



ASSOCIATED TRANSPORTATION ENGINEERS

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Since 1978

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June 20, 2019

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TRAFFIC ASSESSMENT FOR THE SANTA CLAUS LANE STREETScape IMPROVEMENT PROJECT, COUNTY OF SANTA BARBARA

Associated Transportation Engineer's (ATE) has prepared the following traffic assessment for the Santa Claus Lane Streetscape Improvement Project (the "Project") located near Carpinteria in the unincorporated area of Santa Barbara County.

PROJECT DESCRIPTION

The County of Santa Barbara (County) is proposing streetscape improvements along Santa Claus Lane to improve access to the beach, provide new recreational amenities to the public, increase accessibility for all users in the project area, and improve drainage and safety along Santa Claus Lane. The proposed streetscape improvements include: additional public restrooms, rinse stations, trash/recycle bins, bike racks, multi-use path, crosswalks, sidewalks, traffic calming measures, beach and business patron parking, landscaping, and a new roundabout at the intersection of Santa Claus Lane and Spindrift Lane.

BACKGROUND

The South Coast 101 HOV Lanes Project Draft Revised Environmental Impact Report (DEIR) showed that one of the study-area intersections at the US 101/South Padaro Lane interchange was forecast to operate at a poor level of service (LOS) under Year 2020 No Build conditions. County staff therefore requested an assessment of existing traffic conditions within the Project study area to determine current volumes, vehicles delays, and levels of service.

CALTRANS LEVEL OF SERVICE FORECASTS

The South Coast 101 HOV Lanes Project EIR includes level of service forecasts for the Year 2020 No Build scenario for the intersections comprising the US 101/South Padaro Lane interchange. Table 1 summarizes the PM peak hour levels of service presented in the EIR.

**Table 1
South Coast 101 HOV Lanes Project EIR
Year 2020 No Build LOS Forecasts – PM Peak Hour**

Intersection	Control Type	Highest Approach Delay & LOS(a)
<i>Via Real/Padaro Ln</i> Vehicle Delay Level of Service	TWSC	Northbound Left + Right = 17.7 Seconds LOS C
<i>US 101 NB Ramps/Padaro Ln</i> Vehicle Delay Level of Service	TWSC	Westbound Left + Right = 12.2 Seconds LOS B
<i>US 101 SB Off-Ramp/Padaro Ln/Santa Claus Ln</i> Vehicle Delay Level of Service	TWSC	Southbound Left + Thru = 53.0 Seconds LOS F

(a) LOS based on average delay per vehicle for worst approach.

As shown, the South Coast 101 HOV Lanes Project EIR forecast that the US 101 SB Off-Ramp/Padaro Lane/Santa Claus Lane intersection would operate at LOS F during the PM peak hour in the Year 2020. It is important to note that 1) the Caltrans levels of service are based on “traffic forecasts” not actual traffic counts, and 2) the Caltrans levels of service cited for each intersection are based on the highest vehicle delay on the worst approach at the intersections. The County determines levels of service for stop-sign controlled intersections based on the average vehicle delays for all movements that are required to stop or yield when using the intersections.

CURRENT TRAFFIC COUNTS AND LEVELS OF SERVICE

Traffic counts were conducted on Santa Claus Lane to determine existing Average Daily Traffic (ADT) volumes during the weekday and weekend periods. Intersection turning movement counts and delay studies were also conducted at 4 intersections that provide access to Santa Claus Lane. The traffic counts were collected in May and June of 2019 (count data attached). Figures 1 & 2 (attached) shows the existing traffic volumes for weekdays and weekends.

Existing Roadway Operations

Table 1 lists the existing traffic volumes on Santa Claus Lane for the weekday and weekend periods, along with the existing operations based on County criteria. As shown, Santa Claus Lane currently carries relatively low traffic volumes and operates at LOS A on weekdays and weekends.

**Table 1
Existing Roadway Operations**

Roadway Segment	Roadway Classification	Existing ADT	Acceptable Capacity	LOS
Santa Claus Lane e/o Padaro Lane	P-3 Roadway	2,950 (Weekday) 3,200 (Weekend)	10,990 ADT	LOS A LOS A

(b) LOS based on ADT volume using Toro Canyon Plan capacity criteria of 10,990 ADT for P-3 roadways.

Existing Intersection Operations

Table 2 lists the existing levels of service for the study-area intersections. Levels of service were calculated using the operations method contained in the Highway Capacity Manual.¹ Since levels of service for stop-sign controlled intersections are based on the average delay per vehicle, delay data was also collected at the study-area intersections to accurately calculate the levels of service (level of service worksheets and delay calculations are attached for reference). As noted, the County determines levels of service for stop-sign controlled intersections based on the average delay per vehicle for all vehicles that are required to stop and wait for a gap in the opposing traffic streams. As shown in Table 2, the study- area intersections currently operate at LOS A during the weekday PM peak hour and LOS A-B during the weekend peak hour, which meet the County’s LOS C standard.

¹ Highway Capacity Manual, Transportation Research Board, 6th Edition, 2016.

**Table 2
Existing Intersection Operations**

Intersection / Movement	Peak Hour Delay / LOS(a)	
	Weekday PM Peak	Weekend Peak
Via Real/Padaro Ln: NB Left + Right WB Left Overall Intersection:	7.8 Sec. / LOS A 8.7 Sec. / LOS A 8.2 Sec. / LOS A	11.0 Sec. / LOS B 7.7 Sec. / LOS A 10.4 Sec. / LOS B
US 101 NB/Padaro Ln: WB Left + Right NB Left Overall Intersection:	10.7 Sec. / LOS B 8.2 Sec. / LOS A 9.8 Sec. / LOS A	7.6 Sec. / LOS A 7.6 Sec. / LOS A 7.6 Sec. / LOS A
US 101 SB/Padaro Ln/Santa Claus Ln: EB Left EB Thru + Right WB Left + Right SB Left Overall Intersection:	11.9 Sec. / LOS B 11.5 Sec. / LOS B 9.1 Sec. / LOS A 7.9 Sec. / LOS A 9.1 Sec. / LOS A	8.5 Sec. / LOS A 8.2 Sec. / LOS A 9.2 Sec. / LOS A 7.5 Sec. / LOS A 8.2 Sec. / LOS A
Santa Claus Lane/Spindrift: NB Left + Right Overall Intersection:	7.6 Sec. / LOS A 7.6 Sec. / LOS A	7.6 Sec. / LOS A 7.6 Sec. / LOS A

(a) Level of service based on average delay per vehicle pursuant to HCM operations method.

PROJECT TRIP GENERATION & POTENTIAL IMPACTS

The Santa Claus Lane Streetscape Improvements Project is intended to enhance vehicle, pedestrian, and bicycle circulation along Santa Claus Lane; and provide additional parking resources for peak beach visitation days. The Project does not include construction of any new visitor-serving buildings or businesses that would attract new traffic. It is anticipated that these improvements will not result in significant increases in traffic at the US 101/Padaro Lane interchange during the typical weekday AM and PM peak hour commute periods since the existing beach parking is generally not full during these periods. Thus, adding vehicle parking and enhancing pedestrian and bike facilities would not attract new automobiles during the weekday peak periods. The improvements will improve traffic, pedestrian, and bike flows along Santa Claus Lane during weekend peak visitation periods because the additional parking spaces will reduce the frequency of vehicles circling through the area attempting to find open parking spots.

As shown in Tables 1 and 2, Santa Claus Lane and the study-area intersections currently operate at LOS A during the weekday PM peak hour and LOS A-B during the weekend peak hour, which meet the County's LOS C standard. Any minor increases in traffic that would result from the Project would not significantly degrade traffic operations in the study-area and therefore not generate significant traffic impacts. For reference, the study-area intersections would continue to operate at LOS A-B during the weekday AM/PM peak hours and weekend peak hour assuming a 25% increase in traffic to the Santa Claus Lane area.

The Santa Claus Lane Streetscape Improvement Project include a new single-lane roundabout at the Santa Claus Lane/Spindrift Lane intersection. Levels of service for this new intersection feature were calculated using the roundabout operations method contained in the Highway Capacity Manual. The results show that the new roundabout is forecast to operate at LOS A during the weekday PM peak hour and weekend peak hour.

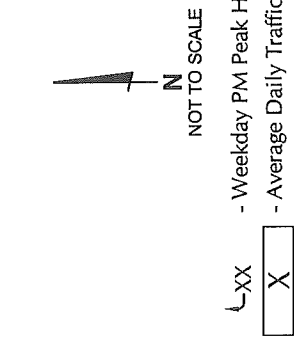
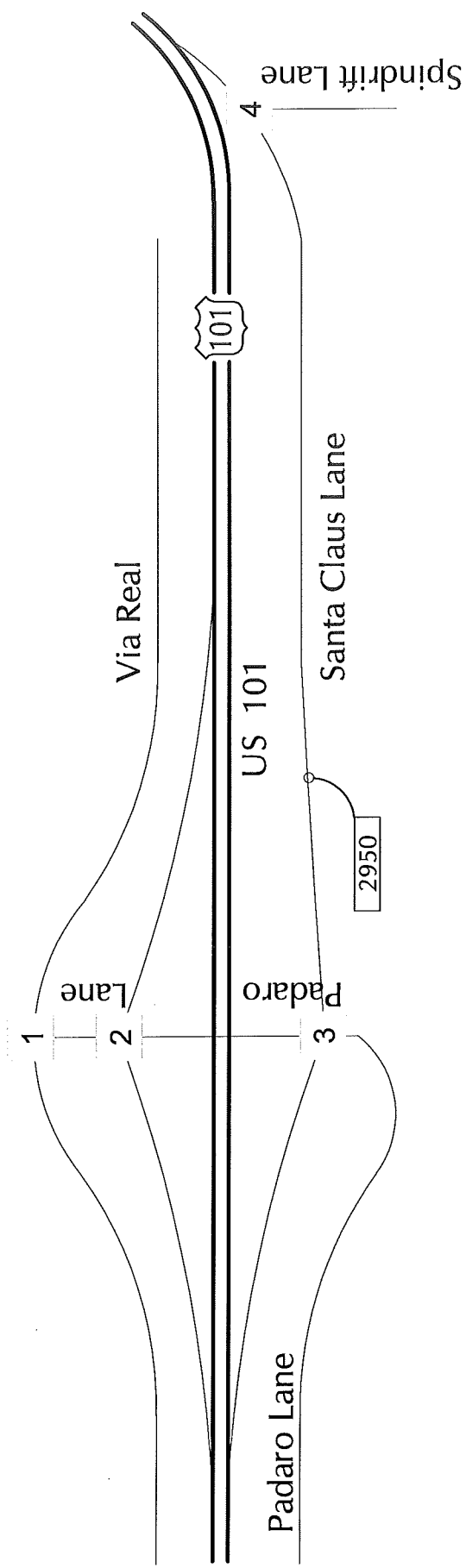
This concludes our traffic assessment for the Santa Claus Lane Streetscape Improvement Project. Thank you for your assistance during the course of the work effort.

Associated Transportation Engineers



Dan Dawson
Supervising Transportation Planner

Attachments



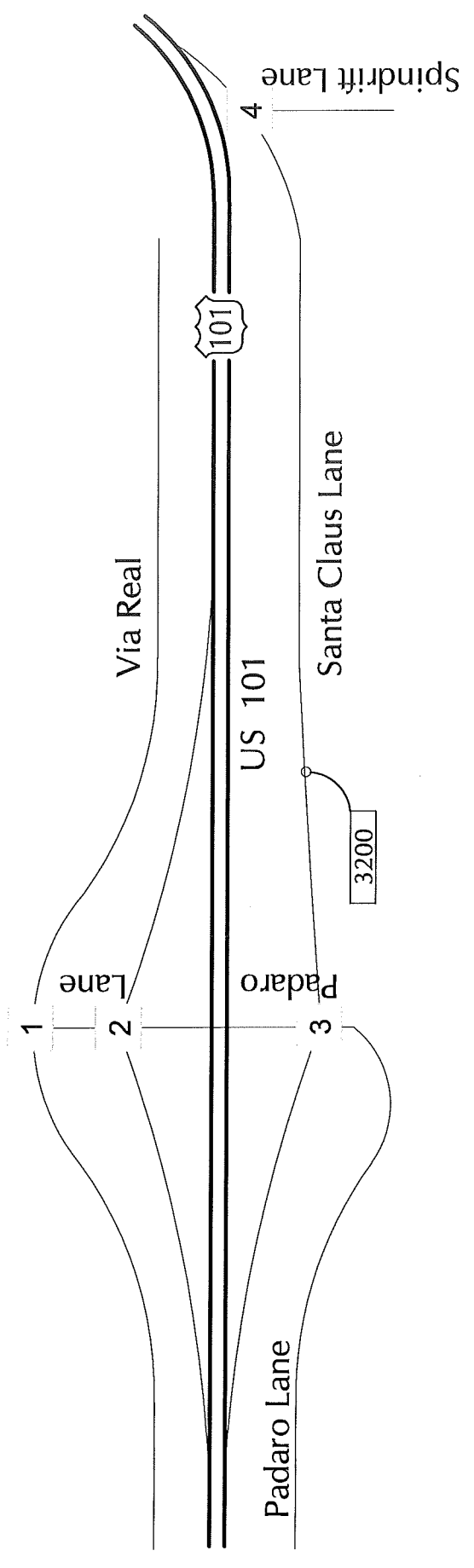
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WEEKDAY TRAFFIC VOLUMES

FIGURE 1



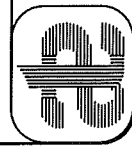
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73	77						
196	66						

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71	71								
114	49								

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158	10				
6	7				

N
 NOT TO SCALE
 ↳XX - Weekend Peak Hour Volume
 ↳X - Average Daily Traffic Volume



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WEEKEND TRAFFIC VOLUMES

VOLUME

Santa Claus Ln Bet. Padaro Ln & Spindrif Ln

Day: Thursday
Date: 5/30/2019

City: Carpinteria
Project #: CA19_2037_001

DAILY TOTALS					NB	SB	EB	WB	Total					
					0	0	2,510	438	2,948					
AM Period	NB	SB	EB	WB	TOTAL	PM Period	NB	SB	EB	WB	TOTAL			
00:00			3	0	3	12:00			63	13	76			
00:15			0	0	0	12:15			49	9	58			
00:30			0	0	0	12:30			49	11	60			
00:45			0	3	0	12:45			42	203	10	43	52	246
01:00			2	0	2	13:00			57	14	71			
01:15			2	0	2	13:15			46	17	63			
01:30			3	0	3	13:30			45	13	58			
01:45			0	7	0	13:45			34	182	12	56	46	238
02:00			1	0	1	14:00			33	9	42			
02:15			0	2	2	14:15			32	10	42			
02:30			0	0	0	14:30			41	12	53			
02:45			0	1	0	14:45			39	145	13	44	52	189
03:00			6	0	6	15:00			41	16	57			
03:15			0	0	0	15:15			54	7	61			
03:30			0	0	0	15:30			81	10	91			
03:45			2	8	1	15:45			69	245	10	43	79	288
04:00			4	0	4	16:00			108	11	119			
04:15			0	0	0	16:15			74	6	80			
04:30			0	0	0	16:30			120	8	128			
04:45			1	5	0	16:45			85	387	11	36	96	423
05:00			2	5	7	17:00			103	14	117			
05:15			3	0	3	17:15			62	7	69			
05:30			4	0	4	17:30			55	13	68			
05:45			18	27	2	17:45			53	273	7	41	60	314
06:00			12	0	12	18:00			55	2	57			
06:15			8	0	8	18:15			29	5	34			
06:30			10	1	11	18:30			31	6	37			
06:45			13	43	2	18:45			23	138	7	20	30	158
07:00			14	0	14	19:00			20	3	23			
07:15			16	3	19	19:15			20	1	21			
07:30			32	3	35	19:30			16	5	21			
07:45			31	93	4	19:45			13	69	5	14	18	83
08:00			28	8	36	20:00			14	5	19			
08:15			49	3	52	20:15			13	1	14			
08:30			30	3	33	20:30			13	0	13			
08:45			38	145	2	20:45			10	50	1	7	11	57
09:00			40	5	45	21:00			5	1	6			
09:15			39	5	44	21:15			8	2	10			
09:30			27	7	34	21:30			5	1	6			
09:45			41	147	3	21:45			3	21	0	4	3	25
10:00			24	2	26	22:00			7	0	7			
10:15			32	10	42	22:15			2	0	2			
10:30			41	11	52	22:30			2	0	2			
10:45			37	134	8	22:45			4	15	0	4	15	
11:00			37	8	45	23:00			3	0	3			
11:15			29	4	33	23:15			3	0	3			
11:30			40	10	50	23:30			0	0	0			
11:45			52	158	18	23:45			5	11	0	5	11	
TOTALS			771	130	901	TOTALS			1739	308	2047			
SPLIT %			85.6%	14.4%	30.6%	SPLIT %			85.0%	15.0%	69.4%			

DAILY TOTALS					NB	SB	EB	WB	Total		
					0	0	2,510	438	2,948		
AM Peak Hour			11:45	11:45	11:45	PM Peak Hour			16:00	13:00	16:00
AM Pk Volume			213	51	264	PM Pk Volume			387	56	423
Pk Hr Factor			0.845	0.708	0.868	Pk Hr Factor			0.806	0.824	0.826
7 - 9 Volume			238	26	264	4 - 6 Volume			660	77	737
7 - 9 Peak Hour			08:00	07:15	08:00	4 - 6 Peak Hour			16:00	16:45	16:00
7 - 9 Pk Volume			145	18	161	4 - 6 Pk Volume			387	45	423
Pk Hr Factor			0.740	0.563	0.774	Pk Hr Factor			0.806	0.804	0.826

VOLUME

Santa Claus Ln Bet. Padaro Ln & Spindrift Ln

Day: Saturday
Date: 6/8/2019

City: Carpinteria
Project #: CA19_2037_001

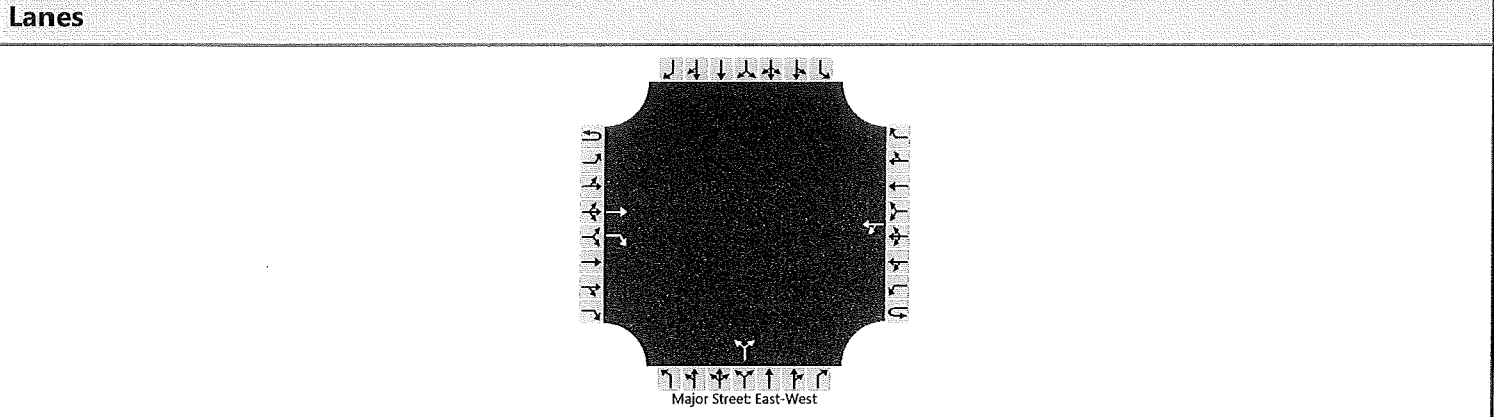
DAILY TOTALS					NB	SB	EB	WB	Total		
					0	0	2,585	583	3,168		
AM Period	NB	SB	EB	WB	TOTAL	PM Period	NB	SB	EB	WB	TOTAL
00:00			4	0	4	12:00			88	19	107
00:15			2	1	3	12:15			68	14	82
00:30			3	0	3	12:30			61	17	78
00:45			6	15	21	12:45		274	57	20	344
01:00			2	0	2	13:00			54	21	75
01:15			1	2	3	13:15			55	24	79
01:30			0	0	0	13:30			70	25	95
01:45			1	4	5	13:45		240	61	17	327
02:00			6	1	7	14:00			50	12	62
02:15			1	1	2	14:15			60	14	74
02:30			0	0	0	14:30			54	29	83
02:45			2	9	11	14:45		214	50	21	290
03:00			3	0	3	15:00			45	10	55
03:15			0	0	0	15:15			53	12	65
03:30			3	0	3	15:30			43	16	59
03:45			0	6	6	15:45		194	53	15	247
04:00			2	2	4	16:00			31	8	39
04:15			0	0	0	16:15			70	5	75
04:30			1	0	1	16:30			51	8	59
04:45			3	6	9	16:45		207	55	9	237
05:00			3	1	4	17:00			53	11	64
05:15			3	0	3	17:15			62	8	70
05:30			4	0	4	17:30			80	8	88
05:45			11	21	32	17:45		260	65	8	295
06:00			5	3	8	18:00			46	4	50
06:15			5	2	7	18:15			52	0	52
06:30			12	3	15	18:30			56	8	64
06:45			4	26	30	18:45		197	43	8	217
07:00			9	1	10	19:00			29	8	37
07:15			15	2	17	19:15			37	14	51
07:30			10	3	13	19:30			22	11	33
07:45			14	48	62	19:45		111	23	12	156
08:00			24	3	27	20:00			18	8	26
08:15			10	3	13	20:15			9	5	14
08:30			23	5	28	20:30			12	4	16
08:45			33	90	123	20:45		46	7	8	71
09:00			25	2	27	21:00			15	6	21
09:15			24	2	26	21:15			14	4	18
09:30			31	2	33	21:30			11	2	13
09:45			38	118	156	21:45		43	3	1	56
10:00			32	6	38	22:00			10	1	11
10:15			35	8	43	22:15			9	2	11
10:30			43	6	49	22:30			7	1	8
10:45			52	162	214	22:45		34	8	5	43
11:00			54	5	59	23:00			6	5	11
11:15			56	2	58	23:15			10	1	11
11:30			65	11	76	23:30			1	2	3
11:45			66	241	307	23:45		19	2	11	30
TOTALS			746	109	855	TOTALS		1839	474		2313
SPLIT %			87.3%	12.7%	27.0%	SPLIT %		79.5%	20.5%		73.0%

DAILY TOTALS					NB	SB	EB	WB	Total
					0	0	2,585	583	3,168

AM Peak Hour	11:30	11:45	11:45	PM Peak Hour	12:00	12:45	12:00
AM Pk Volume	287	62	345	PM Pk Volume	274	90	344
Pk Hr Factor	0.815	0.816	0.806	Pk Hr Factor	0.778	0.900	0.804
7 - 9 Volume	138	22	160	4 - 6 Volume	467	65	532
7 - 9 Peak Hour	08:00	07:45	08:00	4 - 6 Peak Hour	17:00	16:30	17:00
7 - 9 Pk Volume	90	14	103	4 - 6 Pk Volume	260	36	295
Pk Hr Factor	0.682	0.700	0.736	Pk Hr Factor	0.813	0.818	0.838

HCS7 Two-Way Stop-Control Report

General Information				Site Information			
Analyst	KAB	Intersection	VIA REAL/PADARO LANE				
Agency/Co.	ATE	Jurisdiction	CARPINTERIA				
Date Performed	6/20/2019	East/West Street	VIA REAL				
Analysis Year		North/South Street	PADARO LANE				
Time Analyzed	WEEKDAY PM PEAK HOUR	Peak Hour Factor	1.00				
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25				
Project Description	EXISTING CONDITIONS						



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	1	0		0	1	0		0	0	0
Configuration			T	R			LT					LR				
Volume (veh/h)			188	236			156	103			61		142			
Percent Heavy Vehicles (%)							3				3		3			
Proportion Time Blocked																
Percent Grade (%)	0															
Right Turn Channelized	No															
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)						4.1					5.0		5.0			
Critical Headway (sec)						4.13					4.90		4.90			
Base Follow-Up Headway (sec)						2.2					2.9		2.9			
Follow-Up Headway (sec)						2.23					2.93		2.93			

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)						156					203					
Capacity, c (veh/h)						1130					1468					
v/c Ratio						0.14					0.14					
95% Queue Length, Q ₉₅ (veh)						0.5					0.5					
Control Delay (s/veh)						8.7					7.8					
Level of Service (LOS)						A					A					
Approach Delay (s/veh)					5.7				7.8							
Approach LOS					A				A							

AWD = 8.2 = LOS A

National Data & Surveying Services

Intersection Turning Movement Count

Location: Via Real & Padaro Ln
 City: Carpinteria
 Control: 1-Way Stop (EB)

Project ID: 19-02036-001
 Date: 2019-05-30

Total

NS/EW Streets:	Via Real				Via Real				Padaro Ln				Padaro Ln				TOTAL				
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND								
PM	0	1	0	0	0	1	1	0	0	1	0	0	0	0	0	0	0	0	0	0	
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU					
4:00 PM	37	29	0	0	0	48	78	0	23	0	36	1	0	0	0	0					252
4:15 PM	30	29	0	0	0	37	37	0	13	0	33	2	0	0	0	0					181
4:30 PM	73	26	0	0	0	48	63	0	8	0	35	1	0	0	0	0					254
4:45 PM	16	19	0	0	0	55	58	0	13	0	38	0	0	0	0	0					199
5:00 PM	20	21	0	0	0	73	74	0	9	0	30	0	0	0	0	0					227
5:15 PM	25	18	0	0	0	52	54	0	7	0	26	0	0	0	0	0					182
5:30 PM	20	16	0	0	0	56	35	0	7	0	30	1	0	0	0	0					165
5:45 PM	17	16	0	0	0	53	29	0	9	0	25	0	0	0	0	0					149
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU					TOTAL
APPROACH %'s :	238	174	0	0	0	422	428	0	89	0	253	5	0	0	0	0					1609
	57.77%	42.23%	0.00%	0.00%	0.00%	49.65%	50.35%	0.00%	25.65%	0.00%	72.91%	1.44%									
PEAK HR :	04:00 PM - 05:00 PM				04:00 PM	289	289	296	04:30 PM												TOTAL
PEAK HR VOL :	156	103	0	0	0	188	236	0	57	0	142	4	0	0	0	0					886
PEAK HR FACTOR :	0.534	0.888	0.000	0.000	0.000	0.855	0.756	0.000	0.620	0.000	0.934	0.500	0.000	0.000	0.000	0.000					0.872
	0.654				0.841				0.846												

INTERSECTION DELAY WORKSHEET

Carpinteria

VIA REAL/PADERO

PM PEAK HOUR: 4:00 - 5:00

THURSDAY, 5/30/19

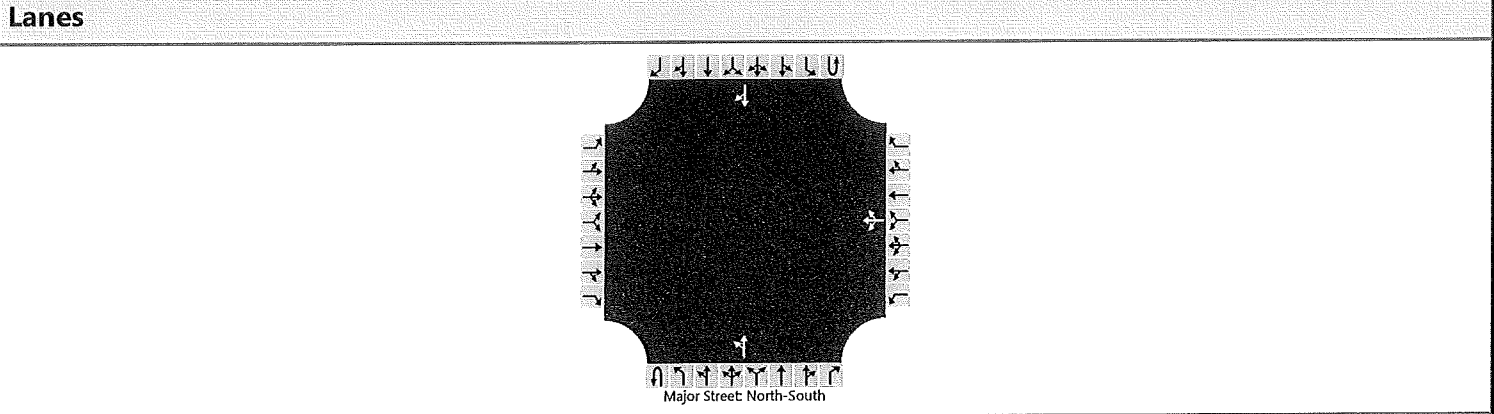
15 SECOND INTERVALS

APPROACH: NORTHBOUND PADERO LANE

Time Ending	Vehicles In Queue															Total Approach							
4:05 PM	0	0	0	0	0	0	0	0	0	1	3	3	2	0	0	0	0	0	0	0	0	0	60
4:10 PM	0	0	0	0	0	0	0	0	1	0	0	2	0	0	0	1	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	1	60
4:20 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
4:25 PM	1	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	48
4:35 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	1	0
4:40 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	1	0	0	0	1	0	0	44
4:50 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:55 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	1	0	0	0	0	0	0	2	0	0	2	0	0	0	1	0	0	51
Subtotal:	1	0	0	0	1	1	0	2	3	4	8	4	2	1	3	3	0	0	1	4	203		
Total Vehicles In Queue:															38								
<p>Total Delay = 38 vehicles x 15 seconds = 570 seconds</p> <p>Average Delay Per Vehicle = 570 seconds / 203 vehicles</p> <p>2.8 Stop delay (seconds per vehicle) + 5 seconds control = 7.8</p>																							

HCS7 Two-Way Stop-Control Report

General Information				Site Information			
Analyst	KAB			Intersection	US 101 NB/PADARO LANE		
Agency/Co.	ATE			Jurisdiction	CARPINTERIA		
Date Performed	6/20/2019			East/West Street	US101 NB RAMP		
Analysis Year				North/South Street	PADARO LANE		
Time Analyzed	WEEKDAY PM PEAK HOUR			Peak Hour Factor	1.00		
Intersection Orientation	North-South			Analysis Time Period (hrs)	0.25		
Project Description	EXISTING CONDITIONS						



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement																
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LTR				LT					TR
Volume (veh/h)						19	4	57		49	114				329	67
Percent Heavy Vehicles (%)						3	3	3		3						
Proportion Time Blocked																
Percent Grade (%)	0															
Right Turn Channelized																
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)						6.5	6.5	6.2		4.1						
Critical Headway (sec)						6.53	6.53	6.23		4.13						
Base Follow-Up Headway (sec)						3.5	4.0	3.3		2.2						
Follow-Up Headway (sec)						3.53	4.03	3.33		2.23						

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)						80				49						
Capacity, c (veh/h)						708				1157						
v/c Ratio						0.11				0.04						
95% Queue Length, Q ₉₅ (veh)						0.4				0.1						
Control Delay (s/veh)						10.7				8.2						
Level of Service (LOS)						B				A						
Approach Delay (s/veh)					10.7				2.7							
Approach LOS					B											

AND = 9.8 = LOS A

National Data & Surveying Services

Intersection Turning Movement Count

Location: US 101 NB Ramp & Padaro Ln
 City: Carpinteria
 Control: 1-Way Stop (NB)

Project ID: 19-02036-002
 Date: 2019-05-30

Total

NS/EW Streets:	US 101 NB Ramp				US 101 NB Ramp				Padaro Ln				Padaro Ln				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
PM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
4:00 PM	4	1	24	0	0	0	0	0	12	36	0	0	0	98	18	0	193
4:15 PM	8	2	10	0	0	0	0	0	7	35	0	0	0	53	17	0	132
4:30 PM	4	1	10	0	0	0	0	0	7	36	0	0	0	110	27	0	195
4:45 PM	3	0	13	0	0	0	0	0	6	37	0	0	0	68	5	0	132
5:00 PM	5	1	9	0	0	0	0	0	11	29	0	0	0	80	13	0	148
5:15 PM	4	0	7	0	0	0	0	0	8	26	0	0	0	60	19	0	124
5:30 PM	5	0	7	0	0	0	0	0	14	32	0	0	0	42	14	0	114
5:45 PM	4	1	7	0	0	0	0	0	7	26	0	0	0	37	10	0	92
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
APPROACH %'s :	28.46%	4.62%	66.92%	0.00%	0.00%	0.00%	0.00%	0.00%	21.88%	78.12%	0.00%	0.00%	0.00%	81.67%	18.33%	0.00%	1130
PEAK HR :	04:00 PM - 05:00 PM				04:00 PM				04:30 PM								TOTAL
PEAK HR VOL :	19	4	57	0	0	0	0	0	32	144	0	0	0	329	67	0	652
PEAK HR FACTOR :	0.594	0.500	0.594	0.000	0.000	0.000	0.000	0.000	0.667	0.973	0.000	0.000	0.000	0.748	0.620	0.000	0.836
	0.690								0.917				0.723				

INTERSECTION DELAY WORKSHEET
 Carpinteria
 US 101 NB RAMPS/PADARO LANE
 PM PEAK HOUR: 4:00 - 5:00
 THURSDAY, 5/30/19
 15 SECOND INTERVALS
 APPROACH: US 101 NB OFF-RAMP

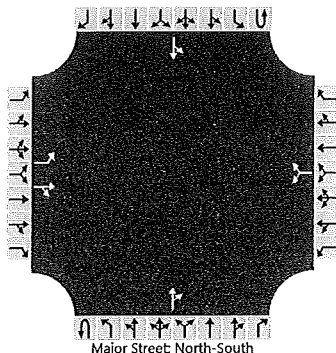
Time Ending	Vehicles in Queue														Total Approach					
4:05 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0
4:10 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0
4:20 PM	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
4:25 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	11
4:35 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0
4:40 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	2	0
4:50 PM	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
4:55 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	12
Subtotal:	0	0	1	3	1	1	2	1	2	0	0	0	0	1	1	1	0	0	4	1
Total Vehicles In Queue:	19																			

Total Delay = 19 vehicles x 15 seconds = 285 seconds
 Average Delay Per Vehicle = 285 seconds / 50 vehicles
 5.7 Stop delay (seconds per vehicle) + 5 seconds control = 10.7

HCS7 Two-Way Stop-Control Report

General Information				Site Information			
Analyst	KAB			Intersection	US101SB/PADARO LANE-SANTA		
Agency/Co.	ATE			Jurisdiction	CARPINTERIA		
Date Performed	6/20/2019			East/West Street	US101 NB RAMP		
Analysis Year				North/South Street	PADARO LANE		
Time Analyzed	WEEKDAY PM PEAK HOUR			Peak Hour Factor	1.00		
Intersection Orientation	North-South			Analysis Time Period (hrs)	0.25		
Project Description	EXISTING CONDITIONS						

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound				
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		1	1	0		0	1	0	0	0	1	0	0	0	1	0	
Configuration		L		TR			LR					TR		LT			
Volume (veh/h)		123	30	0		2		38			16	23		344	6		
Percent Heavy Vehicles (%)		3	3	3		3		3						3			
Proportion Time Blocked																	
Percent Grade (%)		0				0											
Right Turn Channelized																	
Median Type Storage		Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)		4.8	5.0	5.5		7.1		6.2							4.1	
Critical Headway (sec)		4.83	5.03	5.53		7.13		6.23							4.13	
Base Follow-Up Headway (sec)		2.0	2.1	2.1		3.5		3.3							2.2	
Follow-Up Headway (sec)		2.03	2.13	2.13		3.53		3.33							2.23	

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		123		30		40								344			
Capacity, c (veh/h)		643		584		912								1565			
v/c Ratio		0.19		0.05		0.04								0.22			
95% Queue Length, Q ₉₅ (veh)		0.7		0.2		0.1								0.8			
Control Delay (s/veh)		11.9		11.5		9.1								7.9			
Level of Service (LOS)		B		B		A								A			
Approach Delay (s/veh)		11.8				9.1								7.8			
Approach LOS		B				A											

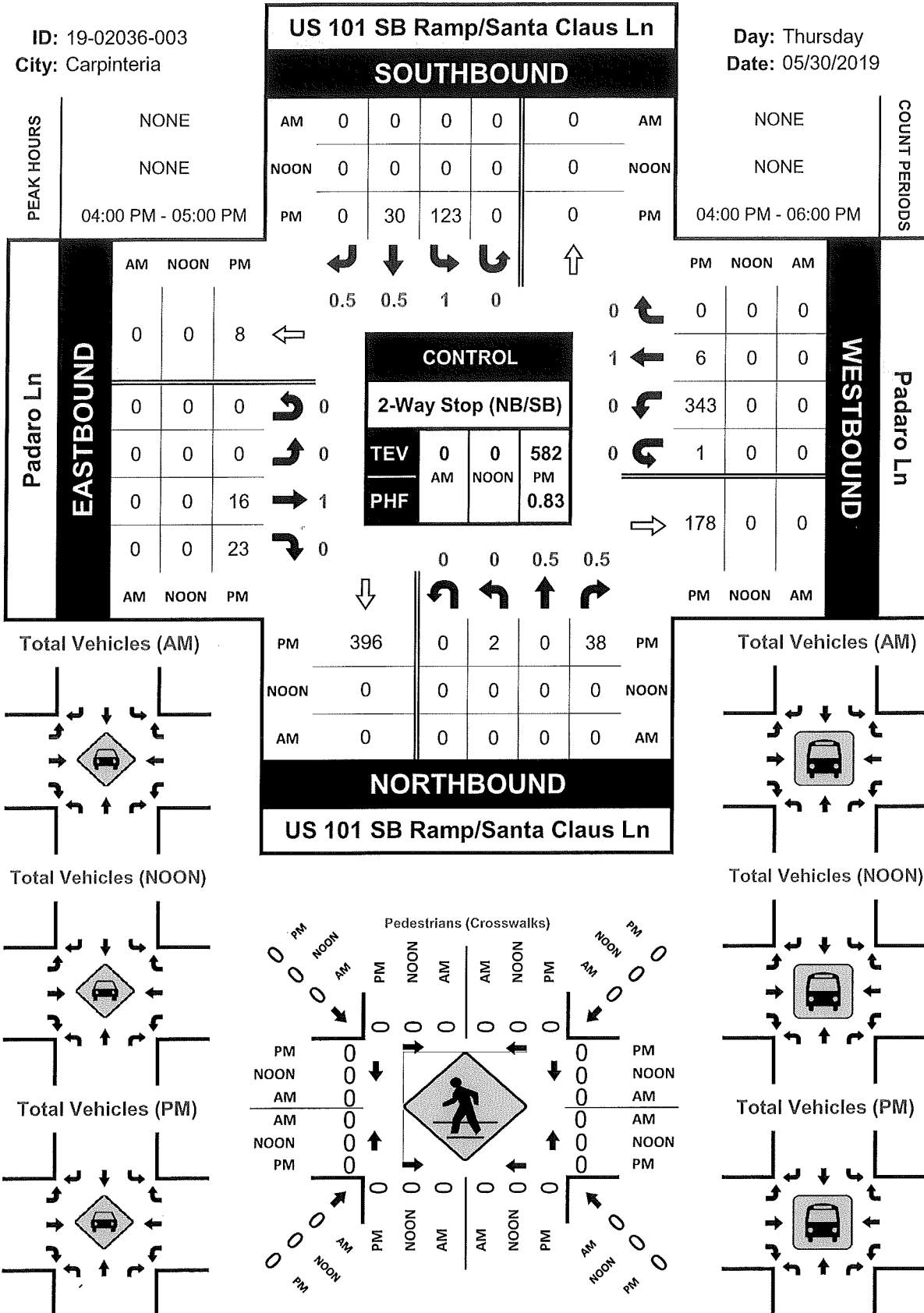
AWD = 9.1 = LOS A

US 101 SB Ramp/Santa Claus Ln & Padaro Ln

Peak Hour Turning Movement Count

ID: 19-02036-003
City: Carpinteria

Day: Thursday
Date: 05/30/2019



National Data & Surveying Services

Intersection Turning Movement Count

Location: US 101 SB Ramp/Santa Claus Ln & Padaro Ln
 City: Carpinteria
 Control: 2-Way Stop (NB/SB)

Project ID: 19-02036-003
 Date: 2019-05-30

Total

NS/EW Streets:	US 101 SB Ramp/Santa Claus Ln				US 101 SB Ramp/Santa Claus Ln				Padaro Ln				Padaro Ln				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
PM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
4:00 PM	0	0	12	0	32	8	0	0	0	5	6	0	99	2	0	0	164
4:15 PM	0	0	8	0	31	3	0	0	0	2	9	0	60	2	0	0	115
4:30 PM	1	0	8	0	31	10	0	0	0	5	5	0	115	0	0	1	176
4:45 PM	1	0	10	0	29	9	0	0	0	4	3	0	69	2	0	0	127
5:00 PM	1	0	17	0	21	12	1	0	0	2	3	0	84	1	0	0	142
5:15 PM	1	0	10	0	21	5	0	0	0	2	3	0	64	2	0	0	108
5:30 PM	0	0	14	0	32	7	1	0	0	1	2	0	43	3	0	0	103
5:45 PM	0	0	8	0	23	12	1	0	0	1	3	0	39	2	0	0	89
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
APPROACH %'s :	4	0	87	0	220	66	3	0	0	22	34	0	573	14	0	1	1024
	4.40%	0.00%	95.60%	0.00%	76.12%	22.84%	1.04%	0.00%	0.00%	39.29%	60.71%	0.00%	97.45%	2.38%	0.00%	0.17%	
PEAK HR :	04:00 PM - 05:00 PM				04:00 PM	289	289	296	04:30 PM								TOTAL
PEAK HR VOL :	2	0	38	0	123	30	0	0	0	16	23	0	343	6	0	1	582
PEAK HR FACTOR :	0.500	0.000	0.792	0.000	0.961	0.750	0.000	0.000	0.000	0.800	0.639	0.000	0.746	0.750	0.000	0.250	0.827
	0.833				0.933				0.886				0.754				

INTERSECTION DELAY WORKSHEET

Carpinteria

US 101 SB/PADARO LANE

PM PEAK HOUR: 4:00 - 5:00

THURSDAY, 5/30/19

15 SECOND INTERVALS

APPROACH: US 101 SB OFF RAMP LEFTS

Time Ending	Vehicles in Queue																Total Approach					
4:05 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	
4:10 PM	1	0	0	0	0	0	0	0	0	1	1	2	0	0	0	0	0	0	0	0	0	
4:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	0	32
4:20 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	1
4:25 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0
4:30 PM	1	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	1	0	31
4:35 PM	1	0	1	0	0	0	0	0	0	1	0	1	1	1	1	0	0	0	0	0	0	0
4:40 PM	0	1	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	1	0	0	0	0	0	1	0	0	1	1	1	0	0	0	31
4:50 PM	0	0	0	1	2	2	0	0	0	0	1	0	0	1	0	1	2	0	0	0	0	0
4:55 PM	0	0	0	0	1	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	1	0	0	0	0	29
Subtotal:	3	2	2	1	3	2	1	3	1	3	3	3	4	3	0	3	5	3	2	1	123	
Total Vehicles in Queue:																					48	

Total Delay = 48 vehicles x 15 seconds = 720 seconds

Average Delay Per Vehicle = 720 seconds / 123 vehicles

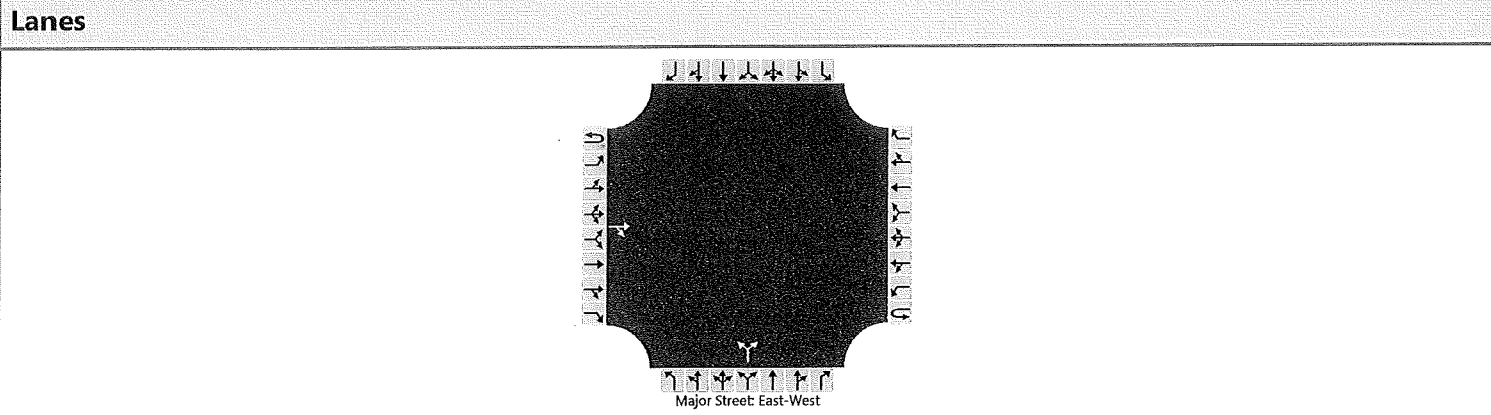
5.9 seconds per vehicle + 5 SEC CONTROL = 11.9

INTERSECTION DELAY WORKSHEET
 Carpinteria
 US 101 SB/PADARO LANE
 PM PEAK HOUR: 4:00 - 5:00
 THURSDAY, 5/30/19
 15 SECOND INTERVALS
 APPROACH: US 101 SB OFF RAMP THRU+RIGHT

Time Ending	Vehicles in Queue														Total Approach						
4:05 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
4:10 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8
4:20 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:25 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3
4:35 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
4:40 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	1
4:45 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	0	0	10
4:50 PM	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:55 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	9
Subtotal:	0	0	0	0	0	2	2	0	0	1	1	0	0	1	0	1	2	0	0	3	30
Total Vehicles in Queue:																					13
Total Delay =	13	vehicles x 15 seconds =	195	seconds																	
Average Delay Per Vehicle =	195	seconds /	30	vehicles																	
				6.5	seconds per vehicle	+	5	SEC CONTROL	=	11.5											

HCS7 Two-Way Stop-Control Report

General Information				Site Information			
Analyst	KAB	Intersection	US101SB/PADARO LANE-SANTA				
Agency/Co.	ATE	Jurisdiction	CARPINTERIA				
Date Performed	6/20/2019	East/West Street	US101 NB RAMP				
Analysis Year		North/South Street	PADARO LANE				
Time Analyzed	WEEKDAY PM PEAK HOUR	Peak Hour Factor	1.00				
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25				
Project Description	EXISTING CONDITIONS						



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	0	0	0	0	0		0	1	0		0	0	0
Configuration				TR							LR					
Volume (veh/h)			360	7						8		3				
Percent Heavy Vehicles (%)										3		3				
Proportion Time Blocked																
Percent Grade (%)										0						
Right Turn Channelized																
Median Type Storage	Undivided															

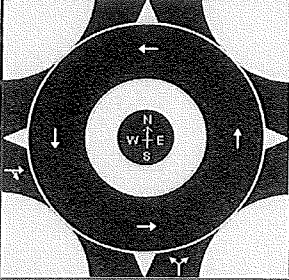
Critical and Follow-up Headways

Base Critical Headway (sec)										7.1		6.2				
Critical Headway (sec)										6.43		6.23				
Base Follow-Up Headway (sec)										3.5		3.3				
Follow-Up Headway (sec)										3.53		3.33				

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)										11						
Capacity, c (veh/h)										646						
v/c Ratio										0.02						
95% Queue Length, Q ₉₅ (veh)										0.1						
Control Delay (s/veh)										10.7						
Level of Service (LOS)										B						
Approach Delay (s/veh)	10.7															
Approach LOS	B															

HCS7 Roundabouts Report

General Information				Site Information				
Analyst	DLD				Intersection		SANTA CLAUS/SPINDRIFT	
Agency or Co.	ATE				E/W Street Name		SANTA CLAUS	
Date Performed	6/20/2019				N/S Street Name		SPINDRIFT	
Analysis Year					Analysis Time Period (hrs)		0.25	
Time Analyzed	PM PEAK HOUR				Peak Hour Factor		1.00	
Project Description	EXISTING WEEKDAY PM PEAK				Jurisdiction		SB COUNTY	

Volume Adjustments and Site Characteristics																
Approach	EB				WB				NB				SB			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement																
Number of Lanes (N)	0	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0
Lane Assignment	TR								LR							
Volume (V), veh/h	0		360	7					0	8		3				
Percent Heavy Vehicles, %	3		3	3					3	3		3				
Flow Rate (v _{PCE}), pc/h	0		371	7					0	8		3				
Right-Turn Bypass	None				None				None				None			
Conflicting Lanes	1								1							
Pedestrians Crossing, p/h	0								0							

Critical and Follow-Up Headway Adjustment												
Approach	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Critical Headway (s)		4.9763						4.9763				
Follow-Up Headway (s)		2.6087						2.6087				

Flow Computations, Capacity and v/c Ratios												
Approach	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Entry Flow (v _e), pc/h		378						11				
Entry Volume, veh/h		367						11				
Circulating Flow (v _c), pc/h		0			8			371			8	
Exiting Flow (v _{ex}), pc/h		374			8			0			7	
Capacity (C _{PCE}), pc/h		1380						945				
Capacity (c), veh/h		1340						918				
v/c Ratio (x)		0.27						0.01				

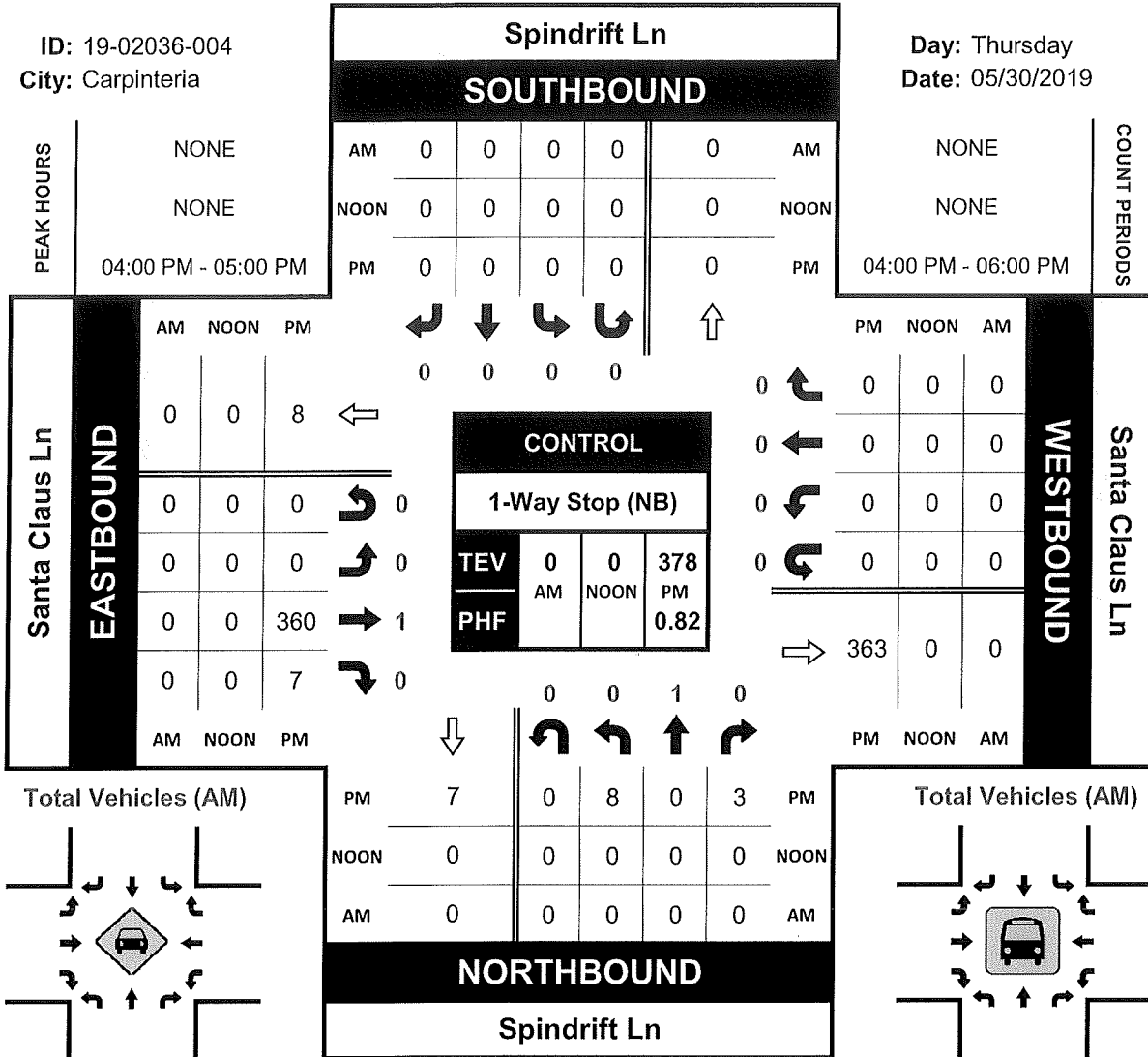
Delay and Level of Service												
Approach	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Lane Control Delay (d), s/veh		5.1						4.0				
Lane LOS		A						A				
95% Queue, veh		1.1						0.0				
Approach Delay, s/veh	5.1						4.0					
Approach LOS	A						A					
Intersection Delay, s/veh LOS	5.0						A					

Spindrift Ln & Santa Claus Ln

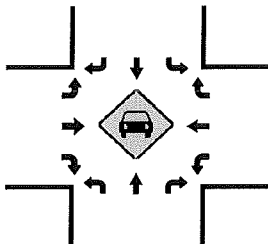
Peak Hour Turning Movement Count

ID: 19-02036-004
City: Carpinteria

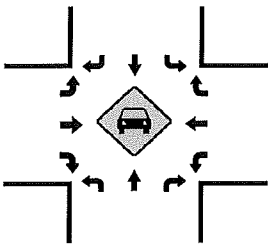
Day: Thursday
Date: 05/30/2019



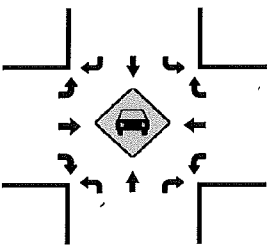
Total Vehicles (AM)



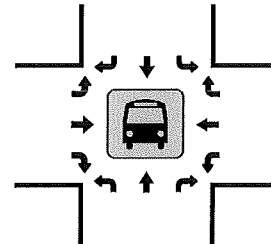
Total Vehicles (NOON)



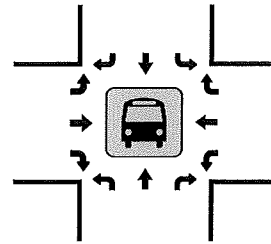
Total Vehicles (PM)



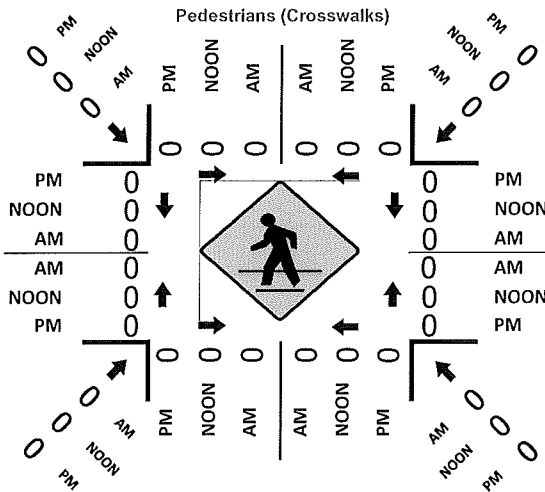
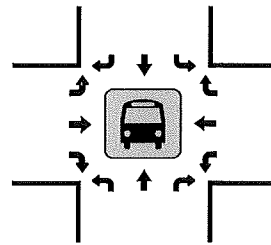
Total Vehicles (AM)



Total Vehicles (NOON)



Total Vehicles (PM)



National Data & Surveying Services

Intersection Turning Movement Count

Location: Spindrift Ln & Santa Claus Ln
 City: Carpinteria
 Control: 1-Way Stop (NB)

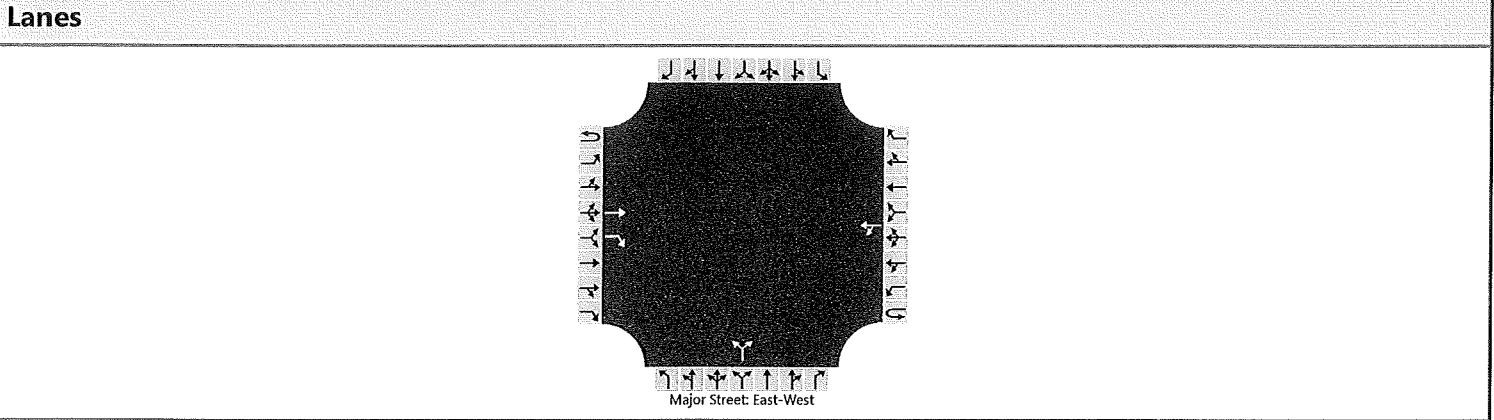
Project ID: 19-02036-004
 Date: 2019-05-30

Total

NS/EW Streets:	Spindrift Ln				Spindrift Ln				Santa Claus Ln				Santa Claus Ln				TOTAL			
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND							
PM	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU				
4:00 PM	4	0	1	0	0	0	0	0	0	108	2	0	0	0	0	0				
4:15 PM	1	0	0	0	0	0	0	0	0	60	2	0	0	0	0	0				
4:30 PM	2	0	0	0	0	0	0	0	0	111	1	0	0	0	0	0				
4:45 PM	1	0	2	0	0	0	0	0	0	81	2	0	0	0	0	0				
5:00 PM	1	0	2	0	0	0	0	0	0	91	2	0	0	0	0	0				
5:15 PM	1	0	0	0	0	0	0	0	0	73	0	0	0	0	0	0				
5:30 PM	1	0	1	0	0	0	0	0	0	52	1	0	0	0	0	0				
5:45 PM	3	0	0	0	0	0	0	0	0	43	2	0	0	0	0	0				
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL			
APPROACH %'s :	14	0	6	0	0	0	0	0	0	619	12	0	0	0	0	0	651			
	70.00%	0.00%	30.00%	0.00%					0.00%	98.10%	1.90%	0.00%								
PEAK HR :	04:00 PM - 05:00 PM				04:00 PM	289	289	296	04:00 PM											
PEAK HR VOL :	8	0	3	0	0	0	0	0	0	360	7	0	0	0	0	0	378			
PEAK HR FACTOR :	0.500	0.000	0.375	0.000	0.000	0.000	0.000	0.000	0.000	0.811	0.875	0.000	0.000	0.000	0.000	0.000	0.822			
	0.550								0.819											

HCS7 Two-Way Stop-Control Report

General Information				Site Information			
Analyst	KAB	Intersection	VIA REAL/PADARO LANE				
Agency/Co.	ATE	Jurisdiction	CARPINTERIA				
Date Performed	6/20/2019	East/West Street	VIA REAL				
Analysis Year		North/South Street	PADARO LANE				
Time Analyzed	WEEKEND PM PEAK HOUR	Peak Hour Factor	1.00				
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25				
Project Description	EXISTING CONDITIONS						



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	1	0	0	1	0	0	1	0		0	0	0	
Configuration			T	R			LT				LR					
Volume (veh/h)			73	77			70	197			198					93
Percent Heavy Vehicles (%)							3				3					3
Proportion Time Blocked																
Percent Grade (%)									0							
Right Turn Channelized	No															
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)						4.1					6.5			6.2		
Critical Headway (sec)						4.13					5.83			6.23		
Base Follow-Up Headway (sec)						2.2					3.5			3.3		
Follow-Up Headway (sec)						2.23					3.53			3.33		

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)						70						291				
Capacity, c (veh/h)						1425						886				
v/c Ratio						0.05						0.33				
95% Queue Length, Q ₉₅ (veh)						0.2						1.4				
Control Delay (s/veh)						7.7						11.0				
Level of Service (LOS)						A						B				
Approach Delay (s/veh)					2.3				11.0							
Approach LOS									B							

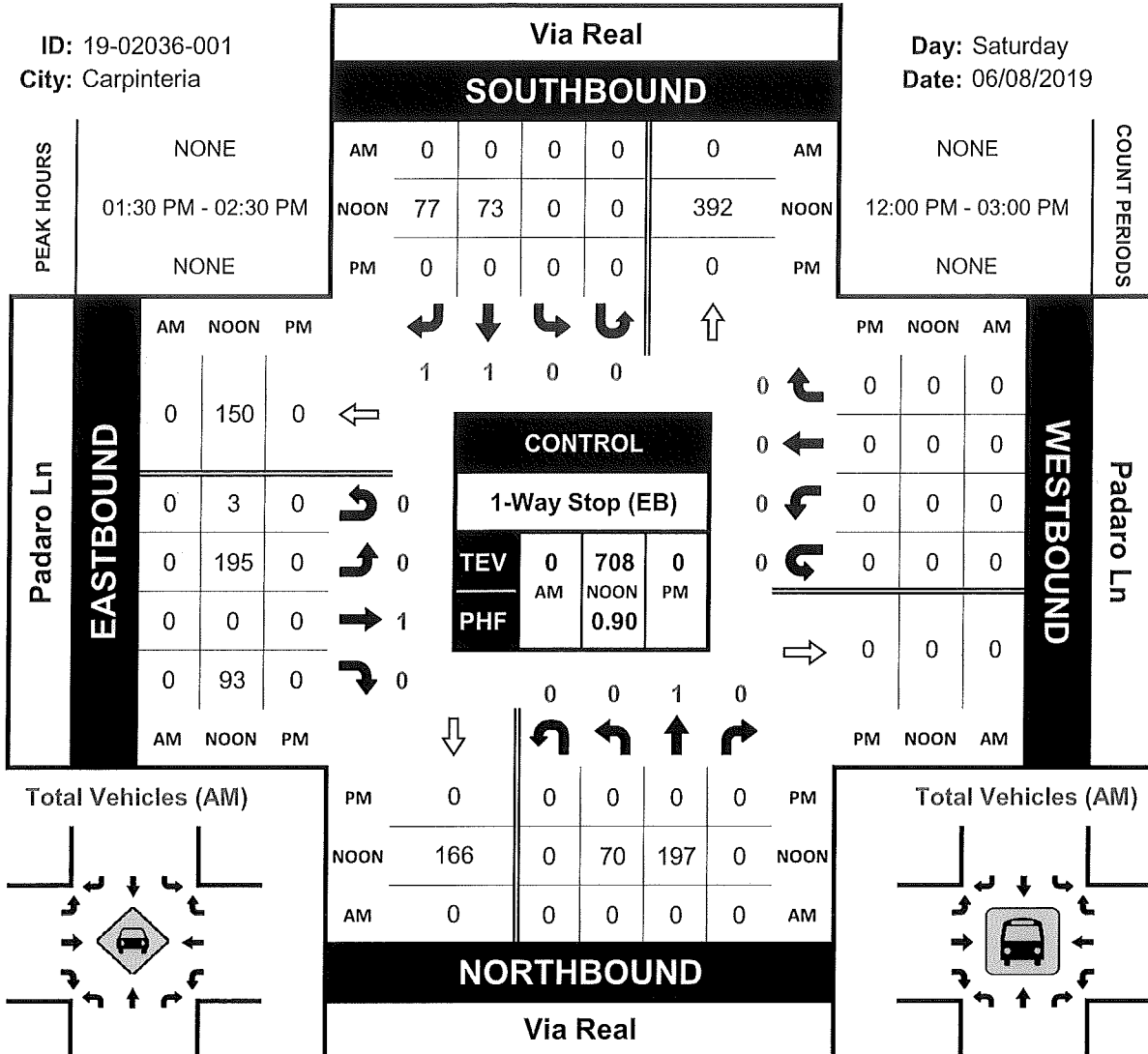
AWD = 10.4 = LOS B

Via Real & Padaro Ln

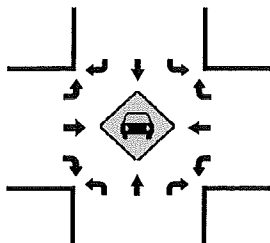
Peak Hour Turning Movement Count

ID: 19-02036-001
City: Carpinteria

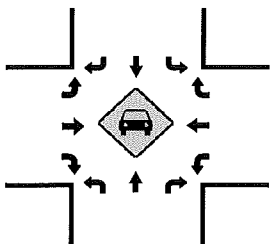
Day: Saturday
Date: 06/08/2019



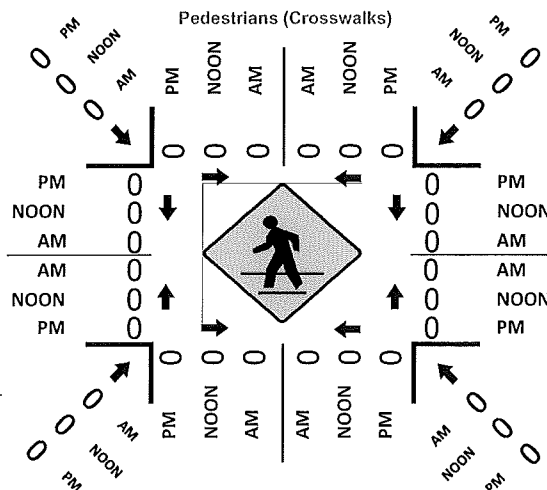
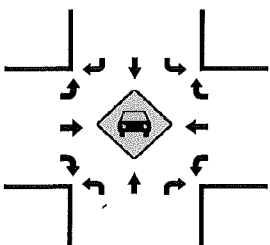
Total Vehicles (AM)



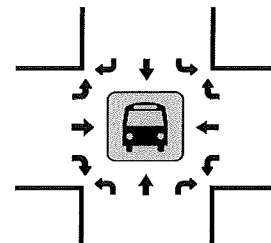
Total Vehicles (NOON)



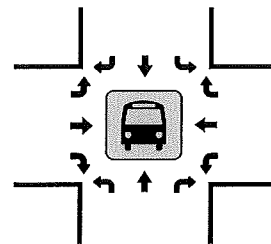
Total Vehicles (PM)



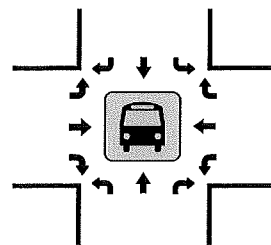
Total Vehicles (AM)



Total Vehicles (NOON)



Total Vehicles (PM)



National Data & Surveying Services

Intersection Turning Movement Count

Location: Via Real & Padaro Ln
 City: Carpinteria
 Control: 1-Way Stop (EB)

Project ID: 19-02036-001
 Date: 2019-06-08

Total

NS/EW Streets:	Via Real				Via Real				Padaro Ln				Padaro Ln				TOTAL	
NOON	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU		
12:00 PM	36	34	0	0	0	51	40	0	16	0	22	0	0	0	0	0	199	
12:15 PM	19	36	0	0	0	28	15	0	20	0	18	0	0	0	0	0	136	
12:30 PM	19	20	0	0	0	21	21	0	25	0	25	0	0	0	0	0	131	
12:45 PM	21	17	0	0	0	17	14	0	19	0	24	0	0	0	0	0	112	
1:00 PM	16	28	0	0	0	20	18	0	26	0	27	0	0	0	0	0	135	
1:15 PM	22	24	0	0	0	21	18	0	62	0	34	0	0	0	0	0	181	
1:30 PM	24	47	0	0	0	25	18	0	49	0	19	0	0	0	0	0	182	
1:45 PM	11	48	0	0	0	16	19	0	57	0	28	2	0	0	0	0	181	
2:00 PM	12	35	0	0	0	22	16	0	41	0	23	0	0	0	0	0	149	
2:15 PM	23	67	0	0	0	10	24	0	48	0	23	1	0	0	0	0	196	
2:30 PM	19	45	0	0	0	17	13	0	36	0	26	0	0	0	0	0	156	
2:45 PM	20	22	0	0	0	15	12	0	30	0	22	0	0	0	0	0	121	
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
APPROACH %'s :	242	423	0	0	0	263	228	0	429	0	291	3	0	0	0	0	1879	
	36.39%	63.61%	0.00%	0.00%	0.00%	53.56%	46.44%	0.00%	59.34%	0.00%	40.25%	0.41%						
PEAK HR :	01:30 PM - 02:30 PM				01:30 PM	171	165	176	02:15 PM									TOTAL
PEAK HR VOL :	70	197	0	0	0	73	77	0	195	0	93	3	0	0	0	0	0	708
PEAK HR FACTOR :	0.729	0.735	0.000	0.000	0.000	0.730	0.802	0.000	0.855	0.000	0.830	0.375	0.000	0.000	0.000	0.000	0.000	0.903
	0.742				0.872				0.836									

INTERSECTION DELAY WORKSHEET

Carpinteria

VIA REAL/PADARO

SATURDAY PEAK HOUR: 1:30-2:30 PM

SATURDAY, 6/8/19

15 SECOND INTERVALS

APPROACH: NORTHBOUND PADARO LANE

Time Ending	Vehicles in Queue															Total Approach					
1:35 PM	1	0	2	1	0	3	5	0	0	0	0	0	0	0	1	0	1	0	0	0	
1:40 PM	1	0	2	1	0	3	5	0	0	0	0	0	0	0	1	0	1	0	0	0	
1:45 PM	0	1	2	2	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	68	
1:50 PM	1	0	3	0	0	0	0	2	0	0	0	0	0	1	0	0	1	0	0		
1:55 PM	0	0	0	0	0	2	2	0	4	1	0	0	0	1	0	0	0	0	0	2	
2:00 PM	0	0	0	0	0	0	0	0	2	1	0	0	2	2	1	0	0	0	1	87	
2:05 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0		
2:10 PM	0	0	3	0	2	0	0	0	0	0	0	0	0	0	0	1	0	0	0		
2:15 PM	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	64	
2:20 PM	3	5	4	4	2	4	1	0	2	0	0	1	2	1	1	0	0	0	0		
2:25 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	2	0	
2:30 PM	0	1	1	0	0	0	1	0	0	0	0	0	0	0	1	0	0	0	0	52	
Subtotal:	6	7	17	8	4	12	15	3	8	2	0	1	4	5	5	2	3	1	4	2	271
Total Vehicles in Queue:																109					

Total Delay = 109 vehicles x 15 seconds = 1635 seconds

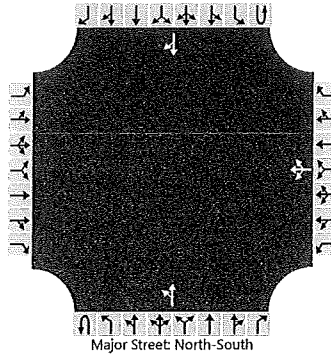
Average Delay Per Vehicle = 1635 seconds / 271 vehicles

6.0 Stop delay (seconds per vehicle) + 5 seconds control = 11.0

HCS7 Two-Way Stop-Control Report

General Information				Site Information			
Analyst	KAB			Intersection	US 101 NB/PADARO LANE		
Agency/Co.	ATE			Jurisdiction	CARPINTERIA		
Date Performed	6/20/2019			East/West Street	US101 NB RAMP		
Analysis Year				North/South Street	PADARO LANE		
Time Analyzed	WEEKEND PM PEAK HOUR			Peak Hour Factor	1.00		
Intersection Orientation	North-South			Analysis Time Period (hrs)	0.25		
Project Description	EXISTING CONDITIONS						

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement																
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	0	0		0	1	0	0	0	1	0	0	0	1	0
Configuration							LTR			LT						TR
Volume (veh/h)						71	14	190		49	114				116	32
Percent Heavy Vehicles (%)						3	3	3		3						
Proportion Time Blocked																
Percent Grade (%)	0															
Right Turn Channelized																
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)					2.0	2.0	2.0		4.1							
Critical Headway (sec)					2.03	2.03	2.03		4.13							
Base Follow-Up Headway (sec)					2.0	2.0	2.0		2.2							
Follow-Up Headway (sec)					2.03	2.03	2.03		2.23							

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)					275			49								
Capacity, c (veh/h)					1667			1427								
v/c Ratio					0.16			0.03								
95% Queue Length, Q ₉₅ (veh)					0.6			0.1								
Control Delay (s/veh)					7.6			7.6								
Level of Service (LOS)					A			A								
Approach Delay (s/veh)					7.6				2.5							
Approach LOS					A											

AWD = 7.6 = LOS = A

National Data & Surveying Services

Intersection Turning Movement Count

Location: US 101 NB Ramp & Padaro Ln
 City: Carpinteria
 Control: 1-Way Stop (NB)

Project ID: 19-02036-002
 Date: 2019-06-08

Total

NS/EW Streets:	US 101 NB Ramp				US 101 NB Ramp				Padaro Ln				Padaro Ln				TOTAL
NOON	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
12:00 PM	20	0	16	0	0	0	0	0	12	22	0	0	0	56	20	0	146
12:15 PM	22	4	23	0	0	0	0	0	9	15	0	0	0	24	10	0	107
12:30 PM	16	4	27	0	0	0	0	0	11	24	0	0	0	26	13	0	121
12:45 PM	22	2	16	0	0	0	0	0	11	26	0	0	0	24	12	0	113
1:00 PM	17	2	25	0	0	0	0	0	15	27	0	0	0	25	9	0	120
1:15 PM	12	2	67	0	0	0	0	0	14	31	0	1	0	30	9	0	166
1:30 PM	24	3	44	0	0	0	0	0	14	23	0	0	0	34	9	0	151
1:45 PM	18	7	54	0	0	0	0	0	5	33	0	0	0	27	5	0	149
2:00 PM	15	4	36	0	0	0	0	0	8	28	0	0	0	23	4	0	118
2:15 PM	9	2	49	0	0	0	0	0	11	22	0	0	0	41	8	0	142
2:30 PM	12	4	31	0	0	0	0	0	17	33	0	0	0	28	4	0	129
2:45 PM	7	3	27	0	0	0	0	0	11	24	0	0	0	25	7	0	104
TOTAL VOLUMES :	NL 194	NT 37	NR 415	NU 0	SL 0	ST 0	SR 0	SU 0	EL 138	ET 308	ER 0	EU 1	WL 0	WT 363	WR 110	WU 0	TOTAL 1566
APPROACH %'s :	30.03%	5.73%	64.24%	0.00%					30.87%	68.90%	0.00%	0.22%	0.00%	76.74%	23.26%	0.00%	
PEAK HR :	01:00 PM - 02:00 PM				01:00 PM	169	165	176	01:15 PM								TOTAL
PEAK HR VOL :	71	14	190	0	0	0	0	0	48	114	0	1	0	116	32	0	586
PEAK HR FACTOR :	0.740	0.500	0.709	0.000	0.000	0.000	0.000	0.000	0.800	0.864	0.000	0.250	0.000	0.853	0.889	0.000	0.883
	0.849								0.886				0.860				

INTERSECTION DELAY WORKSHEET

Carpinteria

US 101 NB RAMPS/PADARO LANE

SATURDAY PEAK HOUR: 1:00-2:00 PM

SATURDAY, 6/8/19

15 SECOND INTERVALS

US 101 NB OFF RAMP

Time Ending	Vehicles in Queue															Total Approach									
1:05 PM	1	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0
1:10 PM	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:20 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:25 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	1	2	0	0	0	0	0	0	0	0	0	0
1:30 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:35 PM	1	0	0	0	2	0	0	0	0	1	0	0	0	0	3	2	0	0	0	0	0	0	0	0	0
1:40 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:50 PM	0	0	0	0	1	0	2	0	0	1	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0
1:55 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0
2:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0
Subtotal:	2	3	0	2	3	0	2	2	2	2	0	2	2	0	3	3	3	0	0	0	0	0	0	0	265

Total Vehicles in Queue: 31

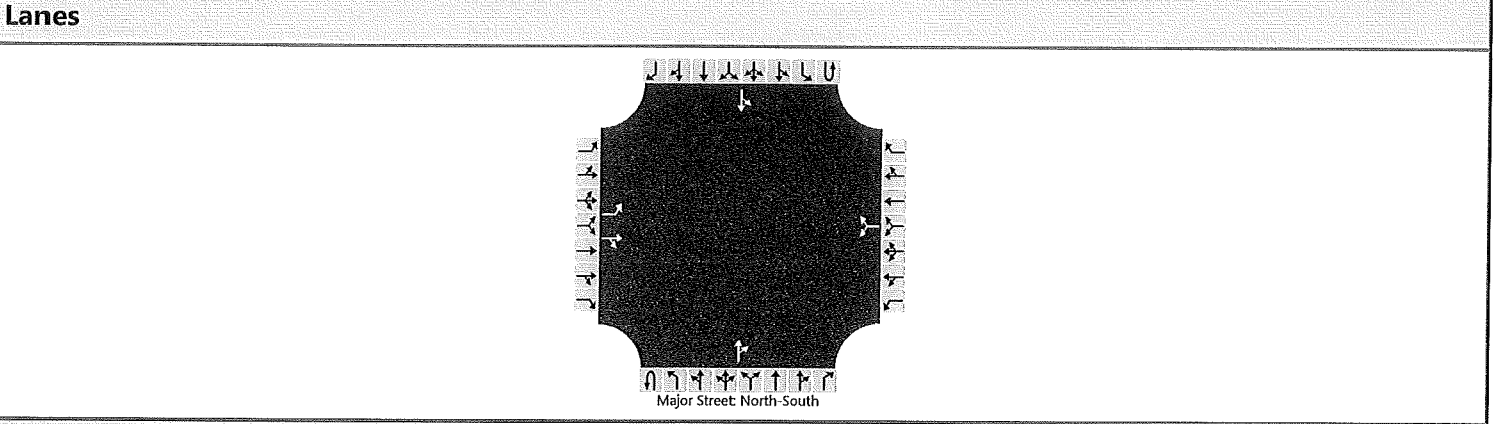
Total Delay = 31 vehicles x 15 seconds = 465 seconds

Average Delay Per Vehicle = 465 seconds / 265 vehicles

$$1.8 \text{ Stop delay (seconds per vehicle) + 5 seconds control} = 6.8$$

HCS7 Two-Way Stop-Control Report

General Information				Site Information			
Analyst	KAB			Intersection	US101SB/PADARO LANE-SANTA		
Agency/Co.	ATE			Jurisdiction	CARPINTERIA		
Date Performed	6/20/2019			East/West Street	US101 NB RAMP		
Analysis Year				North/South Street	PADARO LANE		
Time Analyzed	WEEKEND PM PEAK HOUR			Peak Hour Factor	1.00		
Intersection Orientation	North-South			Analysis Time Period (hrs)	0.25		
Project Description	EXISTING CONDITIONS						



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound				
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	
Movement																	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		1	1	0		0	1	0	0	0	1	0	0	0	1	0	
Configuration		L		TR			LR					TR		LT			
Volume (veh/h)		69	71	6		8		80			12	18		152	35		
Percent Heavy Vehicles (%)		3	3	3		3		3						3			
Proportion Time Blocked																	
Percent Grade (%)	0				0												
Right Turn Channelized																	
Median Type Storage	Undivided																

Critical and Follow-up Headways

Base Critical Headway (sec)		4.0	4.0	4.0		7.1		6.2							4.1		
Critical Headway (sec)		4.03	4.03	4.03		7.13		6.23							4.13		
Base Follow-Up Headway (sec)		2.0	2.0	2.0		3.5		3.3							2.2		
Follow-Up Headway (sec)		2.03	2.03	2.03		3.53		3.33							2.23		

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		69		77				88							152		
Capacity, c (veh/h)		1083		1203				955							1576		
v/c Ratio		0.06		0.06				0.09							0.10		
95% Queue Length, Q ₉₅ (veh)		0.2		0.2				0.3							0.3		
Control Delay (s/veh)		8.5		8.2				9.2							7.5		
Level of Service (LOS)		A		A				A							A		
Approach Delay (s/veh)		8.4					9.2					6.3					
Approach LOS		A					A					A					

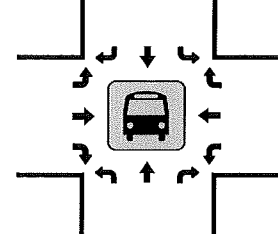
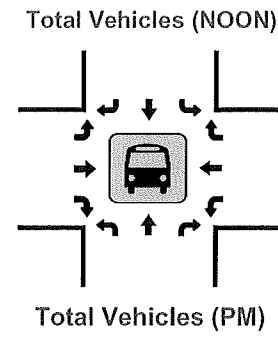
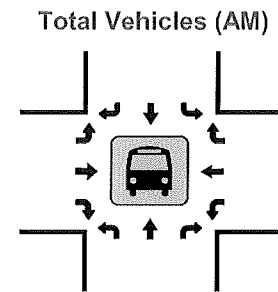
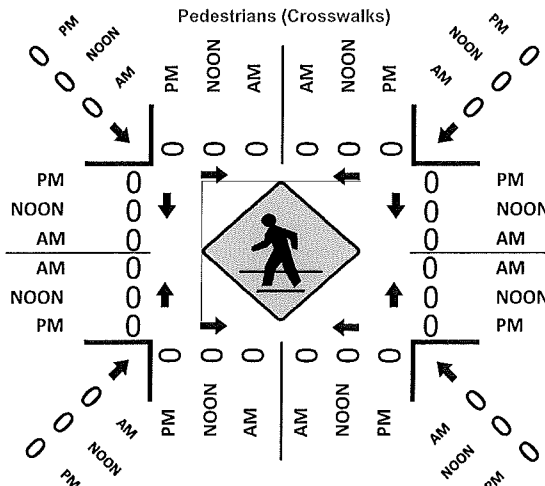
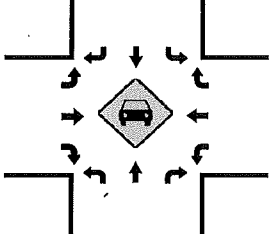
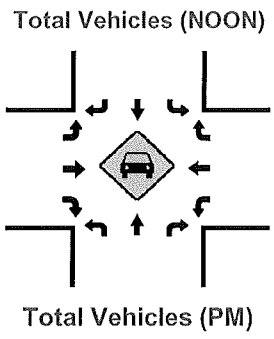
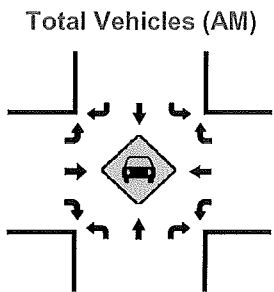
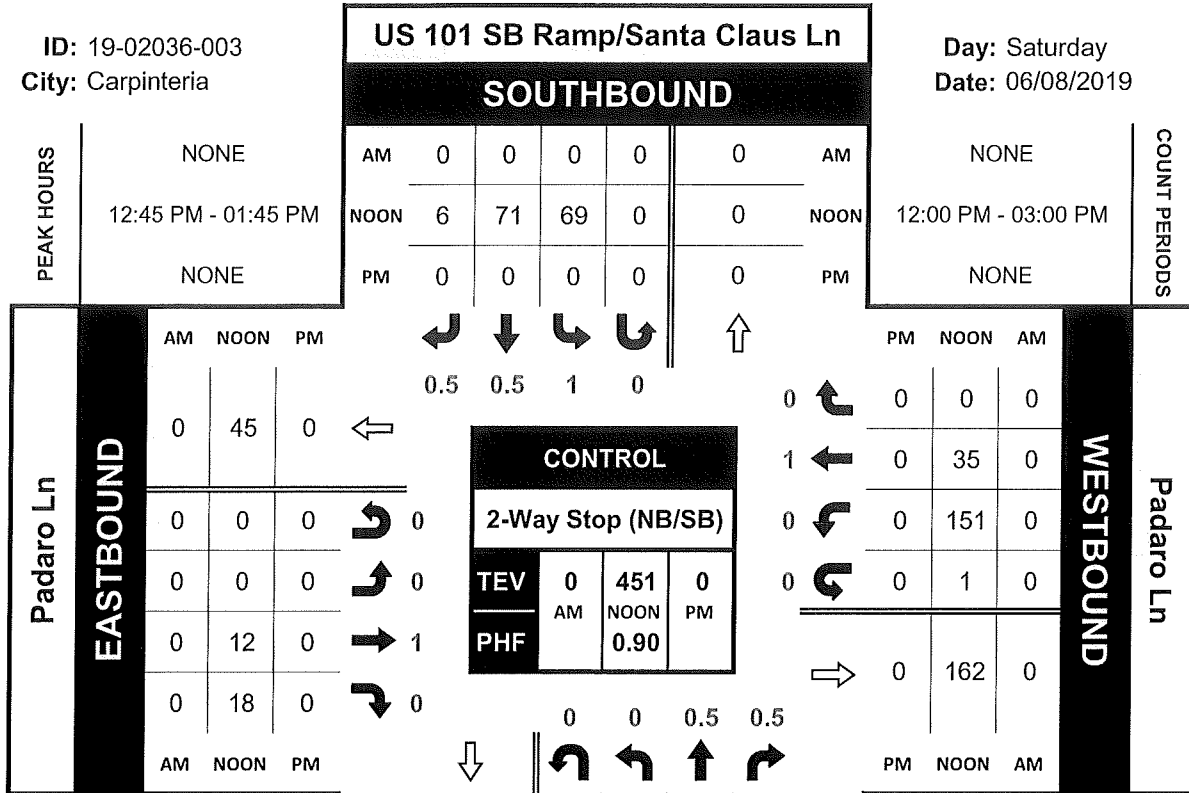
LOS = 8.2 = LOS A

US 101 SB Ramp/Santa Claus Ln & Padaro Ln

Peak Hour Turning Movement Count

ID: 19-02036-003
City: Carpinteria

Day: Saturday
Date: 06/08/2019



National Data & Surveying Services

Intersection Turning Movement Count

Location: US 101 SB Ramp/Santa Claus Ln & Padaro Ln
 City: Carpinteria
 Control: 2-Way Stop (NB/SB)

Project ID: 19-02036-003
 Date: 2019-06-08

Total

NS/EW Streets:	US 101 SB Ramp/Santa Claus Ln				US 101 SB Ramp/Santa Claus Ln				Padaro Ln				Padaro Ln				
NOON	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
12:00 PM	1	0	16	1	16	18	1	0	0	2	3	0	71	4	0	0	133
12:15 PM	3	0	10	0	13	17	0	0	0	0	6	0	41	6	0	0	96
12:30 PM	3	0	14	1	18	20	1	0	0	3	7	0	34	8	0	1	110
12:45 PM	2	0	18	0	16	22	2	0	0	2	3	0	32	11	0	1	109
1:00 PM	0	0	21	0	17	14	0	0	0	4	4	0	37	6	0	0	103
1:15 PM	2	0	20	1	22	16	3	0	0	3	4	0	35	8	0	0	114
1:30 PM	0	0	21	3	14	19	1	0	0	3	7	0	47	10	0	0	125
1:45 PM	4	0	15	0	22	14	0	0	0	0	7	0	40	6	0	0	108
2:00 PM	1	0	14	0	21	13	1	0	0	2	5	0	33	3	0	0	93
2:15 PM	1	0	14	1	19	17	1	0	0	0	3	0	47	6	0	0	109
2:30 PM	1	0	27	3	21	14	2	0	0	2	2	0	36	3	0	0	111
2:45 PM	2	0	11	3	21	15	1	0	0	3	4	0	30	2	0	0	92
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
APPROACH %'s :	20	0	201	13	220	199	13	0	0	24	55	0	483	73	0	2	1303
	8.55%	0.00%	85.90%	5.56%	50.93%	46.06%	3.01%	0.00%	0.00%	30.38%	69.62%	0.00%	86.56%	13.08%	0.00%	0.36%	
PEAK HR :	12:45 PM - 01:45 PM				12:45 PM				01:30 PM								TOTAL
PEAK HR VOL :	4	0	80	4	69	71	6	0	0	12	18	0	151	35	0	1	451
PEAK HR FACTOR :	0.500	0.000	0.952	0.333	0.784	0.807	0.500	0.000	0.000	0.750	0.643	0.000	0.803	0.795	0.000	0.250	0.902
	0.917				0.890				0.750				0.820				

INTERSECTION DELAY WORKSHEET
 Carpinteria
 US 101 SB/PADARO LANE
 SATURDAY PEAK HOUR: 12:34-1:45 PM
 SATURDAY, 6/8/19
 15 SECOND INTERVALS
 APPROACH: US 101 SB OFF RAMP LEFTS

Time Ending	Vehicles in Queue																Total Approach					
12:50 PM	1	0	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0
12:55 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	16
1:05 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0
1:10 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	17
1:20 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:25 PM	0	0	0	0	0	0	0	2	0	0	0	0	0	1	1	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	22
1:35 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0
1:40 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	14
Subtotal:	1	0	0	0	0	0	2	0	3	1	0	0	1	1	1	0	0	0	0	1	0	69
Total Vehicles in Queue:																	11					

Total Delay = 11 vehicles x 15 seconds = 165 seconds

Average Delay Per Vehicle = 165 seconds / 69 vehicles

2.4 seconds per vehicle + 5 SEC CONTROL = 7.4

INTERSECTION DELAY WORKSHEET

Carpinteria

US 101 SB/PADARO LANE

SATURDAY PEAK HOUR: 12:34-1:45 PM

SATURDAY, 6/8/19

15 SECOND INTERVALS

APPROACH: US 101 SB OFF RAMP THRU+RIGHT

Time Ending	Vehicles In Queue														Total Approach							
12:50 PM	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	
12:55 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	24
1:05 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:10 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	14
1:20 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:25 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:30 PM	1	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	19
1:35 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:40 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	20
Subtotal:	2	1	0	1	0	2	2	0	0	1	0	0	1	0	0	0	0	0	0	0	0	77
Total Vehicles In Queue:														10								

Total Delay = 10 vehicles x 15 seconds = 150 seconds

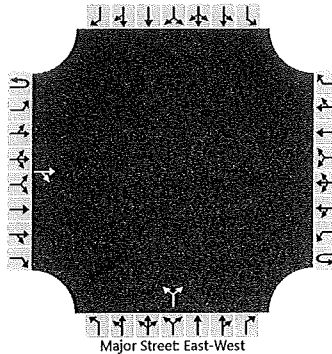
Average Delay Per Vehicle = 150 seconds / 77 vehicles

1.9 seconds per vehicle + S.D. SE - CHANGE = 6.9

HCS7 Two-Way Stop-Control Report

General Information				Site Information			
Analyst	KAB			Intersection	US101SB/PADARO LANE-SANTA		
Agency/Co.	ATE			Jurisdiction	CARPINTERIA		
Date Performed	6/20/2019			East/West Street	US101 NB RAMP		
Analysis Year				North/South Street	PADARO LANE		
Time Analyzed	WEEKEND PM PEAK HOUR			Peak Hour Factor	1.00		
Intersection Orientation	East-West			Analysis Time Period (hrs)	0.25		
Project Description	EXISTING CONDITIONS						

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound				
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	
Movement																	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	1	0	0	0	0	0		0	1	0		0	0	0	
Configuration				TR							LR						
Volume (veh/h)			158	10						6		7					
Percent Heavy Vehicles (%)										3		3					
Proportion Time Blocked																	
Percent Grade (%)										0							
Right Turn Channelized																	
Median Type Storage	Undivided																

Critical and Follow-up Headways

Base Critical Headway (sec)											7.1		6.2			
Critical Headway (sec)											6.43		6.23			
Base Follow-Up Headway (sec)											3.5		3.3			
Follow-Up Headway (sec)											3.53		3.33			

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)											13				
Capacity, c (veh/h)											853				
v/c Ratio											0.02				
95% Queue Length, Q ₉₅ (veh)											0.0				
Control Delay (s/veh)											9.3				
Level of Service (LOS)											A				
Approach Delay (s/veh)											9.3				
Approach LOS											A				

HCS7 Roundabouts Report

General Information

Site Information

Analyst	DLD		Intersection	SANTA CLAUS/SPINDRIFT
Agency or Co.	ATE		E/W Street Name	SANTA CLAUS
Date Performed	6/20/2019		N/S Street Name	SPINDRIFT
Analysis Year			Analysis Time Period (hrs)	0.25
Time Analyzed	WEEKEND PEAK HOUR		Peak Hour Factor	1.00
Project Description	EXISTING WEEKEND PEAK H...		Jurisdiction	SB COUNTY

Volume Adjustments and Site Characteristics

Approach	EB				WB				NB				SB			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement																
Number of Lanes (N)	0	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0
Lane Assignment	TR								LR							
Volume (V), veh/h	0		158	10					0	6		7				
Percent Heavy Vehicles, %	3		3	3					3	3		3				
Flow Rate (V _{pcu}), pc/h	0		163	10					0	6		7				
Right-Turn Bypass	None				None				None				None			
Conflicting Lanes	1								1							
Pedestrians Crossing, p/h	0								0							

Critical and Follow-Up Headway Adjustment

Approach	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Critical Headway (s)		4.9763						4.9763				
Follow-Up Headway (s)		2.6087						2.6087				

Flow Computations, Capacity and v/c Ratios

Approach	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Entry Flow (v _e), pc/h		173						13				
Entry Volume, veh/h		168						13				
Circulating Flow (v _c), pc/h	0			6			163			6		
Exiting Flow (v _{ex}), pc/h	170			6			0			10		
Capacity (C _{pcu}), pc/h		1380						1169				
Capacity (c), veh/h		1340						1135				
v/c Ratio (x)		0.13						0.01				

Delay and Level of Service

Approach	EB			WB			NB			SB		
	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass	Left	Right	Bypass
Lane Control Delay (d), s/veh		3.7						3.3				
Lane LOS		A						A				
95% Queue, veh		0.4						0.0				
Approach Delay, s/veh	3.7						3.3					
Approach LOS	A						A					
Intersection Delay, s/veh LOS	3.7						A					

