
 ↑**1=C-L4** (Outlet Controls 1.52 cfs @ 1.83 fps)

**Summary for Pond C-L05: C-L05**

Inflow Area = 3.080 ac, 0.00% Impervious, Inflow Depth = 0.74" for 50-Year event  
 Inflow = 4.76 cfs @ 0.10 hrs, Volume= 0.190 af  
 Outflow = 4.76 cfs @ 0.10 hrs, Volume= 0.190 af, Atten= 0%, Lag= 0.0 min  
 Primary = 4.76 cfs @ 0.10 hrs, Volume= 0.190 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 63.61' @ 0.10 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	62.68'	<b>30.0" Round Pipe</b> L= 514.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 62.68' / 59.50' S= 0.0062 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=4.76 cfs @ 0.10 hrs HW=63.61' TW=60.83' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 4.76 cfs @ 4.23 fps)

**Summary for Pond C-L06: C-L06**

Inflow Area = 6.280 ac, 0.00% Impervious, Inflow Depth = 0.73" for 50-Year event  
 Inflow = 9.57 cfs @ 0.09 hrs, Volume= 0.382 af  
 Outflow = 9.57 cfs @ 0.09 hrs, Volume= 0.382 af, Atten= 0%, Lag= 0.0 min  
 Primary = 9.57 cfs @ 0.09 hrs, Volume= 0.382 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 60.83' @ 0.10 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	59.40'	<b>30.0" Round C-L6</b> L= 109.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 59.40' / 58.64' S= 0.0070 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=9.55 cfs @ 0.09 hrs HW=60.83' TW=59.89' (Dynamic Tailwater)  
 ↑**1=C-L6** (Outlet Controls 9.55 cfs @ 4.76 fps)

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**Summary for Pond C-L07: C-L07**

Inflow Area = 6.280 ac, 0.00% Impervious, Inflow Depth = 0.73" for 50-Year event  
 Inflow = 9.57 cfs @ 0.09 hrs, Volume= 0.382 af  
 Outflow = 9.57 cfs @ 0.11 hrs, Volume= 0.382 af, Atten= 0%, Lag= 1.2 min  
 Primary = 9.57 cfs @ 0.11 hrs, Volume= 0.382 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 59.89' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	58.54'	<b>30.0" Round C-L7</b> L= 261.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 58.54' / 56.58' S= 0.0075 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=9.57 cfs @ 0.11 hrs HW=59.89' TW=58.07' (Dynamic Tailwater)  
 ↑**1=C-L7** (Outlet Controls 9.57 cfs @ 5.14 fps)

**Summary for Pond C-L08: C-L08**

Inflow Area = 6.280 ac, 0.00% Impervious, Inflow Depth = 0.73" for 50-Year event  
 Inflow = 9.57 cfs @ 0.11 hrs, Volume= 0.382 af  
 Outflow = 9.57 cfs @ 0.13 hrs, Volume= 0.382 af, Atten= 0%, Lag= 1.2 min  
 Primary = 9.57 cfs @ 0.13 hrs, Volume= 0.382 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 58.07' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.48'	<b>30.0" Round Pipe</b> L= 76.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.48' / 56.10' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=9.57 cfs @ 0.13 hrs HW=58.07' TW=57.47' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 9.57 cfs @ 4.16 fps)

**Summary for Pond C-L09: C-L09**

Inflow Area = 11.610 ac, 7.92% Impervious, Inflow Depth = 0.73" for 50-Year event  
 Inflow = 17.72 cfs @ 0.11 hrs, Volume= 0.708 af  
 Outflow = 17.72 cfs @ 0.11 hrs, Volume= 0.708 af, Atten= 0%, Lag= 0.0 min  
 Primary = 17.72 cfs @ 0.11 hrs, Volume= 0.708 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 57.47' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	55.80'	<b>36.0" Round Pipe</b> L= 273.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 55.80' / 53.23' S= 0.0094 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=17.72 cfs @ 0.11 hrs HW=57.47' TW=54.66' (Dynamic Tailwater)  
 ↑**1=Pipe** (Inlet Controls 17.72 cfs @ 4.40 fps)



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**Summary for Pond C-L10: C-L10**

Inflow Area = 3.880 ac, 0.00% Impervious, Inflow Depth = 0.56" for 50-Year event  
 Inflow = 4.56 cfs @ 0.09 hrs, Volume= 0.182 af  
 Outflow = 4.56 cfs @ 0.10 hrs, Volume= 0.182 af, Atten= 0%, Lag= 0.6 min  
 Primary = 4.56 cfs @ 0.10 hrs, Volume= 0.182 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 60.60' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	59.62'	<b>30.0" Round Pipe</b> L= 80.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 59.62' / 59.26' S= 0.0045 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=4.56 cfs @ 0.10 hrs HW=60.60' TW=59.86' (Dynamic Tailwater)  
 ↑**1=Pipe** (Barrel Controls 4.56 cfs @ 3.77 fps)

**Summary for Pond C-L11: C-L11**

Inflow Area = 3.880 ac, 0.00% Impervious, Inflow Depth = 0.56" for 50-Year event  
 Inflow = 4.56 cfs @ 0.10 hrs, Volume= 0.182 af  
 Outflow = 4.56 cfs @ 0.10 hrs, Volume= 0.182 af, Atten= 0%, Lag= 0.0 min  
 Primary = 4.56 cfs @ 0.10 hrs, Volume= 0.182 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 59.86' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	59.06'	<b>36.0" Round Pipe</b> L= 102.8' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 59.06' / 47.80' S= 0.1095 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=4.56 cfs @ 0.10 hrs HW=59.86' TW=51.15' (Dynamic Tailwater)  
 ↑**1=Pipe** (Inlet Controls 4.56 cfs @ 3.04 fps)

**Summary for Pond C-L13: C-L13**

Inflow Area = 0.380 ac, 0.00% Impervious, Inflow Depth = 0.84" for 50-Year event  
 Inflow = 0.67 cfs @ 0.09 hrs, Volume= 0.027 af  
 Outflow = 0.67 cfs @ 0.10 hrs, Volume= 0.027 af, Atten= 0%, Lag= 0.6 min  
 Primary = 0.67 cfs @ 0.10 hrs, Volume= 0.027 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 54.69' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	53.55'	<b>24.0" Round C-L13</b> L= 38.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 53.55' / 53.31' S= 0.0063 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.67 cfs @ 0.10 hrs HW=54.69' TW=54.68' (Dynamic Tailwater)  
 ↑**1=C-L13** (Outlet Controls 0.67 cfs @ 0.52 fps)

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**Summary for Pond C-L14: C-L14**

Inflow Area = 0.810 ac, 0.00% Impervious, Inflow Depth = 0.85" for 50-Year event  
 Inflow = 1.43 cfs @ 0.09 hrs, Volume= 0.057 af  
 Outflow = 1.43 cfs @ 0.09 hrs, Volume= 0.057 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.43 cfs @ 0.09 hrs, Volume= 0.057 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 54.68' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	53.21'	<b>24.0" Round C-L14</b> L= 16.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 53.21' / 53.11' S= 0.0063 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.43 cfs @ 0.09 hrs HW=54.68' TW=54.66' (Dynamic Tailwater)  
 ↑**1=C-L14** (Outlet Controls 1.43 cfs @ 0.80 fps)

**Summary for Pond C-L16: B-L16**

Inflow Area = 12.420 ac, 7.41% Impervious, Inflow Depth = 0.74" for 50-Year event  
 Inflow = 19.15 cfs @ 0.11 hrs, Volume= 0.765 af  
 Outflow = 19.15 cfs @ 0.12 hrs, Volume= 0.765 af, Atten= 0%, Lag= 0.6 min  
 Primary = 19.15 cfs @ 0.12 hrs, Volume= 0.765 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 54.66' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	53.03'	<b>42.0" Round Pipe</b> L= 98.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 53.03' / 50.40' S= 0.0268 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=19.15 cfs @ 0.12 hrs HW=54.66' TW=52.16' (Dynamic Tailwater)  
 ↑**1=Pipe** (Inlet Controls 19.15 cfs @ 4.35 fps)

**Summary for Pond C-L17: C-L17**

Inflow Area = 12.420 ac, 7.41% Impervious, Inflow Depth = 0.74" for 50-Year event  
 Inflow = 19.15 cfs @ 0.12 hrs, Volume= 0.765 af  
 Outflow = 19.15 cfs @ 0.26 hrs, Volume= 0.765 af, Atten= 0%, Lag= 8.4 min  
 Primary = 19.15 cfs @ 0.26 hrs, Volume= 0.765 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 52.64' @ 0.46 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	50.30'	<b>42.0" Round Pipe</b> L= 102.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 50.30' / 47.80' S= 0.0245 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=18.96 cfs @ 0.26 hrs HW=52.32' TW=51.55' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 18.96 cfs @ 4.75 fps)

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**Summary for Pond C-L19: C-L19**

Inflow Area = 26.680 ac, 3.45% Impervious, Inflow Depth = 0.62" for 50-Year event  
 Inflow = 34.51 cfs @ 0.44 hrs, Volume= 1.378 af  
 Outflow = 34.51 cfs @ 0.44 hrs, Volume= 1.378 af, Atten= 0%, Lag= 0.0 min  
 Primary = 34.51 cfs @ 0.44 hrs, Volume= 1.378 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 52.12' @ 0.45 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	47.70'	<b>42.0" Round Pipe</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 47.70' / 47.20' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=34.51 cfs @ 0.44 hrs HW=52.12' TW=51.56' (Dynamic Tailwater)  
 ↑**1=Pipe** (Inlet Controls 34.51 cfs @ 3.59 fps)

**Summary for Pond C-L20: C-L20**

Inflow Area = 26.680 ac, 3.45% Impervious, Inflow Depth = 0.62" for 50-Year event  
 Inflow = 34.51 cfs @ 0.44 hrs, Volume= 1.378 af  
 Outflow = 34.51 cfs @ 0.44 hrs, Volume= 1.378 af, Atten= 0%, Lag= 0.0 min  
 Primary = 34.51 cfs @ 0.44 hrs, Volume= 1.378 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 51.56' @ 0.45 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	47.10'	<b>42.0" Round Pipe</b> L= 50.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 47.10' / 46.60' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=34.51 cfs @ 0.44 hrs HW=51.56' TW=51.01' (Dynamic Tailwater)  
 ↑**1=Pipe** (Inlet Controls 34.51 cfs @ 3.59 fps)

**Summary for Pond C-L22: C-L22**

Inflow Area = 0.110 ac, 100.00% Impervious, Inflow Depth = 0.88" for 50-Year event  
 Inflow = 0.20 cfs @ 0.09 hrs, Volume= 0.008 af  
 Outflow = 0.20 cfs @ 0.10 hrs, Volume= 0.008 af, Atten= 0%, Lag= 0.6 min  
 Primary = 0.20 cfs @ 0.10 hrs, Volume= 0.008 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 67.26' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	67.03'	<b>24.0" Round C-L22</b> L= 45.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 67.03' / 66.89' S= 0.0031 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.20 cfs @ 0.10 hrs HW=67.26' TW=66.92' (Dynamic Tailwater)  
 ↑**1=C-L22** (Barrel Controls 0.20 cfs @ 1.50 fps)



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**Summary for Pond C-L23: C-L23**

Inflow Area = 0.110 ac, 100.00% Impervious, Inflow Depth = 0.88" for 50-Year event  
 Inflow = 0.20 cfs @ 0.10 hrs, Volume= 0.008 af  
 Outflow = 0.20 cfs @ 0.10 hrs, Volume= 0.008 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.20 cfs @ 0.10 hrs, Volume= 0.008 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 66.92' @ 0.10 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	66.69'	<b>24.0" Round Pipe</b> L= 311.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 66.69' / 64.98' S= 0.0055 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.20 cfs @ 0.10 hrs HW=66.92' TW=65.70' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 0.20 cfs @ 1.58 fps)**Summary for Pond C-L24: C-L24**

Inflow Area = 1.490 ac, 7.38% Impervious, Inflow Depth = 0.75" for 50-Year event  
 Inflow = 2.33 cfs @ 0.10 hrs, Volume= 0.093 af  
 Outflow = 2.33 cfs @ 0.15 hrs, Volume= 0.093 af, Atten= 0%, Lag= 3.0 min  
 Primary = 2.33 cfs @ 0.15 hrs, Volume= 0.093 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 65.70' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	64.88'	<b>24.0" Round CB A01</b> L= 71.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 64.88' / 64.52' S= 0.0051 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.33 cfs @ 0.15 hrs HW=65.70' TW=65.34' (Dynamic Tailwater)↑**1=CB A01** (Outlet Controls 2.33 cfs @ 2.84 fps)**Summary for Pond C-L25: C-L25**

Inflow Area = 1.380 ac, 0.00% Impervious, Inflow Depth = 0.70" for 50-Year event  
 Inflow = 2.02 cfs @ 0.09 hrs, Volume= 0.081 af  
 Outflow = 2.02 cfs @ 0.09 hrs, Volume= 0.081 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.02 cfs @ 0.09 hrs, Volume= 0.081 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 65.62' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	64.83'	<b>24.0" Round C-L25</b> L= 61.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 64.83' / 64.52' S= 0.0051 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.02 cfs @ 0.09 hrs HW=65.62' TW=65.34' (Dynamic Tailwater)↑**1=C-L25** (Outlet Controls 2.02 cfs @ 2.58 fps)

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**Summary for Pond C-L26: C-L26**

Inflow Area = 2.870 ac, 3.83% Impervious, Inflow Depth = 0.73" for 50-Year event  
 Inflow = 4.35 cfs @ 0.09 hrs, Volume= 0.174 af  
 Outflow = 4.35 cfs @ 0.09 hrs, Volume= 0.174 af, Atten= 0%, Lag= 0.0 min  
 Primary = 4.35 cfs @ 0.09 hrs, Volume= 0.174 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 65.34' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	64.32'	<b>24.0" Round Pipe</b> L= 135.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 64.32' / 63.65' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=4.35 cfs @ 0.09 hrs HW=65.34' TW=64.44' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 4.35 cfs @ 3.96 fps)

**Summary for Pond C-L28: C-L28**

Inflow Area = 2.870 ac, 3.83% Impervious, Inflow Depth = 0.73" for 50-Year event  
 Inflow = 4.35 cfs @ 0.09 hrs, Volume= 0.174 af  
 Outflow = 4.35 cfs @ 0.10 hrs, Volume= 0.174 af, Atten= 0%, Lag= 0.6 min  
 Primary = 4.35 cfs @ 0.10 hrs, Volume= 0.174 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 64.44' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	63.55'	<b>24.0" Round Pipe</b> L= 324.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 63.55' / 59.99' S= 0.0110 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=4.35 cfs @ 0.10 hrs HW=64.44' TW=60.86' (Dynamic Tailwater)  
 ↑**1=Pipe** (Inlet Controls 4.35 cfs @ 3.21 fps)

**Summary for Pond C-L29: C-L29**

Inflow Area = 0.400 ac, 100.00% Impervious, Inflow Depth = 0.88" for 50-Year event  
 Inflow = 0.73 cfs @ 0.09 hrs, Volume= 0.029 af  
 Outflow = 0.73 cfs @ 0.13 hrs, Volume= 0.029 af, Atten= 0%, Lag= 2.4 min  
 Primary = 0.73 cfs @ 0.13 hrs, Volume= 0.029 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 61.89' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	61.50'	<b>24.0" Round C-L29</b> L= 36.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 61.50' / 61.10' S= 0.0111 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.73 cfs @ 0.13 hrs HW=61.89' TW=61.56' (Dynamic Tailwater)  
 ↑**1=C-L29** (Outlet Controls 0.73 cfs @ 2.56 fps)

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**Summary for Pond C-L30: C-L30**

Inflow Area = 0.960 ac, 41.67% Impervious, Inflow Depth = 0.78" for 50-Year event  
 Inflow = 1.56 cfs @ 0.09 hrs, Volume= 0.062 af  
 Outflow = 1.56 cfs @ 0.09 hrs, Volume= 0.062 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.56 cfs @ 0.09 hrs, Volume= 0.062 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 61.56' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	61.00'	<b>24.0" Round C-L30</b> L= 92.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 61.00' / 59.99' S= 0.0110 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.56 cfs @ 0.09 hrs HW=61.56' TW=60.86' (Dynamic Tailwater)  
 ↑**1=C-L30** (Outlet Controls 1.56 cfs @ 3.23 fps)

**Summary for Pond C-L32: C-L32**

Inflow Area = 3.830 ac, 13.32% Impervious, Inflow Depth = 0.74" for 50-Year event  
 Inflow = 5.91 cfs @ 0.10 hrs, Volume= 0.236 af  
 Outflow = 5.91 cfs @ 0.10 hrs, Volume= 0.236 af, Atten= 0%, Lag= 0.0 min  
 Primary = 5.91 cfs @ 0.10 hrs, Volume= 0.236 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 60.86' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	59.89'	<b>30.0" Round Pipe</b> L= 74.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 59.89' / 56.73' S= 0.0427 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=5.91 cfs @ 0.10 hrs HW=60.86' TW=58.04' (Dynamic Tailwater)  
 ↑**1=Pipe** (Inlet Controls 5.91 cfs @ 3.35 fps)

**Summary for Pond C-L33: C-L33**

Inflow Area = 0.410 ac, 100.00% Impervious, Inflow Depth = 0.88" for 50-Year event  
 Inflow = 0.75 cfs @ 0.09 hrs, Volume= 0.030 af  
 Outflow = 0.75 cfs @ 0.11 hrs, Volume= 0.030 af, Atten= 0%, Lag= 1.2 min  
 Primary = 0.75 cfs @ 0.11 hrs, Volume= 0.030 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 58.13' @ 0.10 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	57.14'	<b>24.0" Round C-L33</b> L= 38.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 57.14' / 56.95' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.75 cfs @ 0.11 hrs HW=58.13' TW=58.11' (Dynamic Tailwater)  
 ↑**1=C-L33** (Outlet Controls 0.75 cfs @ 0.71 fps)



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**Summary for Pond C-L34: C-L34**

Inflow Area = 1.500 ac, 27.33% Impervious, Inflow Depth = 0.72" for 50-Year event  
 Inflow = 2.24 cfs @ 0.09 hrs, Volume= 0.090 af  
 Outflow = 2.24 cfs @ 0.09 hrs, Volume= 0.090 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.24 cfs @ 0.09 hrs, Volume= 0.090 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 58.11' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.85'	<b>24.0" Round C-L34</b> L= 24.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.85' / 56.73' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.24 cfs @ 0.09 hrs HW=58.11' TW=58.04' (Dynamic Tailwater)↑**1=C-L34** (Outlet Controls 2.24 cfs @ 1.54 fps)**Summary for Pond C-L36: C-L36**

Inflow Area = 5.330 ac, 17.26% Impervious, Inflow Depth = 0.73" for 50-Year event  
 Inflow = 8.15 cfs @ 0.10 hrs, Volume= 0.326 af  
 Outflow = 8.15 cfs @ 0.14 hrs, Volume= 0.326 af, Atten= 0%, Lag= 2.4 min  
 Primary = 8.15 cfs @ 0.14 hrs, Volume= 0.326 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 58.04' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.63'	<b>36.0" Round Pipe</b> L= 126.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.63' / 56.00' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=8.15 cfs @ 0.14 hrs HW=58.04' TW=57.47' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 8.15 cfs @ 3.65 fps)**Summary for Subcatchment C01: C01**

Runoff = 0.77 cfs @ 0.09 hrs, Volume= 0.031 af, Depth= 0.86"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

Area (ac)	C	Description
0.430	0.93	Mixed Use, HSG D
0.430		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

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**Summary for Subcatchment C02: C02**

Runoff = 2.47 cfs @ 0.09 hrs, Volume= 0.098 af, Depth= 0.75"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

Area (ac)	C	Description
1.580	0.81	Mixed Use, HSG D
1.580		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment C03: C03**

Runoff = 1.53 cfs @ 0.09 hrs, Volume= 0.061 af, Depth= 0.68"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

Area (ac)	C	Description
1.070	0.74	Mixed Use, HSG D
1.070		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment C04: C04**

Runoff = 4.81 cfs @ 0.09 hrs, Volume= 0.192 af, Depth= 0.72"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

Area (ac)	C	Description
3.200	0.78	Mixed Use, HSG D
3.200		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment C05: C05**

Runoff = 4.56 cfs @ 0.09 hrs, Volume= 0.182 af, Depth= 0.56"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

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Area (ac)	C	Description
3.880	0.61	Mixed Use, HSG D
3.880		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment C06: C06**

Runoff = 0.67 cfs @ 0.09 hrs, Volume= 0.027 af, Depth= 0.84"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

Area (ac)	C	Description
0.380	0.91	Mixed Use, HSG D
0.380		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment C07: C07**

Runoff = 0.76 cfs @ 0.09 hrs, Volume= 0.030 af, Depth= 0.85"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

Area (ac)	C	Description
0.430	0.92	Mixed Use, HSG D
0.430		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment C08: C08**

Runoff = 11.77 cfs @ 0.09 hrs, Volume= 0.470 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

Area (ac)	C	Description
11.110	0.55	Mixed Use, HSG D
11.110		100.00% Pervious Area



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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment C10: C10**

Runoff = 0.20 cfs @ 0.09 hrs, Volume= 0.008 af, Depth= 0.88"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

Area (ac)	C	Description
0.110	0.95	Mixed Use, HSG D
0.110		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment C11: C11**

Runoff = 2.13 cfs @ 0.09 hrs, Volume= 0.085 af, Depth= 0.74"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

Area (ac)	C	Description
1.380	0.80	Mixed Use, HSG D
1.380		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment C12: C12**

Runoff = 2.02 cfs @ 0.09 hrs, Volume= 0.081 af, Depth= 0.70"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

Area (ac)	C	Description
1.380	0.76	Mixed Use, HSG D
1.380		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Proposed System C**

City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

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**Summary for Subcatchment C13: C13**

Runoff = 0.73 cfs @ 0.09 hrs, Volume= 0.029 af, Depth= 0.88"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

Area (ac)	C	Description
0.400	0.95	Mixed Use, HSG D
0.400		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment C14: C14**

Runoff = 0.83 cfs @ 0.09 hrs, Volume= 0.033 af, Depth= 0.71"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

Area (ac)	C	Description
0.560	0.77	Mixed Use, HSG D
0.560		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment C15: C15**

Runoff = 0.75 cfs @ 0.09 hrs, Volume= 0.030 af, Depth= 0.88"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

Area (ac)	C	Description
0.410	0.95	Mixed Use, HSG D
0.410		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment C16: C16**

Runoff = 1.49 cfs @ 0.09 hrs, Volume= 0.060 af, Depth= 0.66"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

**Proposed System C**

City of San Diego 50-Year Duration=29 min, Inten=1.91 in/hr

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Area (ac)	C	Description
1.090	0.71	Mixed Use, HSG D
1.090		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Pond OUTFALL C: OUTFALL C**

Inflow Area = 37.790 ac, 2.43% Impervious, Inflow Depth = 0.59" for 50-Year event  
 Inflow = 46.29 cfs @ 0.45 hrs, Volume= 1.848 af  
 Primary = 46.29 cfs @ 0.45 hrs, Volume= 1.848 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3



100-year Rainfall Intensity

ELEV.	FACTOR
0-1500	1.00
1500-3000	1.25
3000-4000	1.42
4000-5000	1.60
5000-6000	1.7
DESERT	1.25

TO OBTAIN CORRECT INTENSITY, MULTIPLY INTENSITY ON CHART BY FACTOR FOR DESIGN ELEVATION.

COUNTY OF SAN DIEGO  
FOR  
INTENSITY - DURATION - FREQUENCY  
CURVES  
RAINFALL

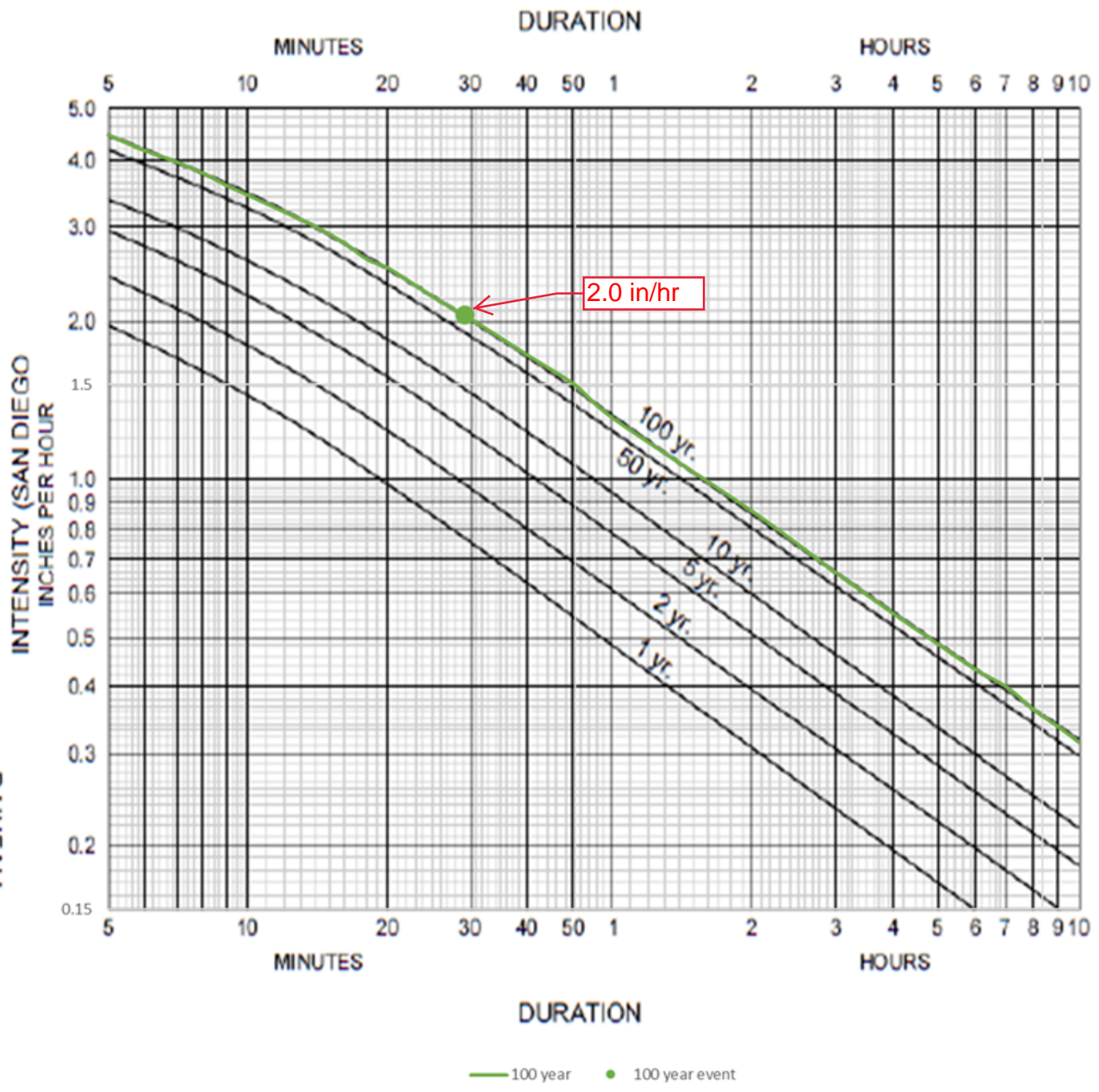


Figure A-1. Intensity-Duration-Frequency Design Chart

— 100 year    ● 100 year event

**Proposed System C**

City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

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Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points x 3  
Runoff by Rational method, Rise/Fall=1.0/1.0 xTc  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Subcatchment BASIN C1: BASIN C1</b>	Runoff Area=10.380 ac 0.00% Impervious Runoff Depth=0.53" Tc=26.0 min C=0.54 Runoff=11.51 cfs 0.460 af
<b>Pond C-BUBBLER: C-BUBBLER</b>	Peak Elev=51.28' Storage=240 cf Inflow=49.31 cfs 1.969 af 34.0" Round Culvert n=0.013 L=110.0' S=0.0027 '/' Outflow=49.32 cfs 1.969 af
<b>Pond C-L01: C-L01</b>	Peak Elev=68.15' Inflow=0.82 cfs 0.033 af 24.0" Round Culvert n=0.013 L=41.0' S=0.0388 '/' Outflow=0.82 cfs 0.033 af
<b>Pond C-L02: C-L02</b>	Peak Elev=66.37' Inflow=0.82 cfs 0.033 af 30.0" Round Culvert n=0.013 L=377.0' S=0.0070 '/' Outflow=0.82 cfs 0.033 af
<b>Pond C-L03: C-L03</b>	Peak Elev=64.12' Inflow=3.45 cfs 0.138 af 30.0" Round Culvert n=0.013 L=66.0' S=0.0071 '/' Outflow=3.45 cfs 0.138 af
<b>Pond C-L04: C-L04</b>	Peak Elev=63.80' Inflow=1.63 cfs 0.065 af 24.0" Round Culvert n=0.013 L=74.0' S=0.0008 '/' Outflow=1.63 cfs 0.065 af
<b>Pond C-L05: C-L05</b>	Peak Elev=63.65' Inflow=5.07 cfs 0.203 af 30.0" Round Culvert n=0.013 L=514.0' S=0.0062 '/' Outflow=5.07 cfs 0.203 af
<b>Pond C-L06: C-L06</b>	Peak Elev=60.89' Inflow=10.20 cfs 0.407 af 30.0" Round Culvert n=0.013 L=109.0' S=0.0070 '/' Outflow=10.20 cfs 0.407 af
<b>Pond C-L07: C-L07</b>	Peak Elev=59.94' Inflow=10.20 cfs 0.407 af 30.0" Round Culvert n=0.013 L=261.0' S=0.0075 '/' Outflow=10.20 cfs 0.407 af
<b>Pond C-L08: C-L08</b>	Peak Elev=58.13' Inflow=10.20 cfs 0.407 af 30.0" Round Culvert n=0.013 L=76.0' S=0.0050 '/' Outflow=10.20 cfs 0.407 af
<b>Pond C-L09: C-L09</b>	Peak Elev=57.53' Inflow=18.88 cfs 0.754 af 36.0" Round Culvert n=0.013 L=273.0' S=0.0094 '/' Outflow=18.88 cfs 0.754 af
<b>Pond C-L10: C-L10</b>	Peak Elev=60.64' Inflow=4.86 cfs 0.194 af 30.0" Round Culvert n=0.013 L=80.0' S=0.0045 '/' Outflow=4.86 cfs 0.194 af
<b>Pond C-L11: C-L11</b>	Peak Elev=59.88' Inflow=4.86 cfs 0.194 af 36.0" Round Culvert n=0.013 L=102.8' S=0.1095 '/' Outflow=4.86 cfs 0.194 af
<b>Pond C-L13: C-L13</b>	Peak Elev=54.75' Inflow=0.71 cfs 0.028 af 24.0" Round Culvert n=0.013 L=38.0' S=0.0063 '/' Outflow=0.71 cfs 0.028 af
<b>Pond C-L14: C-L14</b>	Peak Elev=54.74' Inflow=1.52 cfs 0.061 af 24.0" Round Culvert n=0.013 L=16.0' S=0.0063 '/' Outflow=1.52 cfs 0.061 af
<b>Pond C-L16: B-L16</b>	Peak Elev=54.72' Inflow=20.41 cfs 0.815 af 42.0" Round Culvert n=0.013 L=98.0' S=0.0268 '/' Outflow=20.41 cfs 0.815 af
<b>Pond C-L17: C-L17</b>	Peak Elev=52.95' Inflow=20.41 cfs 0.815 af 42.0" Round Culvert n=0.013 L=102.0' S=0.0245 '/' Outflow=20.41 cfs 0.815 af

**Proposed System C**

City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

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<b>Pond C-L19: C-L19</b>	Peak Elev=52.54' Inflow=36.77 cfs 1.468 af 42.0" Round Culvert n=0.013 L=200.0' S=0.0025 '/' Outflow=36.77 cfs 1.468 af
<b>Pond C-L20: C-L20</b>	Peak Elev=51.91' Inflow=36.77 cfs 1.468 af 42.0" Round Culvert n=0.013 L=50.0' S=0.0100 '/' Outflow=36.77 cfs 1.468 af
<b>Pond C-L22: C-L22</b>	Peak Elev=67.27' Inflow=0.21 cfs 0.009 af 24.0" Round Culvert n=0.013 L=45.0' S=0.0031 '/' Outflow=0.21 cfs 0.009 af
<b>Pond C-L23: C-L23</b>	Peak Elev=66.92' Inflow=0.21 cfs 0.009 af 24.0" Round Culvert n=0.013 L=311.0' S=0.0055 '/' Outflow=0.21 cfs 0.009 af
<b>Pond C-L24: C-L24</b>	Peak Elev=65.74' Inflow=2.48 cfs 0.099 af 24.0" Round Culvert n=0.013 L=71.0' S=0.0051 '/' Outflow=2.48 cfs 0.099 af
<b>Pond C-L25: C-L25</b>	Peak Elev=65.66' Inflow=2.15 cfs 0.086 af 24.0" Round Culvert n=0.013 L=61.0' S=0.0051 '/' Outflow=2.15 cfs 0.086 af
<b>Pond C-L26: C-L26</b>	Peak Elev=65.37' Inflow=4.63 cfs 0.185 af 24.0" Round Culvert n=0.013 L=135.0' S=0.0050 '/' Outflow=4.63 cfs 0.185 af
<b>Pond C-L28: C-L28</b>	Peak Elev=64.47' Inflow=4.63 cfs 0.185 af 24.0" Round Culvert n=0.013 L=324.0' S=0.0110 '/' Outflow=4.63 cfs 0.185 af
<b>Pond C-L29: C-L29</b>	Peak Elev=61.91' Inflow=0.78 cfs 0.031 af 24.0" Round Culvert n=0.013 L=36.0' S=0.0111 '/' Outflow=0.78 cfs 0.031 af
<b>Pond C-L30: C-L30</b>	Peak Elev=61.59' Inflow=1.67 cfs 0.066 af 24.0" Round Culvert n=0.013 L=92.0' S=0.0110 '/' Outflow=1.67 cfs 0.066 af
<b>Pond C-L32: C-L32</b>	Peak Elev=60.89' Inflow=6.30 cfs 0.252 af 30.0" Round Culvert n=0.013 L=74.0' S=0.0427 '/' Outflow=6.30 cfs 0.252 af
<b>Pond C-L33: C-L33</b>	Peak Elev=58.18' Inflow=0.80 cfs 0.032 af 24.0" Round Culvert n=0.013 L=38.0' S=0.0050 '/' Outflow=0.80 cfs 0.032 af
<b>Pond C-L34: C-L34</b>	Peak Elev=58.17' Inflow=2.39 cfs 0.095 af 24.0" Round Culvert n=0.013 L=24.0' S=0.0050 '/' Outflow=2.39 cfs 0.095 af
<b>Pond C-L36: C-L36</b>	Peak Elev=58.10' Inflow=8.69 cfs 0.347 af 36.0" Round Culvert n=0.013 L=126.0' S=0.0050 '/' Outflow=8.69 cfs 0.347 af
<b>Subcatchment C01: C01</b>	Runoff Area=0.430 ac 0.00% Impervious Runoff Depth=0.91" Tc=5.0 min C=0.93 Runoff=0.82 cfs 0.033 af
<b>Subcatchment C02: C02</b>	Runoff Area=1.580 ac 0.00% Impervious Runoff Depth=0.80" Tc=5.0 min C=0.81 Runoff=2.63 cfs 0.105 af
<b>Subcatchment C03: C03</b>	Runoff Area=1.070 ac 0.00% Impervious Runoff Depth=0.73" Tc=5.0 min C=0.74 Runoff=1.63 cfs 0.065 af
<b>Subcatchment C04: C04</b>	Runoff Area=3.200 ac 0.00% Impervious Runoff Depth=0.77" Tc=5.0 min C=0.78 Runoff=5.12 cfs 0.205 af



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<b>Subcatchment C05: C05</b>	Runoff Area=3.880 ac 0.00% Impervious Runoff Depth=0.60" Tc=5.0 min C=0.61 Runoff=4.86 cfs 0.194 af
<b>Subcatchment C06: C06</b>	Runoff Area=0.380 ac 0.00% Impervious Runoff Depth=0.90" Tc=5.0 min C=0.91 Runoff=0.71 cfs 0.028 af
<b>Subcatchment C07: C07</b>	Runoff Area=0.430 ac 0.00% Impervious Runoff Depth=0.90" Tc=5.0 min C=0.92 Runoff=0.81 cfs 0.032 af
<b>Subcatchment C08: C08</b>	Runoff Area=11.110 ac 0.00% Impervious Runoff Depth=0.54" Tc=5.0 min C=0.55 Runoff=12.54 cfs 0.501 af
<b>Subcatchment C10: C10</b>	Runoff Area=0.110 ac 100.00% Impervious Runoff Depth=0.93" Tc=5.0 min C=0.95 Runoff=0.21 cfs 0.009 af
<b>Subcatchment C11: C11</b>	Runoff Area=1.380 ac 0.00% Impervious Runoff Depth=0.79" Tc=5.0 min C=0.80 Runoff=2.27 cfs 0.090 af
<b>Subcatchment C12: C12</b>	Runoff Area=1.380 ac 0.00% Impervious Runoff Depth=0.75" Tc=5.0 min C=0.76 Runoff=2.15 cfs 0.086 af
<b>Subcatchment C13: C13</b>	Runoff Area=0.400 ac 100.00% Impervious Runoff Depth=0.93" Tc=5.0 min C=0.95 Runoff=0.78 cfs 0.031 af
<b>Subcatchment C14: C14</b>	Runoff Area=0.560 ac 0.00% Impervious Runoff Depth=0.76" Tc=5.0 min C=0.77 Runoff=0.89 cfs 0.035 af
<b>Subcatchment C15: C15</b>	Runoff Area=0.410 ac 100.00% Impervious Runoff Depth=0.93" Tc=5.0 min C=0.95 Runoff=0.80 cfs 0.032 af
<b>Subcatchment C16: C16</b>	Runoff Area=1.090 ac 0.00% Impervious Runoff Depth=0.70" Tc=5.0 min C=0.71 Runoff=1.59 cfs 0.063 af
<b>Pond OUTFALL C: OUTFALL C</b>	Inflow=49.32 cfs 1.969 af Primary=49.32 cfs 1.969 af

**Total Runoff Area = 37.790 ac Runoff Volume = 1.969 af Average Runoff Depth = 0.63"**  
**97.57% Pervious = 36.870 ac 2.43% Impervious = 0.920 ac**

**Proposed System C**

City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

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**Summary for Subcatchment BASIN C1: BASIN C1**

Runoff = 11.51 cfs @ 0.44 hrs, Volume= 0.460 af, Depth= 0.53"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

Area (ac)	C	Description
10.380	0.54	Mixed Use, HSG D
10.380		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
26.0					<b>Direct Entry, BASIN C1</b>

**Summary for Pond C-BUBBLER: C-BUBBLER**

Inflow Area = 37.790 ac, 2.43% Impervious, Inflow Depth = 0.63" for 100-Year event  
 Inflow = 49.31 cfs @ 0.44 hrs, Volume= 1.969 af  
 Outflow = 49.32 cfs @ 0.45 hrs, Volume= 1.969 af, Atten= 0%, Lag= 0.6 min  
 Primary = 49.32 cfs @ 0.45 hrs, Volume= 1.969 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 51.28' @ 0.45 hrs Surf.Area= 91 sf Storage= 240 cf

Plug-Flow detention time= 0.1 min calculated for 1.969 af (100% of inflow)  
 Center-of-Mass det. time= 0.1 min ( 19.5 - 19.5 )

Volume	Invert	Avail.Storage	Storage Description
#1	46.00'	113,562 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
46.00	0	0	0
51.80	100	290	290
53.00	45,302	27,241	27,531
54.00	126,760	86,031	113,562

Device	Routing	Invert	Outlet Devices
#1	Primary	46.50'	<b>34.0" Round Bubbler</b> L= 110.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 46.50' / 46.20' S= 0.0027 '/' Cc= 0.900 n= 0.013, Flow Area= 6.31 sf

**Primary OutFlow** Max=49.32 cfs @ 0.45 hrs HW=51.28' TW=0.00' (Dynamic Tailwater)  
 ↑**1=Bubbler** (Barrel Controls 49.32 cfs @ 7.82 fps)

**Proposed System C**

City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

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**Summary for Pond C-L01: C-L01**

Inflow Area = 0.430 ac, 0.00% Impervious, Inflow Depth = 0.91" for 100-Year event  
 Inflow = 0.82 cfs @ 0.09 hrs, Volume= 0.033 af  
 Outflow = 0.82 cfs @ 0.09 hrs, Volume= 0.033 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.82 cfs @ 0.09 hrs, Volume= 0.033 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 68.15' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	67.78'	<b>24.0" Round C-L1</b> L= 41.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 67.78' / 66.19' S= 0.0388 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.82 cfs @ 0.09 hrs HW=68.15' TW=66.37' (Dynamic Tailwater)  
 ↑**1=C-L1** (Inlet Controls 0.82 cfs @ 2.07 fps)

**Summary for Pond C-L02: C-L02**

Inflow Area = 0.430 ac, 0.00% Impervious, Inflow Depth = 0.91" for 100-Year event  
 Inflow = 0.82 cfs @ 0.09 hrs, Volume= 0.033 af  
 Outflow = 0.82 cfs @ 0.09 hrs, Volume= 0.033 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.82 cfs @ 0.09 hrs, Volume= 0.033 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 66.37' @ 0.11 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	65.99'	<b>30.0" Round Pipe</b> L= 377.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 65.99' / 63.35' S= 0.0070 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=0.82 cfs @ 0.09 hrs HW=66.37' TW=64.12' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 0.82 cfs @ 2.68 fps)

**Summary for Pond C-L03: C-L03**

Inflow Area = 2.010 ac, 0.00% Impervious, Inflow Depth = 0.82" for 100-Year event  
 Inflow = 3.45 cfs @ 0.09 hrs, Volume= 0.138 af  
 Outflow = 3.45 cfs @ 0.09 hrs, Volume= 0.138 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.45 cfs @ 0.09 hrs, Volume= 0.138 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 64.12' @ 0.10 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	63.25'	<b>30.0" Round C-L3</b> L= 66.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 63.25' / 62.78' S= 0.0071 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=3.45 cfs @ 0.09 hrs HW=64.12' TW=63.65' (Dynamic Tailwater)  
 ↑**1=C-L3** (Outlet Controls 3.45 cfs @ 3.37 fps)



**Proposed System C**

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**Summary for Pond C-L04: C-L04**

Inflow Area = 1.070 ac, 0.00% Impervious, Inflow Depth = 0.73" for 100-Year event  
 Inflow = 1.63 cfs @ 0.09 hrs, Volume= 0.065 af  
 Outflow = 1.63 cfs @ 0.09 hrs, Volume= 0.065 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.63 cfs @ 0.09 hrs, Volume= 0.065 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 63.80' @ 0.10 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	62.94'	<b>24.0" Round C-L4</b> L= 74.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 62.94' / 62.88' S= 0.0008 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.62 cfs @ 0.09 hrs HW=63.80' TW=63.65' (Dynamic Tailwater)↑**1=C-L4** (Outlet Controls 1.62 cfs @ 1.85 fps)**Summary for Pond C-L05: C-L05**

Inflow Area = 3.080 ac, 0.00% Impervious, Inflow Depth = 0.79" for 100-Year event  
 Inflow = 5.07 cfs @ 0.09 hrs, Volume= 0.203 af  
 Outflow = 5.07 cfs @ 0.09 hrs, Volume= 0.203 af, Atten= 0%, Lag= 0.0 min  
 Primary = 5.07 cfs @ 0.09 hrs, Volume= 0.203 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 63.65' @ 0.10 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	62.68'	<b>30.0" Round Pipe</b> L= 514.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 62.68' / 59.50' S= 0.0062 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=5.07 cfs @ 0.09 hrs HW=63.65' TW=60.89' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 5.07 cfs @ 4.29 fps)**Summary for Pond C-L06: C-L06**

Inflow Area = 6.280 ac, 0.00% Impervious, Inflow Depth = 0.78" for 100-Year event  
 Inflow = 10.20 cfs @ 0.09 hrs, Volume= 0.407 af  
 Outflow = 10.20 cfs @ 0.13 hrs, Volume= 0.407 af, Atten= 0%, Lag= 2.4 min  
 Primary = 10.20 cfs @ 0.13 hrs, Volume= 0.407 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 60.89' @ 0.11 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	59.40'	<b>30.0" Round C-L6</b> L= 109.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 59.40' / 58.64' S= 0.0070 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=10.20 cfs @ 0.13 hrs HW=60.89' TW=59.94' (Dynamic Tailwater)↑**1=C-L6** (Outlet Controls 10.20 cfs @ 4.82 fps)

**Proposed System C**

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**Summary for Pond C-L07: C-L07**

Inflow Area = 6.280 ac, 0.00% Impervious, Inflow Depth = 0.78" for 100-Year event  
 Inflow = 10.20 cfs @ 0.13 hrs, Volume= 0.407 af  
 Outflow = 10.20 cfs @ 0.13 hrs, Volume= 0.407 af, Atten= 0%, Lag= 0.0 min  
 Primary = 10.20 cfs @ 0.13 hrs, Volume= 0.407 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 59.94' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	58.54'	<b>30.0" Round C-L7</b> L= 261.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 58.54' / 56.58' S= 0.0075 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=10.20 cfs @ 0.13 hrs HW=59.94' TW=58.13' (Dynamic Tailwater)↑**1=C-L7** (Outlet Controls 10.20 cfs @ 5.19 fps)**Summary for Pond C-L08: C-L08**

Inflow Area = 6.280 ac, 0.00% Impervious, Inflow Depth = 0.78" for 100-Year event  
 Inflow = 10.20 cfs @ 0.13 hrs, Volume= 0.407 af  
 Outflow = 10.20 cfs @ 0.13 hrs, Volume= 0.407 af, Atten= 0%, Lag= 0.0 min  
 Primary = 10.20 cfs @ 0.13 hrs, Volume= 0.407 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 58.13' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.48'	<b>30.0" Round Pipe</b> L= 76.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.48' / 56.10' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=10.20 cfs @ 0.13 hrs HW=58.13' TW=57.53' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 10.20 cfs @ 4.20 fps)**Summary for Pond C-L09: C-L09**

Inflow Area = 11.610 ac, 7.92% Impervious, Inflow Depth = 0.78" for 100-Year event  
 Inflow = 18.88 cfs @ 0.13 hrs, Volume= 0.754 af  
 Outflow = 18.88 cfs @ 0.13 hrs, Volume= 0.754 af, Atten= 0%, Lag= 0.0 min  
 Primary = 18.88 cfs @ 0.13 hrs, Volume= 0.754 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 57.53' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	55.80'	<b>36.0" Round Pipe</b> L= 273.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 55.80' / 53.23' S= 0.0094 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=18.88 cfs @ 0.13 hrs HW=57.53' TW=54.72' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 18.88 cfs @ 4.48 fps)

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**Summary for Pond C-L10: C-L10**

Inflow Area = 3.880 ac, 0.00% Impervious, Inflow Depth = 0.60" for 100-Year event  
 Inflow = 4.86 cfs @ 0.09 hrs, Volume= 0.194 af  
 Outflow = 4.86 cfs @ 0.09 hrs, Volume= 0.194 af, Atten= 0%, Lag= 0.0 min  
 Primary = 4.86 cfs @ 0.09 hrs, Volume= 0.194 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 60.64' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	59.62'	<b>30.0" Round Pipe</b> L= 80.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 59.62' / 59.26' S= 0.0045 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=4.86 cfs @ 0.09 hrs HW=60.64' TW=59.88' (Dynamic Tailwater)  
 ↑**1=Pipe** (Barrel Controls 4.86 cfs @ 3.83 fps)

**Summary for Pond C-L11: C-L11**

Inflow Area = 3.880 ac, 0.00% Impervious, Inflow Depth = 0.60" for 100-Year event  
 Inflow = 4.86 cfs @ 0.09 hrs, Volume= 0.194 af  
 Outflow = 4.86 cfs @ 0.09 hrs, Volume= 0.194 af, Atten= 0%, Lag= 0.0 min  
 Primary = 4.86 cfs @ 0.09 hrs, Volume= 0.194 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 59.88' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	59.06'	<b>36.0" Round Pipe</b> L= 102.8' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 59.06' / 47.80' S= 0.1095 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=4.86 cfs @ 0.09 hrs HW=59.88' TW=51.35' (Dynamic Tailwater)  
 ↑**1=Pipe** (Inlet Controls 4.86 cfs @ 3.09 fps)

**Summary for Pond C-L13: C-L13**

Inflow Area = 0.380 ac, 0.00% Impervious, Inflow Depth = 0.90" for 100-Year event  
 Inflow = 0.71 cfs @ 0.09 hrs, Volume= 0.028 af  
 Outflow = 0.71 cfs @ 0.09 hrs, Volume= 0.028 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.71 cfs @ 0.09 hrs, Volume= 0.028 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 54.75' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	53.55'	<b>24.0" Round C-L13</b> L= 38.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 53.55' / 53.31' S= 0.0063 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.71 cfs @ 0.09 hrs HW=54.75' TW=54.74' (Dynamic Tailwater)  
 ↑**1=C-L13** (Outlet Controls 0.71 cfs @ 0.52 fps)



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**Summary for Pond C-L14: C-L14**

Inflow Area = 0.810 ac, 0.00% Impervious, Inflow Depth = 0.90" for 100-Year event  
 Inflow = 1.52 cfs @ 0.09 hrs, Volume= 0.061 af  
 Outflow = 1.52 cfs @ 0.09 hrs, Volume= 0.061 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.52 cfs @ 0.09 hrs, Volume= 0.061 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 54.74' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	53.21'	<b>24.0" Round C-L14</b> L= 16.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 53.21' / 53.11' S= 0.0063 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.52 cfs @ 0.09 hrs HW=54.74' TW=54.72' (Dynamic Tailwater)  
 ↑**1=C-L14** (Outlet Controls 1.52 cfs @ 0.82 fps)

**Summary for Pond C-L16: B-L16**

Inflow Area = 12.420 ac, 7.41% Impervious, Inflow Depth = 0.79" for 100-Year event  
 Inflow = 20.41 cfs @ 0.13 hrs, Volume= 0.815 af  
 Outflow = 20.41 cfs @ 0.13 hrs, Volume= 0.815 af, Atten= 0%, Lag= 0.0 min  
 Primary = 20.41 cfs @ 0.13 hrs, Volume= 0.815 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 54.72' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	53.03'	<b>42.0" Round Pipe</b> L= 98.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 53.03' / 50.40' S= 0.0268 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=20.41 cfs @ 0.13 hrs HW=54.72' TW=52.31' (Dynamic Tailwater)  
 ↑**1=Pipe** (Inlet Controls 20.41 cfs @ 4.43 fps)

**Summary for Pond C-L17: C-L17**

Inflow Area = 12.420 ac, 7.41% Impervious, Inflow Depth = 0.79" for 100-Year event  
 Inflow = 20.41 cfs @ 0.13 hrs, Volume= 0.815 af  
 Outflow = 20.41 cfs @ 0.13 hrs, Volume= 0.815 af, Atten= 0%, Lag= 0.0 min  
 Primary = 20.41 cfs @ 0.13 hrs, Volume= 0.815 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 52.95' @ 0.46 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	50.30'	<b>42.0" Round Pipe</b> L= 102.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 50.30' / 47.80' S= 0.0245 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=20.16 cfs @ 0.13 hrs HW=52.31' TW=51.44' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 20.16 cfs @ 5.07 fps)

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**Summary for Pond C-L19: C-L19**

Inflow Area = 26.680 ac, 3.45% Impervious, Inflow Depth = 0.66" for 100-Year event  
 Inflow = 36.77 cfs @ 0.45 hrs, Volume= 1.468 af  
 Outflow = 36.77 cfs @ 0.45 hrs, Volume= 1.468 af, Atten= 0%, Lag= 0.0 min  
 Primary = 36.77 cfs @ 0.45 hrs, Volume= 1.468 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 52.54' @ 0.45 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	47.70'	<b>42.0" Round Pipe</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 47.70' / 47.20' S= 0.0025 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=36.77 cfs @ 0.45 hrs HW=52.54' TW=51.91' (Dynamic Tailwater)  
 ↑**1=Pipe** (Inlet Controls 36.77 cfs @ 3.82 fps)

**Summary for Pond C-L20: C-L20**

Inflow Area = 26.680 ac, 3.45% Impervious, Inflow Depth = 0.66" for 100-Year event  
 Inflow = 36.77 cfs @ 0.45 hrs, Volume= 1.468 af  
 Outflow = 36.77 cfs @ 0.46 hrs, Volume= 1.468 af, Atten= 0%, Lag= 0.6 min  
 Primary = 36.77 cfs @ 0.46 hrs, Volume= 1.468 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 51.91' @ 0.45 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	47.10'	<b>42.0" Round Pipe</b> L= 50.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 47.10' / 46.60' S= 0.0100 '/ Cc= 0.900 n= 0.013, Flow Area= 9.62 sf

**Primary OutFlow** Max=36.77 cfs @ 0.46 hrs HW=51.91' TW=51.28' (Dynamic Tailwater)  
 ↑**1=Pipe** (Inlet Controls 36.77 cfs @ 3.82 fps)

**Summary for Pond C-L22: C-L22**

Inflow Area = 0.110 ac, 100.00% Impervious, Inflow Depth = 0.93" for 100-Year event  
 Inflow = 0.21 cfs @ 0.09 hrs, Volume= 0.009 af  
 Outflow = 0.21 cfs @ 0.09 hrs, Volume= 0.009 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.21 cfs @ 0.09 hrs, Volume= 0.009 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 67.27' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	67.03'	<b>24.0" Round C-L22</b> L= 45.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 67.03' / 66.89' S= 0.0031 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.21 cfs @ 0.09 hrs HW=67.27' TW=66.92' (Dynamic Tailwater)  
 ↑**1=C-L22** (Barrel Controls 0.21 cfs @ 1.53 fps)

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**Summary for Pond C-L23: C-L23**

Inflow Area = 0.110 ac, 100.00% Impervious, Inflow Depth = 0.93" for 100-Year event  
 Inflow = 0.21 cfs @ 0.09 hrs, Volume= 0.009 af  
 Outflow = 0.21 cfs @ 0.09 hrs, Volume= 0.009 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.21 cfs @ 0.09 hrs, Volume= 0.009 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 66.92' @ 0.10 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	66.69'	<b>24.0" Round Pipe</b> L= 311.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 66.69' / 64.98' S= 0.0055 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.21 cfs @ 0.09 hrs HW=66.92' TW=65.74' (Dynamic Tailwater)↑**1=Pipe** (Outlet Controls 0.21 cfs @ 1.60 fps)**Summary for Pond C-L24: C-L24**

Inflow Area = 1.490 ac, 7.38% Impervious, Inflow Depth = 0.80" for 100-Year event  
 Inflow = 2.48 cfs @ 0.09 hrs, Volume= 0.099 af  
 Outflow = 2.48 cfs @ 0.09 hrs, Volume= 0.099 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.48 cfs @ 0.09 hrs, Volume= 0.099 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 65.74' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	64.88'	<b>24.0" Round CB A01</b> L= 71.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 64.88' / 64.52' S= 0.0051 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.48 cfs @ 0.09 hrs HW=65.74' TW=65.37' (Dynamic Tailwater)↑**1=CB A01** (Outlet Controls 2.48 cfs @ 2.85 fps)**Summary for Pond C-L25: C-L25**

Inflow Area = 1.380 ac, 0.00% Impervious, Inflow Depth = 0.75" for 100-Year event  
 Inflow = 2.15 cfs @ 0.09 hrs, Volume= 0.086 af  
 Outflow = 2.15 cfs @ 0.10 hrs, Volume= 0.086 af, Atten= 0%, Lag= 0.6 min  
 Primary = 2.15 cfs @ 0.10 hrs, Volume= 0.086 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 65.66' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	64.83'	<b>24.0" Round C-L25</b> L= 61.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 64.83' / 64.52' S= 0.0051 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.15 cfs @ 0.10 hrs HW=65.66' TW=65.37' (Dynamic Tailwater)↑**1=C-L25** (Outlet Controls 2.15 cfs @ 2.59 fps)



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**Summary for Pond C-L26: C-L26**

Inflow Area = 2.870 ac, 3.83% Impervious, Inflow Depth = 0.77" for 100-Year event  
 Inflow = 4.63 cfs @ 0.10 hrs, Volume= 0.185 af  
 Outflow = 4.63 cfs @ 0.09 hrs, Volume= 0.185 af, Atten= 0%, Lag= 0.0 min  
 Primary = 4.63 cfs @ 0.09 hrs, Volume= 0.185 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 65.37' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	64.32'	<b>24.0" Round Pipe</b> L= 135.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 64.32' / 63.65' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=4.63 cfs @ 0.09 hrs HW=65.37' TW=64.47' (Dynamic Tailwater)  
 ↑**1=Pipe** (Outlet Controls 4.63 cfs @ 4.02 fps)

**Summary for Pond C-L28: C-L28**

Inflow Area = 2.870 ac, 3.83% Impervious, Inflow Depth = 0.77" for 100-Year event  
 Inflow = 4.63 cfs @ 0.09 hrs, Volume= 0.185 af  
 Outflow = 4.63 cfs @ 0.09 hrs, Volume= 0.185 af, Atten= 0%, Lag= 0.0 min  
 Primary = 4.63 cfs @ 0.09 hrs, Volume= 0.185 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 64.47' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	63.55'	<b>24.0" Round Pipe</b> L= 324.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 63.55' / 59.99' S= 0.0110 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=4.63 cfs @ 0.09 hrs HW=64.47' TW=60.89' (Dynamic Tailwater)  
 ↑**1=Pipe** (Inlet Controls 4.63 cfs @ 3.27 fps)

**Summary for Pond C-L29: C-L29**

Inflow Area = 0.400 ac, 100.00% Impervious, Inflow Depth = 0.93" for 100-Year event  
 Inflow = 0.78 cfs @ 0.09 hrs, Volume= 0.031 af  
 Outflow = 0.78 cfs @ 0.09 hrs, Volume= 0.031 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.78 cfs @ 0.09 hrs, Volume= 0.031 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 61.91' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	61.50'	<b>24.0" Round C-L29</b> L= 36.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 61.50' / 61.10' S= 0.0111 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.78 cfs @ 0.09 hrs HW=61.91' TW=61.59' (Dynamic Tailwater)  
 ↑**1=C-L29** (Outlet Controls 0.78 cfs @ 2.57 fps)

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**Summary for Pond C-L30: C-L30**

Inflow Area = 0.960 ac, 41.67% Impervious, Inflow Depth = 0.83" for 100-Year event  
 Inflow = 1.67 cfs @ 0.09 hrs, Volume= 0.066 af  
 Outflow = 1.67 cfs @ 0.09 hrs, Volume= 0.066 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.67 cfs @ 0.09 hrs, Volume= 0.066 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 61.59' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	61.00'	<b>24.0" Round C-L30</b> L= 92.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 61.00' / 59.99' S= 0.0110 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=1.67 cfs @ 0.09 hrs HW=61.59' TW=60.89' (Dynamic Tailwater)↑**1=C-L30** (Outlet Controls 1.67 cfs @ 3.25 fps)**Summary for Pond C-L32: C-L32**

Inflow Area = 3.830 ac, 13.32% Impervious, Inflow Depth = 0.79" for 100-Year event  
 Inflow = 6.30 cfs @ 0.09 hrs, Volume= 0.252 af  
 Outflow = 6.30 cfs @ 0.10 hrs, Volume= 0.252 af, Atten= 0%, Lag= 0.6 min  
 Primary = 6.30 cfs @ 0.10 hrs, Volume= 0.252 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 60.89' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	59.89'	<b>30.0" Round Pipe</b> L= 74.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 59.89' / 56.73' S= 0.0427 '/ Cc= 0.900 n= 0.013, Flow Area= 4.91 sf

**Primary OutFlow** Max=6.30 cfs @ 0.10 hrs HW=60.89' TW=58.10' (Dynamic Tailwater)↑**1=Pipe** (Inlet Controls 6.30 cfs @ 3.41 fps)**Summary for Pond C-L33: C-L33**

Inflow Area = 0.410 ac, 100.00% Impervious, Inflow Depth = 0.93" for 100-Year event  
 Inflow = 0.80 cfs @ 0.09 hrs, Volume= 0.032 af  
 Outflow = 0.80 cfs @ 0.09 hrs, Volume= 0.032 af, Atten= 0%, Lag= 0.0 min  
 Primary = 0.80 cfs @ 0.09 hrs, Volume= 0.032 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 58.18' @ 0.10 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	57.14'	<b>24.0" Round C-L33</b> L= 38.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 57.14' / 56.95' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=0.19 cfs @ 0.09 hrs HW=58.17' TW=58.17' (Dynamic Tailwater)↑**1=C-L33** (Outlet Controls 0.19 cfs @ 0.17 fps)

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**Summary for Pond C-L34: C-L34**

Inflow Area = 1.500 ac, 27.33% Impervious, Inflow Depth = 0.76" for 100-Year event  
 Inflow = 2.39 cfs @ 0.09 hrs, Volume= 0.095 af  
 Outflow = 2.39 cfs @ 0.09 hrs, Volume= 0.095 af, Atten= 0%, Lag= 0.0 min  
 Primary = 2.39 cfs @ 0.09 hrs, Volume= 0.095 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 58.17' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.85'	<b>24.0" Round C-L34</b> L= 24.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.85' / 56.73' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 3.14 sf

**Primary OutFlow** Max=2.39 cfs @ 0.09 hrs HW=58.17' TW=58.10' (Dynamic Tailwater)  
 ↑1=C-L34 (Outlet Controls 2.39 cfs @ 1.54 fps)

**Summary for Pond C-L36: C-L36**

Inflow Area = 5.330 ac, 17.26% Impervious, Inflow Depth = 0.78" for 100-Year event  
 Inflow = 8.69 cfs @ 0.09 hrs, Volume= 0.347 af  
 Outflow = 8.69 cfs @ 0.10 hrs, Volume= 0.347 af, Atten= 0%, Lag= 0.6 min  
 Primary = 8.69 cfs @ 0.10 hrs, Volume= 0.347 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 58.10' @ 0.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	56.63'	<b>36.0" Round Pipe</b> L= 126.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 56.63' / 56.00' S= 0.0050 '/ Cc= 0.900 n= 0.013, Flow Area= 7.07 sf

**Primary OutFlow** Max=8.69 cfs @ 0.10 hrs HW=58.10' TW=57.53' (Dynamic Tailwater)  
 ↑1=Pipe (Outlet Controls 8.69 cfs @ 3.68 fps)

**Summary for Subcatchment C01: C01**

Runoff = 0.82 cfs @ 0.09 hrs, Volume= 0.033 af, Depth= 0.91"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
 City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

Area (ac)	C	Description
0.430	0.93	Mixed Use, HSG D
0.430		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>



**Proposed System C**

City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

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**Summary for Subcatchment C02: C02**

Runoff = 2.63 cfs @ 0.09 hrs, Volume= 0.105 af, Depth= 0.80"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

Area (ac)	C	Description
1.580	0.81	Mixed Use, HSG D
1.580		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment C03: C03**

Runoff = 1.63 cfs @ 0.09 hrs, Volume= 0.065 af, Depth= 0.73"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

Area (ac)	C	Description
1.070	0.74	Mixed Use, HSG D
1.070		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment C04: C04**

Runoff = 5.12 cfs @ 0.09 hrs, Volume= 0.205 af, Depth= 0.77"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

Area (ac)	C	Description
3.200	0.78	Mixed Use, HSG D
3.200		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment C05: C05**

Runoff = 4.86 cfs @ 0.09 hrs, Volume= 0.194 af, Depth= 0.60"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

**Proposed System C**

City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

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Area (ac)	C	Description
3.880	0.61	Mixed Use, HSG D
3.880		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment C06: C06**

Runoff = 0.71 cfs @ 0.09 hrs, Volume= 0.028 af, Depth= 0.90"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

Area (ac)	C	Description
0.380	0.91	Mixed Use, HSG D
0.380		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment C07: C07**

Runoff = 0.81 cfs @ 0.09 hrs, Volume= 0.032 af, Depth= 0.90"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

Area (ac)	C	Description
0.430	0.92	Mixed Use, HSG D
0.430		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment C08: C08**

Runoff = 12.54 cfs @ 0.09 hrs, Volume= 0.501 af, Depth= 0.54"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

Area (ac)	C	Description
11.110	0.55	Mixed Use, HSG D
11.110		100.00% Pervious Area

**Proposed System C**

City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment C10: C10**

Runoff = 0.21 cfs @ 0.09 hrs, Volume= 0.009 af, Depth= 0.93"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

Area (ac)	C	Description
0.110	0.95	Mixed Use, HSG D
0.110		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment C11: C11**

Runoff = 2.27 cfs @ 0.09 hrs, Volume= 0.090 af, Depth= 0.79"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

Area (ac)	C	Description
1.380	0.80	Mixed Use, HSG D
1.380		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment C12: C12**

Runoff = 2.15 cfs @ 0.09 hrs, Volume= 0.086 af, Depth= 0.75"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

Area (ac)	C	Description
1.380	0.76	Mixed Use, HSG D
1.380		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>



**Proposed System C**

City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

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**Summary for Subcatchment C13: C13**

Runoff = 0.78 cfs @ 0.09 hrs, Volume= 0.031 af, Depth= 0.93"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

Area (ac)	C	Description
0.400	0.95	Mixed Use, HSG D
0.400		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment C14: C14**

Runoff = 0.89 cfs @ 0.09 hrs, Volume= 0.035 af, Depth= 0.76"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

Area (ac)	C	Description
0.560	0.77	Mixed Use, HSG D
0.560		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment C15: C15**

Runoff = 0.80 cfs @ 0.09 hrs, Volume= 0.032 af, Depth= 0.93"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

Area (ac)	C	Description
0.410	0.95	Mixed Use, HSG D
0.410		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment C16: C16**

Runoff = 1.59 cfs @ 0.09 hrs, Volume= 0.063 af, Depth= 0.70"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

**Proposed System C**

City of San Diego 100-Year Duration=29 min, Inten=2.04 in/hr

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Area (ac)	C	Description
1.090	0.71	Mixed Use, HSG D
1.090		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

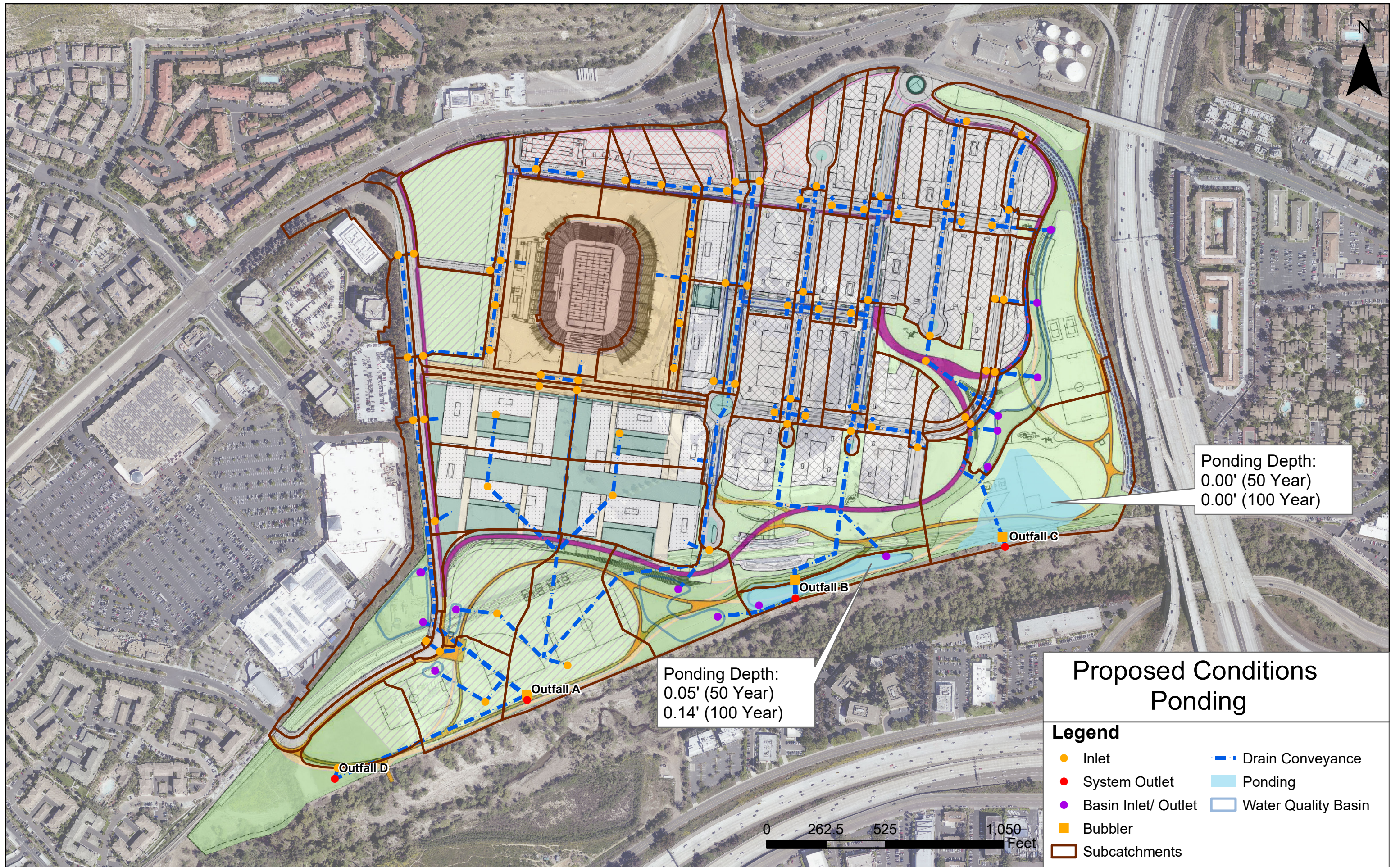
**Summary for Pond OUTFALL C: OUTFALL C**

Inflow Area = 37.790 ac, 2.43% Impervious, Inflow Depth = 0.63" for 100-Year event  
 Inflow = 49.32 cfs @ 0.45 hrs, Volume= 1.969 af  
 Primary = 49.32 cfs @ 0.45 hrs, Volume= 1.969 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs / 3

**APPENDIX B.6**  
Proposed Conditions: Estimated Extent  
Of Ponding Exhibit





Ponding Depth:  
 0.00' (50 Year)  
 0.00' (100 Year)

Ponding Depth:  
 0.05' (50 Year)  
 0.14' (100 Year)

### Proposed Conditions Ponding

- Legend**
- Inlet
  - System Outlet
  - Basin Inlet/ Outlet
  - Bubbler
  - ▭ Subcatchments
  - Drain Conveyance
  - Ponding
  - ▭ Water Quality Basin

0 262.5 525 1,050 Feet



# **APPENDIX C**

## **Offsite Conditions Supporting Material**

# **APPENDIX C.1**

## **Existing and Proposed Conditions: Offsite Area Exhibit**

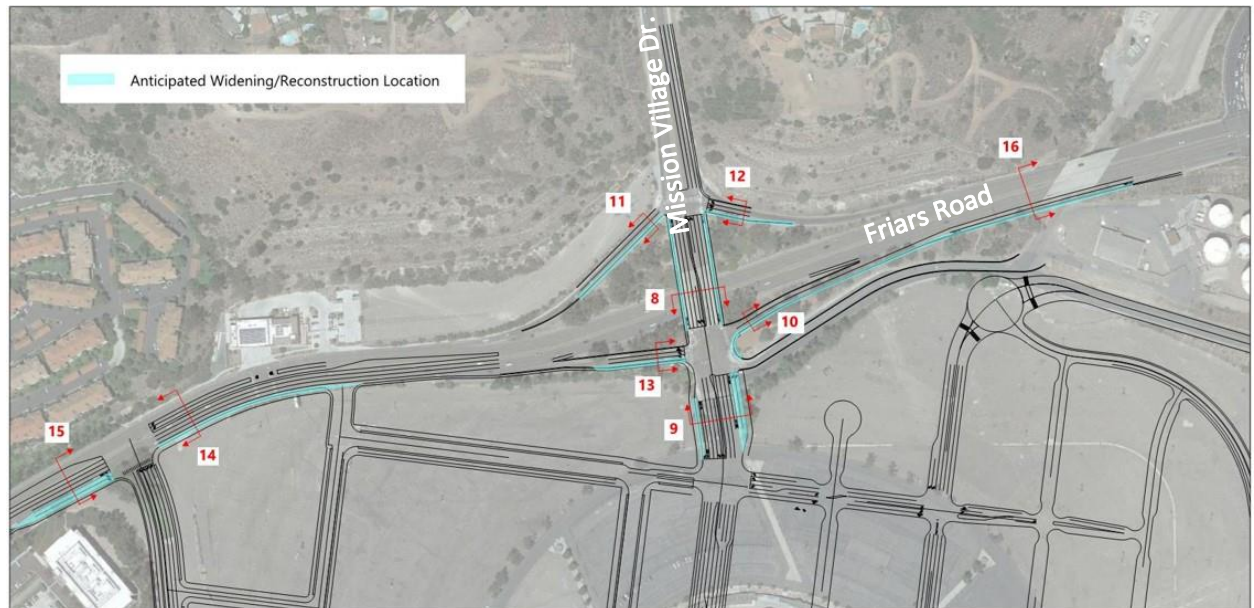


Existing offsite conditions:



\*Google earth V 6.2.2.6613. (August 13, 2018). San Diego, California. 32° 47' 09.64"S, 117° 07' 12.47"W, Eye alt 5292 feet. Landsat/ Copernicus 2018. <http://www.earth.google.com> [April 18, 2019].

Proposed offsite road widening:



\*Carrier Johnson CulturE, Rick Engineering, and Fehr Peers. (n.d.). *SDSU Mission Valley Development Package*. *SDSU Mission Valley Development Package*, tech., 1–84..

# **APPENDIX C.2**

## **Runoff Coefficient**

## APPENDIX A: RATIONAL METHOD AND MODIFIED RATIONAL METHOD

Combining the units for the expression CIA yields:

$$\left( \frac{1 \text{ acre} \times \text{inch}}{\text{hour}} \right) \left( \frac{43,560 \text{ ft}^2}{\text{acre}} \right) \left( \frac{1 \text{ foot}}{12 \text{ inches}} \right) \left( \frac{1 \text{ hour}}{3,600 \text{ seconds}} \right) \Rightarrow 1.008 \text{ cfs}$$

For practical purposes, the unit conversion coefficient difference of 0.8% can be ignored.

The RM formula is based on the assumption that for constant rainfall intensity, the peak discharge rate at a point will occur when the raindrop that falls at the most upstream point in the tributary drainage basin arrives at the point of interest.

Unlike the MRM (discussed in Appendix A.2) or the NRCS hydrologic method (discussed in Appendix B), the RM does not create hydrographs and therefore does not add separate subarea hydrographs at collection points. Instead, the RM develops peak discharges in the main line by increasing the  $T_c$  as flow travels downstream.

Characteristics of, or assumptions inherent to, the RM are listed below:

1. The discharge resulting from any I is maximum when the I lasts as long as or longer than the  $T_c$ .
2. The storm frequency of peak discharges is the same as that of I for the given  $T_c$ .
3. The fraction of rainfall that becomes runoff (or the runoff coefficient, C) is independent of I or precipitation zone number (PZN) condition (PZN Condition is discussed in the NRCS method).
4. The peak rate of runoff is the only information produced by using the RM.

### A.1.2. Runoff Coefficient

The runoff coefficients are based on land use (see Table A-1). Soil type "D" is used throughout the City of San Diego for storm drain conveyance design. An appropriate runoff coefficient (C) for each type of land use in the subarea should be selected from this table and multiplied by the percentage of the total area (A) included in that class. The sum of the products for all land uses is the weighted runoff coefficient ( $\Sigma[CA]$ ). Good engineering judgment should be used when applying the values presented in Table A-1, as adjustments to these values may be appropriate based on site-specific characteristics.



## APPENDIX A: RATIONAL METHOD AND MODIFIED RATIONAL METHOD

**Table A-1. Runoff Coefficients for Rational Method**

Land Use	Runoff Coefficient (C)
	Soil Type <sup>(1)</sup>
<b>Residential:</b>	
Single Family	0.55
Multi-Units	0.70
Mobile Homes	0.65
Rural (lots greater than 1/2 acre)	0.45
<b>Commercial <sup>(2)</sup></b>	
80% Impervious	0.85
<b>Industrial <sup>(2)</sup></b>	
<b>90% Impervious</b>	<b>0.95</b>

**Note:**

<sup>(1)</sup> Type D soil to be used for all areas.

<sup>(2)</sup> Where actual conditions deviate significantly from the tabulated imperviousness values of 80% or 90%, the values given for coefficient C, may be revised by multiplying 80% or 90% by the ratio of actual imperviousness to the tabulated imperviousness. However, in case shall the final coefficient be less than 0.50. For example: Consider commercial property on D soil.

$$\begin{array}{lcl}
 \text{Actual imperviousness} & = & 50\% \\
 \text{Tabulated imperviousness} & = & 80\% \\
 \text{Revised C} & = & (50/80) \times 0.85 = 0.53
 \end{array}$$

The values in Table A-1 are typical for urban areas. However, if the basin contains rural or agricultural land use, parks, golf courses, or other types of nonurban land use that are expected to be permanent, the appropriate value should be selected based upon the soil and cover and approved by the City.

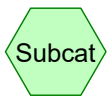
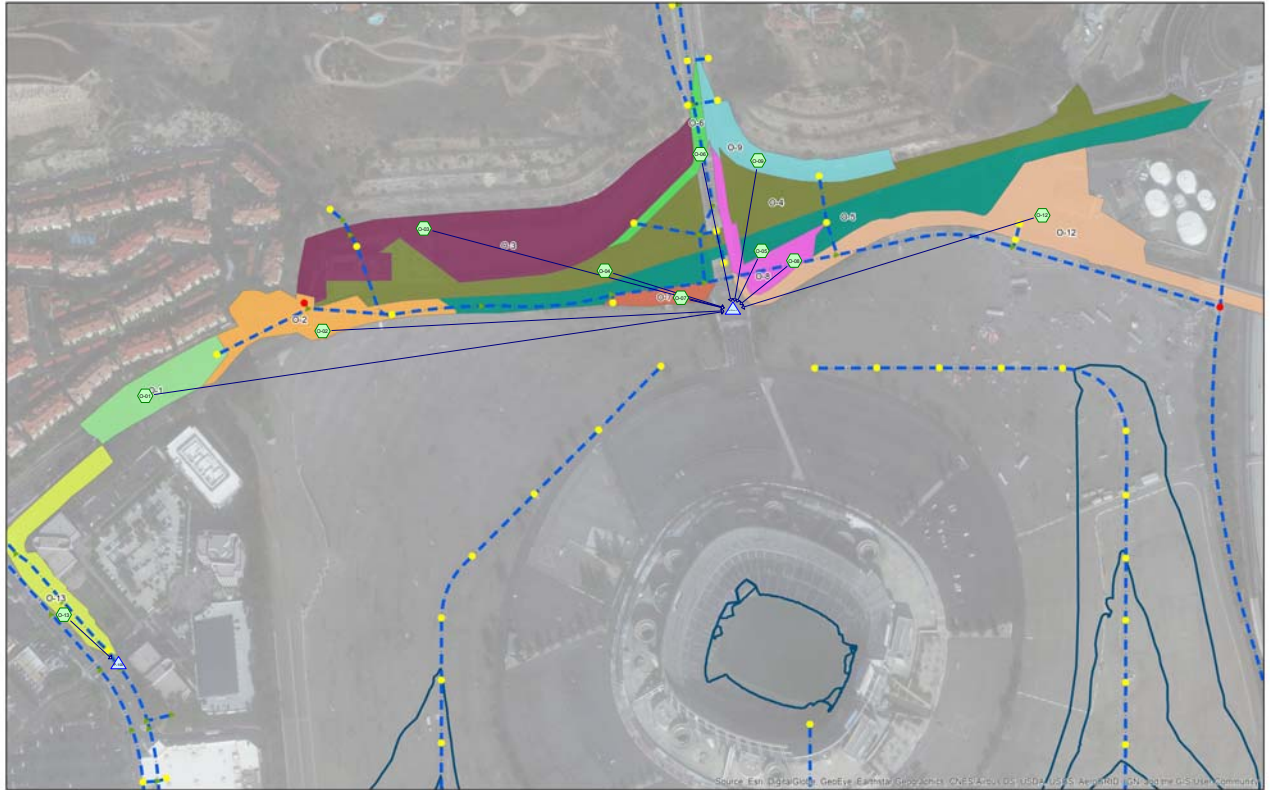
### A.1.3. Rainfall Intensity

The rainfall intensity (I) is the rainfall in inches per hour (in/hr.) for a duration equal to the T<sub>c</sub> for a selected storm frequency. Once a particular storm frequency has been selected for design and a T<sub>c</sub> calculated for the drainage area, the rainfall intensity can be determined from the Intensity-Duration-Frequency Design Chart (Figure A-1).



# **APPENDIX C.3**

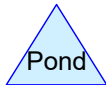
## HydroCAD Reports



Subcat



Reach



Pond



Link

**Routing Diagram for Existing Offsite Runoff**  
 Prepared by SCCM, Printed 5/24/2019  
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## Existing Offsite Runoff

Prepared by SCCM

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### Area Listing (all nodes)

Area (acres)	C	Description (subcatchment-numbers)
15.340	0.95	Mixed Use, HSG D (O-01, O-02, O-03, O-09, O-12, O-13)
5.710	0.78	Mixed Use, HSG D (O-04)
4.350	0.76	Mixed Use, HSG D (O-05)
0.880	0.94	Mixed Use, HSG D (O-06, O-07)
0.880	0.69	Mixed Use, HSG D (O-08)
<b>27.160</b>	<b>0.88</b>	<b>TOTAL AREA</b>

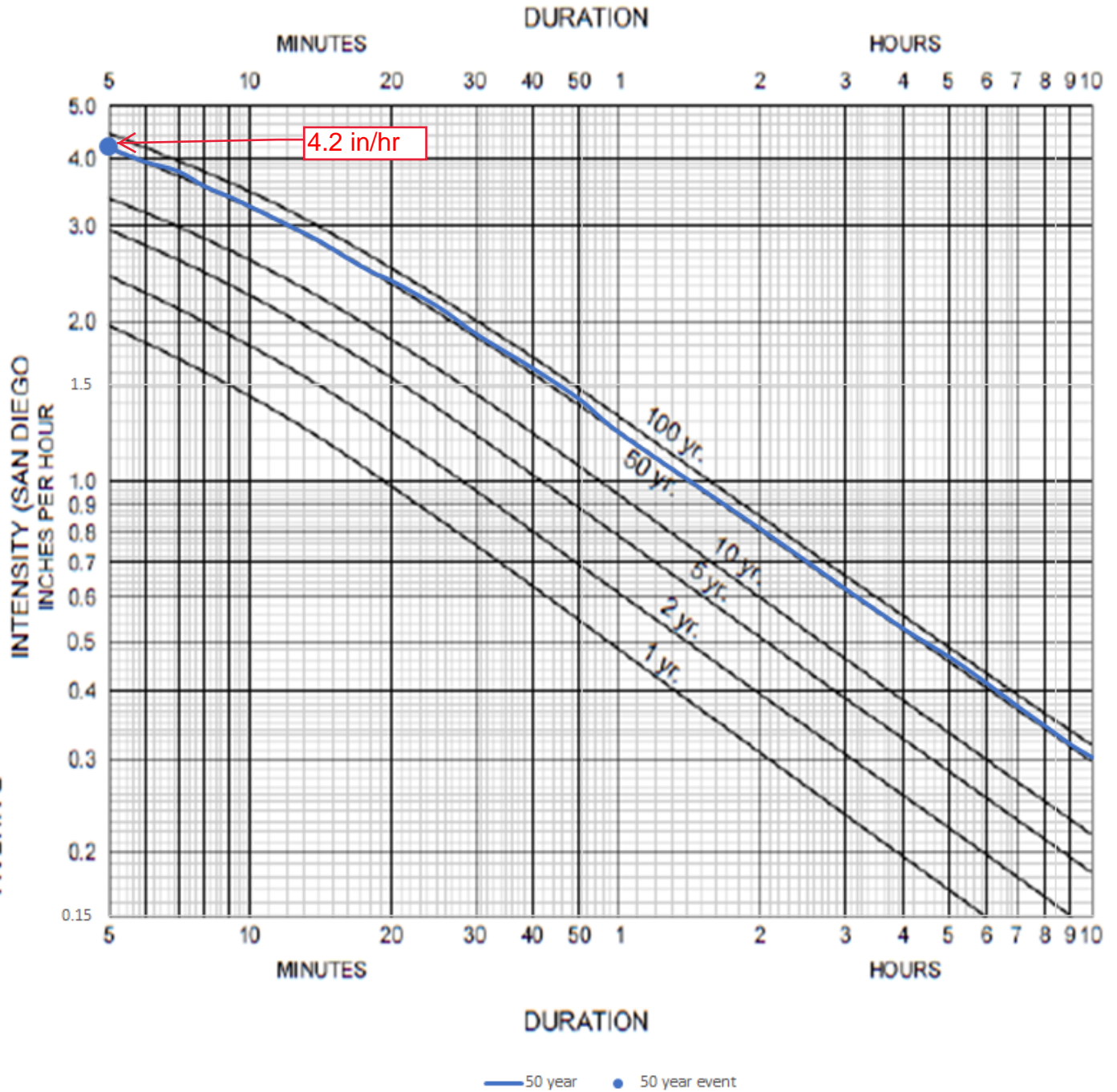
50-year Rainfall Intensity

ELEV.	FACTOR
0-1500	1.00
1500-3000	1.25
3000-4000	1.42
4000-5000	1.60
5000-6000	1.7
DESERT	1.25

TO OBTAIN CORRECT INTENSITY, MULTIPLY INTENSITY ON CHART BY FACTOR FOR DESIGN ELEVATION.

Figure A-1. Intensity-Duration-Frequency Design Chart

COUNTY OF SAN DIEGO  
FOR  
INTENSITY - DURATION - FREQUENCY  
CURVES  
RAINFALL



— 50 year    ● 50 year event

**Existing Offsite Runoff**

City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

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Time span=0.00-4.00 hrs, dt=0.01 hrs, 401 points x 2

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Subcatchment O-01: O-01</b>	Runoff Area=1.320 ac 100.00% Impervious Runoff Depth=0.48" Tc=5.0 min C=0.95 Runoff=4.30 cfs 0.053 af
<b>Subcatchment O-02: O-02</b>	Runoff Area=1.740 ac 100.00% Impervious Runoff Depth=0.48" Tc=5.0 min C=0.95 Runoff=5.67 cfs 0.070 af
<b>Subcatchment O-03: O-03</b>	Runoff Area=5.380 ac 100.00% Impervious Runoff Depth=0.48" Tc=5.0 min C=0.95 Runoff=17.52 cfs 0.217 af
<b>Subcatchment O-04: O-04</b>	Runoff Area=5.710 ac 0.00% Impervious Runoff Depth=0.40" Tc=5.0 min C=0.78 Runoff=15.27 cfs 0.189 af
<b>Subcatchment O-05: O-05</b>	Runoff Area=4.350 ac 0.00% Impervious Runoff Depth=0.39" Tc=5.0 min C=0.76 Runoff=11.33 cfs 0.141 af
<b>Subcatchment O-06: O-06</b>	Runoff Area=0.490 ac 0.00% Impervious Runoff Depth=0.48" Tc=5.0 min C=0.94 Runoff=1.58 cfs 0.020 af
<b>Subcatchment O-07: C07</b>	Runoff Area=0.390 ac 0.00% Impervious Runoff Depth=0.48" Tc=5.0 min C=0.94 Runoff=1.26 cfs 0.016 af
<b>Subcatchment O-08: O-08</b>	Runoff Area=0.880 ac 0.00% Impervious Runoff Depth=0.35" Tc=5.0 min C=0.69 Runoff=2.08 cfs 0.026 af
<b>Subcatchment O-09: C07</b>	Runoff Area=1.370 ac 100.00% Impervious Runoff Depth=0.48" Tc=5.0 min C=0.95 Runoff=4.46 cfs 0.055 af
<b>Subcatchment O-12: O-12</b>	Runoff Area=4.100 ac 100.00% Impervious Runoff Depth=0.48" Tc=5.0 min C=0.95 Runoff=13.35 cfs 0.166 af
<b>Subcatchment O-13: O-13</b>	Runoff Area=1.430 ac 100.00% Impervious Runoff Depth=0.48" Tc=5.0 min C=0.95 Runoff=4.66 cfs 0.058 af
<b>Pond OUTFALL O: OUTFALL O</b>	Inflow=76.82 cfs 0.952 af Primary=76.82 cfs 0.952 af
<b>Pond P-100: P-100</b>	Inflow=4.66 cfs 0.058 af Primary=4.66 cfs 0.058 af

**Total Runoff Area = 27.160 ac Runoff Volume = 1.010 af Average Runoff Depth = 0.45"**  
**43.52% Pervious = 11.820 ac 56.48% Impervious = 15.340 ac**



**Existing Offsite Runoff**

City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

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**Summary for Subcatchment O-01: O-01**

Runoff = 4.30 cfs @ 0.09 hrs, Volume= 0.053 af, Depth= 0.48"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
1.320	0.95	Mixed Use, HSG D
1.320		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-02: O-02**

Runoff = 5.67 cfs @ 0.09 hrs, Volume= 0.070 af, Depth= 0.48"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
1.740	0.95	Mixed Use, HSG D
1.740		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-03: O-03**

Runoff = 17.52 cfs @ 0.09 hrs, Volume= 0.217 af, Depth= 0.48"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
5.380	0.95	Mixed Use, HSG D
5.380		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Existing Offsite Runoff**

City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

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**Summary for Subcatchment O-04: O-04**

Runoff = 15.27 cfs @ 0.09 hrs, Volume= 0.189 af, Depth= 0.40"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
5.710	0.78	Mixed Use, HSG D
5.710		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-05: O-05**

Runoff = 11.33 cfs @ 0.09 hrs, Volume= 0.141 af, Depth= 0.39"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
4.350	0.76	Mixed Use, HSG D
4.350		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-06: O-06**

Runoff = 1.58 cfs @ 0.09 hrs, Volume= 0.020 af, Depth= 0.48"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
0.490	0.94	Mixed Use, HSG D
0.490		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Existing Offsite Runoff**

City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

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**Summary for Subcatchment O-07: C07**

Runoff = 1.26 cfs @ 0.09 hrs, Volume= 0.016 af, Depth= 0.48"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
0.390	0.94	Mixed Use, HSG D
0.390		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-08: O-08**

Runoff = 2.08 cfs @ 0.09 hrs, Volume= 0.026 af, Depth= 0.35"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
0.880	0.69	Mixed Use, HSG D
0.880		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-09: C07**

Runoff = 4.46 cfs @ 0.09 hrs, Volume= 0.055 af, Depth= 0.48"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
1.370	0.95	Mixed Use, HSG D
1.370		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>



**Existing Offsite Runoff**

City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

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**Summary for Subcatchment O-12: O-12**

Runoff = 13.35 cfs @ 0.09 hrs, Volume= 0.166 af, Depth= 0.48"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
4.100	0.95	Mixed Use, HSG D
4.100		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-13: O-13**

Runoff = 4.66 cfs @ 0.09 hrs, Volume= 0.058 af, Depth= 0.48"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
1.430	0.95	Mixed Use, HSG D
1.430		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Pond OUTFALL O: OUTFALL O**

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 25.730 ac, 54.06% Impervious, Inflow Depth = 0.44" for 50-Year event  
Inflow = 76.82 cfs @ 0.09 hrs, Volume= 0.952 af  
Primary = 76.82 cfs @ 0.09 hrs, Volume= 0.952 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs / 2

**Summary for Pond P-100: P-100**

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.430 ac, 100.00% Impervious, Inflow Depth = 0.48" for 50-Year event  
Inflow = 4.66 cfs @ 0.09 hrs, Volume= 0.058 af  
Primary = 4.66 cfs @ 0.09 hrs, Volume= 0.058 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs / 2

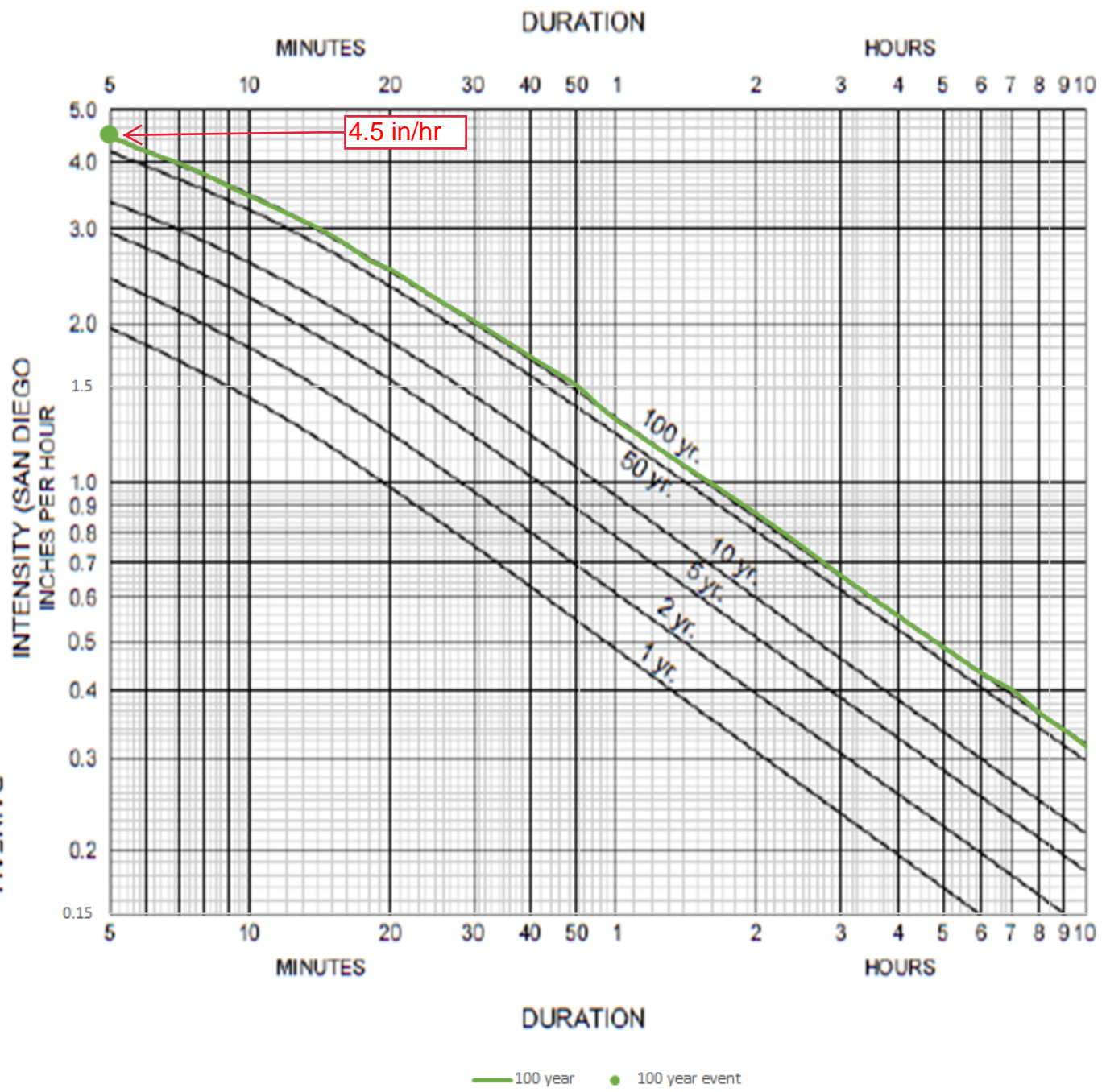
100-year Rainfall Intensity

Figure A-1. Intensity-Duration-Frequency Design Chart

ELEV.	FACTOR
0-1500	1.00
1500-3000	1.25
3000-4000	1.42
4000-5000	1.60
5000-6000	1.7
DESERT	1.25

TO OBTAIN CORRECT INTENSITY, MULTIPLY INTENSITY ON CHART BY FACTOR FOR DESIGN ELEVATION.

COUNTY OF SAN DIEGO  
FOR  
INTENSITY - DURATION - FREQUENCY  
CURVES  
RAINFALL



— 100 year    ● 100 year event

**Existing Offsite Runoff**

City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

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Time span=0.00-4.00 hrs, dt=0.01 hrs, 401 points x 2

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Subcatchment O-01: O-01</b>	Runoff Area=1.320 ac 100.00% Impervious Runoff Depth=0.51" Tc=5.0 min C=0.95 Runoff=4.55 cfs 0.056 af
<b>Subcatchment O-02: O-02</b>	Runoff Area=1.740 ac 100.00% Impervious Runoff Depth=0.51" Tc=5.0 min C=0.95 Runoff=6.00 cfs 0.074 af
<b>Subcatchment O-03: O-03</b>	Runoff Area=5.380 ac 100.00% Impervious Runoff Depth=0.51" Tc=5.0 min C=0.95 Runoff=18.55 cfs 0.230 af
<b>Subcatchment O-04: O-04</b>	Runoff Area=5.710 ac 0.00% Impervious Runoff Depth=0.42" Tc=5.0 min C=0.78 Runoff=16.17 cfs 0.200 af
<b>Subcatchment O-05: O-05</b>	Runoff Area=4.350 ac 0.00% Impervious Runoff Depth=0.41" Tc=5.0 min C=0.76 Runoff=12.00 cfs 0.149 af
<b>Subcatchment O-06: O-06</b>	Runoff Area=0.490 ac 0.00% Impervious Runoff Depth=0.51" Tc=5.0 min C=0.94 Runoff=1.67 cfs 0.021 af
<b>Subcatchment O-07: C07</b>	Runoff Area=0.390 ac 0.00% Impervious Runoff Depth=0.51" Tc=5.0 min C=0.94 Runoff=1.33 cfs 0.016 af
<b>Subcatchment O-08: O-08</b>	Runoff Area=0.880 ac 0.00% Impervious Runoff Depth=0.37" Tc=5.0 min C=0.69 Runoff=2.20 cfs 0.027 af
<b>Subcatchment O-09: C07</b>	Runoff Area=1.370 ac 100.00% Impervious Runoff Depth=0.51" Tc=5.0 min C=0.95 Runoff=4.72 cfs 0.059 af
<b>Subcatchment O-12: O-12</b>	Runoff Area=4.100 ac 100.00% Impervious Runoff Depth=0.51" Tc=5.0 min C=0.95 Runoff=14.14 cfs 0.175 af
<b>Subcatchment O-13: O-13</b>	Runoff Area=1.430 ac 100.00% Impervious Runoff Depth=0.51" Tc=5.0 min C=0.95 Runoff=4.93 cfs 0.061 af
<b>Pond OUTFALL O: OUTFALL O</b>	Inflow=81.34 cfs 1.008 af Primary=81.34 cfs 1.008 af
<b>Pond P-100: P-100</b>	Inflow=4.93 cfs 0.061 af Primary=4.93 cfs 0.061 af

**Total Runoff Area = 27.160 ac Runoff Volume = 1.070 af Average Runoff Depth = 0.47"**  
**43.52% Pervious = 11.820 ac 56.48% Impervious = 15.340 ac**



**Existing Offsite Runoff**

City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

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**Summary for Subcatchment O-01: O-01**

Runoff = 4.55 cfs @ 0.09 hrs, Volume= 0.056 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
1.320	0.95	Mixed Use, HSG D
1.320		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-02: O-02**

Runoff = 6.00 cfs @ 0.09 hrs, Volume= 0.074 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
1.740	0.95	Mixed Use, HSG D
1.740		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-03: O-03**

Runoff = 18.55 cfs @ 0.09 hrs, Volume= 0.230 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
5.380	0.95	Mixed Use, HSG D
5.380		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Existing Offsite Runoff**

City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

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**Summary for Subcatchment O-04: O-04**

Runoff = 16.17 cfs @ 0.09 hrs, Volume= 0.200 af, Depth= 0.42"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
5.710	0.78	Mixed Use, HSG D
5.710		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-05: O-05**

Runoff = 12.00 cfs @ 0.09 hrs, Volume= 0.149 af, Depth= 0.41"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
4.350	0.76	Mixed Use, HSG D
4.350		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-06: O-06**

Runoff = 1.67 cfs @ 0.09 hrs, Volume= 0.021 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
0.490	0.94	Mixed Use, HSG D
0.490		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Existing Offsite Runoff**

City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

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**Summary for Subcatchment O-07: C07**

Runoff = 1.33 cfs @ 0.09 hrs, Volume= 0.016 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
0.390	0.94	Mixed Use, HSG D
0.390		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-08: O-08**

Runoff = 2.20 cfs @ 0.09 hrs, Volume= 0.027 af, Depth= 0.37"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
0.880	0.69	Mixed Use, HSG D
0.880		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-09: C07**

Runoff = 4.72 cfs @ 0.09 hrs, Volume= 0.059 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
1.370	0.95	Mixed Use, HSG D
1.370		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>



**Existing Offsite Runoff**

City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

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**Summary for Subcatchment O-12: O-12**

Runoff = 14.14 cfs @ 0.09 hrs, Volume= 0.175 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
4.100	0.95	Mixed Use, HSG D
4.100		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Subcatchment O-13: O-13**

Runoff = 4.93 cfs @ 0.09 hrs, Volume= 0.061 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
1.430	0.95	Mixed Use, HSG D
1.430		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, PA01

**Summary for Pond OUTFALL O: OUTFALL O**

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 25.730 ac, 54.06% Impervious, Inflow Depth = 0.47" for 100-Year event  
Inflow = 81.34 cfs @ 0.09 hrs, Volume= 1.008 af  
Primary = 81.34 cfs @ 0.09 hrs, Volume= 1.008 af, Atten= 0%, Lag= 0.0 min

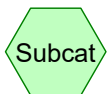
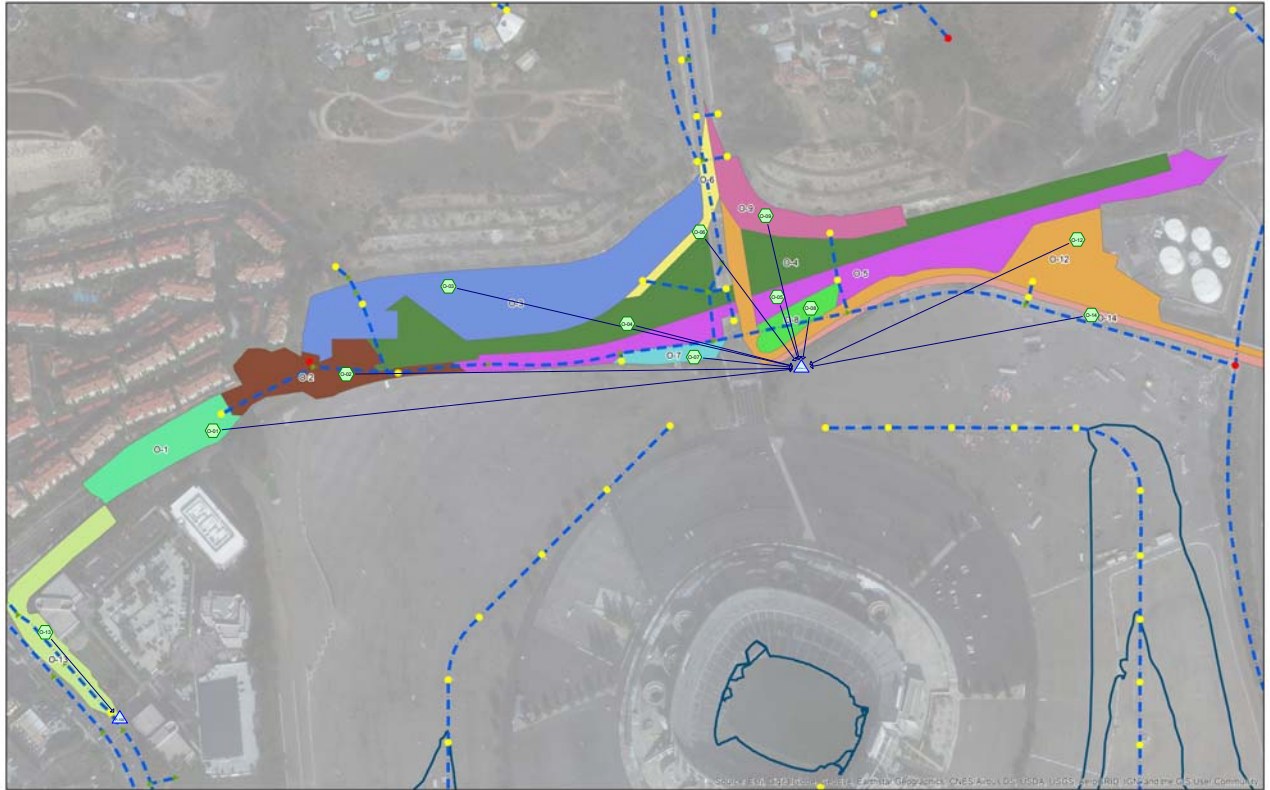
Routing by Dyn-Stor-Ind method, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs / 2

**Summary for Pond P-100: P-100**

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.430 ac, 100.00% Impervious, Inflow Depth = 0.51" for 100-Year event  
Inflow = 4.93 cfs @ 0.09 hrs, Volume= 0.061 af  
Primary = 4.93 cfs @ 0.09 hrs, Volume= 0.061 af, Atten= 0%, Lag= 0.0 min

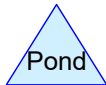
Routing by Dyn-Stor-Ind method, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs / 2



Subcat



Reach



Pond



Link

**Routing Diagram for Proposed Offsite Runoff**  
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## Proposed Offsite Runoff

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### Area Listing (all nodes)

Area (acres)	C	Description (subcatchment-numbers)
11.420	0.95	Mixed Use, HSG D (O-01, O-02, O-03, O-06, O-13, O-14)
5.680	0.80	Mixed Use, HSG D (O-04)
4.180	0.82	Mixed Use, HSG D (O-05)
0.390	0.94	Mixed Use, HSG D (O-07)
0.560	0.72	Mixed Use, HSG D (O-08)
1.430	0.91	Mixed Use, HSG D (O-09)
3.580	0.93	Mixed Use, HSG D (O-12)
<b>27.240</b>	<b>0.89</b>	<b>TOTAL AREA</b>



50-year Rainfall Intensity

ELEV.	FACTOR
0-1500	1.00
1500-3000	1.25
3000-4000	1.42
4000-5000	1.60
5000-6000	1.7
DESERT	1.25

TO OBTAIN CORRECT INTENSITY, MULTIPLY INTENSITY ON CHART BY FACTOR FOR DESIGN ELEVATION.

INTENSITY - DURATION - FREQUENCY CURVES  
FOR  
COUNTY OF SAN DIEGO  
RAINFALL

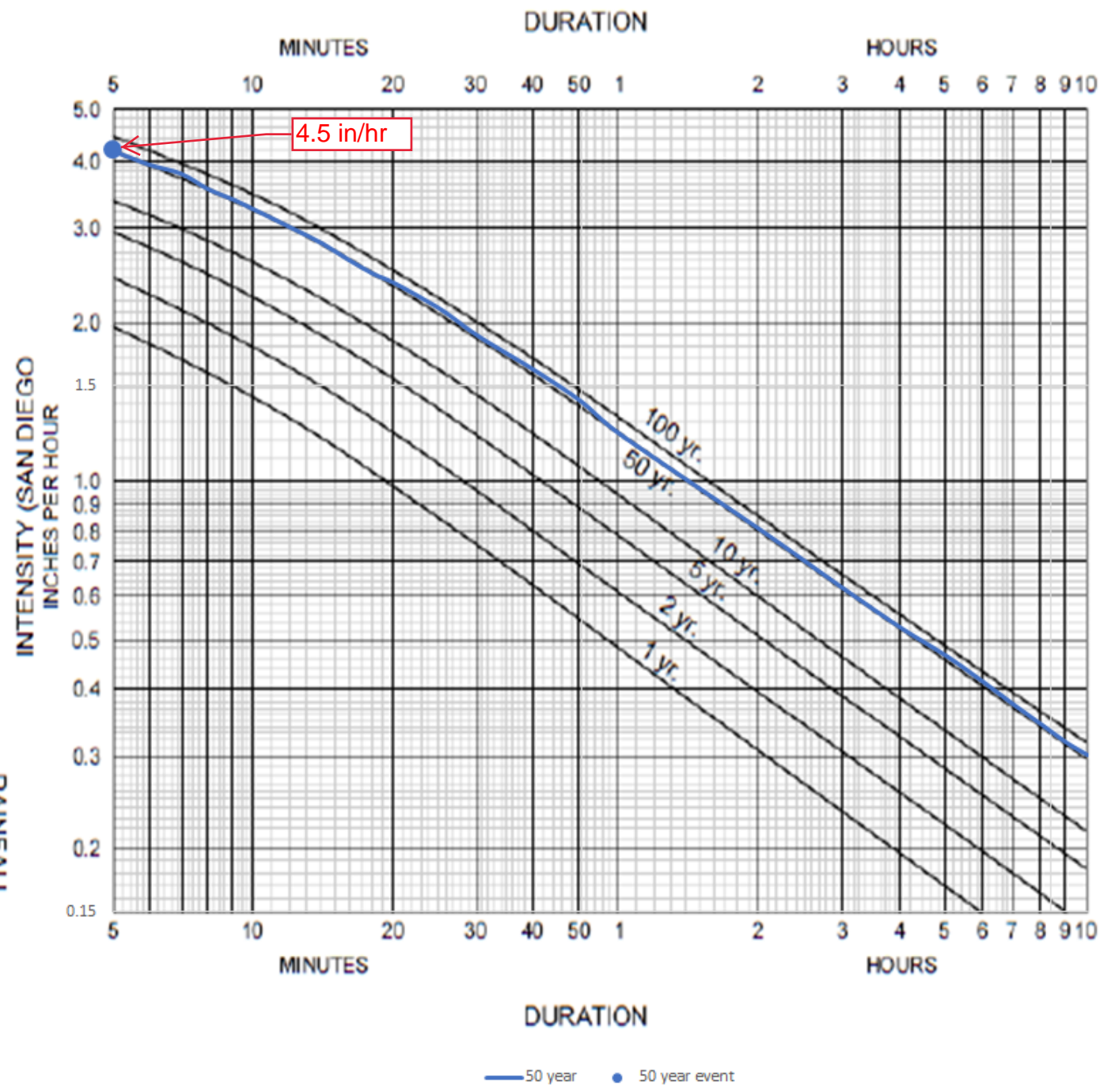


Figure A-1. Intensity-Duration-Frequency Design Chart

**Proposed Offsite Runoff**

City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

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Time span=0.00-4.00 hrs, dt=0.01 hrs, 401 points x 2

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Subcatchment O-01: O-01</b>	Runoff Area=1.390 ac 100.00% Impervious Runoff Depth=0.48" Tc=5.0 min C=0.95 Runoff=4.53 cfs 0.056 af
<b>Subcatchment O-02: O-02</b>	Runoff Area=1.780 ac 100.00% Impervious Runoff Depth=0.48" Tc=5.0 min C=0.95 Runoff=5.80 cfs 0.072 af
<b>Subcatchment O-03: O-03</b>	Runoff Area=5.380 ac 100.00% Impervious Runoff Depth=0.48" Tc=5.0 min C=0.95 Runoff=17.52 cfs 0.217 af
<b>Subcatchment O-04: O-04</b>	Runoff Area=5.680 ac 0.00% Impervious Runoff Depth=0.41" Tc=5.0 min C=0.80 Runoff=15.58 cfs 0.193 af
<b>Subcatchment O-05: O-05</b>	Runoff Area=4.180 ac 0.00% Impervious Runoff Depth=0.42" Tc=5.0 min C=0.82 Runoff=11.75 cfs 0.146 af
<b>Subcatchment O-06: O-06</b>	Runoff Area=0.500 ac 100.00% Impervious Runoff Depth=0.48" Tc=5.0 min C=0.95 Runoff=1.63 cfs 0.020 af
<b>Subcatchment O-07: C07</b>	Runoff Area=0.390 ac 0.00% Impervious Runoff Depth=0.48" Tc=5.0 min C=0.94 Runoff=1.26 cfs 0.016 af
<b>Subcatchment O-08: O-08</b>	Runoff Area=0.560 ac 0.00% Impervious Runoff Depth=0.37" Tc=5.0 min C=0.72 Runoff=1.38 cfs 0.017 af
<b>Subcatchment O-09: C07</b>	Runoff Area=1.430 ac 0.00% Impervious Runoff Depth=0.46" Tc=5.0 min C=0.91 Runoff=4.46 cfs 0.055 af
<b>Subcatchment O-12: O-12</b>	Runoff Area=3.580 ac 0.00% Impervious Runoff Depth=0.47" Tc=5.0 min C=0.93 Runoff=11.41 cfs 0.141 af
<b>Subcatchment O-13: O-13</b>	Runoff Area=1.430 ac 100.00% Impervious Runoff Depth=0.48" Tc=5.0 min C=0.95 Runoff=4.66 cfs 0.058 af
<b>Subcatchment O-14: O-14</b>	Runoff Area=0.940 ac 100.00% Impervious Runoff Depth=0.48" Tc=5.0 min C=0.95 Runoff=3.06 cfs 0.038 af
<b>Pond OUTFALL O: OUTFALL O</b>	Inflow=78.38 cfs 0.972 af Primary=78.38 cfs 0.972 af
<b>Pond P-100: P-100</b>	Inflow=4.66 cfs 0.058 af Primary=4.66 cfs 0.058 af

**Total Runoff Area = 27.240 ac Runoff Volume = 1.029 af Average Runoff Depth = 0.45"**  
**58.08% Pervious = 15.820 ac 41.92% Impervious = 11.420 ac**

**Proposed Offsite Runoff**

City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

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**Summary for Subcatchment O-01: O-01**

Runoff = 4.53 cfs @ 0.09 hrs, Volume= 0.056 af, Depth= 0.48"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
1.390	0.95	Mixed Use, HSG D
1.390		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-02: O-02**

Runoff = 5.80 cfs @ 0.09 hrs, Volume= 0.072 af, Depth= 0.48"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
1.780	0.95	Mixed Use, HSG D
1.780		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-03: O-03**

Runoff = 17.52 cfs @ 0.09 hrs, Volume= 0.217 af, Depth= 0.48"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
5.380	0.95	Mixed Use, HSG D
5.380		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>



**Proposed Offsite Runoff**

City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

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**Summary for Subcatchment O-04: O-04**

Runoff = 15.58 cfs @ 0.09 hrs, Volume= 0.193 af, Depth= 0.41"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
5.680	0.80	Mixed Use, HSG D
5.680		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-05: O-05**

Runoff = 11.75 cfs @ 0.09 hrs, Volume= 0.146 af, Depth= 0.42"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
4.180	0.82	Mixed Use, HSG D
4.180		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-06: O-06**

Runoff = 1.63 cfs @ 0.09 hrs, Volume= 0.020 af, Depth= 0.48"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
0.500	0.95	Mixed Use, HSG D
0.500		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Proposed Offsite Runoff**

City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

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**Summary for Subcatchment O-07: C07**

Runoff = 1.26 cfs @ 0.09 hrs, Volume= 0.016 af, Depth= 0.48"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
0.390	0.94	Mixed Use, HSG D
0.390		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-08: O-08**

Runoff = 1.38 cfs @ 0.09 hrs, Volume= 0.017 af, Depth= 0.37"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
0.560	0.72	Mixed Use, HSG D
0.560		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-09: C07**

Runoff = 4.46 cfs @ 0.09 hrs, Volume= 0.055 af, Depth= 0.46"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
1.430	0.91	Mixed Use, HSG D
1.430		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Proposed Offsite Runoff**

City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

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**Summary for Subcatchment O-12: O-12**

Runoff = 11.41 cfs @ 0.09 hrs, Volume= 0.141 af, Depth= 0.47"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
3.580	0.93	Mixed Use, HSG D
3.580		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-13: O-13**

Runoff = 4.66 cfs @ 0.09 hrs, Volume= 0.058 af, Depth= 0.48"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
1.430	0.95	Mixed Use, HSG D
1.430		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-14: O-14**

Runoff = 3.06 cfs @ 0.09 hrs, Volume= 0.038 af, Depth= 0.48"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

Area (ac)	C	Description
0.940	0.95	Mixed Use, HSG D
0.940		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>



**Proposed Offsite Runoff**

City of San Diego 50-Year Duration=9 min, Inten=3.40 in/hr

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**Summary for Pond OUTFALL O: OUTFALL O**

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 25.810 ac, 38.71% Impervious, Inflow Depth = 0.45" for 50-Year event  
 Inflow = 78.38 cfs @ 0.09 hrs, Volume= 0.972 af  
 Primary = 78.38 cfs @ 0.09 hrs, Volume= 0.972 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs / 2

**Summary for Pond P-100: P-100**

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.430 ac, 100.00% Impervious, Inflow Depth = 0.48" for 50-Year event  
 Inflow = 4.66 cfs @ 0.09 hrs, Volume= 0.058 af  
 Primary = 4.66 cfs @ 0.09 hrs, Volume= 0.058 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs / 2

100-year Rainfall Intensity

ELEV.	FACTOR
0-1500	1.00
1500-3000	1.25
3000-4000	1.42
4000-5000	1.60
5000-6000	1.7
DESERT	1.25

TO OBTAIN CORRECT INTENSITY, MULTIPLY INTENSITY ON CHART BY FACTOR FOR DESIGN ELEVATION.

COUNTY OF SAN DIEGO  
FOR  
INTENSITY - DURATION - FREQUENCY  
CURVES  
RAINFALL

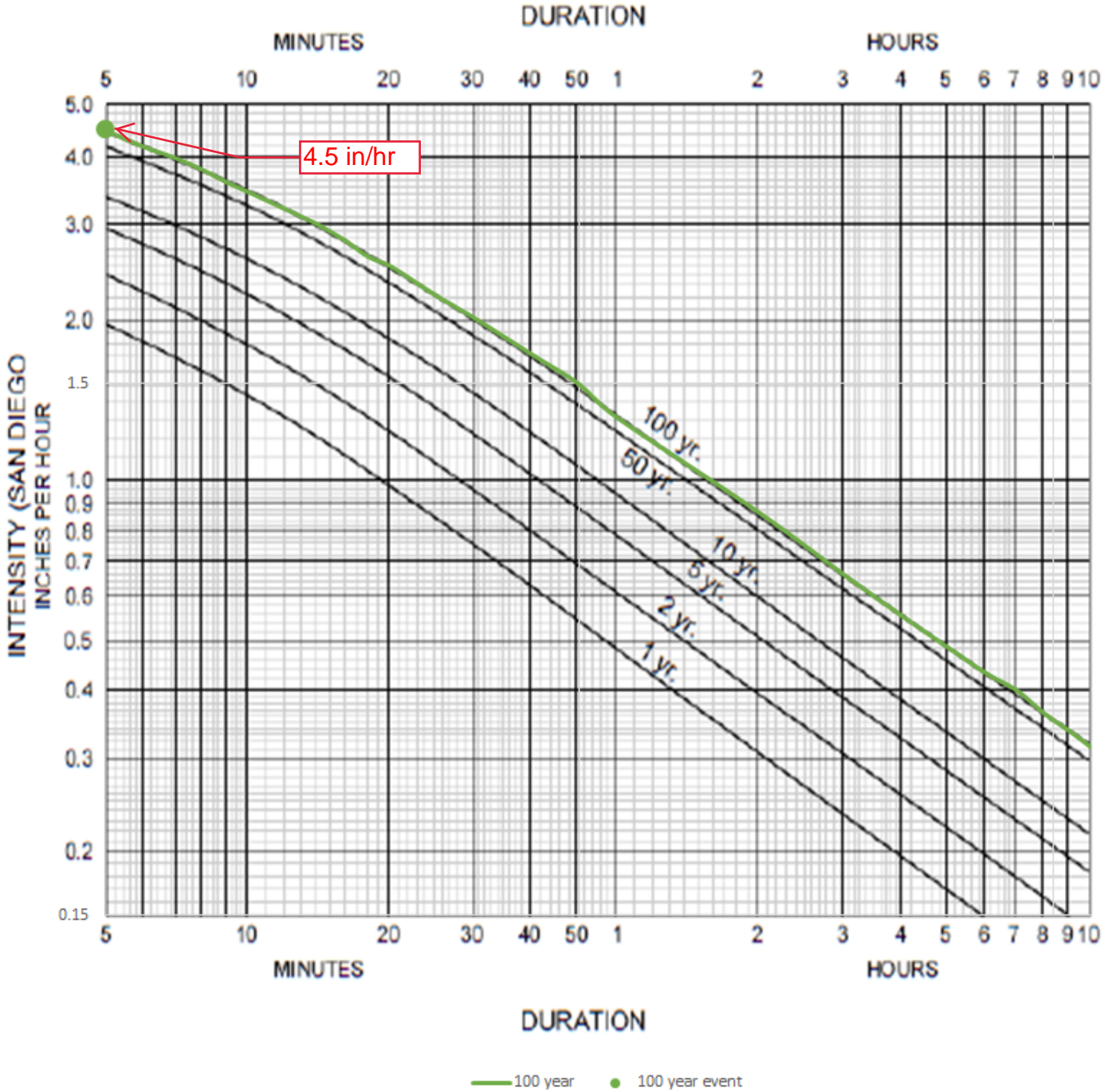


Figure A-1. Intensity-Duration-Frequency Design Chart

**Proposed Offsite Runoff**

City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

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Time span=0.00-4.00 hrs, dt=0.01 hrs, 401 points x 2

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Subcatchment O-01: O-01</b>	Runoff Area=1.390 ac 100.00% Impervious Runoff Depth=0.51" Tc=5.0 min C=0.95 Runoff=4.79 cfs 0.059 af
<b>Subcatchment O-02: O-02</b>	Runoff Area=1.780 ac 100.00% Impervious Runoff Depth=0.51" Tc=5.0 min C=0.95 Runoff=6.14 cfs 0.076 af
<b>Subcatchment O-03: O-03</b>	Runoff Area=5.380 ac 100.00% Impervious Runoff Depth=0.51" Tc=5.0 min C=0.95 Runoff=18.55 cfs 0.230 af
<b>Subcatchment O-04: O-04</b>	Runoff Area=5.680 ac 0.00% Impervious Runoff Depth=0.43" Tc=5.0 min C=0.80 Runoff=16.49 cfs 0.204 af
<b>Subcatchment O-05: O-05</b>	Runoff Area=4.180 ac 0.00% Impervious Runoff Depth=0.44" Tc=5.0 min C=0.82 Runoff=12.44 cfs 0.154 af
<b>Subcatchment O-06: O-06</b>	Runoff Area=0.500 ac 100.00% Impervious Runoff Depth=0.51" Tc=5.0 min C=0.95 Runoff=1.72 cfs 0.021 af
<b>Subcatchment O-07: C07</b>	Runoff Area=0.390 ac 0.00% Impervious Runoff Depth=0.51" Tc=5.0 min C=0.94 Runoff=1.33 cfs 0.016 af
<b>Subcatchment O-08: O-08</b>	Runoff Area=0.560 ac 0.00% Impervious Runoff Depth=0.39" Tc=5.0 min C=0.72 Runoff=1.46 cfs 0.018 af
<b>Subcatchment O-09: C07</b>	Runoff Area=1.430 ac 0.00% Impervious Runoff Depth=0.49" Tc=5.0 min C=0.91 Runoff=4.72 cfs 0.059 af
<b>Subcatchment O-12: O-12</b>	Runoff Area=3.580 ac 0.00% Impervious Runoff Depth=0.50" Tc=5.0 min C=0.93 Runoff=12.09 cfs 0.150 af
<b>Subcatchment O-13: O-13</b>	Runoff Area=1.430 ac 100.00% Impervious Runoff Depth=0.51" Tc=5.0 min C=0.95 Runoff=4.93 cfs 0.061 af
<b>Subcatchment O-14: O-14</b>	Runoff Area=0.940 ac 100.00% Impervious Runoff Depth=0.51" Tc=5.0 min C=0.95 Runoff=3.24 cfs 0.040 af
<b>Pond OUTFALL O: OUTFALL O</b>	Inflow=82.99 cfs 1.029 af Primary=82.99 cfs 1.029 af
<b>Pond P-100: P-100</b>	Inflow=4.93 cfs 0.061 af Primary=4.93 cfs 0.061 af

**Total Runoff Area = 27.240 ac Runoff Volume = 1.090 af Average Runoff Depth = 0.48"**  
**58.08% Pervious = 15.820 ac 41.92% Impervious = 11.420 ac**



**Proposed Offsite Runoff**

City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

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**Summary for Subcatchment O-01: O-01**

Runoff = 4.79 cfs @ 0.09 hrs, Volume= 0.059 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
1.390	0.95	Mixed Use, HSG D
1.390		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-02: O-02**

Runoff = 6.14 cfs @ 0.09 hrs, Volume= 0.076 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
1.780	0.95	Mixed Use, HSG D
1.780		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-03: O-03**

Runoff = 18.55 cfs @ 0.09 hrs, Volume= 0.230 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
5.380	0.95	Mixed Use, HSG D
5.380		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Proposed Offsite Runoff**

City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

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**Summary for Subcatchment O-04: O-04**

Runoff = 16.49 cfs @ 0.09 hrs, Volume= 0.204 af, Depth= 0.43"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
5.680	0.80	Mixed Use, HSG D
5.680		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-05: O-05**

Runoff = 12.44 cfs @ 0.09 hrs, Volume= 0.154 af, Depth= 0.44"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
4.180	0.82	Mixed Use, HSG D
4.180		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-06: O-06**

Runoff = 1.72 cfs @ 0.09 hrs, Volume= 0.021 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
0.500	0.95	Mixed Use, HSG D
0.500		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Proposed Offsite Runoff**

City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

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**Summary for Subcatchment O-07: C07**

Runoff = 1.33 cfs @ 0.09 hrs, Volume= 0.016 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
0.390	0.94	Mixed Use, HSG D
0.390		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-08: O-08**

Runoff = 1.46 cfs @ 0.09 hrs, Volume= 0.018 af, Depth= 0.39"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
0.560	0.72	Mixed Use, HSG D
0.560		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-09: C07**

Runoff = 4.72 cfs @ 0.09 hrs, Volume= 0.059 af, Depth= 0.49"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
1.430	0.91	Mixed Use, HSG D
1.430		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>



**Proposed Offsite Runoff**

City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

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**Summary for Subcatchment O-12: O-12**

Runoff = 12.09 cfs @ 0.09 hrs, Volume= 0.150 af, Depth= 0.50"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
3.580	0.93	Mixed Use, HSG D
3.580		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-13: O-13**

Runoff = 4.93 cfs @ 0.09 hrs, Volume= 0.061 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
1.430	0.95	Mixed Use, HSG D
1.430		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Summary for Subcatchment O-14: O-14**

Runoff = 3.24 cfs @ 0.09 hrs, Volume= 0.040 af, Depth= 0.51"

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs  
City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr

Area (ac)	C	Description
0.940	0.95	Mixed Use, HSG D
0.940		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					<b>Direct Entry, PA01</b>

**Proposed Offsite Runoff***City of San Diego 100-Year Duration=9 min, Inten=3.60 in/hr*

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**Summary for Pond OUTFALL O: OUTFALL O**

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 25.810 ac, 38.71% Impervious, Inflow Depth = 0.48" for 100-Year event  
Inflow = 82.99 cfs @ 0.09 hrs, Volume= 1.029 af  
Primary = 82.99 cfs @ 0.09 hrs, Volume= 1.029 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs / 2

**Summary for Pond P-100: P-100**

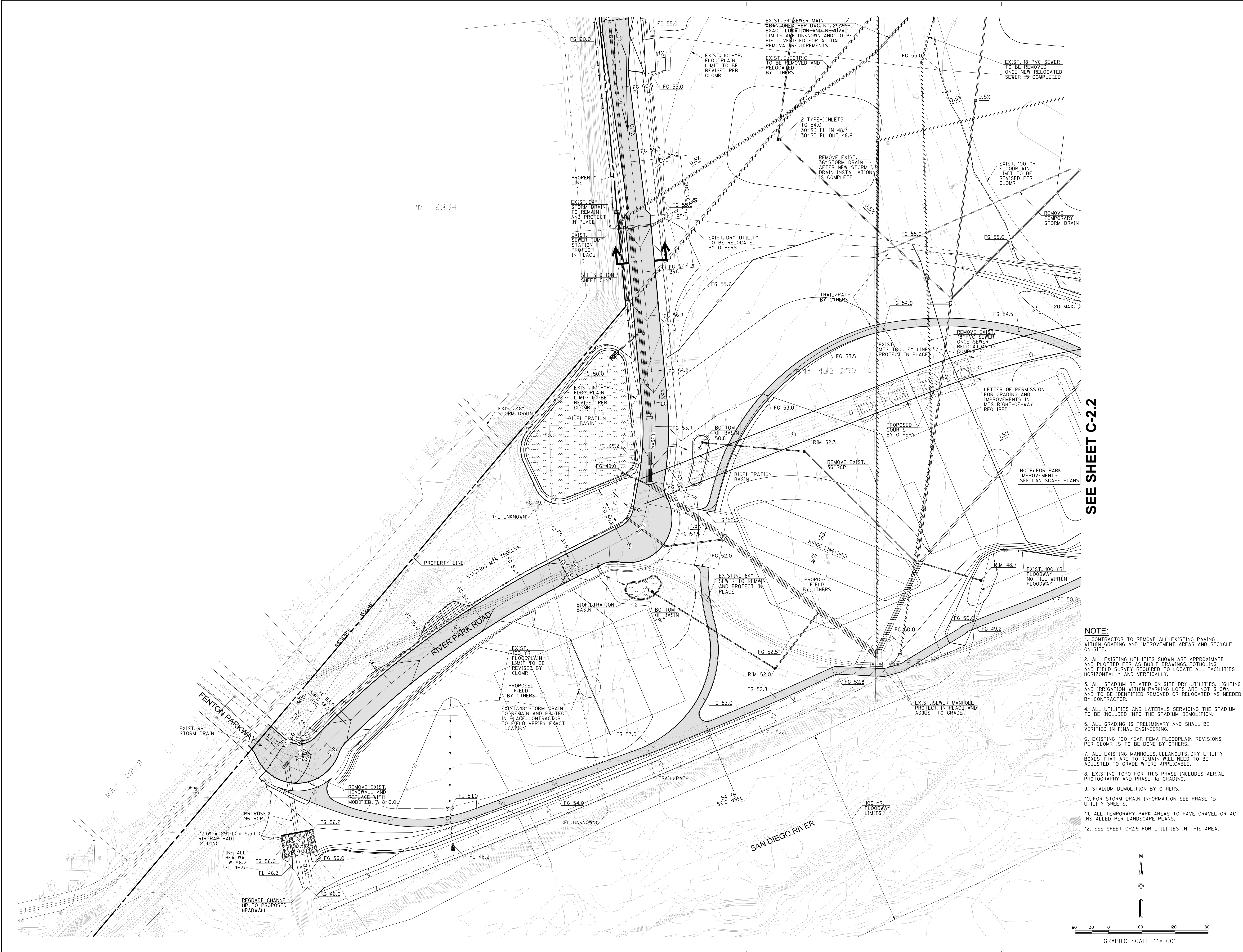
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.430 ac, 100.00% Impervious, Inflow Depth = 0.51" for 100-Year event  
Inflow = 4.93 cfs @ 0.09 hrs, Volume= 0.061 af  
Primary = 4.93 cfs @ 0.09 hrs, Volume= 0.061 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-4.00 hrs, dt= 0.01 hrs / 2

**APPENDIX C.4**  
96" Storm Drain Extension  
Drawing

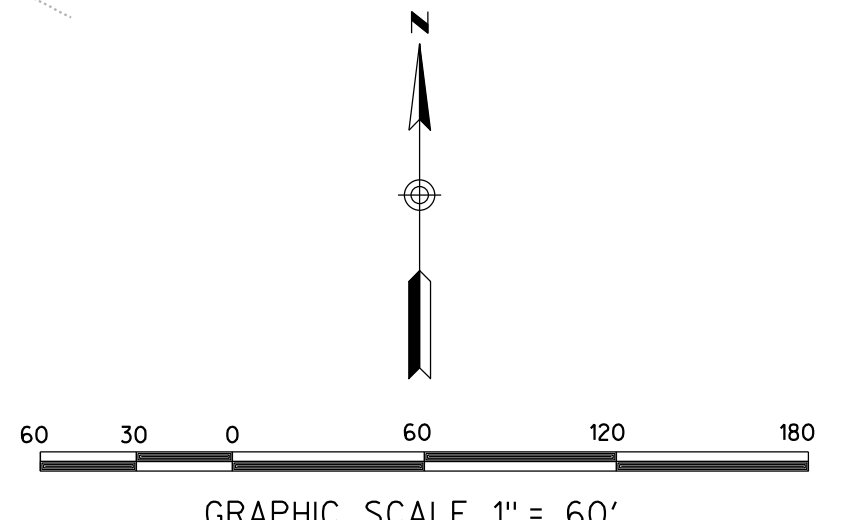




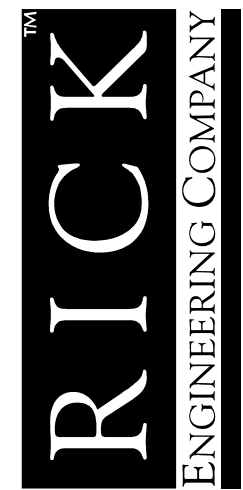
PM 18354

SEE SHEET C-2.2

- NOTE:**
1. CONTRACTOR TO REMOVE ALL EXISTING PAVING WITHIN GRADING AND IMPROVEMENT AREAS AND RECYCLE ON-SITE.
  2. ALL EXISTING UTILITIES SHOWN ARE APPROXIMATE AND PLOTTED PER AS-BUILT DRAWINGS. POT-HOLING AND FIELD SURVEY REQUIRED TO LOCATE ALL FACILITIES HORIZONTALLY AND VERTICALLY.
  3. ALL STADIUM RELATED ON-SITE DRY UTILITIES, LIGHTING AND IRRIGATION WITHIN PARKING LOTS ARE NOT SHOWN AND TO BE IDENTIFIED REMOVED OR RELOCATED AS NEEDED BY CONTRACTOR.
  4. ALL UTILITIES AND LATERALS SERVING THE STADIUM TO BE INCLUDED INTO THE STADIUM DEMOLITION.
  5. ALL GRADING IS PRELIMINARY AND SHALL BE VERIFIED IN FINAL ENGINEERING.
  6. EXISTING 100 YEAR FEMA FLOODPLAIN REVISIONS PER CLOMR IS TO BE DONE BY OTHERS.
  7. ALL EXISTING MANHOLES, CLEANOUTS, DRY UTILITY BOXES THAT ARE TO REMAIN WILL NEED TO BE ADJUSTED TO GRADE WHERE APPLICABLE.
  8. EXISTING TOPS FOR THIS PHASE INCLUDES AERIAL PHOTOGRAPHY AND PHASE 1a GRADING.
  9. STADIUM DEMOLITION BY OTHERS.
  10. FOR STORM DRAIN INFORMATION SEE PHASE 1b UTILITY SHEETS.
  11. ALL TEMPORARY PARK AREAS TO HAVE GRAVEL OR AC INSTALLED PER LANDSCAPE PLANS.
  12. SEE SHEET C-2.9 FOR UTILITIES IN THIS AREA.



5620 FRIARS ROAD  
SAN DIEGO, CA 92110  
619-291-0707  
(FAX) 619-291-4165



**SAN DIEGO STATE UNIVERSITY  
MISSION VALLEY  
75% DD SITE DEVELOPMENT PACKAGE**

ISSUED: 01/18/19-RFP  
REV. 2/12/19

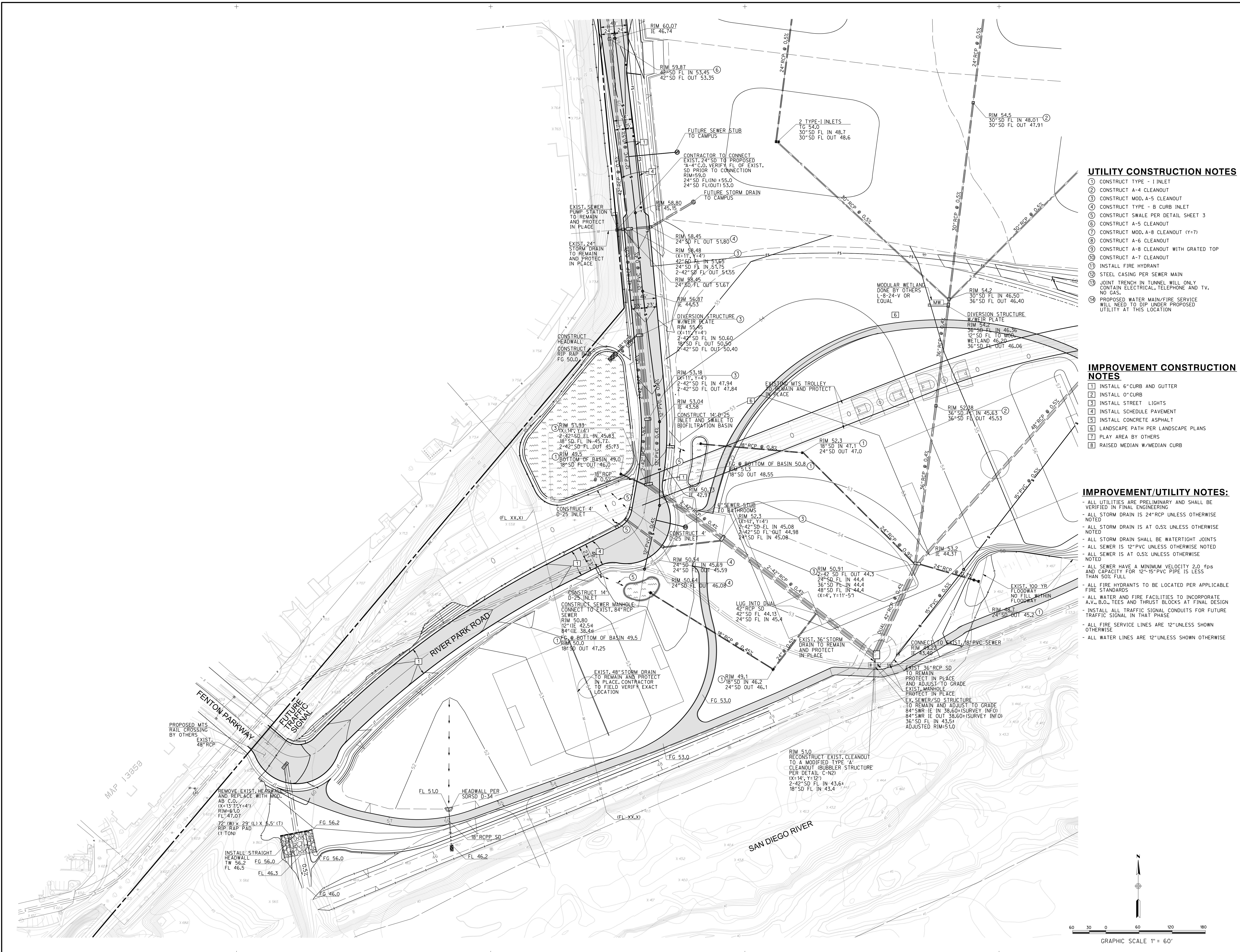
PROJECT NO:  
FILENAME:  
DRAWN BY: CHECKED BY:  
PLOT DATE:  
TITLE:

**PHASE 1B  
GRADING**

11 OF 23  
DRAWING NO:

**C-2.3**





**UTILITY CONSTRUCTION NOTES**

- 1 CONSTRUCT TYPE - I INLET
- 2 CONSTRUCT A-4 CLEANOUT
- 3 CONSTRUCT MOD. A-5 CLEANOUT
- 4 CONSTRUCT TYPE - B CURB INLET
- 5 CONSTRUCT SWALE PER DETAIL SHEET 3
- 6 CONSTRUCT A-5 CLEANOUT
- 7 CONSTRUCT MOD. A-8 CLEANOUT (Y=7)
- 8 CONSTRUCT A-6 CLEANOUT
- 9 CONSTRUCT A-8 CLEANOUT WITH GRATED TOP
- 10 CONSTRUCT A-7 CLEANOUT
- 11 INSTALL FIRE HYDRANT
- 12 STEEL CASING PER SEWER MAIN
- 13 JOINT TRENCH IN TUNNEL WILL ONLY CONTAIN ELECTRICAL, TELEPHONE AND TV. NO GAS.
- 14 PROPOSED WATER MAIN/FIRE SERVICE WILL NEED TO DIP UNDER PROPOSED UTILITY AT THIS LOCATION

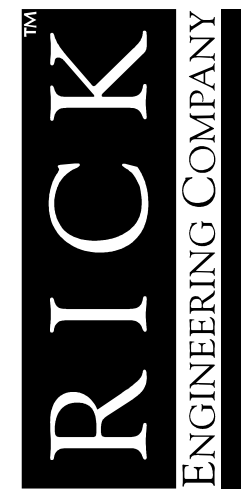
**IMPROVEMENT CONSTRUCTION NOTES**

- 1 INSTALL 6" CURB AND GUTTER
- 2 INSTALL 0" CURB
- 3 INSTALL STREET LIGHTS
- 4 INSTALL SCHEDULE PAVEMENT
- 5 INSTALL CONCRETE ASPHALT
- 6 LANDSCAPE PATH PER LANDSCAPE PLANS
- 7 PLAY AREA BY OTHERS
- 8 RAISED MEDIAN W/MEDIAN CURB

**IMPROVEMENT/UTILITY NOTES:**

- ALL UTILITIES ARE PRELIMINARY AND SHALL BE VERIFIED IN FINAL ENGINEERING
- ALL STORM DRAIN IS 24" RCP UNLESS OTHERWISE NOTED
- ALL STORM DRAIN IS AT 0.5% UNLESS OTHERWISE NOTED
- ALL STORM DRAIN SHALL BE WATERTIGHT JOINTS
- ALL SEWER IS 12" PVC UNLESS OTHERWISE NOTED
- ALL SEWER IS AT 0.5% UNLESS OTHERWISE NOTED
- ALL SEWER HAVE A MINIMUM VELOCITY 2.0 FPS AND CAPACITY FOR 12"-15" PVC PIPE IS LESS THAN 50% FULL
- ALL FIRE HYDRANTS TO BE LOCATED PER APPLICABLE FIRE STANDARDS
- ALL WATER AND FIRE FACILITIES TO INCORPORATE A.V., B.O., TEES AND THRUST BLOCKS AT FINAL DESIGN
- INSTALL ALL TRAFFIC SIGNAL CONDUITS FOR FUTURE TRAFFIC SIGNAL IN THAT PHASE
- ALL FIRE SERVICE LINES ARE 12" UNLESS SHOWN OTHERWISE
- ALL WATER LINES ARE 12" UNLESS SHOWN OTHERWISE

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**SAN DIEGO STATE UNIVERSITY  
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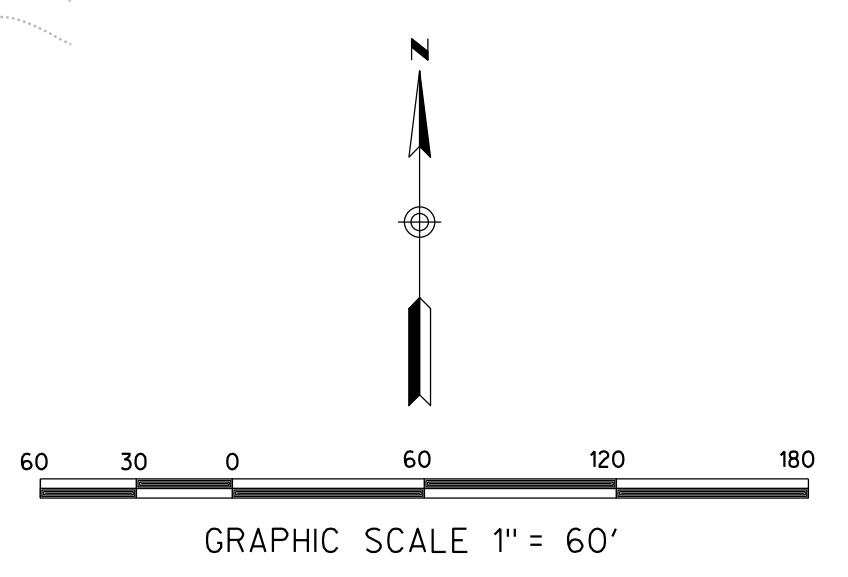
ISSUED: 01/18/19-RFP  
REV. 2/12/19

PROJECT NO:  
FILENAME:  
DRAWN BY: CHECKED BY:  
PLOT DATE:  
TITLE:

**PHASE 1B  
UTILITIES**

16 OF 23  
DRAWING NO:

**C-2.8**

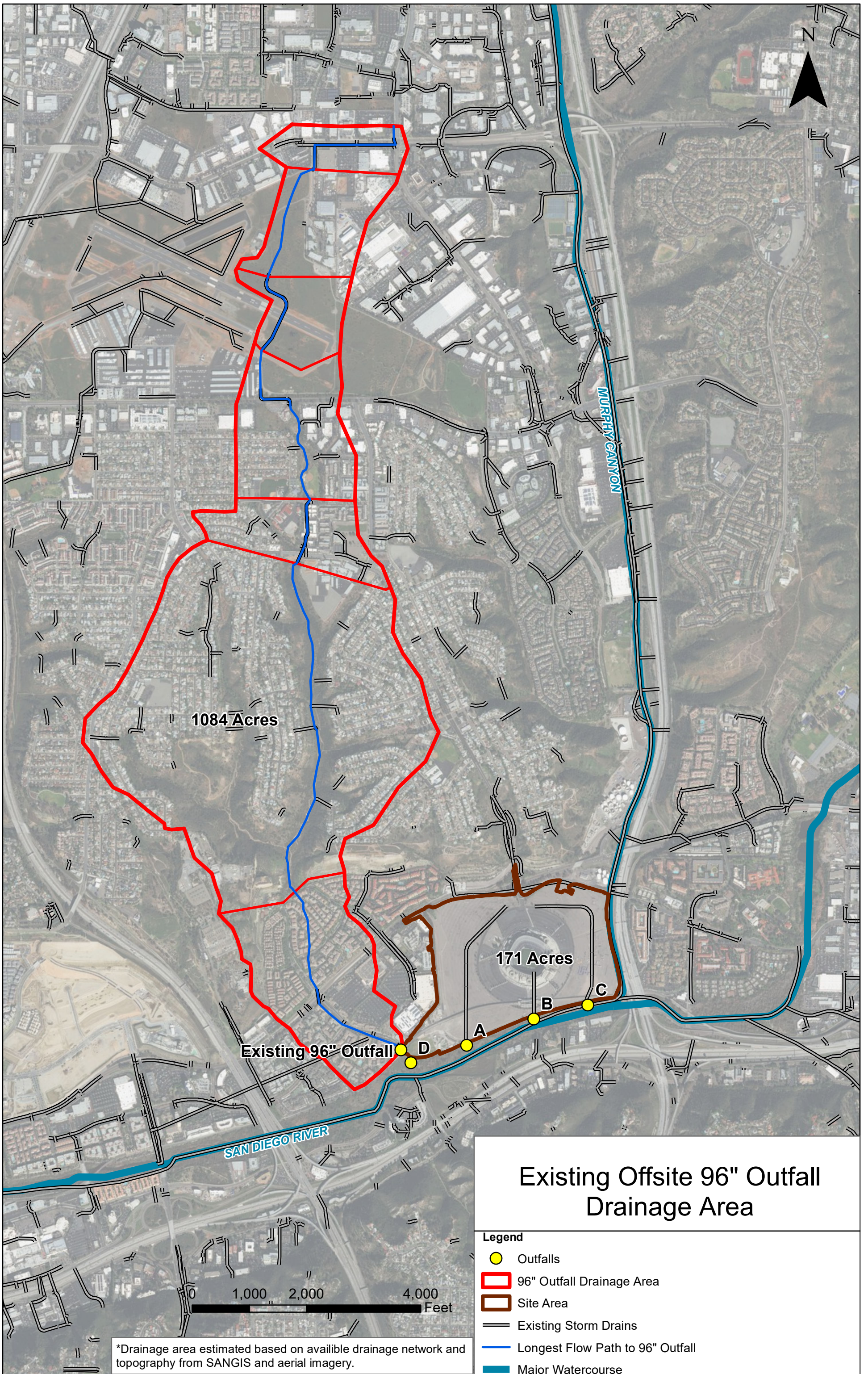




## **APPENDIX C.5**

### 96" Storm Drain Delineation Figure







## **APPENDIX C.6**

### **96" Storm Drain Time of Concentration**

System West				
Storm Drain 1				
Pipe Length	2218	feet		
Pipe Diameter	4.0	feet	Assumes 42" dia	
Pipe Slope	0.005		Averaged over full length of pipe	
manning's n	0.013		concrete	
depth (iterative for 2 year depth)	2.77	feet		
h	1.2			
Theta	2.4			
K	3.3			
Area	9.3		C	0.85
Wetted Perimeter	7.9		A	41
Hydraulic Radius	1.18		Q=CIA	83.64
Culvert velocity	9.0	feet/sec	Q=A*V	83.64
Time through culvert	4.1	minutes		
Tc	5	minutes		
Open Channel 2				
Length	2046	feet		
side slopes	2.0	feet	Assumes 42" dia	
Bottom Width	5.0			
Channel Slope	0.003		Averaged over full length of channel	
manning's n	0.030		Clean, no rifts or deep pools	
depth (iterative for 2 year depth)	4.81	feet		
Area	33.7		C	0.6
Wetted Perimeter	26.5		A	118
Hydraulic Radius	1.27		Q=CIA	106.20
Channel velocity	3.2	feet/sec	Q=A*V	106.22
Time through culvert	10.8	minutes		
Tc	16	minutes		
Storm Drain 3				
Pipe Length	1645	feet		
Pipe Diameter	6.0	feet		
Pipe Slope	0.001			
manning's n	0.013		concrete	
depth (iterative for 2 year depth)	4.19	feet		
h	1.8			
Theta	2.3			
K	7.2			
Area	21.1		C	0.6
Wetted Perimeter	11.9		A	169
Hydraulic Radius	1.78		Q=CIA	123.71
Culvert velocity	5.9	feet/sec	Q=A*V	123.71
Time through culvert	4.7	minutes		
Tc	20	minutes		
Open Channel 4				
Length	3242	feet		
side slopes	2.0	feet		
Bottom Width	30.0			
Channel Slope	0.015			
manning's n	0.040		clean winding some pools	
depth (iterative for 2 year depth)	1.08	feet		
Area	34.6		C	0.6
Wetted Perimeter	34.8		A	276
Hydraulic Radius	0.99		Q=CIA	158.98
Channel velocity	4.6	feet/sec	Q=A*V	158.98
Time through culvert	11.7	minutes		
Tc	32	minutes		
Storm Drain 5				
Pipe Length	1191	feet		
Pipe Diameter	6.0	feet		
Pipe Slope	0.047			
manning's n	0.013		Concrete	
depth (iterative for 2 year depth)	1.98	feet		
h	2.0			
Theta	2.4			
K	8.1			
Area	8.1		C	0.65
Wetted Perimeter	7.3		A	346
Hydraulic Radius	1.11		Q=CIA	215.90
Culvert velocity	26.6	feet/sec	Q=A*V	215.90
Time through culvert	0.7	minutes		
Tc	33	minutes		

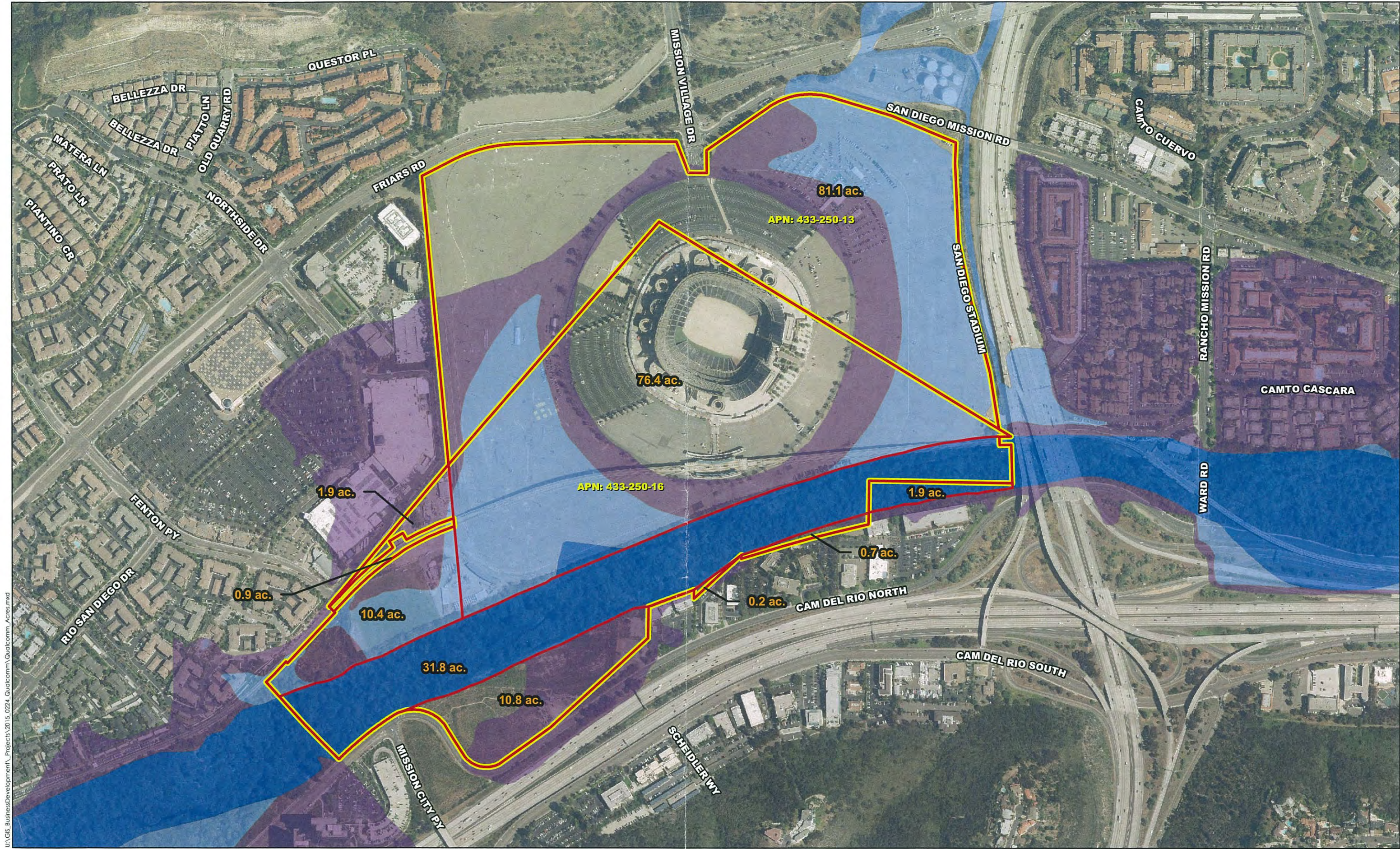


Open Channel 6			
Length	5989	feet	
side slopes	2.0	feet	
Bottom Width	30.0		
Channel Slope	0.029		
manning's n	0.048		more ineffective slopes and sections
depth (iterative for 2 year depth)	1.11	feet	
Area	35.6		C 0.6
Wetted Perimeter	35.0		A 453
Hydraulic Radius	1.02		Q=CIA 190.26
Channel velocity	5.4	feet/sec	Q=A*V 190.26
Time through culvert	18.7	minutes	
Tc	52	minutes	
Storm Drain 7			
Pipe Length	3814	feet	
Pipe Diameter	8.0	feet	
Pipe Slope	0.023		Averaged over full length of pipe
manning's n	0.013		concrete
depth (iterative for 2 year depth)	1.59	feet	
h	1.6		
Theta	1.8		
K	7.1		
Area	7.1		C 0.5
Wetted Perimeter	7.4		A 338.264288
Hydraulic Radius	0.96		Q=CIA 118.39
Culvert velocity	16.7	feet/sec	Q=A*V 118.44
Time through culvert	3.8	minutes	
Tc	55	minutes	
Tc	55	minutes	

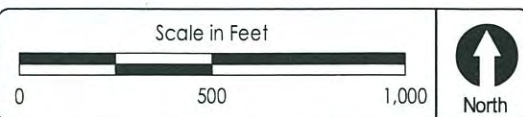
# **APPENDIX D**

## FEMA Flood Plain





U:\GIS\BusinessDevelopment\Projects\2015\_0224\_Qualcomm\Qualcomm\_Acres.mxd



Date of Exhibit: 4/8/2015  
 DigitalGlobe Aerial Image: 04.2013  
 Utilities: SANGIS  
 FEMA NFHL: 06.2014

- Legend**
- Segment
  - Parcels
  - FEMA 100-YR Floodway
  - 100-YR Floodplain
  - 500-YR Floodplain

## Qualcomm Stadium Parcel Acres



**NOTES TO USERS**

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations (BFEs)** and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

**Coastal Base Flood Elevations (BFEs)** shown on this map apply only to landward of 0.0' North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Universal Transverse Mercator (UTM) Zone 11. The horizontal datum was NAD83, GRS1980 spheroid. Differences in datum, spheroid, projection or UTM zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at <http://www.ngs.noaa.gov/> or contact the National Geodetic Survey at the following address:

NGS Information Services  
NOAA, NNGS12  
National Geodetic Survey  
SSM3-3 #9202  
1315 East-West Highway  
Silver Spring, Maryland 20910-3282  
(301) 713-3242

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at <http://www.ngs.noaa.gov/>.

**Base map** information shown on this FIRM was provided in digital format by the USDA National Agriculture Imagery Program (NAIP). This information was photogrammetrically compiled at a scale of 1:24,000 from aerial photography dated 2009.

This map reflects more detailed and up-to-date **stream channel configurations** than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map.

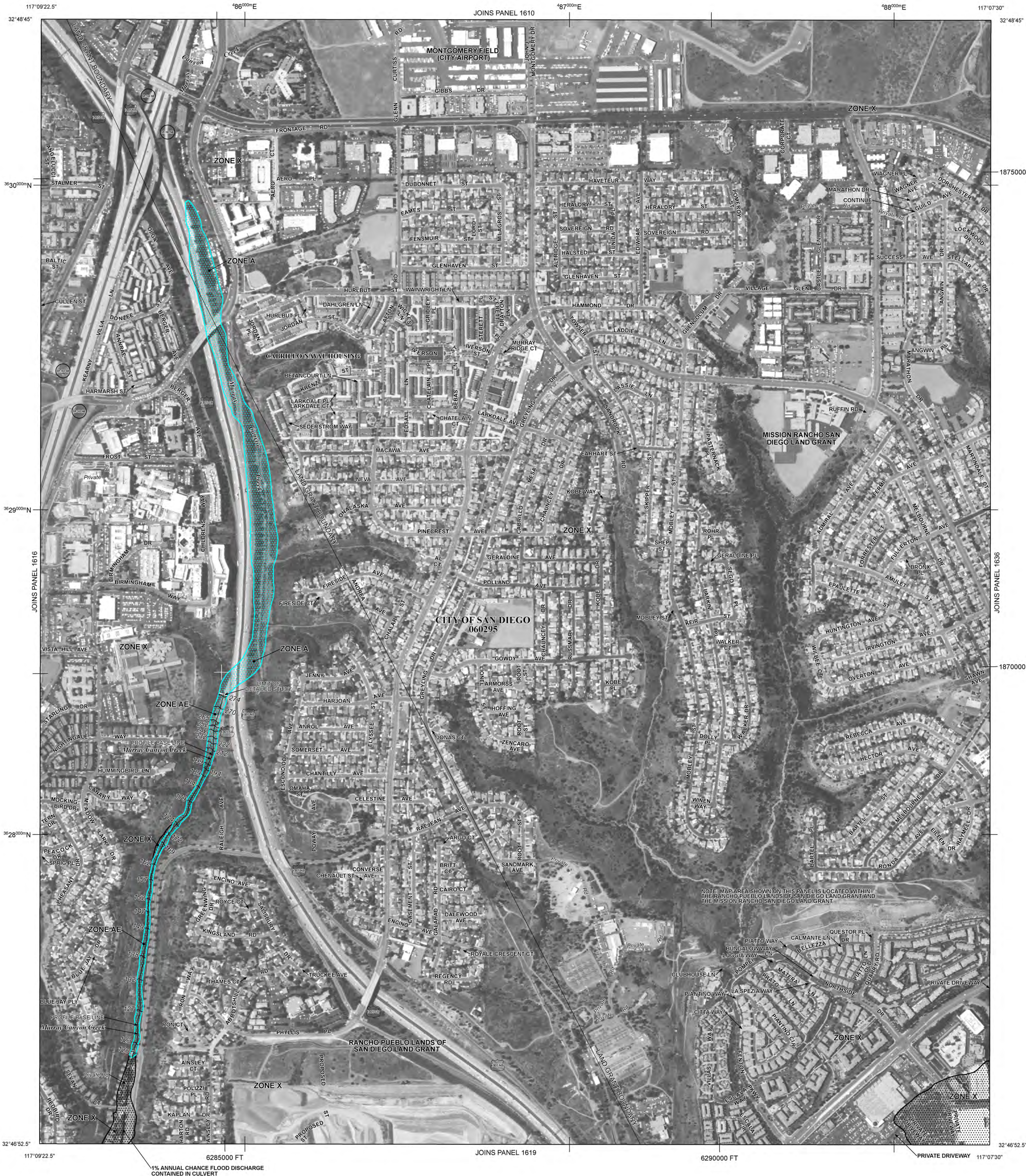
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The **"profile base lines"** depicted on this map represent the hydraulic modeling baselines that match the flood profiles in the FIS report. As a result of improved topographic data, the "profile base line", in some cases, may deviate significantly from the channel centerline or appear outside the SFHA.



**LEGEND**

**SPECIAL FLOOD HAZARD AREAS SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD**

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

- ZONE A** No Base Flood Elevations determined.
- ZONE AE** Base Flood Elevations determined.
- ZONE AH** Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
- ZONE AO** Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
- ZONE AR** Special Flood Hazard Area formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
- ZONE A99** Areas to be protected from the 1% annual chance flood event by a Federal flood protection system under construction; no Base Flood Elevations determined.
- ZONE V** Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
- ZONE VE** Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

**FLOODWAY AREAS IN ZONE AE**

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

**OTHER FLOOD AREAS**

- ZONE X** Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

**OTHER AREAS**

- ZONE X** Areas determined to be outside the 0.2% annual chance floodplain.
- ZONE D** Areas in which flood hazards are undetermined, but possible.

**COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS**

**OTHERWISE PROTECTED AREAS (OPAs)**

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

- 1% annual chance floodplain boundary
- 0.2% annual chance floodplain boundary
- Floodway boundary
- Zone D boundary
- CBRS and OPA boundary
- Boundary dividing Special Flood Hazard Area Zones and boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths, or flood velocities
- Base Flood Elevation line and value; elevation in feet\*
- Base Flood Elevation value where uniform within zone; elevation in feet\*

\* Referenced to the North American Vertical Datum of 1988

**Cross section line**

Transect line

Geographic coordinates referenced to the North American Datum of 1983 (NAD 83), Western Hemisphere

1000-meter Universal Transverse Mercator grid ticks, zone 11

5000-foot grid values; California State Plane coordinate system, Zone VI (FIPSZONE = 405), Lambert projection

Bench mark (see explanation in Notes to Users section of this FIRM panel)

River Mile

**MAP REPOSITORIES**

Refer to Map Repositories list on Map Index

**EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP**

June 19, 1997

**EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL**

May 16, 2012 - to update corporate limits, to add roads and road names, to incorporate previously issued Letters of Map Revision, and to update map elevations to North American Vertical Datum of 1988.

For community map revision history prior to countywide mapping, refer to the Community Map History table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

**MAP SCALE 1" = 500'**

250 0 250 500 750 1,000 FEET  
150 0 150 300 METERS

**NATIONAL FLOOD INSURANCE PROGRAM**

**PANEL 1617G**

**FIRM**

**FLOOD INSURANCE RATE MAP**

**SAN DIEGO COUNTY, CALIFORNIA**

**AND INCORPORATED AREAS**

**PANEL 1617 OF 2375**

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

**CONTAINS:**

<b>COMMUNITY</b>	<b>NUMBER</b>	<b>PANEL</b>	<b>SUFFIX</b>
SAN DIEGO, CITY OF	060295	1617	G

Notice to User: The Map Number shown below should be used when placing map orders; the Community Number shown above should be used on insurance applications for the subject community.

**MAP NUMBER 06073C1617G**

**MAP REVISED MAY 16, 2012**

**Federal Emergency Management Agency**



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This map reflects more detailed and up-to-date stream channel configurations than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map.

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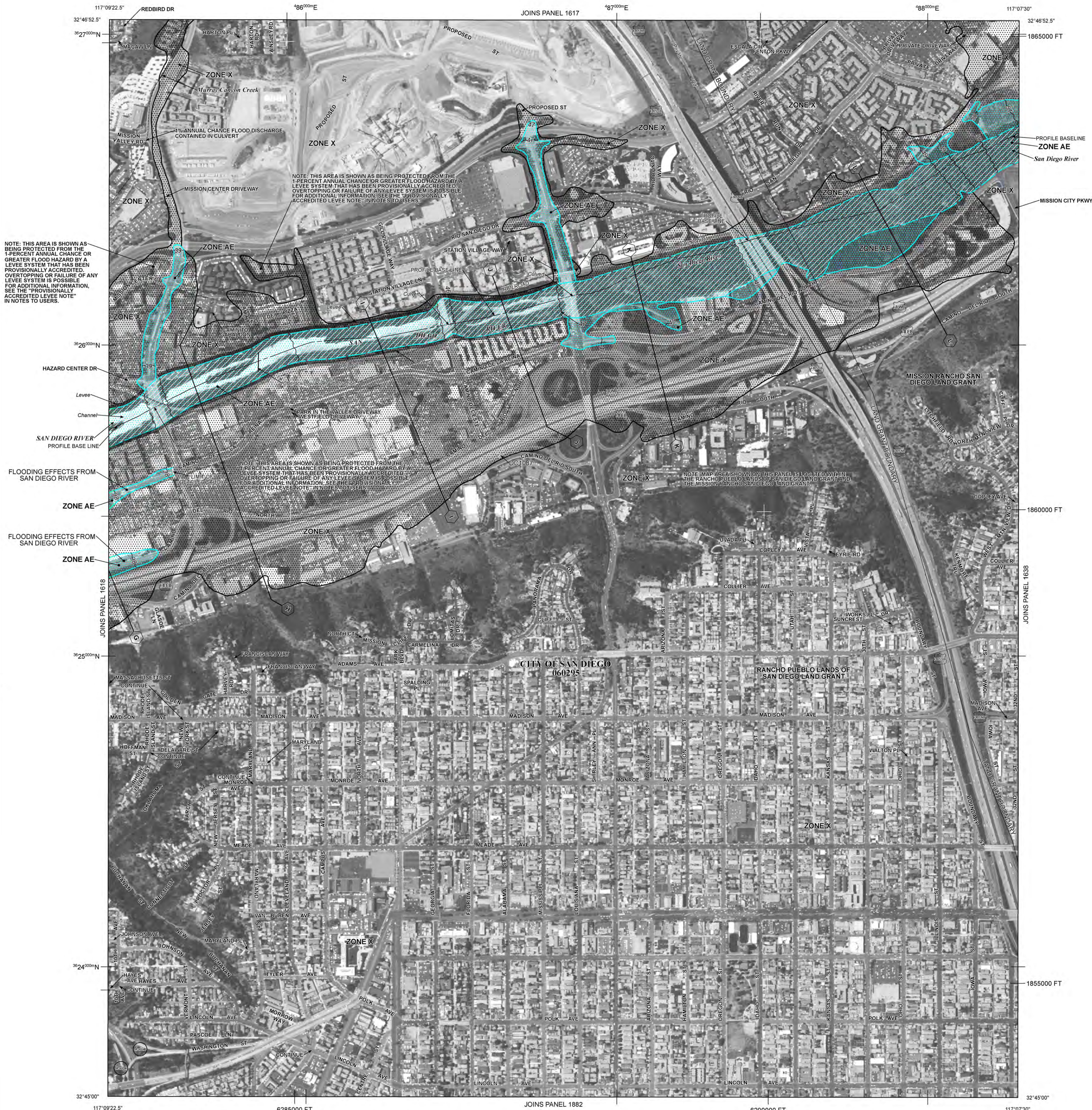
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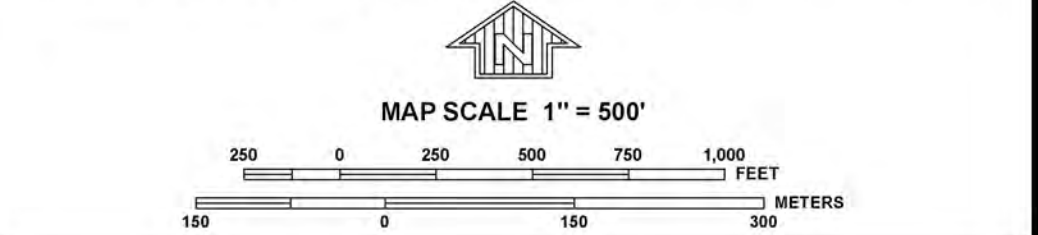
The "profile base lines" depicted on this map represent the hydraulic modeling baselines that match the flood profiles in the FIS report. As a result of improved topographic data, the "profile base line", in some cases, may deviate significantly from the channel centerline or appear outside the SFHA.

**Provisionally Accredited Levee Notes to Users:** Check with your local community to obtain more information, such as the estimated level of protection provided (which may exceed the 1-percent-annual-chance level) and Emergency Action Plan, on the levee system(s) shown as providing protection for areas on this panel. To maintain accreditation, the levee owner or community is required to submit the data and documentation necessary to comply with Section 65.10 of the NFIP regulations by May 16, 2012. If the community or owner does not provide the necessary data and documentation or if the data and documentation provided indicate the levee system does not comply with Section 65.10 requirements, FEMA will revise the flood hazard and risk information for this area to reflect de-accreditation of the levee system. To mitigate flood risk in residual risk areas, property owners and residents are encouraged to consider flood insurance and floodproofing or other protective measures. For more information on flood insurance, interested parties should visit the FEMA Website at <http://www.fema.gov/business/nfip/index.shtml>.



**LEGEND**

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- The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.
- ZONE A** No Base Flood Elevations determined.
- ZONE AE** Base Flood Elevations determined.
- ZONE AH** Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
- ZONE AO** Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
- ZONE AR** Special Flood Hazard Area formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
- ZONE A99** Areas to be protected from 1% annual chance flood event by a federal flood protection system under construction; no Base Flood Elevations determined.
- ZONE V** Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
- ZONE VE** Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.
- FLOODWAY AREAS IN ZONE AE
- The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.
- OTHER FLOOD AREAS**
- ZONE X** Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.
- OTHER AREAS**
- ZONE X** Areas determined to be outside the 0.2% annual chance floodplain.
- ZONE D** Areas in which flood hazards are undetermined, but possible.
- COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS
- OTHERWISE PROTECTED AREAS (OPAs)
- CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.
- 1% annual chance floodplain boundary
- 0.2% annual chance floodplain boundary
- Floodway boundary
- Zone D boundary
- CBRS and OPA boundary
- Boundary dividing Special Flood Hazard Area Zones and boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths, or flood velocities
- Base Flood Elevation line and value; elevation in feet\* (EL 987)
- Base Flood Elevation value where uniform within zone; elevation in feet\*
- \* Referenced to the North American Vertical Datum of 1988
- Cross section line
- Transect line
- Geographic coordinates referenced to the North American Datum of 1983 (NAD 83), Western Hemisphere
- 47°52'00"E 1000-meter Universal Transverse Mercator grid ticks, zone 11
- 6000000 FT 5000-foot grid values; California State Plane coordinate system, Zone VI (FIPSZONE = 406), Lambert projection
- DX5510 Bench mark (see explanation in Notes to Users section of this FIRM panel)
- M1.5 River Mile
- MAP REPOSITORIES**  
Refer to Map Repositories list on Map Index
- EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP**  
June 19, 1997
- EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL**  
May 16, 2012 - to update corporate limits, to add roads and road names, to incorporate previously issued Letters of Map Revision, and to update map elevations to North American Vertical Datum of 1988.



**NATIONAL FLOOD INSURANCE PROGRAM**

**PANEL 1619G**

**FIRM**

**FLOOD INSURANCE RATE MAP**

**SAN DIEGO COUNTY, CALIFORNIA**

**AND INCORPORATED AREAS**

**PANEL 1619 OF 2375**

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

**CONTAINS:**

COMMUNITY	NUMBER	PANEL	SUFFIX
SAN DIEGO, CITY OF	060295	1619	G

Notice to User: The Map Number shown below should be used when placing map orders. The Community Number shown above should be used on insurance applications for the subject community.

**MAP NUMBER**  
06073C1619G

**MAP REVISED**  
MAY 16, 2012

Federal Emergency Management Agency



**NOTES TO USERS**

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations (BFEs)** and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

**Coastal Base Flood Elevations (BFEs)** shown on this map apply only to landward of 0.0' North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was **Universal Transverse Mercator (UTM) Zone 11**. The horizontal datum was **NAD83, GRS1980 spheroid**. Differences in datum, spheroid, projection or UTM zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the **North American Vertical Datum of 1988**. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at <http://www.ngs.noaa.gov/> or contact the National Geodetic Survey at the following address:

NGS Information Services  
NOAA, NNGS12  
National Geodetic Survey  
SSM-C-3 #9202  
1315 East-West Highway  
Silver Spring, Maryland 20910-3282  
(301) 713-3242

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at <http://www.ngs.noaa.gov/>.

**Base map** information shown on this FIRM was provided in digital format by the USDA National Agriculture Imagery Program (NAIP). This information was photogrammetrically compiled at a scale of 1:24,000 from aerial photography dated 2009.

This map reflects more detailed and up-to-date **stream channel configurations** than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map.

**Corporate limits** shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels, community map repository addresses, and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

Contact the **FEMA Map Service Center** at 1-877-FEMA MAP (1-877-336-2627) for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study report, and/or digital versions of this map. The FEMA Map Service Center may also be reached by Fax at 1-800-358-9620 and its website at <http://msc.fema.gov/>.

If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA MAP (1-877-336-2627) or visit the FEMA website at <http://www.fema.gov/business/firm/>.

The "profile base lines" depicted on this map represent the hydraulic modeling baselines that match the flood profiles in the FIS report. As a result of improved topographic data, the "profile base line", in some cases, may deviate significantly from the channel centerline or appear outside the SFHA.



**LEGEND**

**SPECIAL FLOOD HAZARD AREAS SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD**

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

- ZONE A** No Base Flood Elevations determined.
- ZONE AE** Base Flood Elevations determined.
- ZONE AH** Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
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- ZONE V** Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
- ZONE VE** Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

**FLOODWAY AREAS IN ZONE AE**

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

**OTHER FLOOD AREAS**

- ZONE X** Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

**OTHER AREAS**

- ZONE X** Areas determined to be outside the 0.2% annual chance floodplain.
- ZONE D** Areas in which flood hazards are undetermined, but possible.

**COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS**

**OTHERWISE PROTECTED AREAS (OPAs)**

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

- 1% annual chance floodplain boundary
- 0.2% annual chance floodplain boundary
- Floodway boundary
- Zone D boundary
- CBRS and OPA boundary
- Boundary dividing Special Flood Hazard Area Zones and boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths, or flood velocities
- Base Flood Elevation line and value; elevation in feet\*
- Base Flood Elevation value where uniform within zone; elevation in feet\*

\* Referenced to the North American Vertical Datum of 1988

**MAP REPOSITORIES**  
Refer to Map Repositories list on Map Index

**EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP**  
June 19, 1997

**EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL**  
June 16, 1999

May 16, 2012 - to update corporate limits, to add roads and road names, to incorporate previously issued Letters of Map Revision, and to update map elevations to North American Vertical Datum of 1988.

For community map revision history prior to countywide mapping, refer to the Community Map History table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

**MAP SCALE 1" = 500'**

250 0 250 500 1,000 FEET  
150 0 150 300 METERS

**NATIONAL FLOOD INSURANCE PROGRAM**

**PANEL 1636H**

**FIRM**

**FLOOD INSURANCE RATE MAP**

**SAN DIEGO COUNTY, CALIFORNIA**

**AND INCORPORATED AREAS**

**PANEL 1636 OF 2375**

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

**CONTAINS:**

<b>COMMUNITY</b>	<b>NUMBER</b>	<b>PANEL</b>	<b>SUFFIX</b>
SAN DIEGO, CITY OF	060295	1636	H

Notice to User: The Map Number shown below should be used when placing map orders; the Community Number shown above should be used on insurance applications for the subject community.

**MAP NUMBER 06073C1636H**

**MAP REVISED MAY 16, 2012**

**Federal Emergency Management Agency**



**NOTES TO USERS**

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or floodways have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

**Coastal Base Flood Elevations (BFEs)** shown on this map apply only to landward of 0.0' North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Universal Transverse Mercator (UTM) Zone 11. The horizontal datum was NAD83, GRS1980 spheroid. Differences in datum, spheroid, projection or UTM zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at <http://www.ngs.noaa.gov/> or contact the National Geodetic Survey at the following address:

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NOAA, NNGS12  
National Geodetic Survey  
SSM-C-3 #9202  
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Silver Spring, Maryland 20910-3282  
(301) 713-3242

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**Base map** information shown on this FIRM was provided in digital format by the USDA National Agriculture Imagery Program (NAIP). This information was photogrammetrically compiled at a scale of 1:24,000 from aerial photography dated 2009.

This map reflects more detailed and up-to-date stream channel configurations than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map.

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The "profile base lines" depicted on this map represent the hydraulic modeling baselines that match the flood profiles in the FIS report. As a result of improved topographic data, the "profile base line", in some cases, may deviate significantly from the channel centerline or appear outside the SFHA.

**LEGEND**

**SPECIAL FLOOD HAZARD AREAS SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD**

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- ZONE A** No Base Flood Elevations determined.
- ZONE AE** Base Flood Elevations determined.
- ZONE AH** Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
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- ZONE VE** Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

**FLOODWAY AREAS IN ZONE AE**

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

**OTHER FLOOD AREAS**

- ZONE X** Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

**OTHER AREAS**

- ZONE X** Areas determined to be outside the 0.2% annual chance floodplain.
- ZONE D** Areas in which flood hazards are undetermined, but possible.

**COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS**

**OTHERWISE PROTECTED AREAS (OPAs)**

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- 1% annual chance floodplain boundary
- 0.2% annual chance floodplain boundary
- Floodway boundary
- Zone D boundary
- CBRS and OPA boundary
- Boundary dividing Special Flood Hazard Area Zones and boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths, or flood velocities
- Base Flood Elevation line and value; elevation in feet\*
- Base Flood Elevation value where uniform within zone; elevation in feet\*

\* Referenced to the North American Vertical Datum of 1988

**Cross section line**

Transect line

Geographic coordinates referenced to the North American Datum of 1983 (NAD 83), Western Hemisphere

1000-meter Universal Transverse Mercator grid ticks, zone 11

5000-foot grid values; California State Plane coordinate system, Zone VI (FIPSZONE = 406), Lambert projection

Bench mark (see explanation in Notes to Users section of this FIRM panel)

River Mile

**MAP REPOSITORIES**

Refer to Map Repositories list on Map Index

**EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP**

June 19, 1997

**EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL**

June 16, 1999

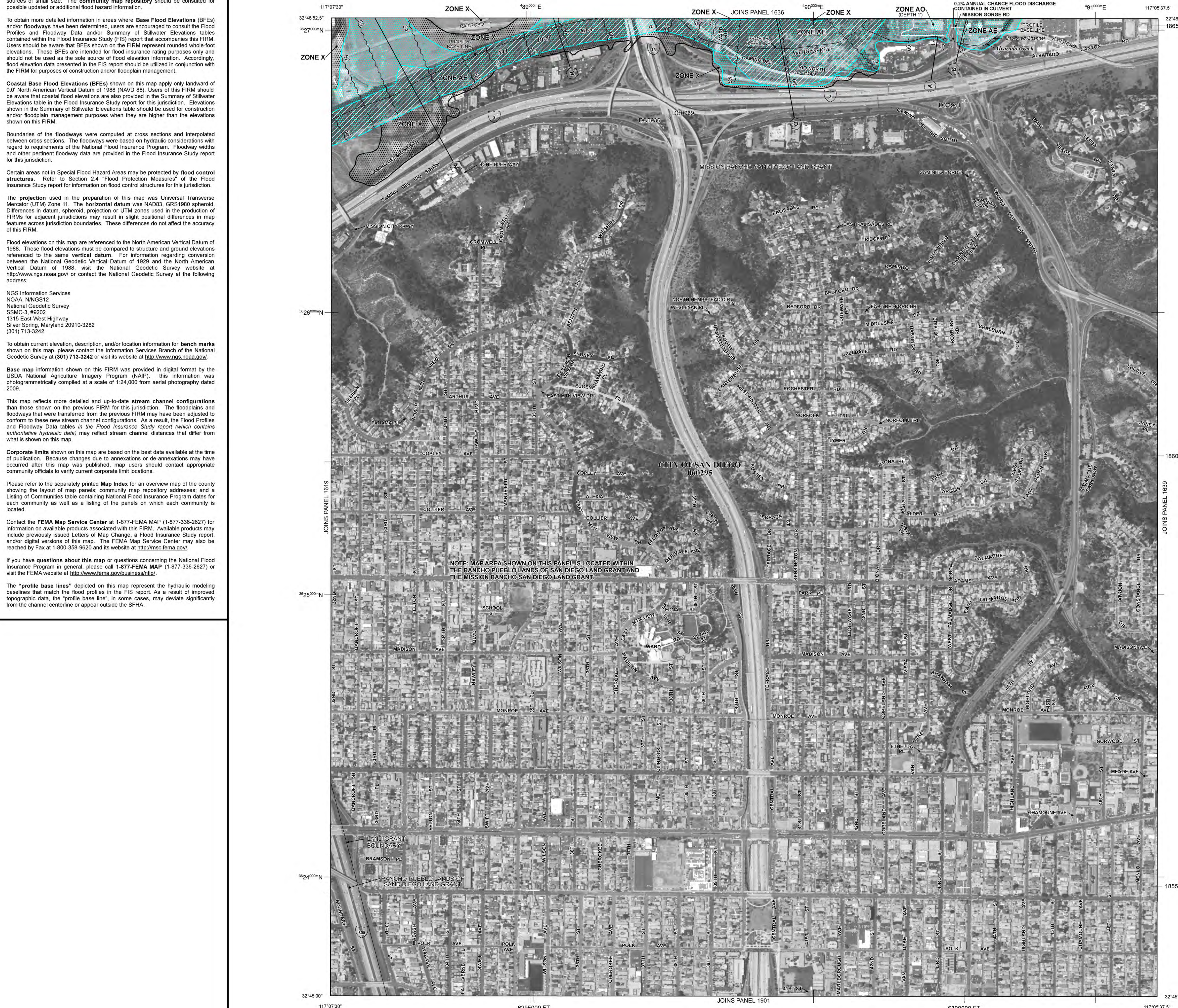
May 16, 2012 - to update corporate limits, to add roads and road names, to incorporate previously issued Letters of Map Revision, and to update map elevations to North American Vertical Datum of 1988.

For community map revision history prior to countywide mapping, refer to the Community Map History table located in the Flood Insurance Study report for this jurisdiction.

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**MAP SCALE 1" = 500'**

250 0 250 500 750 1,000 FEET  
150 0 150 300 METERS



**NATIONAL FLOOD INSURANCE PROGRAM**

**PANEL 1638H**

**FIRM**

**FLOOD INSURANCE RATE MAP**

**SAN DIEGO COUNTY, CALIFORNIA**

**AND INCORPORATED AREAS**

**PANEL 1638 OF 2375**

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

**CONTAINS:**

COMMUNITY	NUMBER	PANEL	SUFFIX
SAN DIEGO, CITY OF	060295	1638	H

Notice to User: The Map Number shown below should be used when placing map orders; the Community Number shown above should be used on insurance applications for the subject community.

**MAP NUMBER 06073C1638H**

**MAP REVISED MAY 16, 2012**

Federal Emergency Management Agency