

GeoMat Testing Laboratories, Inc.

Soil Engineering, Environmental Engineering, Materials Testing, Geology

April 1, 2020

Project No. 20105-01

TO: Sri Jayaram Foundation, Inc.
6549 Pimlico Place
Eastvale, California 92880

SUBJECT: Soil Infiltration Report Update, APN 1016-331-05-0000, 4.83 Acres, 12594 Roswell Avenue, City of Chino, County of San Bernardino, California

REFERENCE: City and County Engineering and Testing, Inc. "Basic Infiltration Testing Report, Proposed Sri Sai Mandir Center, Approximately 4.83 Acres, 12594 Roswell Avenue, City of Chino, County of San Bernardino, California." Project No. J&P2018044.DRI.RPT, Report Dated August 22, 2018.

As requested, we have updated the above referenced soil infiltration report. The purpose of the update is to transfer, to the new provided plan, the previously reported information by City and County Engineering. City and County Engineering is no longer in business.

For easy reference, the previously prepared report by City and County Engineering is attached. There are no changes in the findings, conclusion and recommendation of the previous report except for the following:

1. New site plan which supersedes the previous site plan. The new plan depicts the exploratory boreholes and soil infiltration tests previously conducted by City and County Engineering.
2. New Project Description and usage provided by project representative.

New Project Description

The proposed development is located on a 4.83 acre site at 12594 Roswell Avenue, Chino, CA in the unincorporated area of San Bernardino County. The site is bordered by Roswell Avenue at the East and Walnut Ave at the North. The proposed project is to construct about 32,400 square foot multipurpose building to serve as both a place of worship as well as a facility for various community events & activities. The proposed development also includes about 4,500 square feet of caretaker quarter.

Usage of Proposed Building:

The first level is designed to serve as the main 270- seat congregation area for the purpose of worship and prayer. There will also be a kitchen facility for cooking and a dining hall located adjacent to the main congregation hall at the first floor, as well as classrooms for the youth,

multipurpose meeting rooms, administrative offices and prayer/meditation rooms. A detailed site plan is attached with this document.

The second level will house a prayer hall where devotees can view the idols and perform rituals. There will also be three classrooms for youth to learn about music, dance, yoga, education, etc.

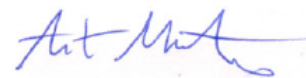
The facility will also be designed to offer spaces for community events and activities. Both the larger hall or the smaller multipurpose rooms and classrooms will function individually for community services such as health fairs, counseling sessions, job search assistance, environmental awareness campaigns, community pantry, food drive, etc.

If you should have any questions regarding this report, please do not hesitate to call our office. We appreciate this opportunity to be of service.

Submitted for GeoMat Testing Laboratories, Inc.



Haytham Nabils, GE 2375
Project Engineer, Exp. 12/31/2020



Art Martinez
Staff Engineer

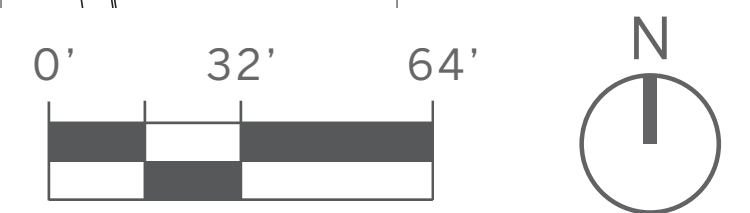
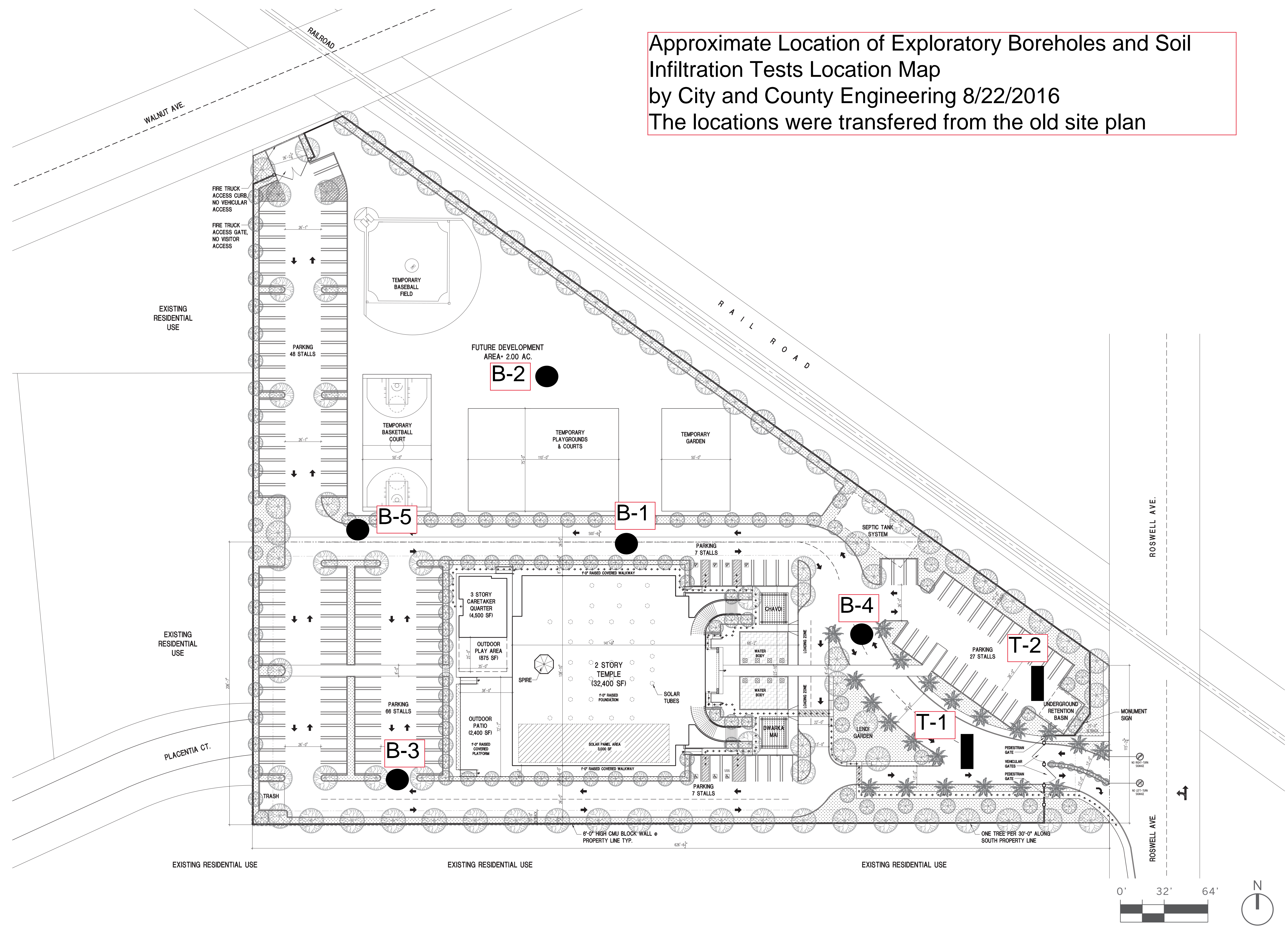


Distribution: (1) Addressee

Attachments:

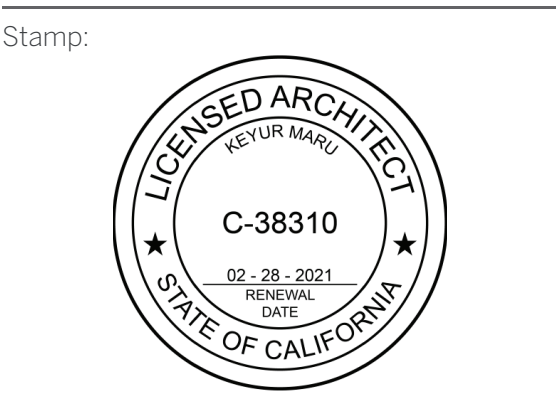
Plate 1 Site Plan
Appendix A Soil Infiltration Report by City and County Engineering and Testing

Approximate Location of Exploratory Boreholes and Soil Infiltration Tests Location Map
 by City and County Engineering 8/22/2016
 The locations were transferred from the old site plan



Revisions:

01	Conceptual Drawings	11.20.2019
02	Conceptual Drawings v2	11.26.2019
03	Conceptual Drawings v3	12.03.2019
04	Conceptual Drawings v4	12.05.2019
05	Conceptual Drawings v5	12.15.2019
06	Conceptual Drawings v6	12.24.2019
07	Conceptual Drawings v7	01.20.2020
08	Conceptual Drawings v8	03.16.2020



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Drawing Data:

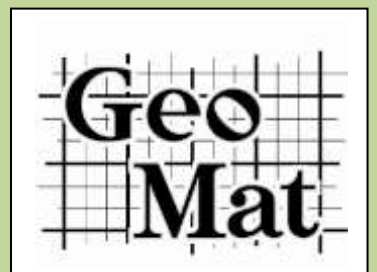
Project No	19026
Issue Date	03.16.2020
Scale	1/32" = 1'-0"
Drawn By	KM
Sheet	22" x 34"

If sheet is less than size indicated above, this is a reduced print. Reduce scale accordingly.

Sheet:

Site Plan

Appendix A



**CITY & COUNTY ENGINEERING
AND TESTING, INC.**

2324 S. Vineyard Ave., Suite B, Ontario, CA 91761-7764, (909)-930-5868

**BASIC INFILTRATION TESTING REPORT, PROPOSED SRI SAI MANDIR CENTER,
APPROXIMATELY 4.83 ACRES, 12594 ROSWELL AVENUE,
CITY OF CHINO, COUNTY OF SAN BERNARDINO, CALIFORNIA,**

APN: 1016- 331- 05-0000

**August 22, 2018
Job #J&P2018044 DRI.RPT**

**Prepared For:
SRI SAI RAM MANDIR
12594 Roswell Avenue
CHINO, CA 91710**

**Prepared By:
CITY & COUNTY ENGINEERING AND TESTING INC.
2324 S. Vineyard Ave. Suite B
Ontario, CA 91761-7764
(909)-930-5868**

CITY & COUNTY ENGINEERING AND TESTING, INC.

2324 s. Vineyard Ave., Suite B, Ontario, CA 91761-7764

August 22, 2018

Job #J&P2018044 DRI.RPT

**SRI SAI RAM MANDIR
12594 Roswell Avenue
CHINO, CA 91710**

Attention: Mrs. Arunasri Reddy. Project Manager
**Subject: BASIC INFILTRATION TESTING REPORT, IN GENERAL ACCORDANCE
WITH ASTM 3385-03 TEST METHOD PROPOSED SRI SAI RAM MANDIR,
APPROXIMATELY 4.83 ACRES, 12594 ROSWELL AVENUE, CITY OF
CHINO, COUNTY OF SAN BERNARDINO, CALIFORNIA,**

APN: 1016-331-050-0000

Reference: Your Work Authorization and Contract dated August 03, 2018

Introduction

This report provides a summary of the geotechnical engineering services conducted to support evaluation of the feasibility of infiltration at the subject site. The purpose of our services was to complete two (2) in-situ infiltration tests utilizing the double-ring infiltrometer to evaluate the feasibility of infiltration for disposal of stormwater runoff following the falling head method.

Project Description

We understand that an infiltration trench/swale will be utilized to capture storm runoff for on-site disposal for the proposed Sri Sai Ram Mandir Center

Scope of Services

City and County Engineering and Testing was retained to provide geotechnical engineering services to support the project. Our scope of work consisted of the following specific tasks:

- 1) Complete two (2) infiltration tests at the site utilizing the double ring infiltrometer. The tests were completed in general accordance with the falling head method.
- 2) Complete data analysis.

3) Preparation of this report summarizing our findings, conclusions, and recommendations. The report includes:

- Site plan showing the location of infiltration tests and exploratory trench.
 - Summary of log of conditions observed at the testing locations.
 - Discussion of the results of insitu infiltration testing.
-
- A discussion of the surficial soil and anticipated groundwater conditions at the site.
 - Evaluation of the feasibility of infiltration.
 - Recommendations for in-situ infiltration rate.

Existing Site Conditions

The site is located in the southwesterly San Bernardino County. The property consists of the irregular-shaped parcel of 4.83 acre is located at 12594 Roswell Avenue within the City of Chino, California. Based on our site reconnaissance, the site is vacant, now. Southeast of the site was previously developed with a single family home and detached garage, which was later used for Armstrong nursery. Most of the site was used for nursery use. The, structure along with garage, plants and goods were since have been removed from the site leaving a stockpile of crushed rock in the southeast corner. The site is bounded to its north and northeast, south, east and west by chain link fence, partly block wall and wood fence. Rail road tract bordered to its north and northeast. There are few residential and industrial structures located around the subject site. No drainage course is located within the site or close by. The site has general slope towards south and southeast.

Groundwater

Groundwater study is not within the scope of this work. However, no groundwater was encountered in the exploratory trenches to 15 feet depth.

Based on the California Department of Water Resources and local water company's website; the depth of groundwater at the site is more than 50 feet.

Please note that the potential for rain or irrigation water locally seeping through from adjacent elevated areas and showing up near grades cannot be precluded. Our experience indicates that surface or near-surface groundwater conditions can develop in areas where groundwater conditions did not exist prior to site development, especially in areas where a substantial increase in surface water infiltration results from landscape irrigation. Fluctuations in perched and static water elevations are likely to occur in the future due to variations in precipitation, temperature, consumptive uses, and other factors including urbanization and development. However; it is not likely to be less than 100 feet.

Subsurface Soils

The subsoil encountered in our exploratory borings and Infiltration trench during the exploration consists of **young alluvium**, brown to olive gray, fine silty sand (SM), silt lenses, poorly graded with grass-vegetation, house hold trash and debris to about 5-feet below the existing ground, slightly moist to moist, and very loose. The underlying soils below 5-feet to about 10-feet were found to be olive gray, fine silty sand (SM) and sandy silt (ML), slightly moist to moist and loose to firm. The sub soils between 10-feet to the end of our borings to a maximum depth of 40-feet were olive gray, fine sandy silt (ML) and fine silty sand (SM), poorly graded, moist and medium dense. Generally, the sub soils are very loose in the upper 5-feet and medium dense below 5-8 feet.

Based on the laboratory test results, the subsurface soils in foundation zone consist of mostly fine poorly graded silty fine sand (SM), possess relatively low cohesive properties, are highly susceptible to hydro consolidation and low in expansion potential.

Groundwater or hard bedrock strata were not encountered in any of our exploratory borings/trenches to a maximum depth of 40.0 feet below the existing ground. Information, based on the local water district, the depth of groundwater in the vicinity of the site should be 50-feet or more.

Test Method and Findings

Two infiltration tests were conducted at 5.0 and 8 feet below ground surface, in native soil. Based on the results of this study, infiltration of stormwater at the site is feasible. The following summarizes the result of the infiltration feasibility study and the recommended field infiltration rate for use in design.

Trench excavation for infiltration testing was conducted utilizing a track mounted DEEREJBE- 310 extended hoe backhoe on August 11, 2018. The bottom of the test trenches were cut level to the desired infiltration depth of 5.0 and 8 feet below the ground surface. The soil profile is described in the form of Exploratory Trench Logs, see Appendix B.

Infiltrometer Device

The double-ring infiltrometer test method consists of driving two open cylinders, one inside the other, into the ground and then partially filling the rings with water to a fixed point. The water is added at the constant mark at every time interval. The volume of water added each time interval is equal to the measure of the volume of liquid that infiltrates into the soil. The volume of water infiltrated during the time intervals can be converted into an infiltration velocity (in^3/hr). The incremental infiltration velocity within the inner test cylinder is equivalent to the infiltration rate (in/hr).

Infiltration Test Result

Based on the (minimum) test result, water infiltration rate stabilized at 1.5 *inch per hour* or 3.81 *cm/hr.* for the tests that were conducted 5.0 and 8 feet below ground surface, see Appendix C. This result is raw test result.

Factors of Safety

Based on **Worksheet “H” in the Technical Guidance Document** for Water Quality Management Plans prepared for The County of San Bernardino Area wide Storm water Program dated June 07, 2016, the minimum safety factor for this suitability assessment is 1. The design engineer should complete Worksheet “H” to determine the Total Safety Factor for the BMP. Minimum safety factor should not be less than 2, but may be higher at the discretion of the design engineer and acceptance of the plan reviewer.

Conclusions/Recommendations

- In our opinion, water infiltration at the site is feasible. Filter fabric should be used whenever aggregates are placed against native soils.
- Infiltration water should not be allowed to saturate pavement and concrete structures sub grade soils.
- The planned infiltration system should extend vertically into native soil. The designer should review the attached geotechnical log for soil classification.
- Please note that soils in infiltration areas should not be subject to compaction during construction.
- The proposed system by the civil engineer should be constructed and maintained in accordance with manufacturer guidelines.

An important consideration for infiltration facilities is that, during construction, great care must be taken not to reduce the infiltrative capacity of the soil in the facility through compaction by heavy equipment or by using the infiltration area as a sediment trap.

Infiltration facilities should be constructed late in the site development after soils (that might erode and clog the units) have been stabilized, or should be protected (by flagging) until site work is completed.

Infiltration facilities should be sited with the following guidelines:

INFILTRATION FACILITY SETBACKS	
Setback From	Distance
Property Lines and Public Right of Way	5 feet
Foundations	15 feet or within a 1:1 plane drawn up from

	the bottom of foundation
Slopes	H/2, 5 feet minimum (H: is slope height)
Private drinking water wells	100 feet

Ferrous metal pipes should be protected from potential corrosion by bituminous coating, etc. We recommend that all utility pipes be nonmetallic and/or corrosion resistant. Recommendations should be verified by soluble sulfate and corrosion testing of soil samples obtained from specific locations during construction.

If applicable, four to six inch diameter with locking caps observation well(s) extending vertically into the system's bottom is suggested as an observation point. Observation well(s) should be checked regularly and after large storm events. Once performance stabilizes, frequency of monitoring may be reduced.

City & County Soil Engineering should observe the basin excavation. Additional laboratory testing including but not limited to grain size analysis, sand equivalent, sulfate content, etc should be conducted during construction.

Use of this Report

This report was prepared for the exclusive use of the owner and their consultants for specific applications to the proposed site. The use by others, or for the purposes other than intended, is at the user's sole risk.

The findings, conclusions, and recommendations presented herein are based on our understanding of the project and on subsurface conditions observed during our site work. Within the limitations of scope, schedule, and budget, the conclusions and recommendations presented in this report were prepared in accordance with generally accepted geotechnical engineering principals and practices in the area at the time the report was prepared. We make no other warranty either expressed or implied.

We appreciate this opportunity to provide geotechnical services on this project and look forward to assisting the Project Team as the design progresses. Please call our office if you have any questions or comments regarding the information contained in this report, or if we may be of further services

Submitted for
City and County Engineering and Testing Inc.



Zen Bhatia, RCE #36150, License Expired on 6/30/2020

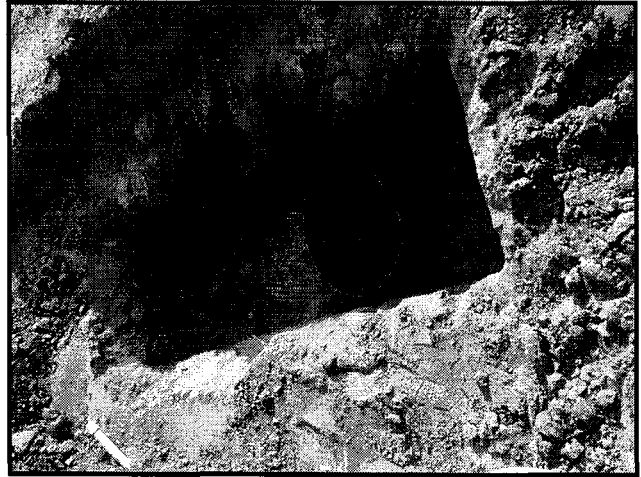
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Attachments: Plate 1 Site Photos
Plate 2 Index Map
Plate 3 Topographic Map
Plate-4 Aerial Map
Plate 5 Infiltration Test Location Map
Appendix A Percolation Data/Graphs

SITE PHOTOGRAPHS



VIEW- EAST TO WEST



VIEW- DOUBLE RING INFILT. TEST



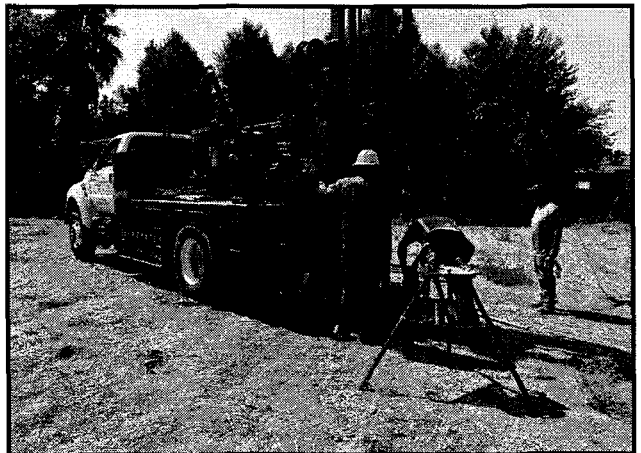
VIEW- NW TO SE



VIEW- SE TO NW



VIEW- SOUTH TO NORTH

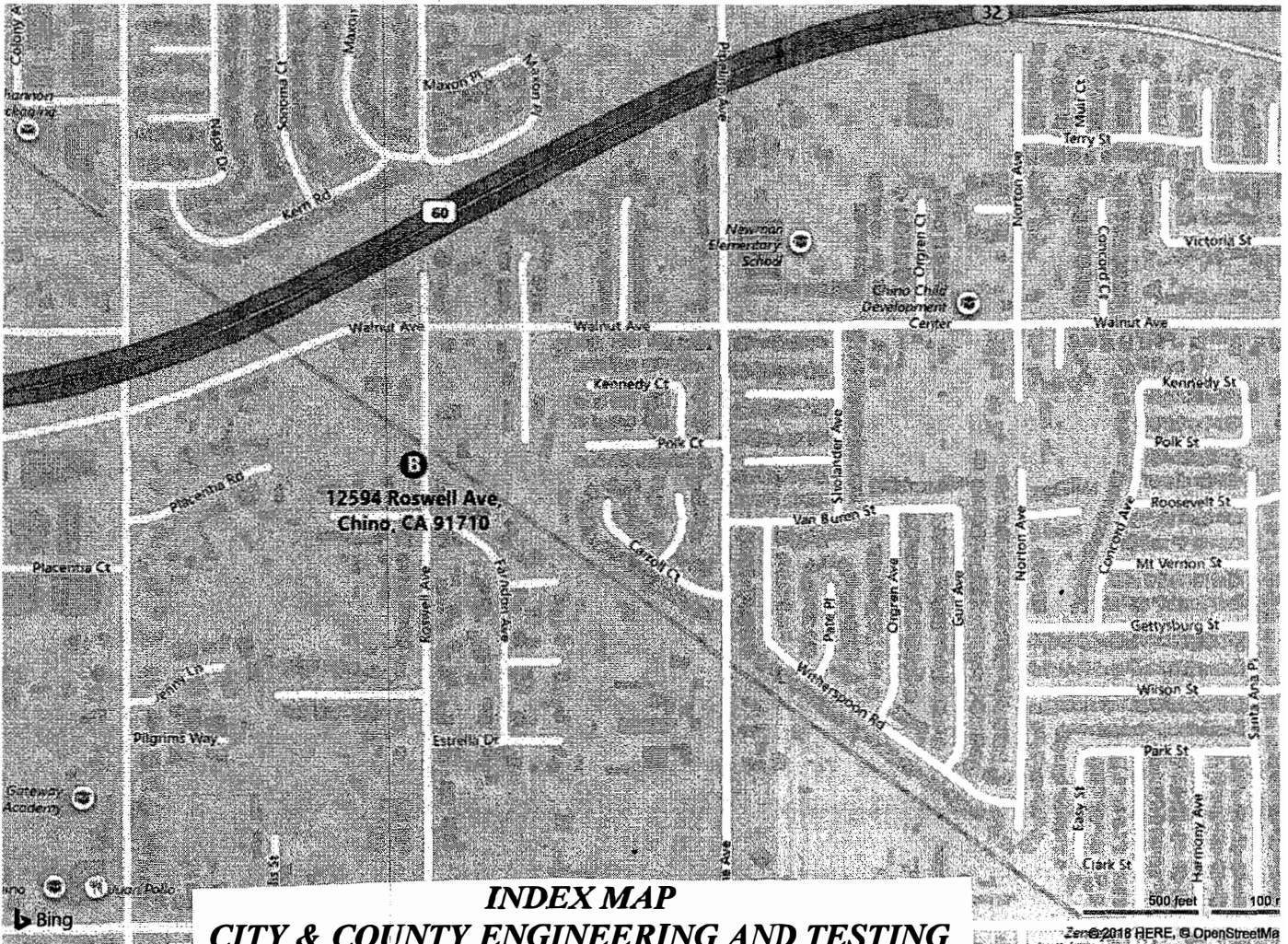
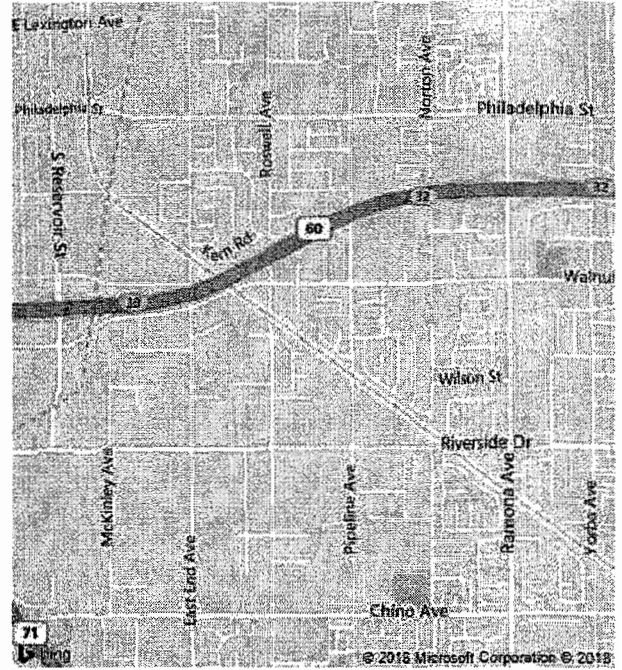


VIEW- WEST TO EAST



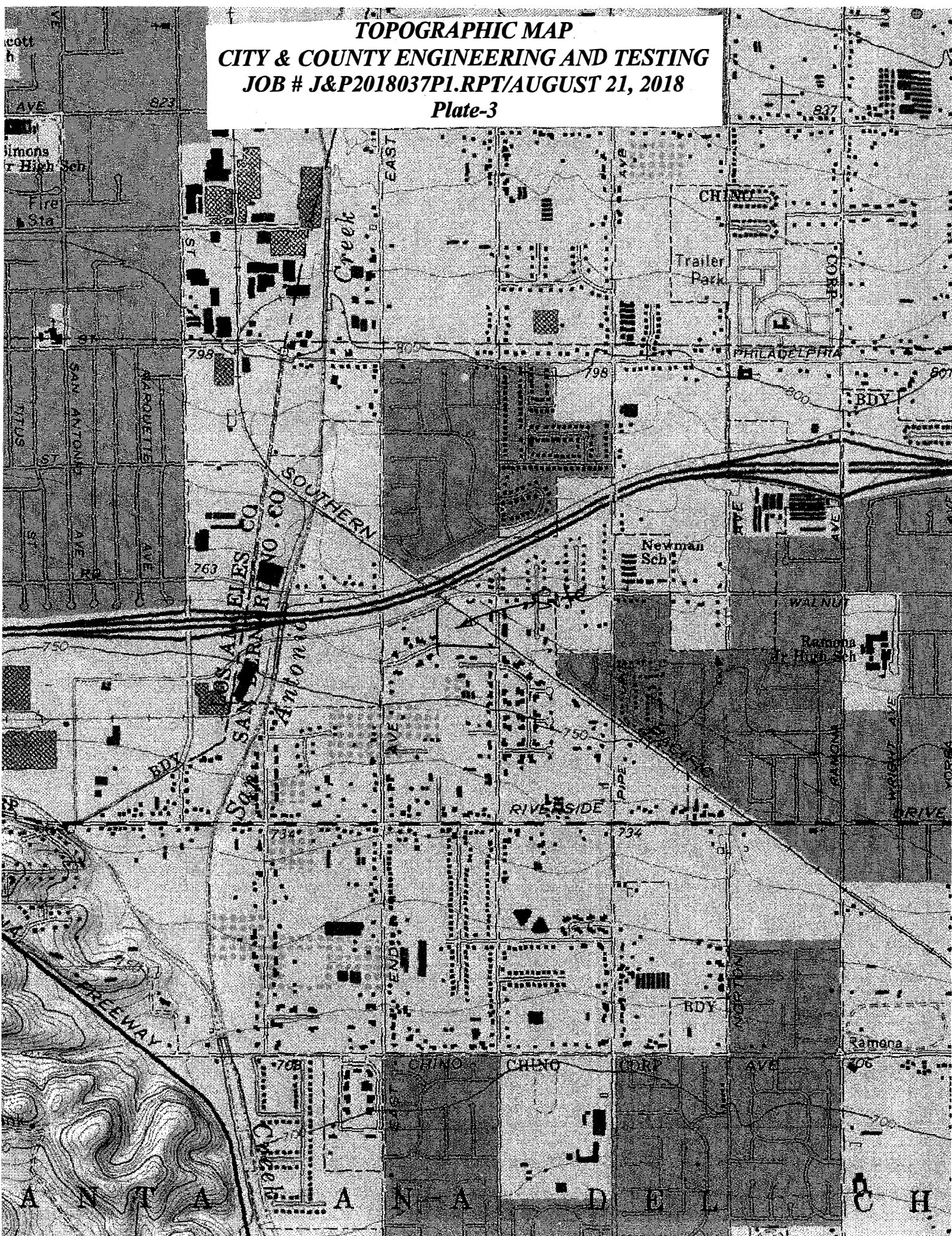
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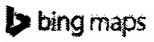
SRI SAI RAM TEMPLE
 JOB #J&P2018037P1
 August 7, 2018
 INDEX MAP
 Plate 2



INDEX MAP
CITY & COUNTY ENGINEERING AND TESTING
JOB # J&P2018037P1.RPT/August 21, 2018
Plate-2

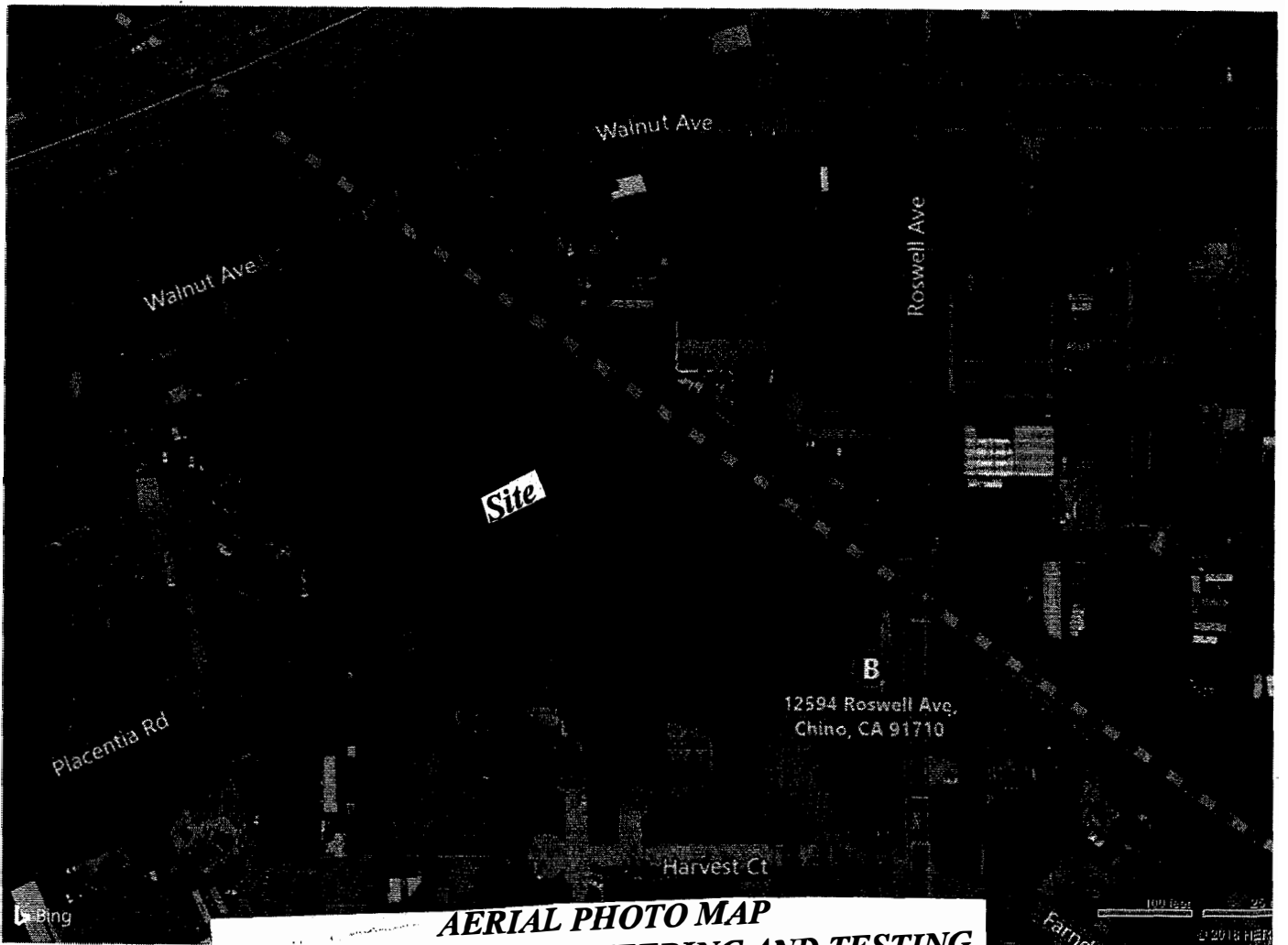
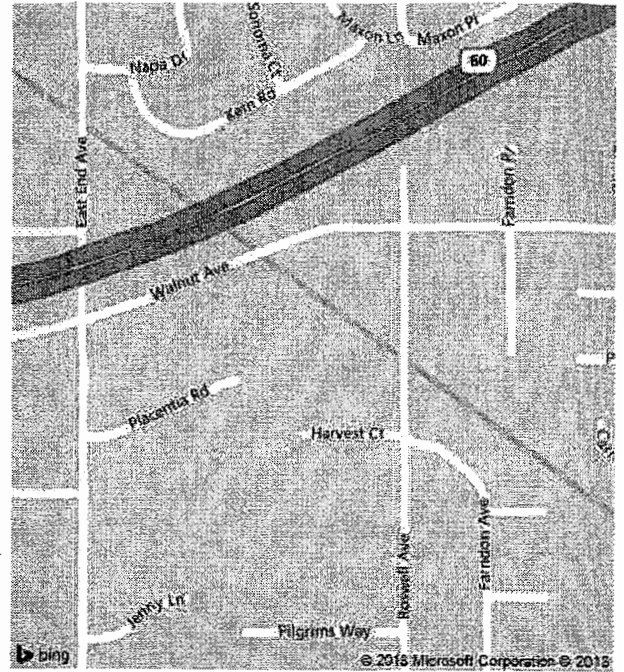
TOPOGRAPHIC MAP
CITY & COUNTY ENGINEERING AND TESTING
JOB # J&P2018037P1.RPT/AUGUST 21, 2018
Plate-3



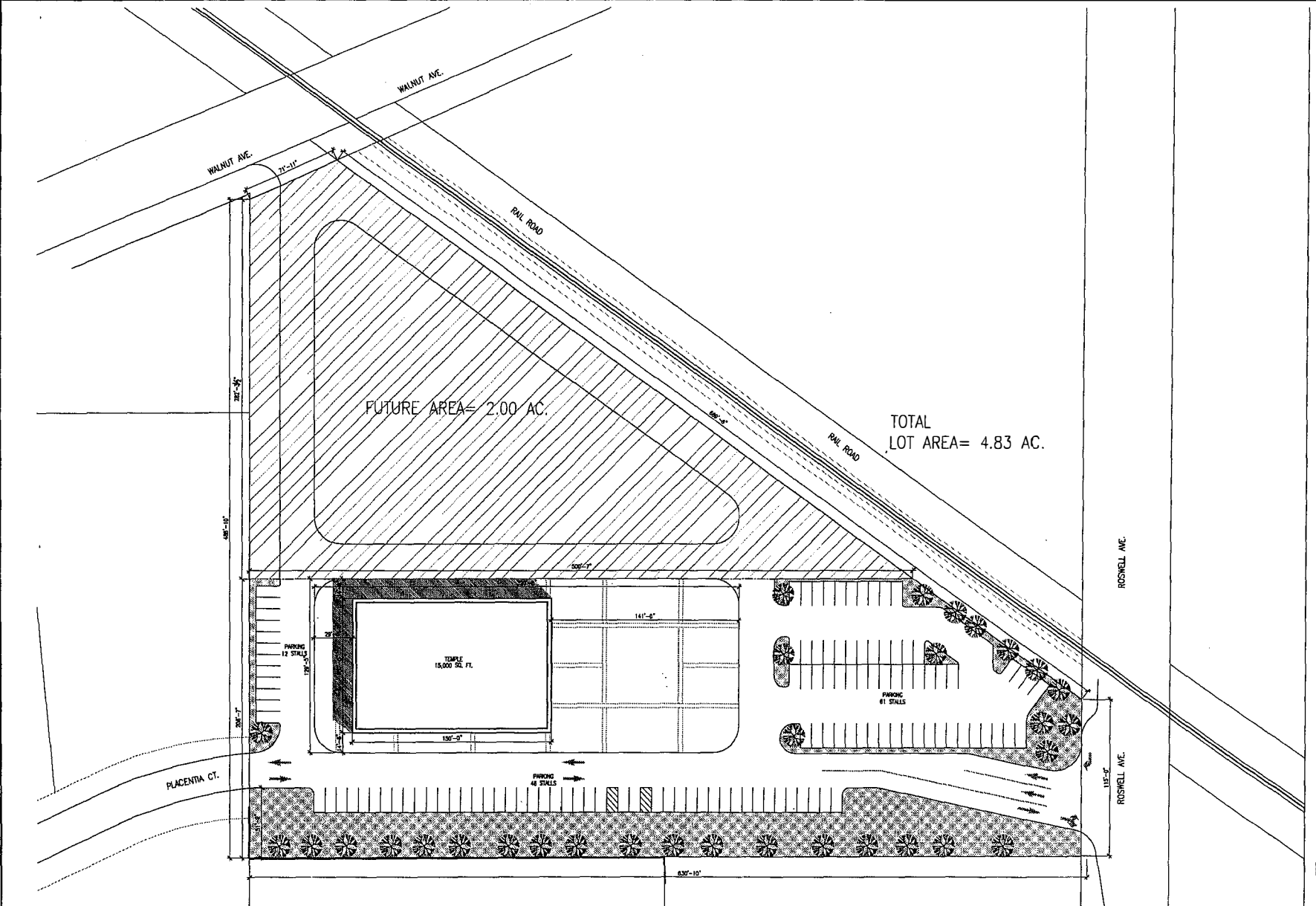


Notes

SRI SAI RAM TEMPLE
 JOB #J&P2018037P1
 August 7, 2018
 AERIAL MAP
 Plate 3



AERIAL PHOTO MAP
CITY & COUNTY ENGINEERING AND TESTING
JOB # J&P2018037P1.RPT/AUGUST 21, 2018
Plate-4



CONCEPTUAL SITE PLAN
SCALE: 1/32" = 1' - 0"



APPROVAL STAMP:

APPROVAL STAMP:



APPROVAL STAMP:

PROJECT:
SHRI SAI RAM MANDIR
CONCEPTUAL DESIGN

CLIENT:

DATE: 8/16/2018
 DRAWN: AM CHECKED: SB
 SCALE: AS NOTED PROJECT NO.:
 TITLE:

CONCEPTUAL SITE PLAN

SHEET:
A-1.0

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REFERENCES

The County of San Bernardino Area Wide Storm Water Program, Technical Guidance Document for Water Quality Management Plans, June 7, 2013.

San Bernardino County Stormwater Program, Model Water Quality Management Plan Guidance, Jun. 9, 2005.

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Carsel, R. F. and R. S. Parrish. 1988. "Developing joint probability distributions of soil water retention characteristics." *Water Resour. Res.*24: 755-769.

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Massmann, JW, Butchart, and S Stolar, Infiltration Characteristics, Performance, and Design of Stormwater Facilities, Final Research Report, Research Project TI 803, Task 12, Washington DOT 2003.

Soilvision Systems, A Knowledge-Based Soils Database, Murray Fredlund, Canada, 2004.

California Stormwater Quality Association (QASCA), California Stormwater BMP Handbook, Infiltration Trench, TC-10 Design Considerations

BMP Handbook, Part B, Planning Activities, Stormwater Mitigation Measures, Watershed Protection Division, City of Los Angeles.

APPENDIX A
PERCOLATION TEST DATA/GRAPH

CITY & COUNTY SOIL ENGINEERING AND TESTING

DOUBLE RING INFILTROMETER TEST DATA- TEST #T-1 (P-1)

SRI SAI RAM		TEMPLE		Constants-		Ring Data		Liquid Container-	
Location: 12594 ROSWELL AVE, CHINO, County of San Bernardino, California				Job #J&P 2018044		Area, A _r (in ²)	Depth of Liquid (in)	Vol. V _r (in ³ /in) #	
				DRI					
Test By: HM/GL USCS class SP/GP				Annular Space: 24" D		113	18"	1-78.54	
Water Table Dept		Penetration of Rings into Soil (in.):				Inner:		Outer: 89 F	
Date Test:	Tape water used:	80F	pH:	Ground Temp (F): 83 f			at Depth: 12" / 78 F		
Liquid Level Maintained by using:		(Flow valve <input type="checkbox"/> Float Valve <input type="checkbox"/> Marriotte Tube <input checked="" type="checkbox"/> Other : Manually_							
Additional Comments:		Soil Description/Pit Location/Project Detail/Weather							
Date Tested: 08/11/18		Gray, fine silty sand, silt (SM-ML)							

Time	Time	Dt	Inner Ring		Annular RING		Liquid Temp "F"	Infiltration Rats. I:**		Remarks
			Elev.: H(in)	AH (in)/Qf	Elev.: H(in)	.AH (in)/Q		Inner in/hr	Outer in/hr	
1 - Start	8:40	15.00	2.0	2.5	2.0	2.7	78 f	3.47	2.81	
End	8:55		4.5	196.35	4.7	477.0	78 f			
2 - Start	9:05	30.00	2.0	2.1	2.0	2.0	78 f	2.91	2.08	
End	9:35		4.1	164.93	4.0	353.4	78 f			
3 - Start	9:37	30.00	2.0	1.75	2.0	2.01	78 f	2.43	2.09	
End	10:07		3.75	137.44	4.0	355.2	78 f			
4 - Start	10:09	30.00	2.0	1.5	2.0	1.75	78 f	2.08	1.82	
End	10:39		3.5	117.81	3.75	309.2	78 f			
5 - Start	10:40	30.00	2.0	1.25	2.0	1.75	78 f	1.73	1.82	
End	11:10		3.25	98.17	3.75	309.2	78 f			
6 - Start	11:11	30.00	2.0	1.1	2.0	1.50	78 f	1.53	1.56	
End	11:41		3.1	86.40	3.5	265.0	78 f			
7 - Start	11:44	30.00	2.0	1.1	2.0	1.5	78 f	1.53	1.56	
End	12:14		3.14	86.40	3.5	265.0	78 f			
8 - Start	12:18	30.00	2.0	1.1	2.0	1.5	78 f	1.53	1.56	
End	12:48		3.1	86.40	3.5	265.0	78 f			

"Flow. Qf = AH x Vr-**" Infiltration I =(Qf/Ar)/At

Table 1 - Test Data Form for Double Ring Infiltrometer
 Test; Riverside County - Low Impact Development BMP Design
 Handbook

DOUBLE RING INFILTRMETER TEST DATA- TEST # T-2 (P-2)

SRI SAI RAM		TEMPLE		Constants-		Ring Data		Liquid Container—	
Location: 12594 ROSWELL AVE, CHINO, County of San Bernardino, California				Job #J&P 2018044 DRI		Area, A _r (in ²)	Depth of Liquid (in)	Vol.V _r (in ³ /in) #	
				Inner Ring: 12" D		113	18"	1-78.54	
Test By:	HM/GL	USCS class	SP/GP	Annular Space: 24" D		339	18"	2-176.7	
Water Table	Dept	Penetration of Rings into Soil (in.):				Inner:		Outer: 89 F	
Date Test:	Tape water used:	80F	pH:	Ground Temp (F): 83 f			at Depth: 12" /78 F		
Liquid Level Maintained by using:		(Flow valve <input type="checkbox"/> Float Valve <input type="checkbox"/> Mariotte Tube <input checked="" type="checkbox"/> Other : Manually							
Additional Comments:		Soil Description/Pit Location/Project Detail/Weather							
Date Tested: 08/11/18		Gray, fine silty sand, silt (SM-ML)							

Time	Time	Dt	Inner Ring		Annular RING		Liquid Temp "F"	Infiltration Rats. I>**		Remarks
			Elev. H(in)	AH (in)/Qf	Elev., H(in)	.AH (in)/Q		Inner in/hr	Outer in/hr	
I - Start	8:42	15.00	2.0	2.75	2.0	2.75	78f	3.82	2.87	
End	8:57		4.75	215.9	4.75	485.9	78f			
2 - Start	9.05	30.00	2.0	2.25	2.0	2.25	78f	3.12	2.34	
End	9.35		4.25	176.7	4.25	397.6	78f			
3 - Start	9:40	30.00	2.0	2.00	2.0	2.1	78f	2.78	2.19	
End	10:10		4.00	157.1	4.1	371.1	78f			
4 - Start	10:12	30.00	2.0	1.75	2.0	2.00	78f	2.43	2.08	
End	10: 42		3.75	137.4	4.00	353.4	78f			
5 - Start	10:45	30.00	2.0	1.5	2.0	1.75	78f	2.08	1.81	
End	11:15		3.5	117.8	3.75	309.2	78f			
6 - Start	11:18	30.00	2.0	1.25	2.0	1.50	78f	1.74	1.56	
End	11:48		3.25	98.2	3.5	265.1	78f			
7 - Start	11:50	30.00	2.0	1.1	2.0	1.50	78f	1.52	1.56	
End	12: 20		3.14	86.4	3.25	265.1	78f			
8 - Start	12: 25	30.00	2.0	1.1	2.0	1.50	78f	1.52	1.56	
End	12.55		3.1	86.4	3.25	265.1	78f			

"Flow. Qf = AH x Vr-***'Infiltration I=(Qf/Ar)/At

Table 1 - Test Data Form for Double Ring Infiltrometer
 Test; Riverside County - Low Impact Development BMP Design
 Handbook

CITY COUNTY SOIL ENGINEERING AND TESTING

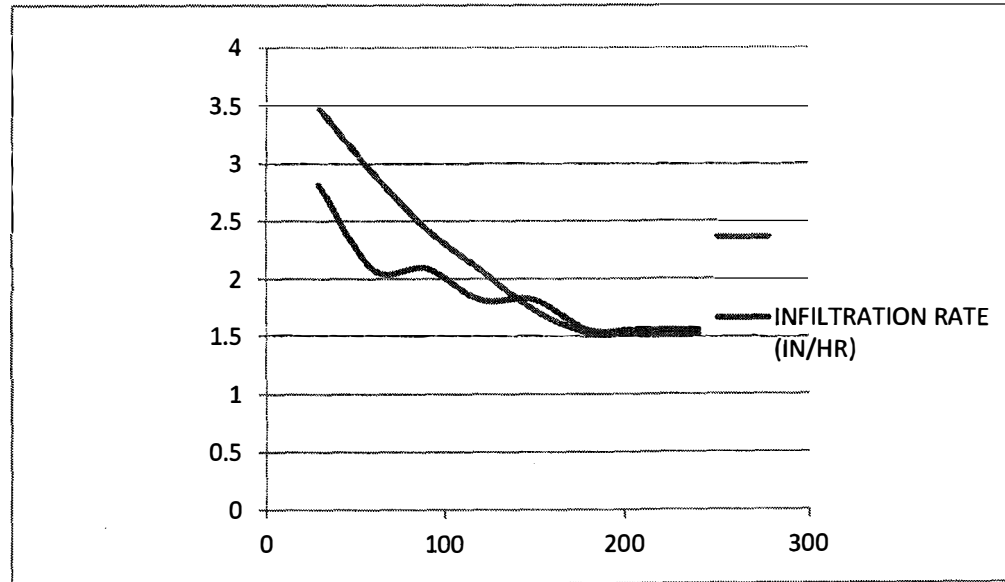
Job #J12018044P1-DRI

INFILTRATION CURVE (P-1)

Site: 12594 ROSWELL AVE., CHINO, CA 91710

ELAPSED TIME (MIN) INFILTRATION RATE (IN/HR)

30	3.47	2.81
60	2.91	2.08
90	2.43	2.09
120	2.08	1.82
150	1.73	1.82
180	1.53	1.56
210	1.53	1.56
240	1.53	1.56



CITY COUNTY SOIL ENGINEERING AND TESTING

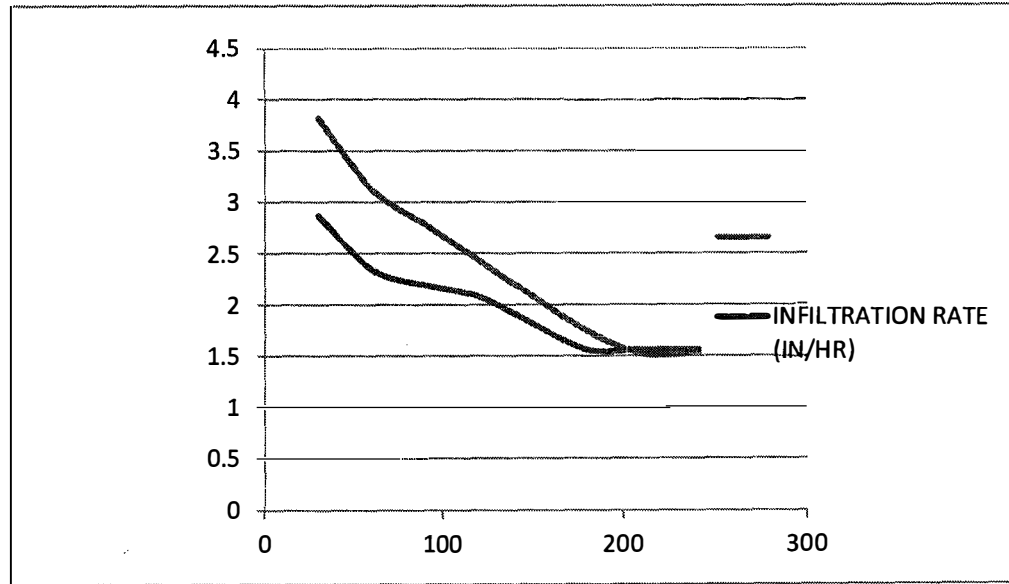
Job #J12018044P1-DRI

INFILTRATION CURVE (P-2)

ELAPSED TIME (MIN) INFILTRATION RATE (IN/HR)

Site: 12594 ROSWELL AVE., CHINO, CA 91710

30	3.82	2.87
60	3.12	2.34
90	2.78	2.19
120	2.43	2.08
150	2.08	1.81
180	1.74	1.56
210	1.52	1.56
240	1.52	1.56



Major division	Group symbols	Typical names	Classification criteria		
Coarse-grained soils more than 50% retained on No. 200 sieve	Gravels 50% or more of coarse fraction retained on No. 4 sieve	GW	Well graded gravels and gravel sand mixtures, little or no fines	$C_u = D_{60}/D_{10}$ greater than 4 $C_z = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3	
		GP	Poorly graded gravels and gravel sand mixtures, little or no fines	Not meeting both criteria for GW	
		GM	Silty gravels, gravel sand silt mixtures	Atterberg limits plot below A-line or plasticity index less than 4	
		GC	Clayey gravels, gravel sand-clay mixtures	Atterberg limits plot above A-line and plasticity index greater than 7	
	Sands more than 50% of coarse fraction passes No. 4 sieve	Clean sands	SW	Well-graded sands and gravelly sands, little or no fines	$C_u = D_{60}/D_{10}$ greater than 6 $C_z = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3
			SP	Poorly graded sands and gravelly sands, little or no fines	Not meeting both criteria for SW
		Sands with fines	SM	Silty sands, sand silt mixtures	Atterberg limits plot below A-line of plasticity index less than 4
			SC	Clayey sands, sand-clay mixtures	Atterberg limits plot above A-line and plasticity index greater than 7
			Classification on basis of percentage of fines GW, GP, SW, SP GM, GC, SM, SC Borderline classification requiring use of dual symbol		
			Less than 5% pass No. 200 sieve More than 12% pass No. 200 sieve 5% to 12% pass No. 200 sieve		

For soils with more than 50% passing No. 200 sieve, classify on Casagrande chart and highly organic soil is PT (peat) →

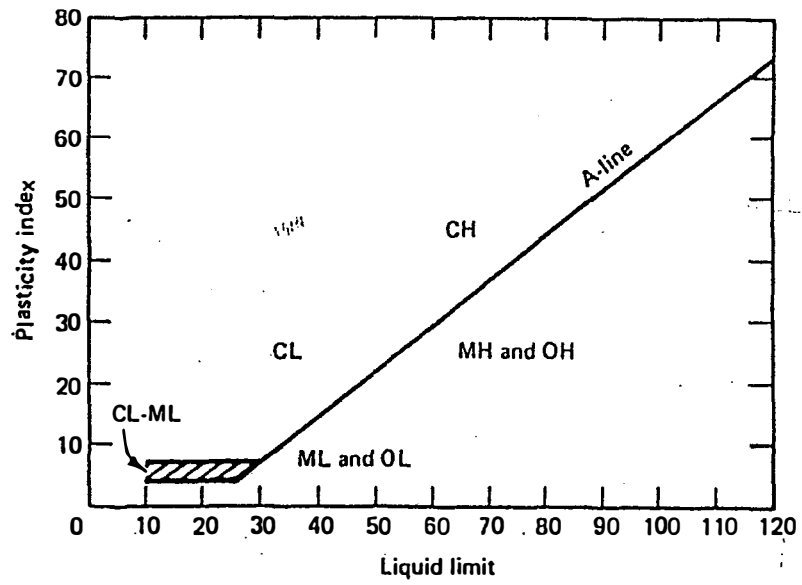





Figure 3-9 Unified classification tabular chart. (Courtesy A. Casagrande, *Classification and Identification of Soils*, Trans. ASCE 1:13, 1948.)

BORING LOG NO. B-1 (P-1)

Job # J&P2018037P1		08/12/18		Client: SRI SAI RAM MANDIR		
Hole Diameter: 8"		Elev. G.L.		Location: 12594 ROSWELL AVE., CHINO, CA		
Sampling Method		Drive Wt. 140#		CME-45		Logged By: ZB
Drop: 30"		Drilling Co: GEOMAT DRILLING, RIVERSIDE, CA				
Dry Density (pcf)	Depth (ft)	# of Blows (ft)	Moist. (%)	Sample Type	Soil Class	Earth Materials Description Top Soil: Dense grass-vegetation-12"
					SM	Dark gray, fine silty sand, grass, vegetation, roots s. moist very loose
99.1		38	12.0	■ ●	SM	Brown, fine silty sand, poorly graded, v. moist, med. loose dense, 27% passing #200 sieve
	5					
116.4		37	1.7	■ ●	SM	Olive gray, fine silty sand, poorly graded, s. moist, med. dense,
	10					
		8		◆	ML	Olive gray, fine sandy clayey silt, moist, stiff
	15					
		22		◆	SM	Olive gray, fine silty sand, poorly graded, moist, medium dense
	20					
		19		◆	ML	Olive gray, fine sandy clayey silt, moist, very tiff
	25					
		15	8.0	◆	SM	Olive gray, fine silty sand, poorly graded, moist, medium dense, 38% passing #200 sieve
	30					


 Undisturbed Ring Sample
 Bulk Sample
 Standard Penetration Test

City & County

**Soil Engineering
And Testing**




BORING LOG NO. B-2

Job # J&P2018037P1		08/12/18		Client: SRI SAI RAM MANDIR		
Hole Diameter: 8"		Elev. G.L.		Location: 12594 ROSWELL AVE., CHINO, CA		
Sampling Method		Drive Wt. 140#		CME-45		Logged By: ZB
Drop: 30"				Drilling Co: GEOMAT DRILLING, RIVERSIDE, CA		
Dry Density (pcf)	Depth (ft)	# of Blows (ft)	Moist. (%)	Sample Type	Soil Class	Earth Materials Description Top Soil: Dense grass-vegetation-12"
					SM	Dark gray, fine silty sand, grass, vegetation, roots s. moist very loose
	5	46		■ ●	SM	Light brown, fine to coarse silty sand, few gravel, s. moist, medium dense, 27% passing #200 sieve
104.0	10	17	7.7	■ ●	SM	Light brown, fine silty sand, moist, med. dense 27% passing #200 sieve
	15	9		◆	ML	Olive gray, fine sandy clayey silt, moist, stiff
	20	13		◆	ML	Olive gray, fine sandy clayey silt, moist, stiff
	25	20		◆	ML	Olive gray, fine sandy clayey silt, moist, very stiff
						<i>End of Boring @ 25 feet Depth</i>
						<i>No Groundwater Encountered</i>
						<i>Boring Backfilled</i>


 Undisturbed Ring Sample
 Bulk Sample
 Standard Penetration Test

BORING LOG NO. B-3

Job # J&P2018037P1		08/12/18	Client: SRI SAI RAM MANDIR			
Hole Diameter: 8"		Elev. G.L.	Location: 12594 ROSWELL AVE., CHINO, CA			
Sampling Method		Drive Wt. 140#	CME-45	Logged By: ZB		
Drop: 30"			Drilling Co: GEOMAT DRILLING, RIVERSIDE, CA			
Dry Density (pcf)	Depth (ft)	# of Blows (ft)	Moist. (%)	Sample Type	Soil Class	Earth Materials Description Top Soil: Dense grass-vegetation-12"
					SM	Dark gray, fine silty sand, grass, vegetation, roots s. moist very loose
	5	31		■ ●	SM	Brown, fine silty sand, poorly graded, v. moist, med. dense, 27% passing #200 sieve
	10	14		■ ●	ML	Olive gray, fine sandy clayey silt, moist, stiff
	15	9		◆	ML	Olive gray, fine sandy clayey silt, moist, stiff
	20	13		◆	ML	Olive gray, fine sandy clayey silt, moist, stiff dense
	25	14		◆	ML	Olive gray, fine sandy clayey silt, moist, stiff
	30	17		◆	ML	Olive gray, fine sandy clayey silt, moist, very stiff

 Undisturbed Ring Sample
 Bulk Sample
 Standard Penetration Test

City & County

**Soil Engineering
And Testing**

BORING LOG NO. B-3 (Continuous from 30')

Job # J&P2018037P1		08/12/18	Client: SRI SAI RAM MANDIR			
Hole Diameter: 8"		Elev. G.L.	Location: 12594 ROSWELL AVE., CHINO, CA			
Sampling Method		Drive Wt. 140#	CME-45	Logged By: ZB		
Drop: 30"			Drilling Co: GEOMAT DRILLING, RIVERSIDE, CA			
Dry Density (pcf)	Depth (ft)	# of Blows (ft)	Moist. (%)	Sample Type	Soil Class	Earth Materials Description Top Soil: Dense grass-vegetation-12"
	30	17		◆	ML	Olive gray, fine sandy clayey silt, moist, very stiff
	35	13		■ ◆	ML	Olive gray, fine sandy clayey silt, moist, stiff <i>dense, 27% passing #200 sieve</i>
	40	11		■ ◆	ML	Olive gray, fine sandy clayey silt, moist, stiff
						<i>End of Boring @ 40 feet Depth</i>
						<i>No Groundwater Encountered</i>
						<i>Boring Backfilled</i>
				◆		

●	Undisturbed Ring Sample
■	Bulk Sample
◆	Standard Penetration Test

City & County

Soil Engineering
And Testing

BORING LOG NO. B-4

Job # J&P2018037P1		08/12/18		Client: SRI SAI RAM MANDIR		
Hole Diameter: 8"		Elev. G.L.		Location: 12594 ROSWELL AVE., CHINO, CA		
Sampling Method		Drive Wt. 140#		CME-45		Logged By: ZB
Drop: 30"		Drilling Co: GEOMAT DRILLING, RIVERSIDE, CA				
Dry Density (pcf)	Depth (ft)	# of Blows (ft)	Moist. (%)	Sample Type	Soil Class	Earth Materials Description Top Soil: Dense grass-vegetation-12"
					SM	Dark gray, fine silty sand, grass, vegetation, roots s. moist very loose
		28		■ ●	SM	Lt. brown, fine silty sand, poorly graded, s. moist, med. dense,
	5					
98.4		33	4.4	■ ●	SM	Olive gray, fine silty sand, poorly graded, s. moist, med. <i>dense, 23% passing #200 sieve</i>
	10					
		13	17.8	◆	ML	Olive gray, fine sandy clayey silt, moist, stiff <i>57% passing #200 sieve</i>
	15					
		21		◆	SM	Olive gray, fine silty sand, poorly graded, moist, medium dense
	20					
		24	7.3	◆	SM	Olive gray, fine silty sand, poorly graded, moist, medium <i>Dense, 33% passing #200 sieve</i>
	25					<i>End of Boring @ 25 feet Depth</i>
						<i>No Groundwater Encountered</i>
						<i>Boring Backfilled</i>






● Undisturbed Ring Sample
 ■ Bulk Sample
 ◆ Standard Penetration Test

City & County

**Soil Engineering
And Testing**

BORING LOG NO. B-5 (P-2)

Job # J&P2018037P1		08/12/18		Client: SRI SAI RAM MANDIR		
Hole Diameter: 8"		Elev. G.L.		Location: 12594 ROSWELL AVE., CHINO, CA		
Sampling Method		Drive Wt. 140#		CME-45		Logged By: ZB
Drop: 30"		Drilling Co: GEOMAT DRILLING, RIVERSIDE, CA				
Dry Density (pcf)	Depth (ft)	# of Blows (ft)	Moist. (%)	Sample Type	Soil Class	Earth Materials Description Top Soil: Dense grass-vegetation-12"
					SM	Dark gray, fine silty sand, grass, vegetation, roots s. moist very loose
	5	21		■ ●	SM	Brown, fine silty sand, poorly graded, v. moist, medium dense, 27% passing #200 sieve
91.6	10	14	16.8	■ ●	ML	Olive gray, fine sandy clayey silt, moist, stiff
101.0	15	21	16.0	■ ●	SM	Olive gray, fine silty sand, poorly graded, moist, medium dense
	20	9		◆	ML	Olive gray, fine sandy clayey silt, moist, stiff
	25	21		◆	SM	Olive gray, fine silty sand, poorly graded, moist, medium dense
	30	29		◆	SM	Olive gray, fine silty sand, poorly graded, moist, dense

 Undisturbed Ring Sample
 Bulk Sample
 Standard Penetration Test

City & County

**Soil Engineering
And Testing**

BORING LOG NO. B-1 (continuous from 30')

Job # J&P2018037P1		08/12/18	Client: SRI SAI RAM MANDIR			
Hole Diameter: 8"		Elev. G.L.	Location: 12594 ROSWELL AVE., CHINO, CA			
Sampling Method	Drive Wt. 140#	CME-45		Logged By: ZB		
Drop: 30"	Drilling Co: GEOMAT DRILLING, RIVERSIDE, CA					
Dry Density (pcf)	Depth (ft)	# of Blows (ft)	Moist. (%)	Sample Type	Soil Class	Earth Materials Description Top Soil: Dense grass-vegetation-12"
	30				SM	Dark gray, fine silty sand, grass, vegetation, roots s. moist very loose
		16	15.7	◆	ML	Olive gray, fine sandy clayey silt, moist, very stiff 53% passing #200 sieve
	35					
		12	20.0	◆	ML	Olive gray, fine sandy clayey silt, moist, stiff 58% passing #200 sieve
	40					
						<i>End of Boring @ 40 feet Depth</i>
						<i>No Groundwater Encountered</i>
						<i>Boring Backfilled after Percolation Testing</i>

● Undisturbed Ring Sample
■ Bulk Sample
◆ Standard Penetration Test

BORING LOG NO. B-5 (continuous from 30')

Job # J&P2018037P1		08/12/18		Client: SRI SAI RAM MANDIR		
Hole Diameter: 8"		Elev. G.L.		Location: 12594 ROSWELL AVE., CHINO, CA		
Sampling Method	Drive Wt. 140#		CME-45		Logged By: ZB	
Drop: 30"				Drilling Co: GEOMAT DRILLING, RIVERSIDE, CA		
Dry Density (pcf)	Depth (ft)	# of Blows (ft)	Moist. (%)	Sample Type	Soil Class	Earth Materials Description Top Soil: Dense grass-vegetation-12"
	30	29		◆	SM	Olive gray, fine silty sand, poorly graded, moist, dense
	35	9		◆	ML	Olive gray, fine sandy clayey silt, moist, stiff
						53% passing #200 sieve
	40	21		◆	ML	Olive gray, fine sandy clayey silt, moist, stiff
						58% passing #200 sieve
						End of Boring @ 40 feet Depth
						No Groundwater Encountered
						Boring Backfilled after Percolation Testing



Undisturbed Ring Sample
 Bulk Sample
 Standard Penetration Test

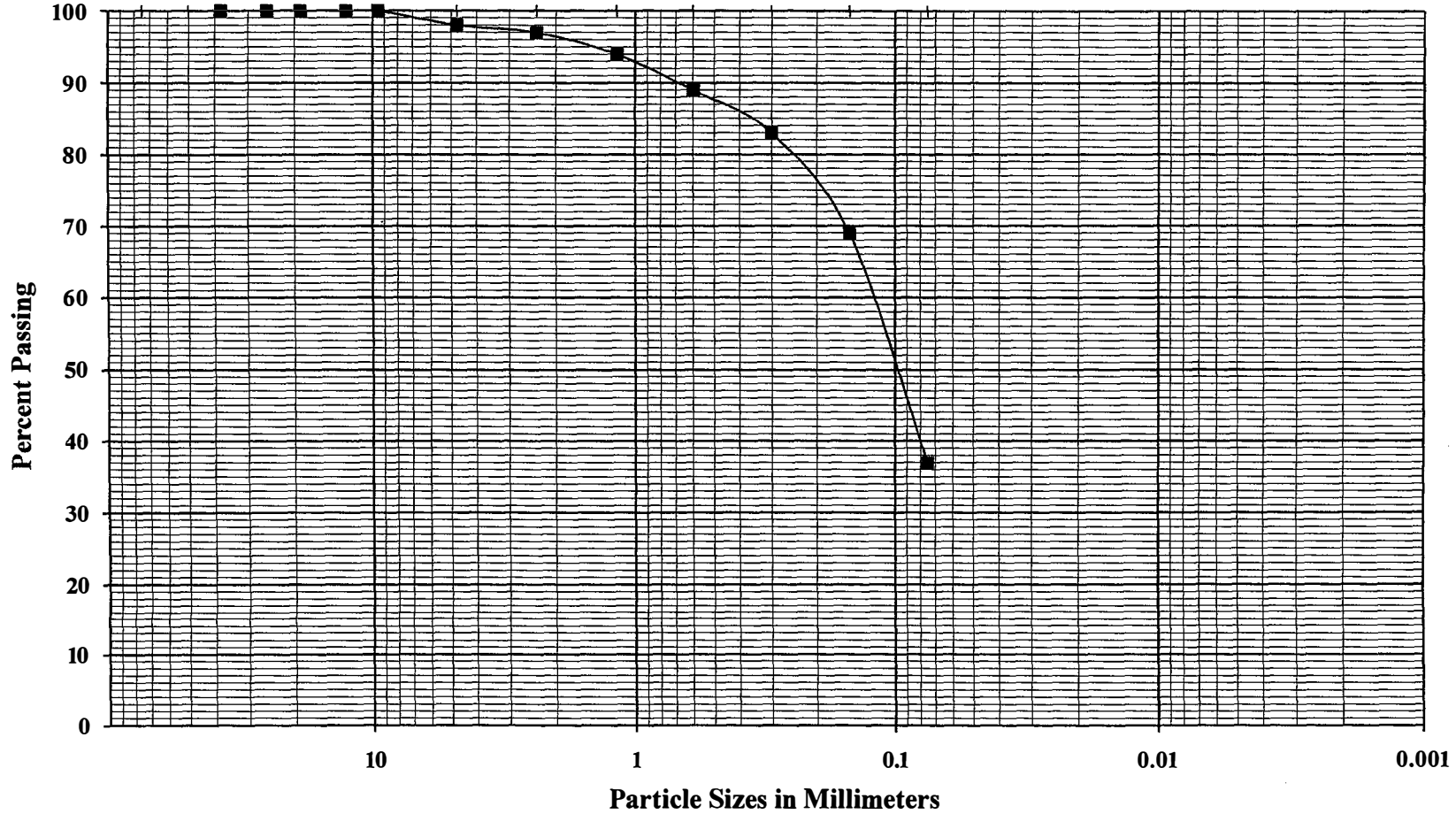
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And Testing**

GRAVEL		SAND			SILT & CLAY
COARSE	FINE	COARSE	MEDIUM	FINE	

3" 2" 1.5" 1" 3/4" 1/2" 3/8" #4 #8 #16 #30 #50 #100 #200

← U.S. Standard Sieves



Sample Identification: T-1 @ -5'

Soil Type: Olive gray, fine silty sand (SM)

Location: 12954 ROISWELL AVE., CHINO, CA

4.90%

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GRAIN SIZE DISTRIBUTION CURVE

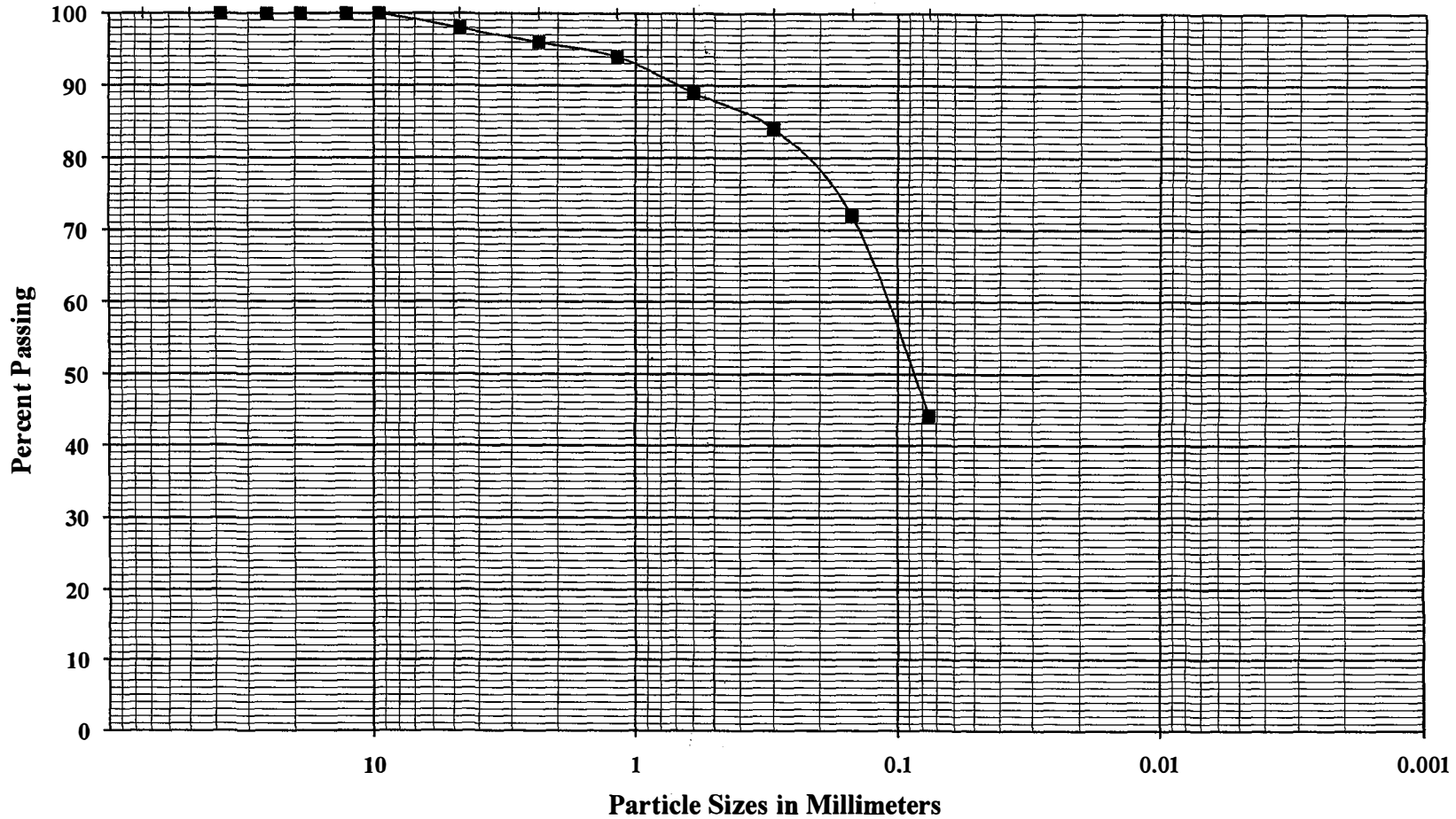
SRI SAI RAM MANDIR

PROJECT No.
J&P2018037P1

GRAVEL		SAND			SILT & CLAY
COARSE	FINE	COARSE	MEDIUM	FINE	

3" 2" 1.5" 1" 3/4" 1/2" 3/8" #4 #8 #16 #30 #50 #100 #200

← U.S. Standard Sieves



Sample Identification: T-2 @ -8'

Soil Type: Olive gray, fine silty sand (SM)

Location: 12954 ROISWELL AVE., CHINO, CA

5.20%

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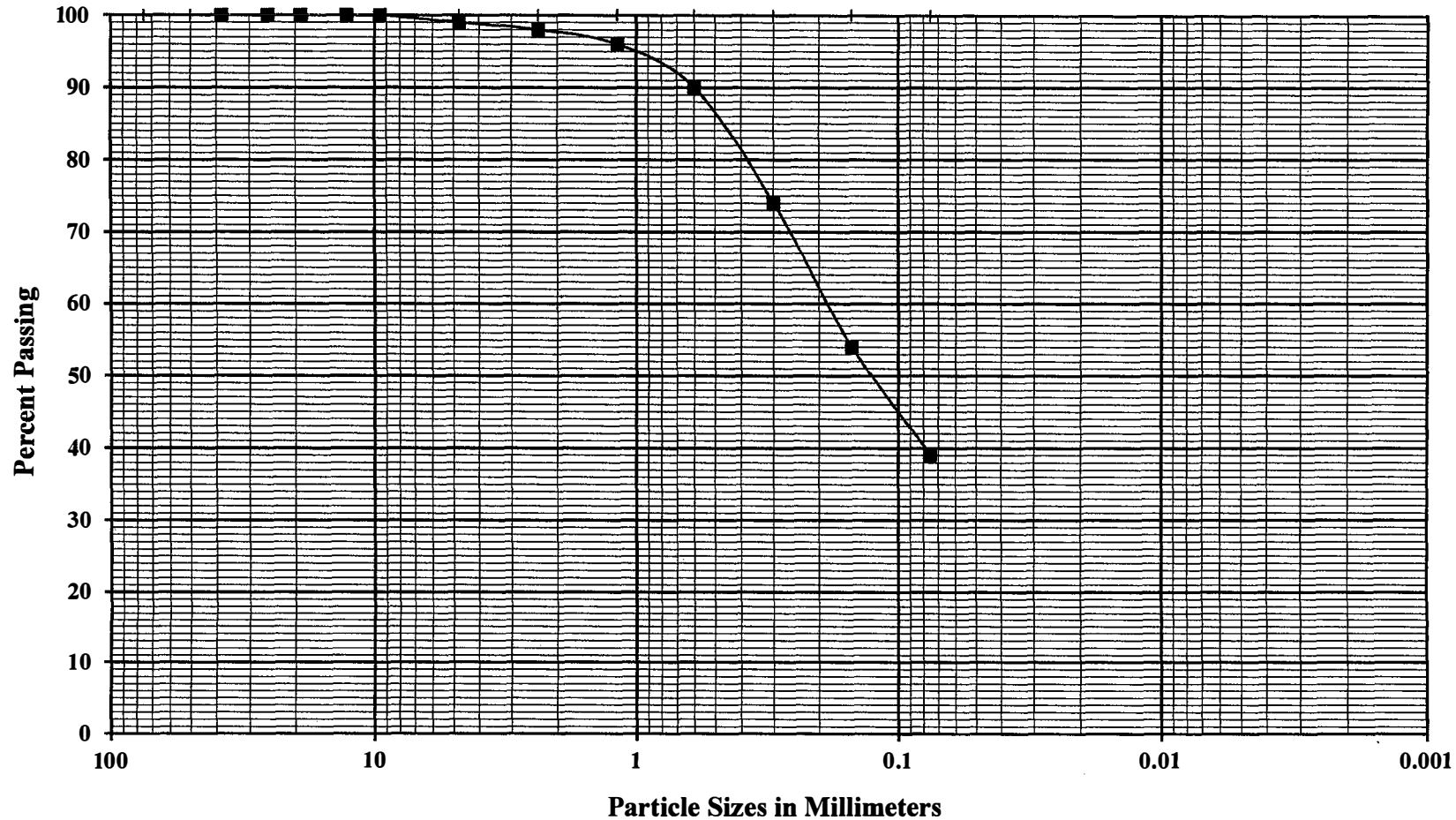
GRAIN SIZE DISTRIBUTION CURVE

SRI SAI RAM MANDIR

PROJECT No.
J&P2018037P1

GRAVEL		SAND					SILT & CLAY
COARSE	FINE	COARSE	MEDIUM	FINE			

3" 2" 1.5" 1" 3/4" 1/2" 3/8" #4 #8 #16 #30 #50 #100 #200 ← U.S. Standard Sieves



Sample Identification: B-1 (P-1) @ -28'

Soil Type: Olive gray, fine silty sand (SM)

Location: 12954 ROISWELL AVE., CHINO, CA

8.00%

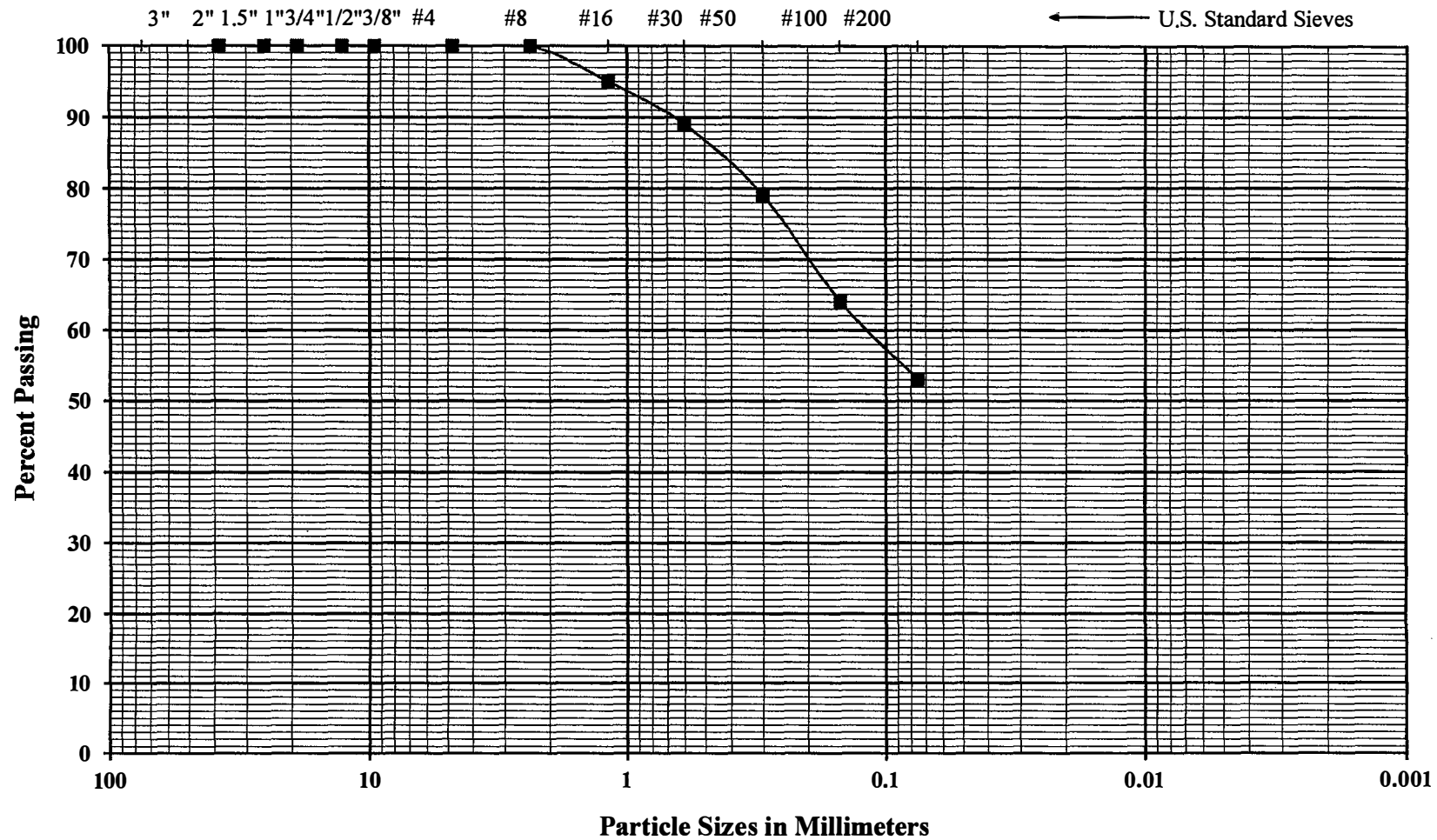
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And Testing

GRAIN SIZE DISTRIBUTION CURVE

SRI SAI RAM MANDIR

PROJECT No.
J&P2018037P1

GRAVEL		SAND			SILT & CLAY
COARSE	FINE	COARSE	MEDIUM	FINE	



Sample Identification: B-1 (P-1) @ -33'

Soil Type: Brown, fine sandy clayey silt (ML)

Location: 12954 ROISWELL AVE., CHINO, CA

15.70%

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GRAIN SIZE DISTRIBUTION CURVE

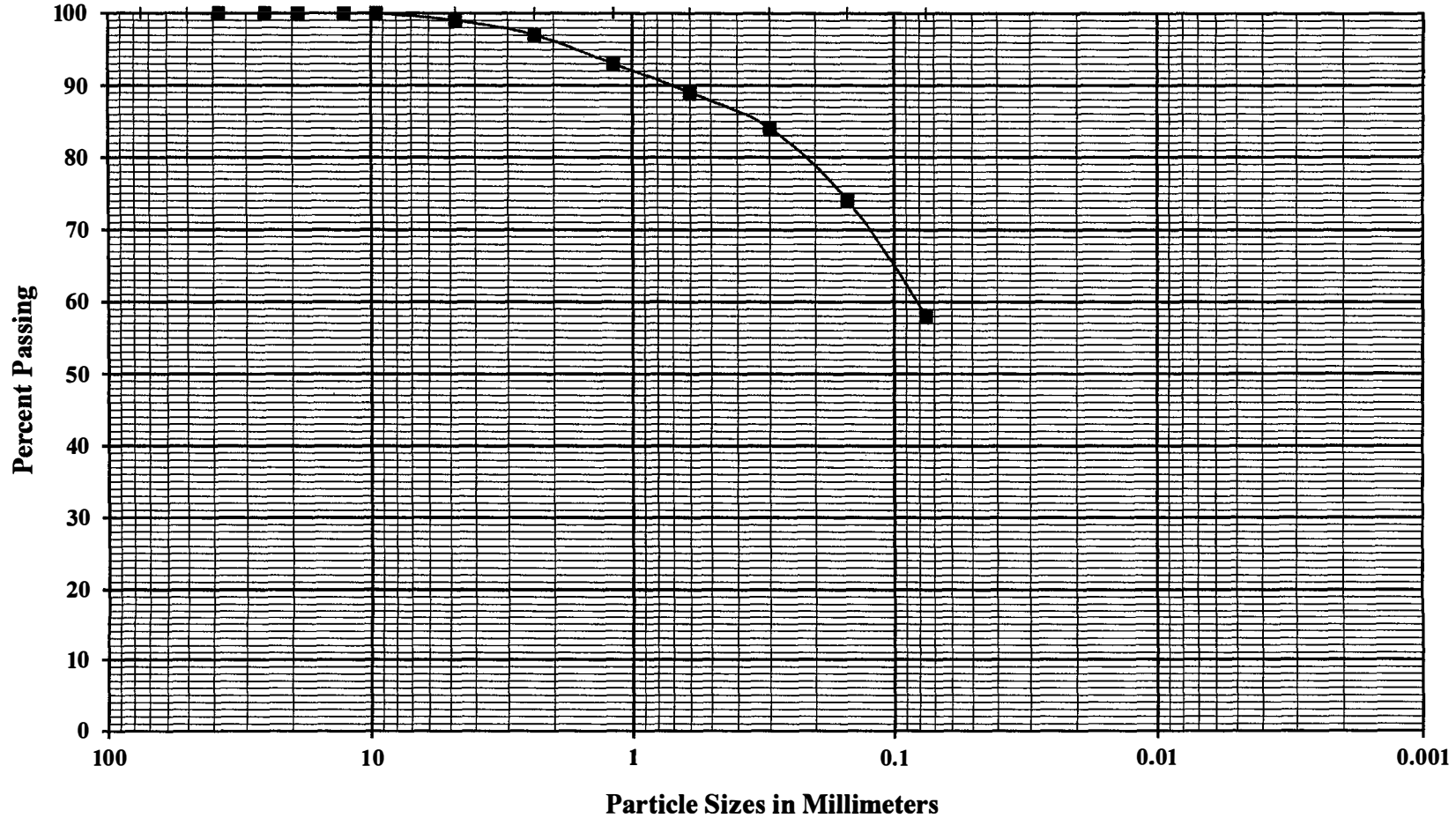
SRI SAI RAM MANDIR

PROJECT No.
J&P2018037P1

GRAVEL		SAND			SILT & CLAY
COARSE	FINE	COARSE	MEDIUM	FINE	

3" 2" 1.5" 1" 3/4" 1/2" 3/8" #4 #8 #16 #30 #50 #100 #200

← U.S. Standard Sieves



Sample Identification: B-1 (P-1) @ -38'

Soil Type: Brown, fine sandy clayey silt (ML)

Location: 12954 ROISWELL AVE., CHINO, CA

20.00%

|

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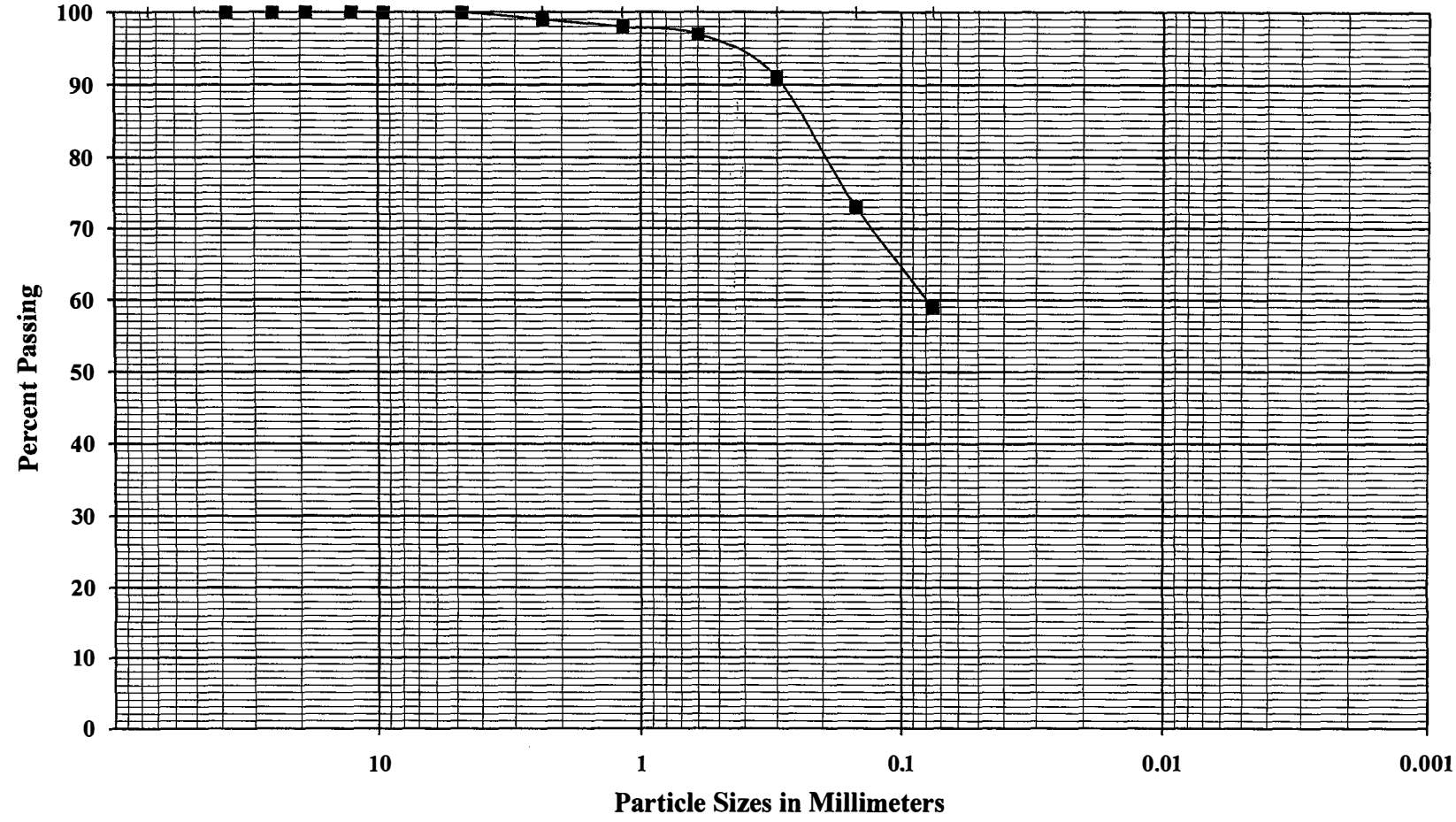
GRAIN SIZE DISTRIBUTION CURVE

SRI SAI RAM MANDIR

PROJECT No.
J&P2018037P1

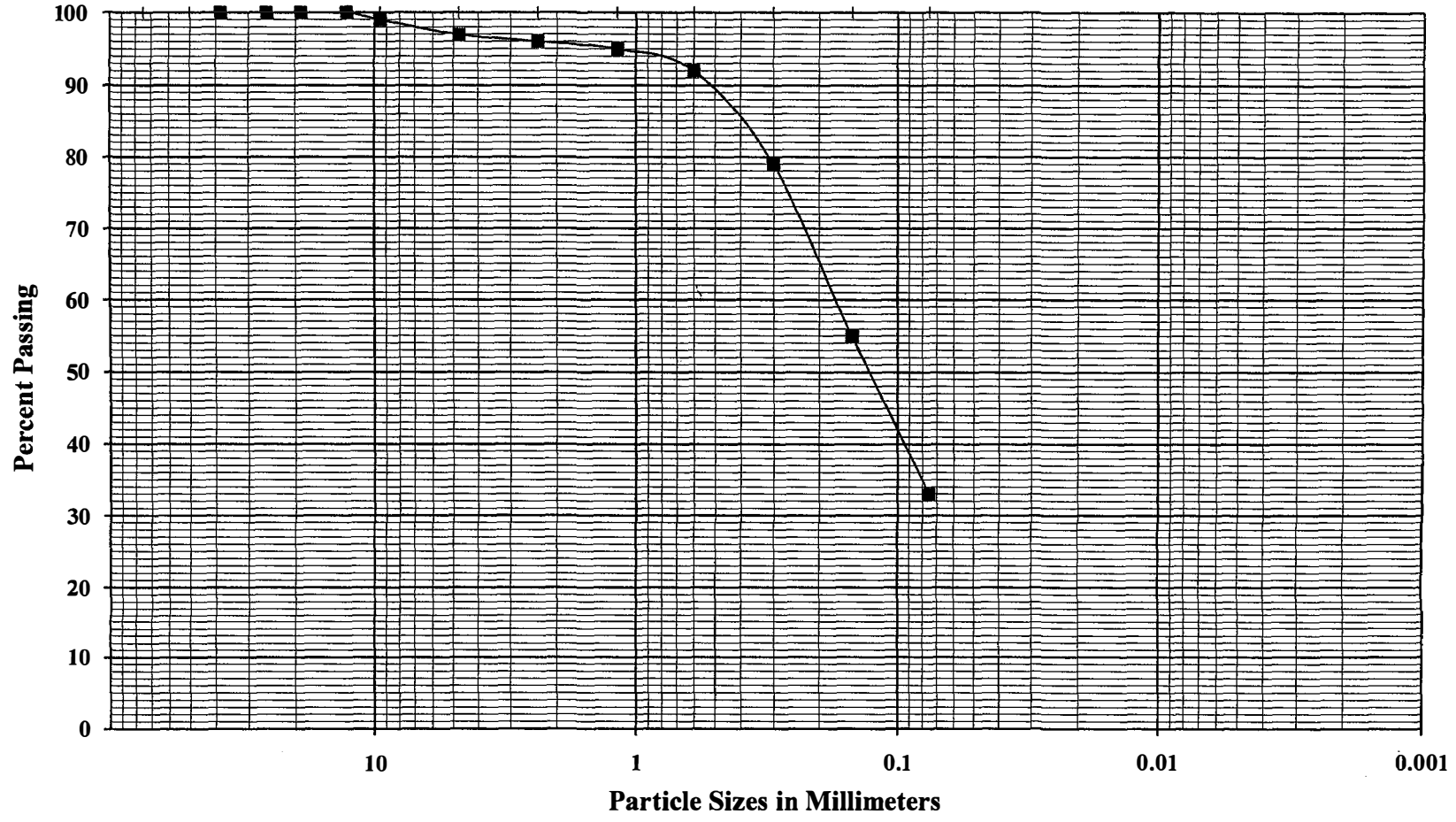
GRAVEL		SAND			SILT & CLAY
COARSE	FINE	COARSE	MEDIUM	FINE	

3" 2" 1.5" 1" 3/4" 1/2" 3/8" #4 #8 #16 #30 #50 #100 #200 ← U.S. Standard Sieves



GRAVEL		SAND			SILT & CLAY
COARSE	FINE	COARSE	MEDIUM	FINE	

3" 2" 1.5" 1" 3/4" 1/2" 3/8" #4 #8 #16 #30 #50 #100 #200 ← U.S. Standard Sieves



Sample Identification: B-4 @ -24'
 Location: 12954 ROISWELL AVE., CHINO, CA

Soil Type: Olive gray, fine silty sand (SM)
 7.30%

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GRAIN SIZE DISTRIBUTION CURVE
 SRI SAI RAM MANDIR

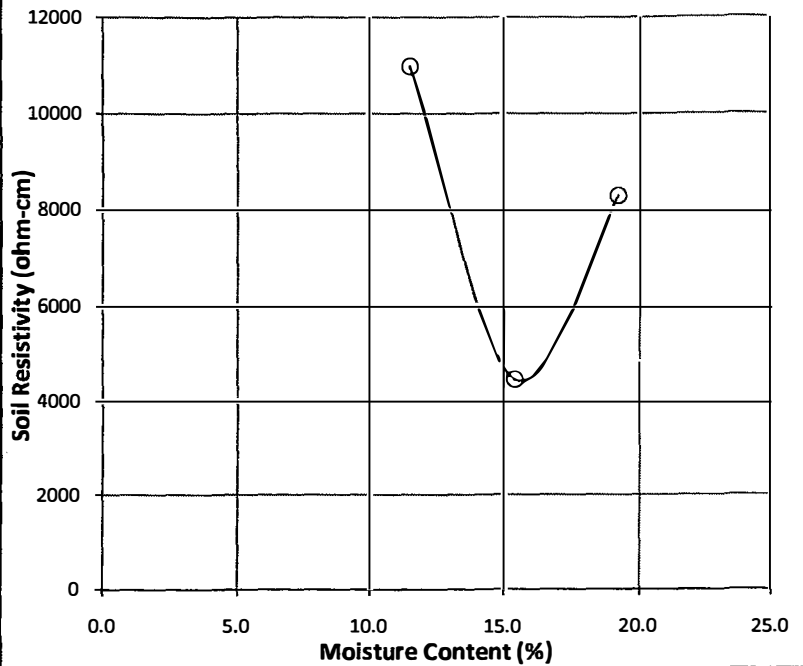
PROJECT No.
 J&P2018037P1

Soil Resistivity, Soluble Sulfate, Soluble Chloride, pH

Project Name:
Project No.:
Sample ID:
Soil Classification:

Sample Collected:
Collected By:
Sample Tested:
Tested by:

Specimen No.	1	2	3	4
Soil Box Constant (cm)	1.00	1.00	1.00	
Water Added (ml)	15.0	5.0	5.0	
Moisture (%)	11.5	15.4	19.2	
Meter Dial Reading	11.0	4.5	8.3	
Multilier Setting (ohm)	1K	1K	1K	
Resistance (ohm)	11000	4500	8300	



Minimum Resistivity (ohm-cm)	4500
Temperature (°C)	18.2
$R_{min, 15.5} = [R_{min-T} * (24.5+T)] / 40$	4803.75

Water increment: 100-150 ml for large box and 5-15 ml for small box

Resistivity = Resistance X Soil Box Constant

Large Soil Box Constant = 6.67 cm

Small Soil Box Constant = 1.00 cm

Rmin 15.5 Corrected Minimum Resistivity to

Standard Ground Temperature of 15.5°C

Soil Corrosivness	Resistivity (ohm-cm)
-------------------	----------------------

Very Severely Corrosive 0 - 900

Severely Corrosive 900 - 2,300

Moderately Corrosive 2,300 - 5,000

Mildly Corrosive 5,000 - 10,000

Very Mildly Corrosive 10,000 - 100,000

Reference: ASTM STP 1013 Titled "Effects of Soil Characteristics on Corrosion" (February, 1989).

Mixing Ratio	Dilution Factor	Sulfate Reading (ppm)	Sulfate Content		Chloride Reading (ppm)	Chloride Content		pH
			ppm	%		ppm	%	
3	1	125	375	0.0375	1	5	0.0005	6.75
Average					Average			

ACI 318-05 Table 4.3.1 Requirements for Concrete Exposed to Sulfate-Containing Solutions

Sulfate Exposure	Water-Soluble Sulfate (SO ₄) in Soil, % by Mass	Sulfate (SO ₄) in Water, ppm	Cement Type	Maximum w/cm by Mass	Minimum Design Compressive Strength f _c , Mpa (psi)
Negligible	< 0.10	< 150	No Special Type	--	--
Moderate (See Water)	0.10 to 0.20	150 to 1500	II IP(MS), IS(MS), P(MS), I(PM)(MS), I(SM)(MS)	0.5	28 (4000)
Severe	0.20 to 2.00	1,500 to 10,000	V	0.45	31 (4500)
Very Severe	> 2.00	> 10,000	V + pozz	0.45	31 (4500)

Caltrans classifies a site as corrosive to structural concrete as an area where soil and/or water contains > 500 ppm chloride, > 2000 ppm sulfate, or has a pH < 5.5. A minimum resistivity of less than 1000 ohm-cm indicates the potential for corrosive environment requiring testing for the above criteria.

The 2007 CBC Section 1904A references ACI 318 for material selection and mix design for reinforced concrete dependant on the onsite corrosion potential, soluble sulfate content, and soluble chloride content in soil.

Comments: Sec. 4.3 of ACI 318 (2005) Soil environment is detrimental to concrete if it has soluble sulfate > 1000 ppm and/or pH < 5.5. Soil environment is corrosive to reinforcement and steel pipes if chloride ion > 500 ppm or pH < 4.0.

The information in this form is not intended for corrosion engineering design. If corrosion is critical, a corrosion specialist should be contacted to provide further recommendations.