



Appendix J

Preliminary Hydrology Report

PRELIMINARY HYDROLOGY REPORT



Lockhart Solar PV II

**San Bernardino County, CA
September 2021**

Prepared For: Lockhart Solar PV II, LLC

Prepared By: Fuscoe Engineering, Inc.

Job Number: 1859-003



PRELIMINARY HYDROLOGY REPORT

Lockhart Solar PV II

San Bernardino County, California

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For

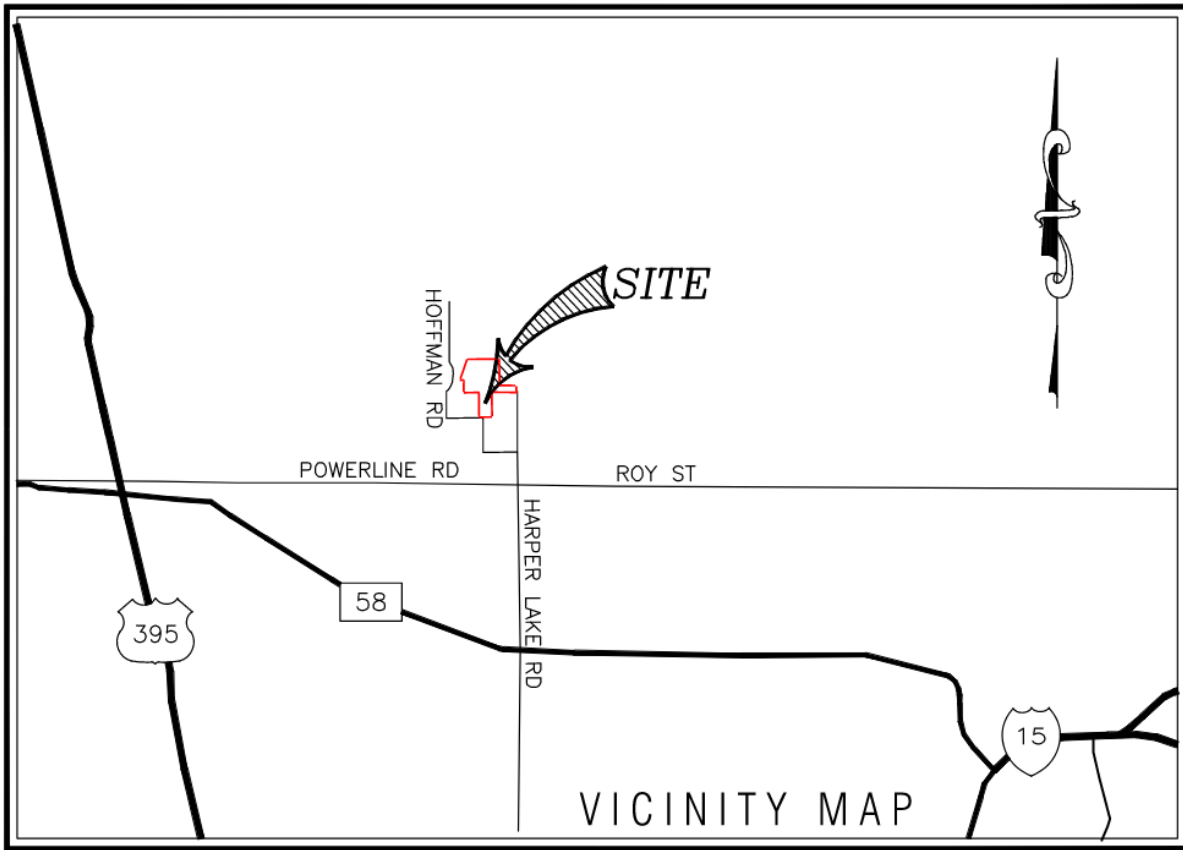
Lockhart Solar PV II, LLC

September 2021

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1. INTRODUCTION

1.1 Purpose

The purpose of this preliminary report is to provide an assessment of the hydrologic conditions of the land which will be used for the Lockhart Solar PV II Project (or Project). This study discusses the retention requirements for the Project and provides an overview of the off-site runoff contribution from adjacent/upstream areas and associated flow depth and velocity determination. Estimations of these numbers are required to properly position Project structures, to size flow mitigation structures, routing channels, and detention structures to ensure the Project can be adequately protected, and to route flow to its original locations so that downstream effects will be negligible.

1.2 Project Overview

The Project proposes to develop solar photovoltaic (PV) facilities, battery energy storage system (BESS), and associated infrastructure. The Project Site is in unincorporated Hinkley, CA, approximately 7 miles north of the intersection of Harper Lake Road and Mojave-Barstow Highway 58. The Project Site, as depicted in Figure 1, is made up of areas within three parcels, which consist of vacant, previously disturbed land, miscellaneous concrete foundations, various electrical lines and poles, as well as existing shared facilities within the Shared Facilities Area. The Project site is bordered on the south by the existing Solar Energy Generating System (SEGS) VIII and IX Solar Thermal Power Plants, which the County approved for repowering to PV solar and battery storage in 2019 as part of the Lockhart Solar Facility (CUP Project #2019001245 approved in 2019); Harper Lake Road to the east; Hoffman Road to the west; and vacant land to the north.

While the Project's development footprint includes up to approximately 690 acres of land, for the purposes of this report, the Project Site/study area is defined as the SEGS X facility site boundary (approximately 600-acres) and the approximately 110-acre Shared Facilities Area, and excludes any areas outside the existing SEGS X property fence line.

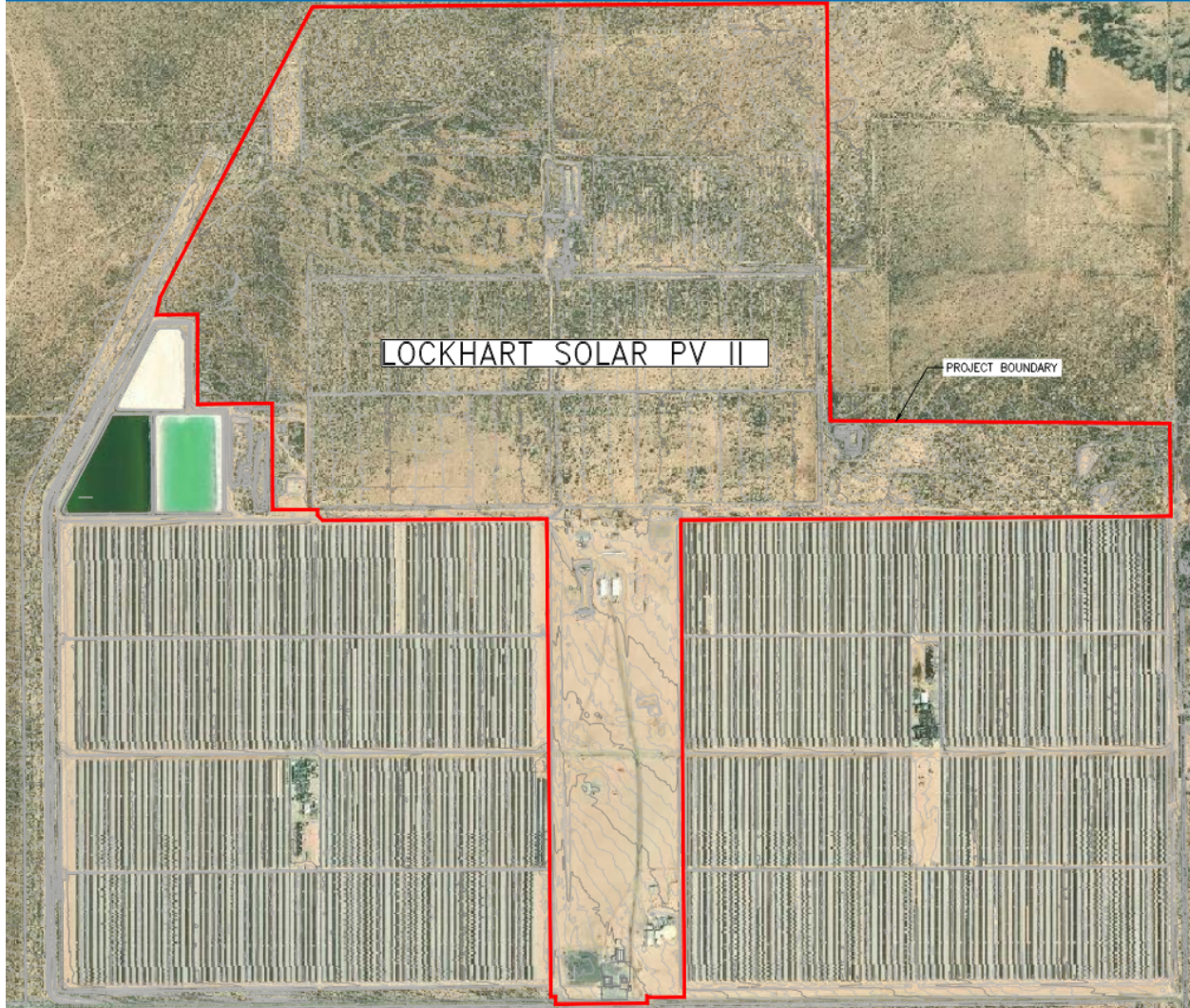
The Project Site, as depicted in Figure 1, will be considered as the study area for this report (approximately 710 acres).

According to the Federal Emergency Management Agency (FEMA) Panel Number 06071C3250H, dated 8/28/2008 the study area resides within a designation of FEMA Flood Zone D. Zone D is defined as "undetermined Flood Hazard" per the Flood Insurance Rate Map (FIRM) panel. This indicates that a formal hydrologic and hydraulic study for the area has not been completed. The area has not been mapped and approved by FEMA with floodplains or floodways.

Flood hazards are undermined in this area. Base Flood Elevations (BFEs) are not provided in D Zones. Flood insurance for properties is not required at the Federal level in Zone D areas. Refer to Appendix 1 for FEMA FIRMette.

The potential hydrologic issues in this general landscape are flooding and erosive velocities. This study will discuss the results of analyzing pre-construction hydraulic conditions associated with flow depth, and flow velocity across the study area.

Figure 1. Project Site/Study Area



1.3 Study Area Description

The majority of the study area is located on rural, undeveloped land. Stormwater runoff enters the study area from southern and western boundary and exits the study area along the northern and eastern boundary. There is an existing earthen berm outside the western boundary of the study area that protects a major portion of the study area by diverting the offsite flow to the northwest corner. The existing berm does not currently extend along the full length of the western boundary of the study area. Flow from the berm confluences with offsite flow coming from the north and begins to pond just outside the northeast corner of the site within the dry lake bed of Harper Lake. The flow from the south crosses the southerly portion of the study area and concentrates at the eastern boundary within Harper Lake.

As seen in Google Earth, there is some evidence of channelization, but most flow is expected to be via shallow overland flow.

2. METHODOLOGY

2.1 Flow Analysis Methodology

Due to the complex and distributary nature of flow paths upstream and through the study area, a FLO-2D model is utilized to determine flow depth and velocities through the study area.

FLO-2D (FLO-2D Software, Inc.) is a dynamic flood routing model that simulates channel flow, unconfined overland flow, and street flow. It can simulate a flood over complex topography and roughness while reporting on volume conservation – the key to accurate flood distribution.

The FLO-2D Grid Developer System (GDS) is a GIS integrated software tool used to facilitate the creation of all required data to run the FLO-2D model. GDS simplifies construction of the finite difference grid system and allows graphical editing of its attributes and components. The GDS will overlay the grid system on a digital terrain map or a Digital Terrain Model (DTM) set of points and will interpolate and assign elevations to the grid elements. It will establish geographical boundaries, import background images and photos, and it will automatically generate the data files required to run the model.

In this study, the offsite hydrology was modeled with Advanced Engineering Software (AES-2016). Runoff hydrographs from AES were used as boundary conditions for FLO-2D Pro. Hydrographs were developed for the 100-year 24-hour rainfall event in accordance with the San Bernardino County Hydrology Manual (SBCHM). Computational flow modeling occurred for a return period rainfall event and results can be seen in Appendix 2 and in Appendix 4.

United States Geological Survey (USGS) StreamStats analysis has been utilized to define the watershed and offsite sub-watershed delineation. The watershed extent and flow channels were compared to Google Earth images for verification. See Offsite Hydrology Map in Appendix 4 for sub-watershed delineation. AES software was used to produce the rainfall hydrographs for each inflow location shown in Appendix 4.

For Flood Routing Analysis the Link-Node Runoff Hydrograph Model for San Bernardino County has been utilized. To construct the synthetic critical storm pattern, area-averaged watershed rainfall values from National Oceanic and Atmospheric Administration's (NOAA) Atlas 14 precipitation data for each sub basin was entered into AES. Lag value was computed per Corps of Engineering LAG formula. Basin factor (Manning's coefficient) $n=0.05$, adjusted loss rate 0.4 in/hr and low loss fraction coefficient 0.9 have been used as an average value for all sub basins.

Two rainfall hydrograph tables have been extracted from AES output files and utilized as inflow points in FLO-2D software.

2.2 Hydrology Study Standards

The requirements and recommendations found in the SBCHM (August 1986) was used as the basis for the methodology and proposed retention storage volume calculations.

The Project will include compacted native on-site access roads, PV arrays, electrical equipment areas, upgrade of substation and switchyard, battery storage and other miscellaneous improvements. These proposed improvements are expected to cause an increase in runoff leaving the study area.

Retention design elements such as retention basins are proposed to attenuate the outgoing flow. The proposed retention basins will satisfy the following conditions:

- Each basin has adequate tributary area to ensure the basin will completely fill during the 100-year, 24-hour storm event.
- Each retention basin will have capacity to retain the calculated proportionate volume increase of the subarea tributary to the basin.
- Each basin will decrease the calculated on-site post-developed outgoing flow compared with pre-developed flow.
- Each basin will completely draw-down within a 72-hour period.

To determine the increase in 100-year, 24-hour runoff volume, the study area was divided into 3 Drainage Management Areas (DMA's). These DMA's were analyzed just for on-site flows. Hydrograph calculations were performed for existing and proposed conditions for the 3 DMA's. Unit hydrograph calculation results have been used to determine the peak flow and rain flow volumes.

Advanced Engineering Software (AES-2016) software has been used to produce the rainfall hydrographs for each DMA.

For Flood Routing Analysis the Single Area Unit-Hydrograph Model for San Bernardino County has been utilized. To construct the synthetic critical storm pattern, area-averaged watershed rainfall values from NOAA Atlas 14 precipitation data for each sub basin was entered to AES. Lag value was computed per Corps of Engineering LAG formula. Basin factor (Manning's coefficient) $n=0.035$ has been used as an average value for all sub basins. An adjusted loss rate and low loss fraction coefficients have been calculated for each drainage area per Section C of the SBCHM. Refer to Appendix 6 for reference pages from the SBCHM. Refer to Appendix 3 for Onsite AES Calculation Results and Appendix 5 for Onsite Hydrology Map.

2.3 Grid Elements

The FLO-2D model's computational area was determined by inclusion of the study area and the tributary areas upstream that are considered alluvial in nature. The simulation was modeled utilizing 40-ft x 40-ft grid. The results of the downstream model are presented in Appendix 7, Flow Depth Results and Appendix 8, Flow Velocity Results.

2.4 Rainfall Intensity

Design storms were developed using NOAA Atlas 14 grids. Precipitation was added to the model using a rainfall value of 2.91" for the 100-year, 24-hour storm event. These depths were used to develop custom storm hydrographs, based on the SCS 24-hours Type II rainfall distribution. The depth area reduction factors were not applied. FLO-2D calculates runoff for each cell of a finite element grid based on a given rainfall distribution. Refer to Appendix 9 for Precipitation Frequency Data.

Precipitation values used in AES study were based on 100-year, 5-min, 30-min, 1-hour, 3-hour, 6-hour, 24-hour storm event.

2.5 Terrain

A Digital Terrain Model from USGS for outside the study area and an aerial topographic map for inside the study area were incorporated into text points files. These XYZ files are read directly into FLO-2D as DTM (elevation data) files.

2.6 Curve Numbers

The combination of United States Department of Agriculture Natural Resources Conservation Service (USDA-NRCS) Soil Survey Geographic Database (SSURGO) soil data and USDA-NRCS State Soil Geographic (STATSGO) soil data provides soil types within the study area and full coverage of the contributing watershed. SSURGO data was used for study area and areas of the contributing watershed where it was available, STATSCO data was supplemented in the contributing watershed where SSURGO data was unavailable.

Soils in the area are primarily classified as Hydrologic Soil Groups A, B and C within the study area. Land cover was obtained from USDA 2013 Crop Data Layer. Based on Soils Group numbers and Land cover data the average soils Curve Number has been determined from SBCHM, Figure C-3. See Appendix 6 for Curve Number tables and Appendix 10 for Hydrologic Soil Group data.

Curve Numbers were applied to each grid cell in the FLO-2D model based on intersection the grid with the curve numbers.

3. CALCULATIONS/RESULTS

The FLO-2D model was run for the pre-construction scenarios, analyzing the 100-year, 24-hour storm event. Exhibits of outputs are provided in the appendices. Below, the results are discussed for analysis of flow depth and flow velocity.

It shall be noted that the results of the FLO-2D model are estimates of velocity and depth resulting from one possible scenario of flooding across the study area. The model shows that for the vast majority of the study area, flow depths are less than 0.5'. In such situations, the actual characteristics of runoff across the study area will vary for each storm event and the results of the model shall be taken to provide an overall estimate of potential flow characteristics that may occur.

Due to the variability discussed above, the model does not provide results with precision adequate enough to enable recommendations for precise placement of maximum flow depth mitigation measures.

3.1 Flow Depth

See Appendix 7 for outputs from the FLO-2D model for flow depth. A maximum flow depth within the study area ranges from 0 to 3.7 feet. The maximum flow depth of 6.3' occurs outside of the study area. Note that two outputs are provided for each scenario, one with maximum flow depth outside of study area and another for the study area with a full flow depth range from 0.1 to 6 feet to better represent the maximum flow depth across the study area.

Within the study area, flows can reach greater depths primarily at locations of flow paths which enter the study area from northwest to northeast directions. A significant portion of runoff from the offsite flow approaches the study area from the southwest. There is an existing earthen berm outside of the study area along the western boundary with a channel on the upstream side of the berm that was constructed for the existing SEGS VIII and IX facilities. A portion of the offsite flow is captured by this channel and is diverted north along the western boundary. The berm does not run the entire length of the western boundary and as the berm terminates, the offsite flow crosses into the study area where flood depths exceed 3 feet. The remaining flow coming from the southwest enters into the study area along the southern boundary generally having flow depths of 2 feet or less within the study area. The offsite flow from the southwest eventually collects in Harper Lake located immediately east of the study area after crossing through the study area.

To address the concern of the flood depths exceeding 3 feet in the northwest corner of the study area, there will need to be mitigation measures proposed during the final design. The offsite run-on will need to be captured and diverted upstream of the solar modules either within or outside of the study area. Once a design has been completed, it is recommended that a post-developed Flo2D analysis be performed to model the mitigation measure and confirm flood depths have been reduced adequately in the area of concern.

3.2 Flow Velocity

See Appendix 8 for outputs from FLO-2D model for flow velocity. The velocity of flow across the study area range from 0 ft/sec to 9 ft/sec. The notable areas of increased velocity are at the locations where the flow enters the study area and along the channels outside of the study area.

3.3 Required Storage

As discussed in Section 2.2, the study area was divided into 3 DMA's. Hydrograph calculations were performed for existing and proposed conditions for these areas, based on input values summarized in Table 1 and Table 2. As described in Section 1.2, for the purposes of this analysis, the acreage reflected in the Tables below includes the 600-acre SEGS X property and the entire 110-acre Shared Facilities Area bringing the study area acreage total to approximately 710 acres (rounded down to 709 acres for technical accuracy).

Based on Soils Group information and Soils Coverage the Soil Conservation Service (SCS) Curve Numbers (Antecedent Moisture Content II) for existing and proposed conditions for all DMA's was determined to be 62. The number was derived from the Curve Number tables in the SBCHM in Appendix 6 and assumes the study area falls within Soil Group 'A' and will be considered poor quality Open Brush for the existing and proposed conditions. The Curve Number is then used in the following equations found within the SBCHM in Appendix 6 and results are shown in the tables below.

Pervious percentages were used to account for future development. While the study area contains miscellaneous concrete foundations, various electrical lines and poles, as well as existing facilities within the Shared Facilities Area, the study area is otherwise largely vacant. Therefore, for the purposes of preparing a conservative analysis, the percentage of impervious area for the existing condition was assumed to be 0%. For the proposed condition, the study area would continue to be almost entirely pervious, with the only new impervious areas being the inverters, battery storage units, and the posts under the arrays. The percentage of impervious area for the proposed condition is determined to be 0.5%.

Table 1. EXISTING CONDITION Input Data

DMA	Area ¹ (AC)	Max Loss Rate	Soil Capacity (S)	Initial Abstraction (Ia)	Storm Runoff Yield (Yi)	Low Loss Rate	Length (ft)	Lca (ft)	Runoff Volume (AF)
1	315	0.35	2.407	0.481	0.380	0.620	5,357	3,838	36.93
2	219	0.35	2.407	0.481	0.380	0.620	5,520	2,488	25.65
3	175	0.35	2.407	0.481	0.380	0.620	8,570	4,508	20.76

⁽¹⁾ Acreage reflected 709 acres

Table 2. PROPOSED CONDITION Input Data

DMA	Area ¹ (AC)	Max Loss Rate	Soil Capacity (S)	Initial Abstraction (Ia)	Storm Runoff Yield (Yi)	Low Loss Rate	Length (ft)	Lca (ft)	Runoff Volume (AF)
1	315	0.348	2.407	0.481	0.380	0.620	5,357	3,838	36.97
2	219	0.348	2.407	0.481	0.380	0.620	5,520	2,488	25.67
3	175	0.348	2.407	0.481	0.380	0.620	8,570	4,508	20.78

⁽¹⁾ Acreage reflected 709 acres

As shown in these tables, because the improvements under the Project would increase the imperviousness of the study area by 0.5%, the runoff volumes in the proposed condition is slightly greater than in existing conditions.

Table 3 below shows the required retention storage for the proposed retention basins calculated as the difference between the existing and proposed runoff volumes.

Table 3. Required Storage for Retention Basins

DMA	Area (AC) ¹	Required Storage (af)	Required Storage (cf)
1	315	0.04	1,742
2	219	0.02	871
3	175	0.02	871

⁽¹⁾ Acreage reflected 709 acres

4. CONCLUSION

As described within this study, the Lockhart Solar PV II Project study area is largely located in a rural, undeveloped area and slopes from southwest to northeast. The Project will collect flows originating south, west, and north of the study area and convey them easterly through the site via overland sheet flow.

The Project is not anticipated to construct traditional storm drain infrastructure and the means of providing the storage required to meet County retention standards will be shallow ponding outside and/or under the arrays. Based on the 100-year, 24-hour design storm, the required storage volume for retention basins has been determined for each DMA.

It is reiterated that the results of the FLO-2D model are estimates of velocity and maximum flow depth resulting from one possible scenario of flooding across the study area. The model shows that for the vast majority of the study area, flow depths are less than 0.5'. In such situations, the actual characteristics of runoff across the study area will vary for each storm event and the results of the model shall be taken to provide an overall estimate of potential flow that may occur.

Due to the variability discussed above, the model does not provide results with a precision adequate enough to enable recommendations for precise placement of scour mitigation measures.

The following statements address the CEQA guidelines for determining whether a significant impact to hydrology will occur as a result of project implementation. Would the project:

- ***Violate any water quality standards or waste discharge requirements or otherwise substantially degrade surface or groundwater quality?***

The Project proposes the use of retention basins for the minor increases in impervious surface due to the development of the Project Site. There are no proposed waste discharges from the Project and California water quality standards are being met with the provided basins. Therefore there is no substantial degrading of the surface or groundwater quality.

- ***Substantially decrease groundwater supplies or interfere substantially with groundwater recharge such that the project may impeded sustainable groundwater management of the basin?***

The Project will maintain a majority of the native soils in the proposed conditions with only slight increases in new impervious surface due to inverters, battery storage units, and posts under the arrays. The impervious increase is assumed to be 0.5%, therefore 99.5% of the study area is assumed to remain native material and infiltration will occur as it did in the existing condition for groundwater recharge.

- ***Substantially alter the existing drainage pattern of the site or area, including through the alteration of the course of a stream or river or through the addition of impervious surfaces, in a manner which would:***

- ***Result in substantial erosion or siltation on- or off-site;***

As part of the existing SEGS VIII and IX facilities, a berm and channel were constructed to divert off-site flows around the SEGS Facilities. The berm does not extend along the full length of the western boundary of the Lockhart Solar PV II Project study area and further diversion elements will need to be constructed as part of the Project to protect the Project from the concentrated off-site flows. The diversion element will be designed to capture and divert the off-site flows from the existing channel and continue on their path around the Project boundary. To match the existing condition, the flows will then spread from the concentrated path in a sheet flow manor to minimize erosion. The proposed diversion of off-site flow will not substantially alter the existing drainage pattern.

On-site flows are anticipated to sheet flow across the Project Site with only minor increases in imperviousness and therefore are not expected to result in substantial erosion.

- ***Substantially increase the rate or amount of surface runoff in a manner which would result in flooding on- or off-site;***

The impervious increase is assumed to be 0.5% for the developed site with retention basins being sized to capture the difference in the pre- vs. post-developed conditions to mitigate the impervious increase so no substantial increases are anticipated for the rate or amount of surface runoff.

- ***Create or contribute runoff water which would exceed the capacity of existing or planned stormwater drainage systems or provide substantial additional sources of polluted runoff;***

Additional runoff generated from increased imperviousness due to the development of the Project will be managed by retention basins. Runoff that leaves the project boundary will collect in Harper Lake directly east of the Project Site, so no stormwater drainage systems will be impacted.

- ***Or impeded or redirect flood flows?***

The overall drainage patterns of off-site flow are maintained in the developed condition with runoff ultimately getting to the dry lake bed Harper Lake directly east of the Project Site.

- ***In flood hazard, tsunami, or seich zones, risk release of pollutants due to project inundation?***

The study area resides within a designation of FEMA Flood Zone D, which is defined as “undetermined Flood Hazard”. This indicates that area has not been mapped and approved by FEMA with floodplains or floodways and flood hazards are undermined in this area.

- ***Conflict with or obstruct implementation of a water quality control plan or sustainable groundwater management plan?***

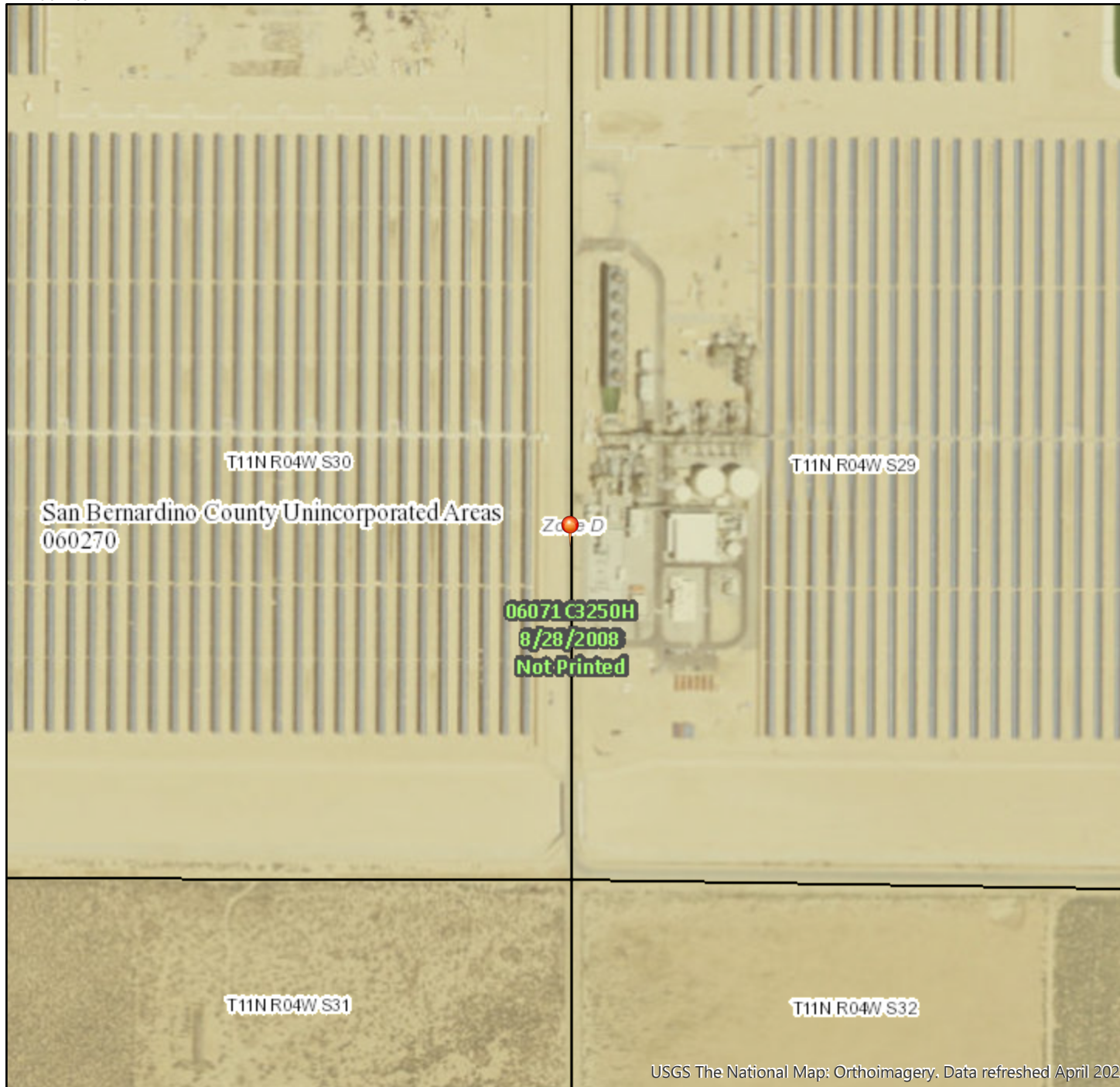
The Project is expected to maintain existing overall drainage patterns with only an assumed 0.5% impervious increase which will be sufficiently managed utilizing retention basins. It is anticipated that existing infiltration of runoff will be maintained in the developed condition and no conflicts or obstructions of a water quality control plan or sustainable groundwater management plan will occur.

APPENDIX 1
FEMA FIRMeTte

National Flood Hazard Layer FIRMette



117°20'8"W 35°1'2"N



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS	
	Without Base Flood Elevation (BFE) <i>Zone A, V, A99</i>
	With BFE or Depth <i>Zone AE, AO, AH, VE, AR</i>
	Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD	
	0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile <i>Zone X</i>
	Future Conditions 1% Annual Chance Flood Hazard <i>Zone X</i>
	Area with Reduced Flood Risk due to Levee. See Notes. <i>Zone X</i>
	Area with Flood Risk due to Levee <i>Zone D</i>

OTHER AREAS	
	NO SCREEN Area of Minimal Flood Hazard <i>Zone X</i>
	Effective LOMRs
	Area of Undetermined Flood Hazard <i>Zone D</i>

GENERAL STRUCTURES	
	Channel, Culvert, or Storm Sewer
	Levee, Dike, or Floodwall

OTHER FEATURES	
	20.2 Cross Sections with 1% Annual Chance Water Surface Elevation
	17.5 Coastal Transect
	Base Flood Elevation Line (BFE)
	Limit of Study
	Jurisdiction Boundary
	Coastal Transect Baseline
	Profile Baseline
	Hydrographic Feature

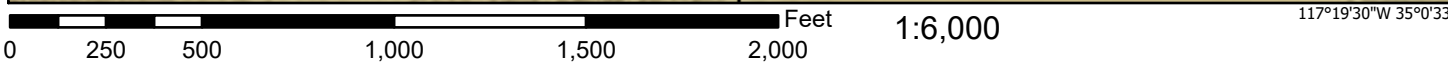
MAP PANELS	
	Digital Data Available
	No Digital Data Available
	Unmapped

The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 6/18/2020 at 7:15 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.



APPENDIX 2
Offsite AES Calculation Results

F L O O D R O U T I N G A N A L Y S I S
USING COUNTY HYDROLOGY MANUAL OF SAN BERNARDINO(1986)
(c) Copyright 1989-2016 Advanced Engineering Software (aes)
Ver. 23.0 Release Date: 07/01/2016 License ID 1355

Analysis prepared by:

***** DESCRIPTION OF STUDY *****

- * LOCKHART SOLAR *
 - * 100-YR STORM EVENT *
 - * INFLOW 1 *
- *****

FILE NAME: LOCEA.DAT
TIME/DATE OF STUDY: 14:37 09/13/2021

** INPUT SUMMARY **

FLOW PROCESS FROM NODE 100.00 TO NODE 101.00 IS CODE = 1

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS) ADDED TO STREAM #1<<<<<

=====

WATERCOURSE LENGTH = 61196.000 FEET
 LENGTH FROM CONCENTRATION POINT TO CENTROID = 28978.000 FEET
 ELEVATION VARIATION ALONG WATERCOURSE = 870.000 FEET
 BASIN FACTOR = 0.050
 WATERSHED AREA = 19853.000 ACRES; BASEFLOW = 0.000 CFS/SQUARE-MILE
 MOUNTAIN S-GRAPH SELECTED
 MAXIMUM WATERSHED LOSS RATE(INCH/HOUR) = 0.400; LOW LOSS FRACTION = 0.900
 SPECIFIED PEAK RAINFALL DEPTHS(INCH):
 5-MINUTE = 0.32; 30-MINUTE = 0.77; 1-HOUR = 1.05
 3-HOUR = 1.51; 6-HOUR = 1.88; 24-HOUR = 3.00
 PRECIPITATION DEPTH-AREA REDUCTION FACTORS:
 5-MINUTE = 0.530; 30-MINUTE = 0.555; 1-HOUR = 0.569
 3-HOUR = 0.890; 6-HOUR = 0.951; 24-HOUR = 0.969

FLOW PROCESS FROM NODE 200.00 TO NODE 101.00 IS CODE = 1

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS) ADDED TO STREAM #2<<<<<

=====

WATERCOURSE LENGTH = 67843.000 FEET
LENGTH FROM CONCENTRATION POINT TO CENTROID = 44169.000 FEET
ELEVATION VARIATION ALONG WATERCOURSE = 870.000 FEET
BASIN FACTOR = 0.050
WATERSHED AREA = 14192.000 ACRES; BASEFLOW = 0.000 CFS/SQUARE-MILE
MOUNTAIN S-GRAPH SELECTED
MAXIMUM WATERSHED LOSS RATE(INCH/HOUR) = 0.400; LOW LOSS FRACTION = 0.900
SPECIFIED PEAK RAINFALL DEPTHS(INCH):
5-MINUTE = 0.32; 30-MINUTE = 0.77; 1-HOUR = 1.05
3-HOUR = 1.51; 6-HOUR = 1.88; 24-HOUR = 3.00
PRECIPITATION DEPTH-AREA REDUCTION FACTORS:
5-MINUTE = 0.601; 30-MINUTE = 0.620; 1-HOUR = 0.627
3-HOUR = 0.919; 6-HOUR = 0.962; 24-HOUR = 0.976

FLOW PROCESS FROM NODE 101.00 TO NODE 101.00 IS CODE = 7

>>>>STREAM NUMBER 2 ADDED TO STREAM NUMBER 1<<<<<

FLOW PROCESS FROM NODE 101.00 TO NODE 101.00 IS CODE = 11

>>>>VIEW STREAM NUMBER 1 HYDROGRAPH<<<<<

FLOW PROCESS FROM NODE 101.00 TO NODE 102.00 IS CODE = 1

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS) ADDED TO STREAM #3<<<<<

=====

WATERCOURSE LENGTH = 64738.000 FEET
LENGTH FROM CONCENTRATION POINT TO CENTROID = 44541.000 FEET
ELEVATION VARIATION ALONG WATERCOURSE = 1790.000 FEET
BASIN FACTOR = 0.050
WATERSHED AREA = 13056.000 ACRES; BASEFLOW = 0.000 CFS/SQUARE-MILE
MOUNTAIN S-GRAPH SELECTED
MAXIMUM WATERSHED LOSS RATE(INCH/HOUR) = 0.400; LOW LOSS FRACTION = 0.900
SPECIFIED PEAK RAINFALL DEPTHS(INCH):
5-MINUTE = 0.32; 30-MINUTE = 0.77; 1-HOUR = 1.05
3-HOUR = 1.51; 6-HOUR = 1.88; 24-HOUR = 3.00
PRECIPITATION DEPTH-AREA REDUCTION FACTORS:
5-MINUTE = 0.620; 30-MINUTE = 0.635; 1-HOUR = 0.641
3-HOUR = 0.925; 6-HOUR = 0.964; 24-HOUR = 0.977

 FLOW PROCESS FROM NODE 102.00 TO NODE 102.00 IS CODE = 7

>>>>STREAM NUMBER 3 ADDED TO STREAM NUMBER 1<<<<<

 FLOW PROCESS FROM NODE 102.00 TO NODE 102.00 IS CODE = 11

>>>>VIEW STREAM NUMBER 1 HYDROGRAPH<<<<<

↑

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+-----+
|                                     | * AES FLOODSCx PROGRAM RESULTS SUMMARY *
|                                     |
| INPUT FILENAME: [LOCEA.DAT        | ]
|           Page: 1 of              |
+-----+
  
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UPSTREAM		DOWNSTREAM				UPSTREAM		DOWNSTREAM	
TIME(2)	TO	MAX. STORAGE		PEAK (CFS)	PEAK (CFS)	PEAK (CFS)	PEAK (CFS)	PEAK (CFS)	PEAK (CFS)
NODE #	NODE #	HYDROLOGIC/HYDRAULIC	PROCESS						
(HR)	MODELED (AF)	FOOTNOTES							
17.417	100.00	101.00	Subarea (UH) Added to Stream #1	0.0	1674.8				
17.583	200.00	101.00	Subarea (UH) Added to Stream #2	0.0	1099.4				
17.583	101.00	101.00	Stream #2 Added to: Stream #1	1674.8	2774.2				
17.583	101.00	101.00	View: Stream #1		2774.2				
17.583	101.00	1361.08	3						
17.417	101.00	102.00	Subarea (UH) Added to Stream #3	0.0	1293.5				
17.583	102.00	102.00	Stream #3 Added to: Stream #1	2774.2	4067.7				
17.583	102.00	102.00	View: Stream #1		4067.7				
17.583	102.00	1934.37	3						

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|Notes: 1 = BASIN MODEL VOLUME EXCEEDED; 2 = TIME IS AT END OF 5-MINUTE UNIT
INTERVAL

| 3 = RUNOFF ESTIMATES DO NOT EXTEND PAST 2 DAYS AFTER THE PEAK DAY OF THE
DESIGN STORM

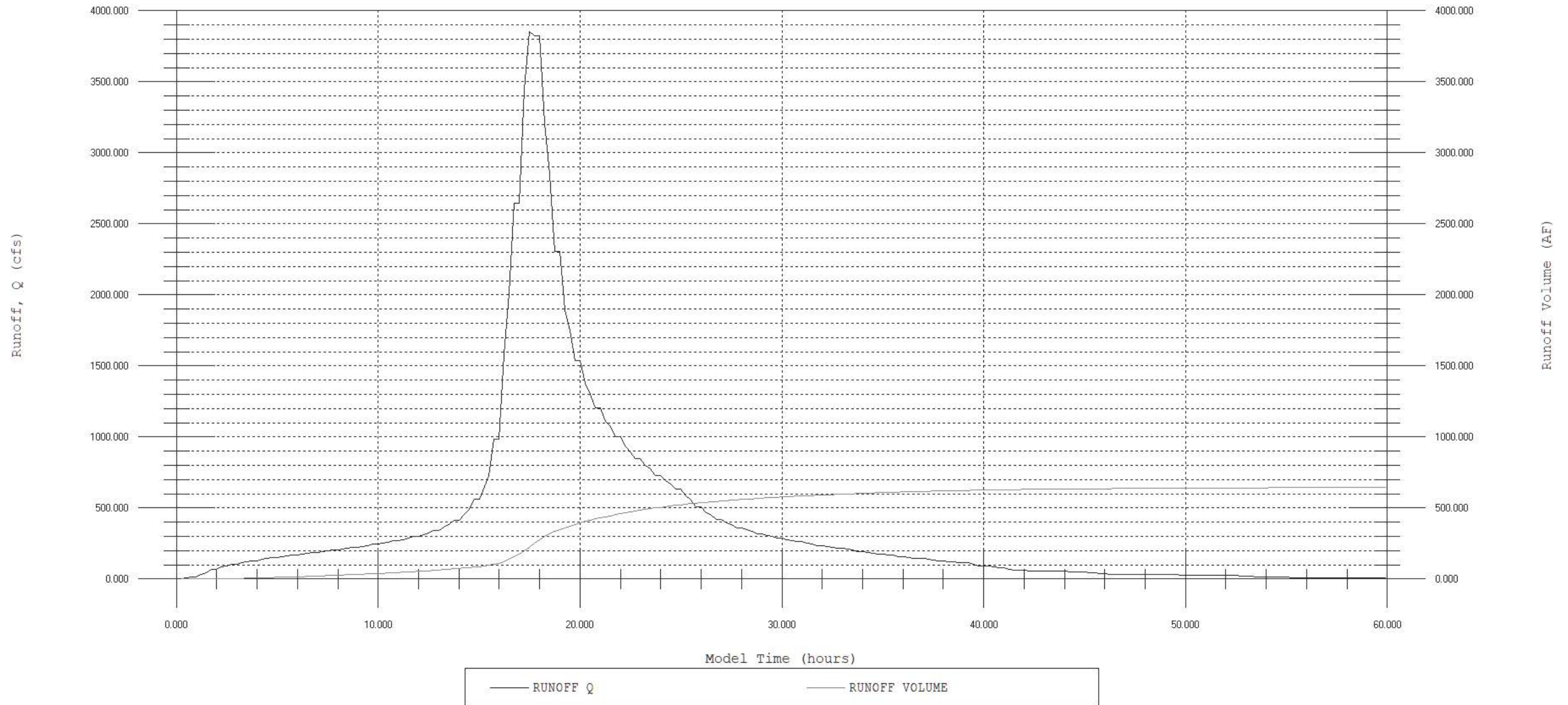
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END OF FLOODSCx ROUTING ANALYSIS

View Stream # 1

From Node 102.00 to Node 102.00



F L O O D R O U T I N G A N A L Y S I S
USING COUNTY HYDROLOGY MANUAL OF SAN BERNARDINO(1986)
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Ver. 23.0 Release Date: 07/01/2016 License ID 1355

Analysis prepared by:

***** DESCRIPTION OF STUDY *****

- * LOCKHART SOLAR *
 - * 100-YR STORM EVENT *
 - * INFLOW 2 *
- *****

FILE NAME: LOCEH.DAT
TIME/DATE OF STUDY: 14:37 09/13/2021

** INPUT SUMMARY **

FLOW PROCESS FROM NODE 400.00 TO NODE 401.00 IS CODE = 1

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS) ADDED TO STREAM #1<<<<<

=====

WATERCOURSE LENGTH = 64426.000 FEET
 LENGTH FROM CONCENTRATION POINT TO CENTROID = 48310.000 FEET
 ELEVATION VARIATION ALONG WATERCOURSE = 625.000 FEET
 BASIN FACTOR = 0.050
 WATERSHED AREA = 30610.000 ACRES; BASEFLOW = 0.000 CFS/SQUARE-MILE
 MOUNTAIN S-GRAPH SELECTED
 MAXIMUM WATERSHED LOSS RATE(INCH/HOUR) = 0.400; LOW LOSS FRACTION = 0.900
 SPECIFIED PEAK RAINFALL DEPTHS(INCH):
 5-MINUTE = 0.32; 30-MINUTE = 0.77; 1-HOUR = 1.05
 3-HOUR = 1.51; 6-HOUR = 1.88; 24-HOUR = 3.00
 PRECIPITATION DEPTH-AREA REDUCTION FACTORS:
 5-MINUTE = 0.440; 30-MINUTE = 0.477; 1-HOUR = 0.500
 3-HOUR = 0.845; 6-HOUR = 0.931; 24-HOUR = 0.958

FLOW PROCESS FROM NODE 401.00 TO NODE 402.00 IS CODE = 1

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS) ADDED TO STREAM #2<<<<<

=====

WATERCOURSE LENGTH = 56381.000 FEET
LENGTH FROM CONCENTRATION POINT TO CENTROID = 32787.000 FEET
ELEVATION VARIATION ALONG WATERCOURSE = 460.000 FEET
BASIN FACTOR = 0.050
WATERSHED AREA = 18575.000 ACRES; BASEFLOW = 0.000 CFS/SQUARE-MILE
MOUNTAIN S-GRAPH SELECTED
MAXIMUM WATERSHED LOSS RATE(INCH/HOUR) = 0.400; LOW LOSS FRACTION = 0.900
SPECIFIED PEAK RAINFALL DEPTHS(INCH):
5-MINUTE = 0.32; 30-MINUTE = 0.77; 1-HOUR = 1.05
3-HOUR = 1.51; 6-HOUR = 1.88; 24-HOUR = 3.00
PRECIPITATION DEPTH-AREA REDUCTION FACTORS:
5-MINUTE = 0.543; 30-MINUTE = 0.568; 1-HOUR = 0.581
3-HOUR = 0.897; 6-HOUR = 0.953; 24-HOUR = 0.971

FLOW PROCESS FROM NODE 402.00 TO NODE 402.00 IS CODE = 7

>>>>STREAM NUMBER 2 ADDED TO STREAM NUMBER 1<<<<<

FLOW PROCESS FROM NODE 402.00 TO NODE 402.00 IS CODE = 11

>>>>VIEW STREAM NUMBER 1 HYDROGRAPH<<<<<

FLOW PROCESS FROM NODE 600.00 TO NODE 402.00 IS CODE = 1

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS) ADDED TO STREAM #3<<<<<

=====

WATERCOURSE LENGTH = 41497.000 FEET
LENGTH FROM CONCENTRATION POINT TO CENTROID = 21606.000 FEET
ELEVATION VARIATION ALONG WATERCOURSE = 360.000 FEET
BASIN FACTOR = 0.050
WATERSHED AREA = 6040.000 ACRES; BASEFLOW = 0.000 CFS/SQUARE-MILE
MOUNTAIN S-GRAPH SELECTED
MAXIMUM WATERSHED LOSS RATE(INCH/HOUR) = 0.400; LOW LOSS FRACTION = 0.900
SPECIFIED PEAK RAINFALL DEPTHS(INCH):
5-MINUTE = 0.32; 30-MINUTE = 0.77; 1-HOUR = 1.05
3-HOUR = 1.51; 6-HOUR = 1.88; 24-HOUR = 3.00
PRECIPITATION DEPTH-AREA REDUCTION FACTORS:
5-MINUTE = 0.759; 30-MINUTE = 0.759; 1-HOUR = 0.759
3-HOUR = 0.962; 6-HOUR = 0.980; 24-HOUR = 0.988

FLOW PROCESS FROM NODE 402.00 TO NODE 402.00 IS CODE = 7

>>>>STREAM NUMBER 3 ADDED TO STREAM NUMBER 1<<<<<
=====

FLOW PROCESS FROM NODE 402.00 TO NODE 402.00 IS CODE = 11

>>>>VIEW STREAM NUMBER 1 HYDROGRAPH<<<<<
=====

FLOW PROCESS FROM NODE 402.00 TO NODE 403.00 IS CODE = 1

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS) ADDED TO STREAM #4<<<<<
=====

WATERCOURSE LENGTH = 55496.000 FEET
LENGTH FROM CONCENTRATION POINT TO CENTROID = 34693.000 FEET
ELEVATION VARIATION ALONG WATERCOURSE = 890.000 FEET
BASIN FACTOR = 0.050
WATERSHED AREA = 10574.000 ACRES; BASEFLOW = 0.000 CFS/SQUARE-MILE
MOUNTAIN S-GRAPH SELECTED
MAXIMUM WATERSHED LOSS RATE(INCH/HOUR) = 0.400; LOW LOSS FRACTION = 0.900
SPECIFIED PEAK RAINFALL DEPTHS(INCH):
5-MINUTE = 0.32; 30-MINUTE = 0.77; 1-HOUR = 1.05
3-HOUR = 1.51; 6-HOUR = 1.88; 24-HOUR = 3.00
PRECIPITATION DEPTH-AREA REDUCTION FACTORS:
5-MINUTE = 0.662; 30-MINUTE = 0.669; 1-HOUR = 0.672
3-HOUR = 0.938; 6-HOUR = 0.969; 24-HOUR = 0.980

FLOW PROCESS FROM NODE 403.00 TO NODE 403.00 IS CODE = 7

>>>>STREAM NUMBER 4 ADDED TO STREAM NUMBER 1<<<<<
=====

FLOW PROCESS FROM NODE 403.00 TO NODE 403.00 IS CODE = 11

>>>>VIEW STREAM NUMBER 1 HYDROGRAPH<<<<<
=====



* AES FLOODSCx PROGRAM RESULTS SUMMARY *

INPUT FILENAME: [LOCEH.DAT]
 Page: 1 of 1

UPSTREAM TIME(2) TO (HR)	DOWNSTREAM MAX. STORAGE NODE # MODELED (AF)	HYDROLOGIC/HYDRAULIC PROCESS FOOTNOTES	UPSTREAM PEAK (CFS)	DOWNSTREAM PEAK (CFS)	PEAK
--------------------------------	--	---	------------------------	--------------------------	------

400.00	401.00	Subarea (UH) Added to Stream #1	0.0	1431.1	18.083
401.00	402.00	Subarea (UH) Added to Stream #2	0.0	1477.0	17.750
402.00	402.00	Stream #2 Added to: Stream #1	1431.1	2840.3	17.750
402.00	402.00	View: Stream #1		2840.3	17.750
600.00	402.00	Subarea (UH) Added to Stream #3	0.0	906.5	17.083

402.00	402.00	Stream #3 Added to: Stream #1	2840.3	3670.0	17.583
402.00	402.00	View: Stream #1		3670.0	17.583
402.00	403.00	Subarea (UH) Added to Stream #4	0.0	1185.7	17.417
403.00	403.00	Stream #4 Added to: Stream #1	3670.0	4855.7	17.583
403.00	403.00	View: Stream #1		4855.7	17.583

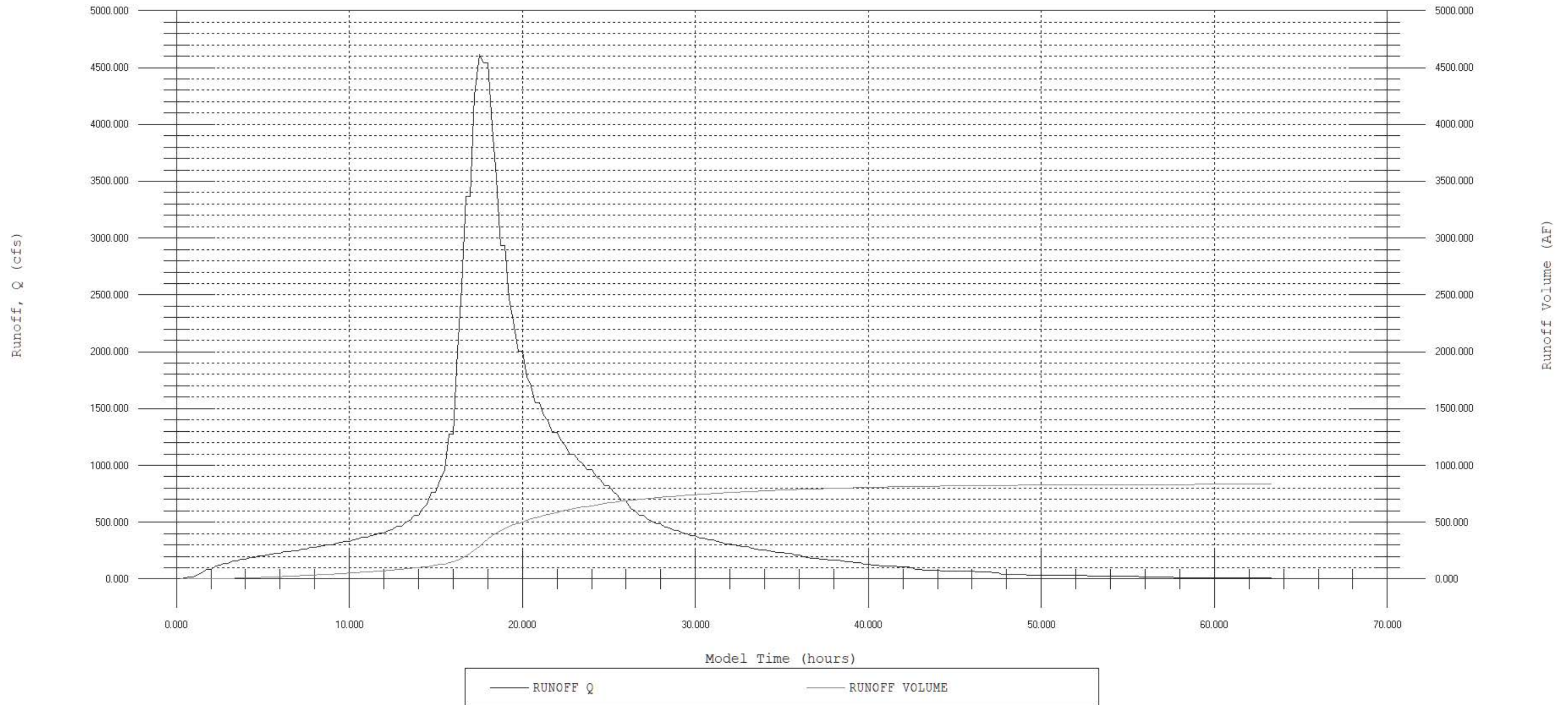
Notes: 1 = BASIN MODEL VOLUME EXCEEDED; 2 = TIME IS AT END OF 5-MINUTE UNIT INTERVAL
 3 = RUNOFF ESTIMATES DO NOT EXTEND PAST 2 DAYS AFTER THE PEAK DAY OF THE DESIGN STORM

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END OF FLOODSCx ROUTING ANALYSIS

View Stream # 1

From Node 403.00 to Node 403.00



APPENDIX 3
Onsite AES Calculation Results

F L O O D R O U T I N G A N A L Y S I S
USING COUNTY HYDROLOGY MANUAL OF SAN BERNARDINO(1986)
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Ver. 23.0 Release Date: 07/01/2016 License ID 1355

Analysis prepared by:

***** DESCRIPTION OF STUDY *****

- * LOCKHART SOLAR *
 - * 100-YR STORM EVENT *
 - * AREA 1 EXISTING CONDITION *
- *****

FILE NAME: LOEX1.DAT
TIME/DATE OF STUDY: 10:09 07/08/2021

FLOW PROCESS FROM NODE 100.00 TO NODE 101.00 IS CODE = 1

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS)<<<<<
=====

(UNIT-HYDROGRAPH ADDED TO STREAM #1)

WATERCOURSE LENGTH = 5357.000 FEET
 LENGTH FROM CONCENTRATION POINT TO CENTROID = 3838.000 FEET
 ELEVATION VARIATION ALONG WATERCOURSE = 29.000 FEET
 BASIN FACTOR = 0.035
 WATERSHED AREA = 314.560 ACRES
 BASEFLOW = 0.000 CFS/SQUARE-MILE
 WATERCOURSE "LAG" TIME = 0.396 HOURS
 CAUTION: LAG TIME IS LESS THAN 0.50 HOURS.
 THE 5-MINUTE PERIOD UH MODEL (USED IN THIS COMPUTER PROGRAM)
 MAY BE TOO LARGE FOR PEAK FLOW ESTIMATES.
 DESERT(UNDEVELOPED) S-GRAPH SELECTED
 MAXIMUM WATERSHED LOSS RATE(INCH/HOUR) = 0.350
 LOW LOSS FRACTION = 0.620
 HYDROGRAPH MODEL #1 SPECIFIED

SPECIFIED PEAK 5-MINUTES RAINFALL(INCH)= 0.33
 SPECIFIED PEAK 30-MINUTES RAINFALL(INCH)= 0.78

SPECIFIED PEAK 1-HOUR RAINFALL(INCH) = 1.05
 SPECIFIED PEAK 3-HOUR RAINFALL(INCH) = 1.50
 SPECIFIED PEAK 6-HOUR RAINFALL(INCH) = 1.85
 SPECIFIED PEAK 24-HOUR RAINFALL(INCH) = 2.91

PRECIPITATION DEPTH-AREA REDUCTION FACTORS:

5-MINUTE FACTOR = 0.986
 30-MINUTE FACTOR = 0.986
 1-HOUR FACTOR = 0.986
 3-HOUR FACTOR = 0.998
 6-HOUR FACTOR = 0.999
 24-HOUR FACTOR = 0.999

UNIT HYDROGRAPH TIME UNIT = 5.000 MINUTES
 UNIT INTERVAL PERCENTAGE OF LAG-TIME = 21.060

=====

UNIT HYDROGRAPH DETERMINATION

INTERVAL NUMBER	"S" GRAPH MEAN VALUES	UNIT HYDROGRAPH ORDINATES(CFS)
1	1.066	40.545
2	4.908	146.176
3	12.648	294.434
4	29.637	646.317
5	46.517	642.147
6	57.607	421.890
7	64.897	277.307
8	70.370	208.196
9	74.680	163.982
10	78.215	134.478
11	81.063	108.352
12	83.454	90.942
13	85.572	80.557
14	87.444	71.224
15	89.009	59.551
16	90.272	48.035
17	91.447	44.701
18	92.488	39.621
19	93.443	36.297
20	94.263	31.192
21	94.979	27.241
22	95.661	25.956
23	96.220	21.264
24	96.762	20.622
25	97.187	16.180

26	97.566	14.423
27	97.880	11.918
28	98.091	8.051
29	98.307	8.220
30	98.555	9.427
31	98.808	9.611
32	99.060	9.610
33	99.313	9.610
34	99.566	9.610
35	99.818	9.610
36	100.000	6.910

TOTAL SOIL-LOSS VOLUME(ACRE-FEET) = 39.2829
TOTAL STORM RUNOFF VOLUME(ACRE-FEET) = 36.9306

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2 4 - H O U R S T O R M
R U N O F F H Y D R O G R A P H

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HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS(CFS)
(Note: Time indicated is at END of Each Unit Intervals)

TIME(HRS)	VOLUME(AF)	Q(CFS)	0.	100.0	200.0	300.0	400.0
0.083	0.0004	0.05	Q
0.167	0.0020	0.23	Q
0.250	0.0061	0.61	Q
0.333	0.0159	1.42	Q
0.417	0.0313	2.23	Q
0.500	0.0503	2.77	Q
0.583	0.0719	3.13	Q
0.667	0.0953	3.40	Q
0.750	0.1202	3.62	Q
0.833	0.1463	3.80	Q
0.917	0.1735	3.95	Q
1.000	0.2016	4.08	Q
1.083	0.2305	4.19	Q
1.167	0.2601	4.30	Q
1.250	0.2903	4.39	Q
1.333	0.3211	4.46	Q
1.417	0.3523	4.54	Q
1.500	0.3840	4.60	Q
1.583	0.4161	4.67	Q
1.667	0.4487	4.72	Q
1.750	0.4816	4.78	Q

1.833	0.5148	4.83	Q
1.917	0.5484	4.87	Q
2.000	0.5822	4.92	Q
2.083	0.6164	4.96	Q
2.167	0.6507	4.99	Q
2.250	0.6854	5.03	Q
2.333	0.7202	5.06	Q
2.417	0.7552	5.09	Q
2.500	0.7905	5.12	Q
2.583	0.8260	5.15	Q
2.667	0.8617	5.19	Q
2.750	0.8976	5.22	Q
2.833	0.9338	5.25	QV
2.917	0.9702	5.28	QV
3.000	1.0068	5.32	QV
3.083	1.0435	5.34	QV
3.167	1.0805	5.36	QV
3.250	1.1175	5.38	QV
3.333	1.1547	5.40	QV
3.417	1.1921	5.43	QV
3.500	1.2297	5.45	QV
3.583	1.2674	5.47	QV
3.667	1.3052	5.50	QV
3.750	1.3433	5.52	QV
3.833	1.3815	5.55	QV
3.917	1.4198	5.57	QV
4.000	1.4583	5.60	QV
4.083	1.4971	5.62	QV
4.167	1.5359	5.65	QV
4.250	1.5750	5.67	QV
4.333	1.6142	5.70	QV
4.417	1.6536	5.72	QV
4.500	1.6932	5.75	QV
4.583	1.7330	5.78	QV
4.667	1.7730	5.80	QV
4.750	1.8131	5.83	QV
4.833	1.8535	5.86	Q V
4.917	1.8940	5.89	Q V
5.000	1.9348	5.91	Q V
5.083	1.9757	5.94	Q V
5.167	2.0168	5.97	Q V
5.250	2.0581	6.00	Q V
5.333	2.0997	6.03	Q V
5.417	2.1414	6.06	Q V
5.500	2.1834	6.09	Q V
5.583	2.2256	6.12	Q V
5.667	2.2680	6.15	Q V
5.750	2.3106	6.19	Q V
5.833	2.3534	6.22	Q V
5.917	2.3964	6.25	Q V

6.000	2.4397	6.28	Q V
6.083	2.4832	6.32	Q V
6.167	2.5270	6.35	Q V
6.250	2.5709	6.39	Q V
6.333	2.6151	6.42	Q V
6.417	2.6596	6.46	Q V
6.500	2.7043	6.49	Q V
6.583	2.7493	6.53	Q V
6.667	2.7945	6.56	Q V
6.750	2.8399	6.60	Q V
6.833	2.8857	6.64	Q V
6.917	2.9316	6.68	Q V
7.000	2.9779	6.72	Q V
7.083	3.0244	6.76	Q V
7.167	3.0712	6.80	Q V
7.250	3.1183	6.84	Q V
7.333	3.1657	6.88	Q V
7.417	3.2133	6.92	Q V
7.500	3.2613	6.96	Q V
7.583	3.3095	7.01	Q V
7.667	3.3581	7.05	Q V
7.750	3.4069	7.09	Q V
7.833	3.4561	7.14	Q V
7.917	3.5056	7.18	Q V
8.000	3.5554	7.23	Q V
8.083	3.6055	7.28	Q V
8.167	3.6560	7.33	Q V
8.250	3.7068	7.38	Q V
8.333	3.7579	7.43	Q V
8.417	3.8094	7.48	Q V
8.500	3.8613	7.53	Q V
8.583	3.9135	7.58	Q V
8.667	3.9661	7.63	Q V
8.750	4.0190	7.69	Q V
8.833	4.0723	7.74	Q V
8.917	4.1261	7.80	Q V
9.000	4.1802	7.86	Q V
9.083	4.2347	7.92	Q V
9.167	4.2896	7.98	Q V
9.250	4.3450	8.04	Q V
9.333	4.4008	8.10	Q V
9.417	4.4570	8.16	Q V
9.500	4.5136	8.23	Q V
9.583	4.5707	8.29	Q V
9.667	4.6283	8.36	Q V
9.750	4.6863	8.43	Q V
9.833	4.7449	8.50	Q V
9.917	4.8039	8.57	Q V
10.000	4.8634	8.64	Q V
10.083	4.9234	8.71	Q V

10.167	4.9839	8.79	Q	V
10.250	5.0450	8.87	Q	V
10.333	5.1066	8.95	Q	V
10.417	5.1688	9.03	Q	V
10.500	5.2315	9.11	Q	V
10.583	5.2949	9.20	Q	V
10.667	5.3588	9.28	Q	V
10.750	5.4233	9.37	Q	V
10.833	5.4885	9.46	Q	V
10.917	5.5543	9.56	Q	V
11.000	5.6208	9.65	Q	V
11.083	5.6879	9.75	Q	V
11.167	5.7558	9.85	Q	V
11.250	5.8243	9.95	Q	V
11.333	5.8936	10.06	.Q	V
11.417	5.9636	10.17	.Q	V
11.500	6.0344	10.28	.Q	V
11.583	6.1060	10.39	.Q	V
11.667	6.1784	10.51	.Q	V
11.750	6.2516	10.63	.Q	V
11.833	6.3257	10.76	.Q	V
11.917	6.4007	10.89	.Q	V
12.000	6.4766	11.02	.Q	V
12.083	6.5534	11.15	.Q	V
12.167	6.6309	11.26	.Q	V
12.250	6.7090	11.34	.Q	V
12.333	6.7871	11.34	.Q	V
12.417	6.8653	11.35	.Q	V
12.500	6.9439	11.42	.Q	V
12.583	7.0232	11.52	.Q	V
12.667	7.1034	11.64	.Q	V
12.750	7.1844	11.77	.Q	V
12.833	7.2666	11.93	.Q	V
12.917	7.3498	12.09	.Q	V
13.000	7.4343	12.26	.Q	V
13.083	7.5200	12.45	.Q	V
13.167	7.6071	12.64	.Q	V
13.250	7.6955	12.85	.Q	V
13.333	7.7855	13.07	.Q	V
13.417	7.8771	13.29	.Q	V
13.500	7.9703	13.54	.Q	V
13.583	8.0653	13.79	.Q	V
13.667	8.1621	14.06	.Q	V
13.750	8.2609	14.34	.Q	V
13.833	8.3617	14.64	.Q	V.
13.917	8.4647	14.96	.Q	V.
14.000	8.5700	15.29	.Q	V.
14.083	8.6779	15.67	.Q	V.
14.167	8.7890	16.12	.Q	V.
14.250	8.9037	16.67	.Q	V.

14.333	9.0238	17.43	.Q	V.	.	.	.
14.417	9.1492	18.21	.Q	V.	.	.	.
14.500	9.2796	18.93	.Q	V	.	.	.
14.583	9.4146	19.61	.Q	V	.	.	.
14.667	9.5545	20.31	. Q	V	.	.	.
14.750	9.6993	21.02	. Q	V	.	.	.
14.833	9.8494	21.79	. Q	V	.	.	.
14.917	10.0050	22.60	. Q	V	.	.	.
15.000	10.1668	23.49	. Q	.V	.	.	.
15.083	10.3351	24.44	. Q	.V	.	.	.
15.167	10.5107	25.50	. Q	.V	.	.	.
15.250	10.6944	26.66	. Q	.V	.	.	.
15.333	10.8871	27.99	. Q	.V	.	.	.
15.417	11.0909	29.59	. Q	. V	.	.	.
15.500	11.3100	31.81	. Q	. V	.	.	.
15.583	11.5495	34.78	. Q	. V	.	.	.
15.667	11.8210	39.42	. Q	. V	.	.	.
15.750	12.1323	45.20	. Q	. V	.	.	.
15.833	12.4959	52.79	. Q	. V	.	.	.
15.917	12.9397	64.44	. Q	. V	.	.	.
16.000	13.5293	85.60	. Q	. V	.	.	.
16.083	14.3759	122.94	.	. Q	V	.	.
16.167	15.6184	180.40	.	.	V	Q	.
16.250	17.3161	246.50	.	.	V	Q	.
16.333	19.6131	333.53	.	.	.V	.	Q
16.417	21.8049	318.24	.	.	.	V	.Q
16.500	23.4743	242.40	.	.	.	QV	.
16.583	24.7400	183.78	.	.	Q	.	V
16.667	25.7576	147.76	.	.	Q	.	V
16.750	26.6039	122.89	.	.	Q	.	V
16.833	27.3279	105.12	.	.	Q	.	V.
16.917	27.9528	90.74	.	.	Q.	.	V
17.000	28.5040	80.03	.	.	Q	.	V
17.083	29.0002	72.05	.	.	Q	.	.V
17.167	29.4473	64.92	.	.	Q	.	.V
17.250	29.8455	57.82	.	.	Q	.	.V
17.333	30.1991	51.35	.	.	Q	.	.V
17.417	30.5242	47.20	.	.	Q	.	.V
17.500	30.8220	43.25	.	.	Q	.	.V
17.583	31.0970	39.92	. QV
17.667	31.3486	36.53	. QV
17.750	31.5801	33.61	. QV
17.833	31.7967	31.45	. QV
17.917	31.9946	28.73	. QV
18.000	32.1799	26.91	. QV
18.083	32.3485	24.48	. QV
18.167	32.5052	22.75	. QV
18.250	32.6504	21.08	. QV
18.333	32.7847	19.50	.QV
18.417	32.9158	19.03	.QV

18.500	33.0458	18.88	.Q	.	.	.	V	.
18.583	33.1731	18.49	.Q	.	.	.	V	.
18.667	33.2971	18.00	.Q	.	.	.	V	.
18.750	33.4168	17.38	.Q	.	.	.	V	.
18.833	33.5316	16.66	.Q	.	.	.	V	.
18.917	33.6397	15.70	.Q	.	.	.	V	.
19.000	33.7362	14.01	.Q	.	.	.	V	.
19.083	33.8164	11.65	.Q	.	.	.	V	.
19.167	33.8925	11.04	.Q	.	.	.	V	.
19.250	33.9659	10.66	.Q	.	.	.	V	.
19.333	34.0372	10.35	.Q	.	.	.	V	.
19.417	34.1067	10.10	.Q	.	.	.	V	.
19.500	34.1746	9.86	Q	.	.	.	V	.
19.583	34.2410	9.65	Q	.	.	.	V	.
19.667	34.3061	9.44	Q	.	.	.	V	.
19.750	34.3698	9.25	Q	.	.	.	V	.
19.833	34.4322	9.07	Q	.	.	.	V	.
19.917	34.4935	8.90	Q	.	.	.	V	.
20.000	34.5536	8.73	Q	.	.	.	V	.
20.083	34.6128	8.58	Q	.	.	.	V	.
20.167	34.6709	8.44	Q	.	.	.	V	.
20.250	34.7280	8.30	Q	.	.	.	V	.
20.333	34.7843	8.17	Q	.	.	.	V	.
20.417	34.8396	8.04	Q	.	.	.	V	.
20.500	34.8941	7.92	Q	.	.	.	V	.
20.583	34.9479	7.80	Q	.	.	.	V	.
20.667	35.0008	7.69	Q	.	.	.	V	.
20.750	35.0530	7.58	Q	.	.	.	V	.
20.833	35.1045	7.47	Q	.	.	.	V	.
20.917	35.1553	7.37	Q	.	.	.	V	.
21.000	35.2054	7.28	Q	.	.	.	V	.
21.083	35.2548	7.18	Q	.	.	.	V	.
21.167	35.3036	7.09	Q	.	.	.	V	.
21.250	35.3518	7.00	Q	.	.	.	V	.
21.333	35.3994	6.91	Q	.	.	.	V	.
21.417	35.4464	6.83	Q	.	.	.	V	.
21.500	35.4929	6.75	Q	.	.	.	V	.
21.583	35.5388	6.67	Q	.	.	.	V	.
21.667	35.5842	6.59	Q	.	.	.	V	.
21.750	35.6290	6.51	Q	.	.	.	V	.
21.833	35.6734	6.44	Q	.	.	.	V	.
21.917	35.7173	6.37	Q	.	.	.	V	.
22.000	35.7607	6.30	Q	.	.	.	V	.
22.083	35.8037	6.24	Q	.	.	.	V	.
22.167	35.8462	6.17	Q	.	.	.	V	.
22.250	35.8882	6.11	Q	.	.	.	V	.
22.333	35.9299	6.05	Q	.	.	.	V	.
22.417	35.9711	5.99	Q	.	.	.	V	.
22.500	36.0119	5.93	Q	.	.	.	V.	.
22.583	36.0524	5.87	Q	.	.	.	V.	.

22.667	36.0924	5.82	Q	.	.	.	V.
22.750	36.1321	5.76	Q	.	.	.	V.
22.833	36.1714	5.71	Q	.	.	.	V.
22.917	36.2104	5.66	Q	.	.	.	V.
23.000	36.2489	5.60	Q	.	.	.	V.
23.083	36.2872	5.55	Q	.	.	.	V.
23.167	36.3251	5.51	Q	.	.	.	V.
23.250	36.3627	5.46	Q	.	.	.	V.
23.333	36.4000	5.41	Q	.	.	.	V.
23.417	36.4370	5.37	Q	.	.	.	V.
23.500	36.4736	5.32	Q	.	.	.	V.
23.583	36.5100	5.28	Q	.	.	.	V.
23.667	36.5460	5.24	Q	.	.	.	V.
23.750	36.5818	5.19	Q	.	.	.	V.
23.833	36.6173	5.15	Q	.	.	.	V.
23.917	36.6525	5.11	Q	.	.	.	V.
24.000	36.6874	5.07	Q	.	.	.	V.
24.083	36.7217	4.98	Q	.	.	.	V.
24.167	36.7545	4.76	Q	.	.	.	V.
24.250	36.7846	4.36	Q	.	.	.	V.
24.333	36.8088	3.51	Q	.	.	.	V.
24.417	36.8272	2.68	Q	.	.	.	V.
24.500	36.8419	2.13	Q	.	.	.	V.
24.583	36.8541	1.77	Q	.	.	.	V.
24.667	36.8644	1.50	Q	.	.	.	V.
24.750	36.8733	1.28	Q	.	.	.	V.
24.833	36.8808	1.10	Q	.	.	.	V.
24.917	36.8874	0.96	Q	.	.	.	V.
25.000	36.8932	0.84	Q	.	.	.	V.
25.083	36.8982	0.73	Q	.	.	.	V.
25.167	36.9026	0.63	Q	.	.	.	V.
25.250	36.9064	0.55	Q	.	.	.	V.
25.333	36.9098	0.49	Q	.	.	.	V.
25.417	36.9127	0.43	Q	.	.	.	V.
25.500	36.9153	0.38	Q	.	.	.	V.
25.583	36.9176	0.33	Q	.	.	.	V.
25.667	36.9196	0.29	Q	.	.	.	V.
25.750	36.9213	0.25	Q	.	.	.	V.
25.833	36.9228	0.22	Q	.	.	.	V.
25.917	36.9241	0.19	Q	.	.	.	V.
26.000	36.9252	0.16	Q	.	.	.	V.
26.083	36.9262	0.14	Q	.	.	.	V.
26.167	36.9270	0.12	Q	.	.	.	V.
26.250	36.9277	0.10	Q	.	.	.	V.
26.333	36.9284	0.09	Q	.	.	.	V.
26.417	36.9289	0.08	Q	.	.	.	V.
26.500	36.9294	0.07	Q	.	.	.	V.
26.583	36.9298	0.06	Q	.	.	.	V.
26.667	36.9301	0.05	Q	.	.	.	V.
26.750	36.9304	0.03	Q	.	.	.	V.

26.833	36.9305	0.02	Q	.	.	.	V.
26.917	36.9306	0.01	Q	.	.	.	V

TIME DURATION(minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:
 (Note: 100% of Peak Flow Rate estimate assumed to have
 an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
=====	=====
0%	1615.0
10%	135.0
20%	70.0
30%	50.0
40%	35.0
50%	30.0
60%	20.0
70%	20.0
80%	10.0
90%	10.0

END OF FLOODSCx ROUTING ANALYSIS

FLOOD ROUTING ANALYSIS
USING COUNTY HYDROLOGY MANUAL OF SAN BERNARDINO(1986)
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Analysis prepared by:

***** DESCRIPTION OF STUDY *****

- * LOCKHART SOLAR *
- * 100-YR STORM EVENT *
- * AREA 2 EXISTING CONDITION *

FILE NAME: LOEX2.DAT
TIME/DATE OF STUDY: 10:25 07/08/2021

FLOW PROCESS FROM NODE 200.00 TO NODE 201.00 IS CODE = 1

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS)<<<<<
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(UNIT-HYDROGRAPH ADDED TO STREAM #1)

WATERCOURSE LENGTH = 5520.000 FEET
 LENGTH FROM CONCENTRATION POINT TO CENTROID = 2488.000 FEET
 ELEVATION VARIATION ALONG WATERCOURSE = 19.000 FEET
 BASIN FACTOR = 0.035
 WATERSHED AREA = 218.070 ACRES
 BASEFLOW = 0.000 CFS/SQUARE-MILE
 WATERCOURSE "LAG" TIME = 0.370 HOURS
 CAUTION: LAG TIME IS LESS THAN 0.50 HOURS.
 THE 5-MINUTE PERIOD UH MODEL (USED IN THIS COMPUTER PROGRAM)
 MAY BE TOO LARGE FOR PEAK FLOW ESTIMATES.
 DESERT(UNDEVELOPED) S-GRAPH SELECTED
 MAXIMUM WATERSHED LOSS RATE(INCH/HOUR) = 0.350
 LOW LOSS FRACTION = 0.620
 HYDROGRAPH MODEL #1 SPECIFIED

SPECIFIED PEAK 5-MINUTES RAINFALL(INCH)= 0.33
 SPECIFIED PEAK 30-MINUTES RAINFALL(INCH)= 0.78

SPECIFIED PEAK 1-HOUR RAINFALL(INCH) = 1.05
 SPECIFIED PEAK 3-HOUR RAINFALL(INCH) = 1.50
 SPECIFIED PEAK 6-HOUR RAINFALL(INCH) = 1.85
 SPECIFIED PEAK 24-HOUR RAINFALL(INCH) = 2.91

PRECIPITATION DEPTH-AREA REDUCTION FACTORS:

5-MINUTE FACTOR = 0.990
 30-MINUTE FACTOR = 0.990
 1-HOUR FACTOR = 0.990
 3-HOUR FACTOR = 0.999
 6-HOUR FACTOR = 0.999
 24-HOUR FACTOR = 1.000

UNIT HYDROGRAPH TIME UNIT = 5.000 MINUTES
 UNIT INTERVAL PERCENTAGE OF LAG-TIME = 22.526

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UNIT HYDROGRAPH DETERMINATION

INTERVAL NUMBER	"S" GRAPH MEAN VALUES	UNIT HYDROGRAPH ORDINATES(CFS)
1	1.170	30.846
2	5.502	114.251
3	14.926	248.541
4	34.349	512.235
5	50.449	424.607
6	60.709	270.595
7	67.517	179.529
8	72.753	138.107
9	76.851	108.071
10	80.147	86.923
11	82.844	71.126
12	85.167	61.262
13	87.218	54.086
14	88.923	44.970
15	90.283	35.855
16	91.533	32.979
17	92.635	29.055
18	93.638	26.464
19	94.469	21.900
20	95.235	20.201
21	95.906	17.713
22	96.492	15.444
23	97.019	13.917
24	97.428	10.780
25	97.803	9.883

26	98.044	6.353
27	98.272	6.023
28	98.533	6.887
29	98.804	7.125
30	99.074	7.130
31	99.344	7.125
32	99.614	7.125
33	99.884	7.125
34	100.000	3.048

TOTAL SOIL-LOSS VOLUME(ACRE-FEET) = 27.1967
TOTAL STORM RUNOFF VOLUME(ACRE-FEET) = 25.6488

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2 4 - H O U R S T O R M
R U N O F F H Y D R O G R A P H

HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS(CFS)
(Note: Time indicated is at END of Each Unit Intervals)

TIME(HRS)	VOLUME(AF)	Q(CFS)	0.	75.0	150.0	225.0	300.0
0.083	0.0003	0.04	Q
0.167	0.0015	0.18	Q
0.250	0.0049	0.50	Q
0.333	0.0128	1.14	Q
0.417	0.0243	1.68	Q
0.500	0.0383	2.02	Q
0.583	0.0538	2.26	Q
0.667	0.0706	2.44	Q
0.750	0.0884	2.58	Q
0.833	0.1070	2.70	Q
0.917	0.1262	2.80	Q
1.000	0.1461	2.89	Q
1.083	0.1665	2.96	Q
1.167	0.1874	3.03	Q
1.250	0.2087	3.09	Q
1.333	0.2303	3.14	Q
1.417	0.2523	3.19	Q
1.500	0.2745	3.23	Q
1.583	0.2971	3.27	Q
1.667	0.3199	3.31	Q
1.750	0.3429	3.35	Q
1.833	0.3662	3.38	Q
1.917	0.3897	3.41	Q

2.000	0.4133	3.44	Q
2.083	0.4372	3.46	Q
2.167	0.4611	3.48	Q
2.250	0.4853	3.50	Q
2.333	0.5096	3.53	Q
2.417	0.5340	3.55	Q
2.500	0.5586	3.57	Q
2.583	0.5834	3.60	Q
2.667	0.6083	3.62	Q
2.750	0.6334	3.64	Q
2.833	0.6586	3.66	QV
2.917	0.6839	3.68	QV
3.000	0.7093	3.69	QV
3.083	0.7348	3.71	QV
3.167	0.7605	3.72	QV
3.250	0.7862	3.74	QV
3.333	0.8121	3.75	QV
3.417	0.8380	3.77	QV
3.500	0.8641	3.79	QV
3.583	0.8903	3.80	QV
3.667	0.9166	3.82	QV
3.750	0.9430	3.83	QV
3.833	0.9695	3.85	QV
3.917	0.9962	3.87	QV
4.000	1.0229	3.89	QV
4.083	1.0498	3.90	QV
4.167	1.0768	3.92	QV
4.250	1.1039	3.94	QV
4.333	1.1312	3.96	QV
4.417	1.1586	3.97	QV
4.500	1.1861	3.99	QV
4.583	1.2137	4.01	QV
4.667	1.2415	4.03	QV
4.750	1.2693	4.05	QV
4.833	1.2974	4.07	Q V
4.917	1.3255	4.09	Q V
5.000	1.3538	4.11	Q V
5.083	1.3823	4.13	Q V
5.167	1.4108	4.15	Q V
5.250	1.4396	4.17	Q V
5.333	1.4684	4.19	Q V
5.417	1.4974	4.21	Q V
5.500	1.5266	4.23	Q V
5.583	1.5559	4.25	Q V
5.667	1.5853	4.28	Q V
5.750	1.6149	4.30	Q V
5.833	1.6447	4.32	Q V
5.917	1.6746	4.34	Q V
6.000	1.7046	4.37	Q V
6.083	1.7349	4.39	Q V

6.167	1.7653	4.41	Q V
6.250	1.7958	4.44	Q V
6.333	1.8265	4.46	Q V
6.417	1.8574	4.49	Q V
6.500	1.8885	4.51	Q V
6.583	1.9197	4.54	Q V
6.667	1.9512	4.56	Q V
6.750	1.9827	4.59	Q V
6.833	2.0145	4.61	Q V
6.917	2.0465	4.64	Q V
7.000	2.0786	4.67	Q V
7.083	2.1110	4.69	Q V
7.167	2.1435	4.72	Q V
7.250	2.1762	4.75	Q V
7.333	2.2091	4.78	Q V
7.417	2.2422	4.81	Q V
7.500	2.2756	4.84	Q V
7.583	2.3091	4.87	Q V
7.667	2.3429	4.90	Q V
7.750	2.3768	4.93	Q V
7.833	2.4110	4.96	Q V
7.917	2.4454	4.99	Q V
8.000	2.4800	5.03	Q V
8.083	2.5149	5.06	Q V
8.167	2.5499	5.09	Q V
8.250	2.5853	5.13	Q V
8.333	2.6208	5.16	Q V
8.417	2.6566	5.20	Q V
8.500	2.6927	5.23	Q V
8.583	2.7290	5.27	Q V
8.667	2.7655	5.31	Q V
8.750	2.8023	5.35	Q V
8.833	2.8394	5.39	Q V
8.917	2.8768	5.42	Q V
9.000	2.9144	5.46	Q V
9.083	2.9523	5.51	Q V
9.167	2.9905	5.55	Q V
9.250	3.0290	5.59	Q V
9.333	3.0678	5.63	Q V
9.417	3.1069	5.68	Q V
9.500	3.1463	5.72	Q V
9.583	3.1860	5.77	Q V
9.667	3.2261	5.81	Q V
9.750	3.2665	5.86	Q V
9.833	3.3072	5.91	Q V
9.917	3.3482	5.96	Q V
10.000	3.3896	6.01	Q V
10.083	3.4314	6.06	Q V
10.167	3.4735	6.12	Q V
10.250	3.5160	6.17	Q V

10.333	3.5589	6.23	Q	V
10.417	3.6021	6.28	Q	V
10.500	3.6458	6.34	Q	V
10.583	3.6899	6.40	Q	V
10.667	3.7344	6.46	Q	V
10.750	3.7793	6.52	Q	V
10.833	3.8246	6.59	Q	V
10.917	3.8704	6.65	Q	V
11.000	3.9167	6.72	Q	V
11.083	3.9635	6.79	Q	V
11.167	4.0107	6.86	Q	V
11.250	4.0584	6.93	Q	V
11.333	4.1067	7.00	Q	V
11.417	4.1554	7.08	Q	V
11.500	4.2047	7.16	Q	V
11.583	4.2546	7.24	Q	V
11.667	4.3050	7.32	Q	V
11.750	4.3560	7.41	Q	V
11.833	4.4076	7.50	Q	V
11.917	4.4599	7.58	.Q	V
12.000	4.5127	7.68	.Q	V
12.083	4.5662	7.77	.Q	V
12.167	4.6202	7.84	.Q	V
12.250	4.6745	7.89	.Q	V
12.333	4.7288	7.87	.Q	V
12.417	4.7831	7.88	.Q	V
12.500	4.8377	7.93	.Q	V
12.583	4.8929	8.01	.Q	V
12.667	4.9486	8.09	.Q	V
12.750	5.0050	8.19	.Q	V
12.833	5.0622	8.30	.Q	V
12.917	5.1201	8.41	.Q	V
13.000	5.1789	8.54	.Q	V
13.083	5.2386	8.67	.Q	V
13.167	5.2992	8.81	.Q	V
13.250	5.3609	8.95	.Q	V
13.333	5.4236	9.11	.Q	V
13.417	5.4874	9.26	.Q	V
13.500	5.5524	9.44	.Q	V
13.583	5.6186	9.61	.Q	V
13.667	5.6861	9.81	.Q	V
13.750	5.7550	10.00	.Q	V
13.833	5.8254	10.22	.Q	V.
13.917	5.8973	10.44	.Q	V.
14.000	5.9709	10.68	.Q	V.
14.083	6.0462	10.94	.Q	V.
14.167	6.1238	11.26	.Q	V.
14.250	6.2041	11.66	.Q	V.
14.333	6.2881	12.20	.Q	V.
14.417	6.3757	12.72	.Q	V.

14.500	6.4667	13.21	.Q	V	.	.	.
14.583	6.5608	13.67	.Q	V	.	.	.
14.667	6.6583	14.16	.Q	V	.	.	.
14.750	6.7592	14.65	.Q	V	.	.	.
14.833	6.8638	15.19	. Q	V	.	.	.
14.917	6.9723	15.76	. Q	V	.	.	.
15.000	7.0852	16.39	. Q	.V	.	.	.
15.083	7.2027	17.06	. Q	.V	.	.	.
15.167	7.3254	17.82	. Q	.V	.	.	.
15.250	7.4537	18.64	. Q	.V	.	.	.
15.333	7.5886	19.58	. Q	.V	.	.	.
15.417	7.7314	20.74	. Q	. V	.	.	.
15.500	7.8856	22.38	. Q	. V	.	.	.
15.583	8.0554	24.67	. Q	. V	.	.	.
15.667	8.2501	28.26	. Q	. V	.	.	.
15.750	8.4734	32.43	. Q	. V	.	.	.
15.833	8.7357	38.09	. Q	. V	.	.	.
15.917	9.0605	47.16	. Q	. V	.	.	.
16.000	9.4982	63.55	. Q	. V	.	.	.
16.083	10.1296	91.67	.	. Q	. V	.	.
16.167	11.0663	136.01	.	.	. VQ	.	.
16.250	12.3647	188.53	.	.	. V.	. Q	.
16.333	14.0801	249.07	.	.	.V	.	. Q
16.417	15.5705	216.41 V	. Q
16.500	16.6812	161.28	.	.	.Q	. V	.
16.583	17.5250	122.52	.	.	. Q	. V	.
16.667	18.2076	99.11	.	.	. Q	. V	.
16.750	18.7742	82.26	.	. Q	.	. V.	.
16.833	19.2564	70.02	.	. Q.	.	. V	.
16.917	19.6746	60.73	.	. Q	.	. V	.
17.000	20.0457	53.88	.	. Q	.	.V	.
17.083	20.3780	48.26	.	. Q	.	.V	.
17.167	20.6724	42.74	.	. Q	.	. V	.
17.250	20.9323	37.75	.	. Q	.	. V	.
17.333	21.1697	34.47	.	. Q	.	. V	.
17.417	21.3857	31.35	.	. Q	.	. V	.
17.500	21.5838	28.77	. Q	.	.	. V	.
17.583	21.7631	26.04	. Q	.	.	. V	.
17.667	21.9290	24.08	. Q	.	.	. V	.
17.750	22.0814	22.14	. Q	.	.	. V	.
17.833	22.2214	20.32	. Q	.	.	. V	.
17.917	22.3506	18.76	. Q	.	.	. V	.
18.000	22.4675	16.98	. Q	.	.	. V	.
18.083	22.5764	15.80	. Q	.	.	. V	.
18.167	22.6749	14.31	.Q	.	.	. V	.
18.250	22.7696	13.74	.Q	.	.	. V	.
18.333	22.8635	13.64	.Q	.	.	. V	.
18.417	22.9557	13.40	.Q	.	.	. V	.
18.500	23.0454	13.02	.Q	.	.	. V	.
18.583	23.1318	12.55	.Q	.	.	. V	.

18.667	23.2142	11.97	.Q	.	.	.	V	.
18.750	23.2913	11.18	.Q	.	.	.	V	.
18.833	23.3569	9.53	.Q	.	.	.	V	.
18.917	23.4138	8.27	.Q	.	.	.	V	.
19.000	23.4680	7.87	.Q	.	.	.	V	.
19.083	23.5204	7.60	.Q	.	.	.	V	.
19.167	23.5712	7.39	Q	.	.	.	V	.
19.250	23.6208	7.20	Q	.	.	.	V	.
19.333	23.6692	7.03	Q	.	.	.	V	.
19.417	23.7165	6.87	Q	.	.	.	V	.
19.500	23.7628	6.72	Q	.	.	.	V	.
19.583	23.8082	6.58	Q	.	.	.	V	.
19.667	23.8526	6.45	Q	.	.	.	V	.
19.750	23.8961	6.32	Q	.	.	.	V	.
19.833	23.9389	6.21	Q	.	.	.	V	.
19.917	23.9808	6.10	Q	.	.	.	V	.
20.000	24.0221	5.99	Q	.	.	.	V	.
20.083	24.0626	5.89	Q	.	.	.	V	.
20.167	24.1025	5.79	Q	.	.	.	V	.
20.250	24.1418	5.70	Q	.	.	.	V	.
20.333	24.1804	5.61	Q	.	.	.	V	.
20.417	24.2184	5.52	Q	.	.	.	V	.
20.500	24.2559	5.44	Q	.	.	.	V	.
20.583	24.2928	5.36	Q	.	.	.	V	.
20.667	24.3292	5.29	Q	.	.	.	V	.
20.750	24.3651	5.21	Q	.	.	.	V	.
20.833	24.4006	5.14	Q	.	.	.	V	.
20.917	24.4355	5.07	Q	.	.	.	V	.
21.000	24.4700	5.01	Q	.	.	.	V	.
21.083	24.5040	4.94	Q	.	.	.	V	.
21.167	24.5376	4.88	Q	.	.	.	V	.
21.250	24.5708	4.82	Q	.	.	.	V	.
21.333	24.6035	4.76	Q	.	.	.	V	.
21.417	24.6359	4.70	Q	.	.	.	V	.
21.500	24.6679	4.65	Q	.	.	.	V	.
21.583	24.6995	4.59	Q	.	.	.	V	.
21.667	24.7308	4.54	Q	.	.	.	V	.
21.750	24.7617	4.49	Q	.	.	.	V	.
21.833	24.7923	4.44	Q	.	.	.	V	.
21.917	24.8225	4.39	Q	.	.	.	V	.
22.000	24.8524	4.34	Q	.	.	.	V	.
22.083	24.8820	4.30	Q	.	.	.	V	.
22.167	24.9113	4.25	Q	.	.	.	V	.
22.250	24.9403	4.21	Q	.	.	.	V	.
22.333	24.9691	4.17	Q	.	.	.	V	.
22.417	24.9975	4.13	Q	.	.	.	V	.
22.500	25.0256	4.09	Q	.	.	.	V.	.
22.583	25.0535	4.05	Q	.	.	.	V.	.
22.667	25.0812	4.01	Q	.	.	.	V.	.
22.750	25.1085	3.97	Q	.	.	.	V.	.

22.833	25.1356	3.94	Q	.	.	.	V.
22.917	25.1625	3.90	Q	.	.	.	V.
23.000	25.1891	3.87	Q	.	.	.	V.
23.083	25.2155	3.83	Q	.	.	.	V.
23.167	25.2417	3.80	Q	.	.	.	V.
23.250	25.2676	3.77	Q	.	.	.	V.
23.333	25.2934	3.74	Q	.	.	.	V.
23.417	25.3189	3.70	Q	.	.	.	V.
23.500	25.3442	3.67	Q	.	.	.	V.
23.583	25.3693	3.64	Q	.	.	.	V.
23.667	25.3942	3.61	Q	.	.	.	V.
23.750	25.4189	3.59	Q	.	.	.	V.
23.833	25.4434	3.56	Q	.	.	.	V.
23.917	25.4677	3.53	Q	.	.	.	V.
24.000	25.4918	3.50	Q	.	.	.	V.
24.083	25.5155	3.44	Q	.	.	.	V.
24.167	25.5380	3.27	Q	.	.	.	V.
24.250	25.5582	2.93	Q	.	.	.	V.
24.333	25.5738	2.27	Q	.	.	.	V.
24.417	25.5856	1.72	Q	.	.	.	V.
24.500	25.5951	1.37	Q	.	.	.	V.
24.583	25.6029	1.13	Q	.	.	.	V.
24.667	25.6094	0.95	Q	.	.	.	V.
24.750	25.6150	0.81	Q	.	.	.	V.
24.833	25.6197	0.69	Q	.	.	.	V.
24.917	25.6239	0.60	Q	.	.	.	V.
25.000	25.6274	0.52	Q	.	.	.	V.
25.083	25.6305	0.45	Q	.	.	.	V.
25.167	25.6332	0.39	Q	.	.	.	V.
25.250	25.6355	0.34	Q	.	.	.	V.
25.333	25.6375	0.29	Q	.	.	.	V.
25.417	25.6393	0.26	Q	.	.	.	V.
25.500	25.6408	0.22	Q	.	.	.	V.
25.583	25.6421	0.19	Q	.	.	.	V.
25.667	25.6432	0.16	Q	.	.	.	V.
25.750	25.6442	0.14	Q	.	.	.	V.
25.833	25.6451	0.12	Q	.	.	.	V.
25.917	25.6458	0.10	Q	.	.	.	V.
26.000	25.6464	0.09	Q	.	.	.	V.
26.083	25.6469	0.08	Q	.	.	.	V.
26.167	25.6473	0.07	Q	.	.	.	V.
26.250	25.6477	0.06	Q	.	.	.	V.
26.333	25.6481	0.05	Q	.	.	.	V.
26.417	25.6484	0.04	Q	.	.	.	V.
26.500	25.6486	0.03	Q	.	.	.	V.
26.583	25.6487	0.02	Q	.	.	.	V.
26.667	25.6488	0.01	Q	.	.	.	V.
26.750	25.6488	0.00	Q	.	.	.	V

TIME DURATION(minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:

(Note: 100% of Peak Flow Rate estimate assumed to have an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
0%	1605.0
10%	120.0
20%	65.0
30%	45.0
40%	30.0
50%	25.0
60%	20.0
70%	15.0
80%	10.0
90%	5.0

END OF FLOODSCx ROUTING ANALYSIS

F L O O D R O U T I N G A N A L Y S I S
USING COUNTY HYDROLOGY MANUAL OF SAN BERNARDINO(1986)
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Analysis prepared by:

***** DESCRIPTION OF STUDY *****

- * LOCKHART SOLAR *
 - * 100-YR STORM EVENT *
 - * AREA 3 EXISTING CONDITIONS *
- *****

FILE NAME: LOEX3.DAT
TIME/DATE OF STUDY: 10:33 07/08/2021

FLOW PROCESS FROM NODE 300.00 TO NODE 301.00 IS CODE = 1

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS)<<<<<
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(UNIT-HYDROGRAPH ADDED TO STREAM #1)

WATERCOURSE LENGTH = 8570.000 FEET
 LENGTH FROM CONCENTRATION POINT TO CENTROID = 4508.000 FEET
 ELEVATION VARIATION ALONG WATERCOURSE = 49.000 FEET
 BASIN FACTOR = 0.035
 WATERSHED AREA = 176.340 ACRES
 BASEFLOW = 0.000 CFS/SQUARE-MILE
 WATERCOURSE "LAG" TIME = 0.498 HOURS
 CAUTION: LAG TIME IS LESS THAN 0.50 HOURS.
 THE 5-MINUTE PERIOD UH MODEL (USED IN THIS COMPUTER PROGRAM)
 MAY BE TOO LARGE FOR PEAK FLOW ESTIMATES.
 DESERT(UNDEVELOPED) S-GRAPH SELECTED
 MAXIMUM WATERSHED LOSS RATE(INCH/HOUR) = 0.350
 LOW LOSS FRACTION = 0.620
 HYDROGRAPH MODEL #1 SPECIFIED

SPECIFIED PEAK 5-MINUTES RAINFALL(INCH)= 0.33
 SPECIFIED PEAK 30-MINUTES RAINFALL(INCH)= 0.78

SPECIFIED PEAK 1-HOUR RAINFALL(INCH) = 1.05
 SPECIFIED PEAK 3-HOUR RAINFALL(INCH) = 1.50
 SPECIFIED PEAK 6-HOUR RAINFALL(INCH) = 1.85
 SPECIFIED PEAK 24-HOUR RAINFALL(INCH) = 2.91

PRECIPITATION DEPTH-AREA REDUCTION FACTORS:

5-MINUTE FACTOR = 0.992
 30-MINUTE FACTOR = 0.992
 1-HOUR FACTOR = 0.992
 3-HOUR FACTOR = 0.999
 6-HOUR FACTOR = 0.999
 24-HOUR FACTOR = 1.000

UNIT HYDROGRAPH TIME UNIT = 5.000 MINUTES
 UNIT INTERVAL PERCENTAGE OF LAG-TIME = 16.744

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UNIT HYDROGRAPH DETERMINATION

INTERVAL NUMBER	"S" GRAPH MEAN VALUES	UNIT HYDROGRAPH ORDINATES(CFS)
1	0.780	16.630
2	3.392	55.702
3	7.808	94.182
4	16.237	179.758
5	31.276	320.724
6	44.847	289.415
7	54.506	205.980
8	61.351	145.989
9	66.456	108.863
10	70.648	89.399
11	74.102	73.676
12	77.046	62.781
13	79.528	52.926
14	81.675	45.779
15	83.521	39.376
16	85.215	36.134
17	86.778	33.332
18	88.141	29.055
19	89.286	24.427
20	90.279	21.174
21	91.222	20.105
22	92.080	18.304
23	92.872	16.881
24	93.613	15.801
25	94.248	13.552

26	94.817	12.142
27	95.385	12.115
28	95.872	10.383
29	96.307	9.279
30	96.742	9.261
31	97.093	7.483
32	97.394	6.428
33	97.694	6.398
34	97.909	4.592
35	98.077	3.572
36	98.245	3.582
37	98.434	4.040
38	98.635	4.279
39	98.836	4.289
40	99.037	4.284
41	99.238	4.284
42	99.438	4.284
43	99.639	4.284
44	99.840	4.284
45	100.000	3.408

TOTAL SOIL-LOSS VOLUME(ACRE-FEET) = 21.9796
TOTAL STORM RUNOFF VOLUME(ACRE-FEET) = 20.7570

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2 4 - H O U R S T O R M
R U N O F F H Y D R O G R A P H

HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS(CFS)
(Note: Time indicated is at END of Each Unit Intervals)

TIME(HRS)	VOLUME(AF)	Q(CFS)	0.	50.0	100.0	150.0	200.0
0.083	0.0001	0.02	Q
0.167	0.0008	0.09	Q
0.250	0.0022	0.21	Q
0.333	0.0052	0.44	Q
0.417	0.0110	0.84	Q
0.500	0.0193	1.21	Q
0.583	0.0294	1.47	Q
0.667	0.0408	1.66	Q
0.750	0.0532	1.80	Q
0.833	0.0665	1.92	Q
0.917	0.0804	2.02	Q
1.000	0.0949	2.10	Q

1.083	0.1099	2.18	Q
1.167	0.1253	2.24	Q
1.250	0.1412	2.30	Q
1.333	0.1574	2.36	Q
1.417	0.1740	2.41	Q
1.500	0.1909	2.45	Q
1.583	0.2080	2.49	Q
1.667	0.2254	2.53	Q
1.750	0.2431	2.56	Q
1.833	0.2609	2.59	Q
1.917	0.2790	2.63	Q
2.000	0.2973	2.66	Q
2.083	0.3158	2.68	Q
2.167	0.3345	2.71	Q
2.250	0.3533	2.73	Q
2.333	0.3723	2.76	Q
2.417	0.3914	2.78	Q
2.500	0.4107	2.80	Q
2.583	0.4302	2.82	Q
2.667	0.4498	2.84	Q
2.750	0.4695	2.86	Q
2.833	0.4893	2.88	Q
2.917	0.5093	2.90	Q
3.000	0.5293	2.91	QV
3.083	0.5495	2.93	QV
3.167	0.5698	2.95	QV
3.250	0.5902	2.96	QV
3.333	0.6107	2.98	QV
3.417	0.6314	3.00	QV
3.500	0.6522	3.02	QV
3.583	0.6731	3.04	QV
3.667	0.6941	3.05	QV
3.750	0.7153	3.07	QV
3.833	0.7365	3.09	QV
3.917	0.7579	3.10	QV
4.000	0.7793	3.11	QV
4.083	0.8009	3.13	QV
4.167	0.8225	3.14	QV
4.250	0.8442	3.15	QV
4.333	0.8660	3.17	QV
4.417	0.8880	3.18	QV
4.500	0.9100	3.20	QV
4.583	0.9321	3.21	QV
4.667	0.9543	3.23	QV
4.750	0.9767	3.24	QV
4.833	0.9991	3.26	QV
4.917	1.0216	3.27	QV
5.000	1.0443	3.29	Q V
5.083	1.0670	3.30	Q V
5.167	1.0899	3.32	Q V

5.250	1.1129	3.34	Q V
5.333	1.1360	3.35	Q V
5.417	1.1592	3.37	Q V
5.500	1.1825	3.39	Q V
5.583	1.2059	3.40	Q V
5.667	1.2295	3.42	Q V
5.750	1.2532	3.44	Q V
5.833	1.2770	3.46	Q V
5.917	1.3009	3.47	Q V
6.000	1.3249	3.49	Q V
6.083	1.3491	3.51	Q V
6.167	1.3734	3.53	Q V
6.250	1.3978	3.55	Q V
6.333	1.4224	3.57	Q V
6.417	1.4471	3.59	Q V
6.500	1.4719	3.60	Q V
6.583	1.4969	3.62	Q V
6.667	1.5220	3.64	Q V
6.750	1.5472	3.67	Q V
6.833	1.5726	3.69	Q V
6.917	1.5981	3.71	Q V
7.000	1.6238	3.73	Q V
7.083	1.6496	3.75	Q V
7.167	1.6756	3.77	Q V
7.250	1.7017	3.79	Q V
7.333	1.7280	3.82	Q V
7.417	1.7545	3.84	Q V
7.500	1.7811	3.86	Q V
7.583	1.8079	3.89	Q V
7.667	1.8348	3.91	Q V
7.750	1.8619	3.94	Q V
7.833	1.8892	3.96	Q V
7.917	1.9166	3.98	Q V
8.000	1.9442	4.01	Q V
8.083	1.9720	4.04	Q V
8.167	2.0000	4.06	Q V
8.250	2.0282	4.09	Q V
8.333	2.0565	4.12	Q V
8.417	2.0851	4.14	Q V
8.500	2.1138	4.17	Q V
8.583	2.1427	4.20	Q V
8.667	2.1719	4.23	Q V
8.750	2.2012	4.26	Q V
8.833	2.2307	4.29	Q V
8.917	2.2605	4.32	Q V
9.000	2.2905	4.35	Q V
9.083	2.3207	4.38	Q V
9.167	2.3511	4.42	Q V
9.250	2.3817	4.45	Q V
9.333	2.4126	4.48	Q V

9.417	2.4437	4.52	Q	V
9.500	2.4751	4.55	Q	V
9.583	2.5067	4.59	Q	V
9.667	2.5385	4.62	Q	V
9.750	2.5706	4.66	Q	V
9.833	2.6030	4.70	Q	V
9.917	2.6356	4.74	Q	V
10.000	2.6685	4.78	Q	V
10.083	2.7017	4.82	Q	V
10.167	2.7352	4.86	Q	V
10.250	2.7689	4.90	Q	V
10.333	2.8030	4.94	Q	V
10.417	2.8373	4.99	Q	V
10.500	2.8720	5.03	.Q	V
10.583	2.9070	5.08	.Q	V
10.667	2.9423	5.13	.Q	V
10.750	2.9779	5.17	.Q	V
10.833	3.0139	5.22	.Q	V
10.917	3.0502	5.27	.Q	V
11.000	3.0869	5.32	.Q	V
11.083	3.1239	5.38	.Q	V
11.167	3.1613	5.43	.Q	V
11.250	3.1991	5.49	.Q	V
11.333	3.2373	5.54	.Q	V
11.417	3.2759	5.60	.Q	V
11.500	3.3149	5.66	.Q	V
11.583	3.3544	5.73	.Q	V
11.667	3.3942	5.79	.Q	V
11.750	3.4346	5.85	.Q	V
11.833	3.4753	5.92	.Q	V
11.917	3.5166	5.99	.Q	V
12.000	3.5584	6.06	.Q	V
12.083	3.6006	6.13	.Q	V
12.167	3.6433	6.20	.Q	V
12.250	3.6863	6.25	.Q	V
12.333	3.7296	6.29	.Q	V
12.417	3.7730	6.30	.Q	V
12.500	3.8165	6.32	.Q	V
12.583	3.8603	6.36	.Q	V
12.667	3.9045	6.41	.Q	V
12.750	3.9491	6.48	.Q	V
12.833	3.9943	6.56	.Q	V
12.917	4.0401	6.64	.Q	V
13.000	4.0864	6.73	.Q	V
13.083	4.1334	6.83	.Q	V
13.167	4.1811	6.93	.Q	V
13.250	4.2296	7.03	.Q	V
13.333	4.2788	7.15	.Q	V
13.417	4.3288	7.27	.Q	V
13.500	4.3797	7.39	.Q	V

13.583	4.4316	7.52	.Q	V
13.667	4.4843	7.66	.Q	V
13.750	4.5381	7.81	.Q	V
13.833	4.5930	7.97	.Q	V
13.917	4.6490	8.13	.Q	V
14.000	4.7062	8.30	.Q	V.
14.083	4.7647	8.50	.Q	V.
14.167	4.8247	8.71	.Q	V.
14.250	4.8865	8.96	.Q	V.
14.333	4.9503	9.26	.Q	V.
14.417	5.0167	9.64	.Q	V.
14.500	5.0857	10.02	. Q	V.
14.583	5.1573	10.39	. Q	V.
14.667	5.2313	10.75	. Q	V
14.750	5.3079	11.13	. Q	V
14.833	5.3872	11.51	. Q	V
14.917	5.4694	11.93	. Q	V
15.000	5.5546	12.37	. Q	V
15.083	5.6432	12.86	. Q	V
15.167	5.7354	13.38	. Q	.V
15.250	5.8315	13.97	. Q	.V
15.333	5.9322	14.61	. Q	.V
15.417	6.0383	15.40	. Q	.V
15.500	6.1514	16.42	. Q	.V
15.583	6.2734	17.72	. Q	. V
15.667	6.4079	19.52	. Q	. V
15.750	6.5616	22.33	. Q	. V
15.833	6.7405	25.97	. Q	. V
15.917	6.9535	30.93	. Q	. V
16.000	7.2218	38.96	. Q	. V
16.083	7.5987	54.72	. Q	. V
16.167	8.1324	77.49	.	. Q
16.250	8.8366	102.25	.	.	. V	. Q	.	.
16.333	9.7658	134.92	.	.	. V	. Q	. Q	.
16.417	10.9265	168.54V	. Q	. Q
16.500	11.9887	154.23 V	. Q	.
16.583	12.8354	122.94 Q	. V	.
16.667	13.5052	97.27	.	.	. Q.	. V	. V	.
16.750	14.0505	79.18	.	.	. Q	.	. V	.
16.833	14.5151	67.46	.	. Q	.	.	. V	.
16.917	14.9174	58.40	.	.Q	.	.	. V	.
17.000	15.2723	51.54	.	. Q	.	.	. V.	.
17.083	15.5873	45.74	.	. Q.	.	.	. V	.
17.167	15.8705	41.12	.	. Q	.	.	. V	.
17.250	16.1266	37.19	.	. Q	.	.	.V	.
17.333	16.3625	34.25	.	. Q	.	.	.V	.
17.417	16.5797	31.53	.	. Q	.	.	.V	.
17.500	16.7769	28.64	.	. Q	.	.	. V	.
17.583	16.9554	25.93	.	. Q	.	.	. V	.
17.667	17.1195	23.82	.	. Q	.	.	. V	.

17.750	17.2736	22.38	.	Q	V	.
17.833	17.4177	20.92	.	Q	V	.
17.917	17.5529	19.63	.	Q	V	.
18.000	17.6799	18.45	.	Q	V	.
18.083	17.7977	17.10	.	Q	V	.
18.167	17.9083	16.05	.	Q	V	.
18.250	18.0138	15.33	.	Q	V	.
18.333	18.1124	14.32	.	Q	V	.
18.417	18.2056	13.52	.	Q	V	.
18.500	18.2948	12.95	.	Q	V	.
18.583	18.3775	12.02	.	Q	V	.
18.667	18.4552	11.28	.	Q	V	.
18.750	18.5294	10.77	.	Q	V	.
18.833	18.5980	9.96	.	.Q	V	.
18.917	18.6628	9.41	.	.Q	V	.
19.000	18.7262	9.21	.	.Q	V	.
19.083	18.7892	9.14	.	.Q	V	.
19.167	18.8513	9.02	.	.Q	V	.
19.250	18.9122	8.85	.	.Q	V	.
19.333	18.9719	8.66	.	.Q	V	.
19.417	19.0301	8.46	.	.Q	V	.
19.500	19.0866	8.20	.	.Q	V	.
19.583	19.1410	7.90	.	.Q	V	.
19.667	19.1925	7.49	.	.Q	V	.
19.750	19.2394	6.80	.	.Q	V	.
19.833	19.2785	5.68	.	.Q	V	.
19.917	19.3157	5.40	.	.Q	V	.
20.000	19.3517	5.23	.	.Q	V	.
20.083	19.3869	5.10	.	.Q	V	.
20.167	19.4212	4.99	.	Q	V	.
20.250	19.4549	4.89	.	Q	V	.
20.333	19.4880	4.80	.	Q	V	.
20.417	19.5204	4.71	.	Q	V	.
20.500	19.5523	4.63	.	Q	V	.
20.583	19.5837	4.55	.	Q	V	.
20.667	19.6145	4.47	.	Q	V	.
20.750	19.6448	4.40	.	Q	V	.
20.833	19.6747	4.34	.	Q	V	.
20.917	19.7041	4.27	.	Q	V	.
21.000	19.7331	4.21	.	Q	V	.
21.083	19.7617	4.15	.	Q	V	.
21.167	19.7899	4.10	.	Q	V	.
21.250	19.8177	4.04	.	Q	V	.
21.333	19.8452	3.99	.	Q	V	.
21.417	19.8723	3.94	.	Q	V	.
21.500	19.8991	3.89	.	Q	V	.
21.583	19.9255	3.84	.	Q	V	.
21.667	19.9517	3.79	.	Q	V	.
21.750	19.9775	3.75	.	Q	V	.
21.833	20.0030	3.71	.	Q	V	.

21.917	20.0282	3.66	Q	.	.	.	V .
22.000	20.0532	3.62	Q	.	.	.	V .
22.083	20.0778	3.58	Q	.	.	.	V .
22.167	20.1022	3.54	Q	.	.	.	V .
22.250	20.1264	3.50	Q	.	.	.	V .
22.333	20.1503	3.47	Q	.	.	.	V .
22.417	20.1739	3.43	Q	.	.	.	V .
22.500	20.1973	3.40	Q	.	.	.	V .
22.583	20.2204	3.36	Q	.	.	.	V .
22.667	20.2434	3.33	Q	.	.	.	V .
22.750	20.2661	3.30	Q	.	.	.	V .
22.833	20.2886	3.26	Q	.	.	.	V .
22.917	20.3108	3.23	Q	.	.	.	V .
23.000	20.3329	3.20	Q	.	.	.	V .
23.083	20.3548	3.17	Q	.	.	.	V .
23.167	20.3764	3.15	Q	.	.	.	V .
23.250	20.3979	3.12	Q	.	.	.	V .
23.333	20.4192	3.09	Q	.	.	.	V .
23.417	20.4403	3.06	Q	.	.	.	V .
23.500	20.4612	3.04	Q	.	.	.	V .
23.583	20.4819	3.01	Q	.	.	.	V .
23.667	20.5025	2.99	Q	.	.	.	V .
23.750	20.5229	2.96	Q	.	.	.	V .
23.833	20.5431	2.94	Q	.	.	.	V .
23.917	20.5632	2.91	Q	.	.	.	V .
24.000	20.5831	2.89	Q	.	.	.	V .
24.083	20.6027	2.85	Q	.	.	.	V .
24.167	20.6216	2.75	Q	.	.	.	V .
24.250	20.6397	2.62	Q	.	.	.	V .
24.333	20.6560	2.37	Q	.	.	.	V .
24.417	20.6694	1.95	Q	.	.	.	V .
24.500	20.6802	1.57	Q	.	.	.	V .
24.583	20.6892	1.30	Q	.	.	.	V .
24.667	20.6969	1.11	Q	.	.	.	V .
24.750	20.7035	0.96	Q	.	.	.	V .
24.833	20.7093	0.84	Q	.	.	.	V .
24.917	20.7144	0.75	Q	.	.	.	V .
25.000	20.7190	0.66	Q	.	.	.	V .
25.083	20.7231	0.59	Q	.	.	.	V .
25.167	20.7267	0.53	Q	.	.	.	V .
25.250	20.7300	0.47	Q	.	.	.	V .
25.333	20.7329	0.43	Q	.	.	.	V .
25.417	20.7355	0.38	Q	.	.	.	V .
25.500	20.7378	0.34	Q	.	.	.	V .
25.583	20.7400	0.31	Q	.	.	.	V .
25.667	20.7419	0.28	Q	.	.	.	V .
25.750	20.7436	0.25	Q	.	.	.	V .
25.833	20.7452	0.23	Q	.	.	.	V .
25.917	20.7466	0.20	Q	.	.	.	V .
26.000	20.7478	0.18	Q	.	.	.	V .

26.083	20.7490	0.16	Q	.	.	.	V.
26.167	20.7500	0.15	Q	.	.	.	V.
26.250	20.7509	0.13	Q	.	.	.	V.
26.333	20.7517	0.12	Q	.	.	.	V.
26.417	20.7524	0.10	Q	.	.	.	V.
26.500	20.7530	0.09	Q	.	.	.	V.
26.583	20.7536	0.08	Q	.	.	.	V.
26.667	20.7541	0.07	Q	.	.	.	V.
26.750	20.7545	0.06	Q	.	.	.	V.
26.833	20.7549	0.06	Q	.	.	.	V.
26.917	20.7553	0.05	Q	.	.	.	V.
27.000	20.7556	0.05	Q	.	.	.	V.
27.083	20.7559	0.04	Q	.	.	.	V.
27.167	20.7562	0.04	Q	.	.	.	V.
27.250	20.7564	0.03	Q	.	.	.	V.
27.333	20.7566	0.03	Q	.	.	.	V.
27.417	20.7567	0.02	Q	.	.	.	V.
27.500	20.7568	0.02	Q	.	.	.	V.
27.583	20.7569	0.01	Q	.	.	.	V.

TIME DURATION(minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:
 (Note: 100% of Peak Flow Rate estimate assumed to have
 an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
=====	=====
0%	1655.0
10%	155.0
20%	85.0
30%	60.0
40%	45.0
50%	30.0
60%	25.0
70%	20.0
80%	15.0
90%	10.0

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END OF FLOODSCx ROUTING ANALYSIS

F L O O D R O U T I N G A N A L Y S I S
USING COUNTY HYDROLOGY MANUAL OF SAN BERNARDINO(1986)
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Ver. 23.0 Release Date: 07/01/2016 License ID 1355

Analysis prepared by:

***** DESCRIPTION OF STUDY *****

- * LOCKHART SOLAR *
 - * 100-YR STORM EVENT *
 - * AREA 1 PROPOSED CONDITIONS *
- *****

FILE NAME: LOPR1.DAT
TIME/DATE OF STUDY: 16:54 07/26/2021

FLOW PROCESS FROM NODE 100.00 TO NODE 101.00 IS CODE = 1

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS)<<<<<
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(UNIT-HYDROGRAPH ADDED TO STREAM #1)

WATERCOURSE LENGTH = 5357.000 FEET
 LENGTH FROM CONCENTRATION POINT TO CENTROID = 3838.000 FEET
 ELEVATION VARIATION ALONG WATERCOURSE = 29.000 FEET
 BASIN FACTOR = 0.035
 WATERSHED AREA = 314.560 ACRES
 BASEFLOW = 0.000 CFS/SQUARE-MILE
 WATERCOURSE "LAG" TIME = 0.396 HOURS
 CAUTION: LAG TIME IS LESS THAN 0.50 HOURS.
 THE 5-MINUTE PERIOD UH MODEL (USED IN THIS COMPUTER PROGRAM)
 MAY BE TOO LARGE FOR PEAK FLOW ESTIMATES.
 DESERT(UNDEVELOPED) S-GRAPH SELECTED
 MAXIMUM WATERSHED LOSS RATE(INCH/HOUR) = 0.348
 LOW LOSS FRACTION = 0.620
 HYDROGRAPH MODEL #1 SPECIFIED

SPECIFIED PEAK 5-MINUTES RAINFALL(INCH)= 0.33
 SPECIFIED PEAK 30-MINUTES RAINFALL(INCH)= 0.78

SPECIFIED PEAK 1-HOUR RAINFALL(INCH) = 1.05
 SPECIFIED PEAK 3-HOUR RAINFALL(INCH) = 1.50
 SPECIFIED PEAK 6-HOUR RAINFALL(INCH) = 1.85
 SPECIFIED PEAK 24-HOUR RAINFALL(INCH) = 2.91

PRECIPITATION DEPTH-AREA REDUCTION FACTORS:

5-MINUTE FACTOR = 0.986
 30-MINUTE FACTOR = 0.986
 1-HOUR FACTOR = 0.986
 3-HOUR FACTOR = 0.998
 6-HOUR FACTOR = 0.999
 24-HOUR FACTOR = 0.999

UNIT HYDROGRAPH TIME UNIT = 5.000 MINUTES
 UNIT INTERVAL PERCENTAGE OF LAG-TIME = 21.060

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UNIT HYDROGRAPH DETERMINATION

INTERVAL NUMBER	"S" GRAPH MEAN VALUES	UNIT HYDROGRAPH ORDINATES(CFS)
1	1.066	40.545
2	4.908	146.176
3	12.648	294.434
4	29.637	646.317
5	46.517	642.147
6	57.607	421.890
7	64.897	277.307
8	70.370	208.196
9	74.680	163.982
10	78.215	134.478
11	81.063	108.352
12	83.454	90.942
13	85.572	80.557
14	87.444	71.224
15	89.009	59.551
16	90.272	48.035
17	91.447	44.701
18	92.488	39.621
19	93.443	36.297
20	94.263	31.192
21	94.979	27.241
22	95.661	25.956
23	96.220	21.264
24	96.762	20.622
25	97.187	16.180

26	97.566	14.423
27	97.880	11.918
28	98.091	8.051
29	98.307	8.220
30	98.555	9.427
31	98.808	9.611
32	99.060	9.610
33	99.313	9.610
34	99.566	9.610
35	99.818	9.610
36	100.000	6.910

TOTAL SOIL-LOSS VOLUME(ACRE-FEET) = 39.2480
TOTAL STORM RUNOFF VOLUME(ACRE-FEET) = 36.9655

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2 4 - H O U R S T O R M
R U N O F F H Y D R O G R A P H

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HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS(CFS)
(Note: Time indicated is at END of Each Unit Intervals)

TIME(HRS)	VOLUME(AF)	Q(CFS)	0.	100.0	200.0	300.0	400.0
0.083	0.0004	0.05	Q
0.167	0.0020	0.23	Q
0.250	0.0061	0.61	Q
0.333	0.0159	1.42	Q
0.417	0.0313	2.23	Q
0.500	0.0503	2.77	Q
0.583	0.0719	3.13	Q
0.667	0.0953	3.40	Q
0.750	0.1202	3.62	Q
0.833	0.1463	3.80	Q
0.917	0.1735	3.95	Q
1.000	0.2016	4.08	Q
1.083	0.2305	4.19	Q
1.167	0.2601	4.30	Q
1.250	0.2903	4.39	Q
1.333	0.3211	4.46	Q
1.417	0.3523	4.54	Q
1.500	0.3840	4.60	Q
1.583	0.4161	4.67	Q
1.667	0.4487	4.72	Q
1.750	0.4816	4.78	Q

1.833	0.5148	4.83	Q
1.917	0.5484	4.87	Q
2.000	0.5822	4.92	Q
2.083	0.6164	4.96	Q
2.167	0.6507	4.99	Q
2.250	0.6854	5.03	Q
2.333	0.7202	5.06	Q
2.417	0.7552	5.09	Q
2.500	0.7905	5.12	Q
2.583	0.8260	5.15	Q
2.667	0.8617	5.19	Q
2.750	0.8976	5.22	Q
2.833	0.9338	5.25	QV
2.917	0.9702	5.28	QV
3.000	1.0068	5.32	QV
3.083	1.0435	5.34	QV
3.167	1.0805	5.36	QV
3.250	1.1175	5.38	QV
3.333	1.1547	5.40	QV
3.417	1.1921	5.43	QV
3.500	1.2297	5.45	QV
3.583	1.2674	5.47	QV
3.667	1.3052	5.50	QV
3.750	1.3433	5.52	QV
3.833	1.3815	5.55	QV
3.917	1.4198	5.57	QV
4.000	1.4583	5.60	QV
4.083	1.4971	5.62	QV
4.167	1.5359	5.65	QV
4.250	1.5750	5.67	QV
4.333	1.6142	5.70	QV
4.417	1.6536	5.72	QV
4.500	1.6932	5.75	QV
4.583	1.7330	5.78	QV
4.667	1.7730	5.80	QV
4.750	1.8131	5.83	QV
4.833	1.8535	5.86	Q V
4.917	1.8940	5.89	Q V
5.000	1.9348	5.91	Q V
5.083	1.9757	5.94	Q V
5.167	2.0168	5.97	Q V
5.250	2.0581	6.00	Q V
5.333	2.0997	6.03	Q V
5.417	2.1414	6.06	Q V
5.500	2.1834	6.09	Q V
5.583	2.2256	6.12	Q V
5.667	2.2680	6.15	Q V
5.750	2.3106	6.19	Q V
5.833	2.3534	6.22	Q V
5.917	2.3964	6.25	Q V

6.000	2.4397	6.28	Q V
6.083	2.4832	6.32	Q V
6.167	2.5270	6.35	Q V
6.250	2.5709	6.39	Q V
6.333	2.6151	6.42	Q V
6.417	2.6596	6.46	Q V
6.500	2.7043	6.49	Q V
6.583	2.7493	6.53	Q V
6.667	2.7945	6.56	Q V
6.750	2.8399	6.60	Q V
6.833	2.8857	6.64	Q V
6.917	2.9316	6.68	Q V
7.000	2.9779	6.72	Q V
7.083	3.0244	6.76	Q V
7.167	3.0712	6.80	Q V
7.250	3.1183	6.84	Q V
7.333	3.1657	6.88	Q V
7.417	3.2133	6.92	Q V
7.500	3.2613	6.96	Q V
7.583	3.3095	7.01	Q V
7.667	3.3581	7.05	Q V
7.750	3.4069	7.09	Q V
7.833	3.4561	7.14	Q V
7.917	3.5056	7.18	Q V
8.000	3.5554	7.23	Q V
8.083	3.6055	7.28	Q V
8.167	3.6560	7.33	Q V
8.250	3.7068	7.38	Q V
8.333	3.7579	7.43	Q V
8.417	3.8094	7.48	Q V
8.500	3.8613	7.53	Q V
8.583	3.9135	7.58	Q V
8.667	3.9661	7.63	Q V
8.750	4.0190	7.69	Q V
8.833	4.0723	7.74	Q V
8.917	4.1261	7.80	Q V
9.000	4.1802	7.86	Q V
9.083	4.2347	7.92	Q V
9.167	4.2896	7.98	Q V
9.250	4.3450	8.04	Q V
9.333	4.4008	8.10	Q V
9.417	4.4570	8.16	Q V
9.500	4.5136	8.23	Q V
9.583	4.5707	8.29	Q V
9.667	4.6283	8.36	Q V
9.750	4.6863	8.43	Q V
9.833	4.7449	8.50	Q V
9.917	4.8039	8.57	Q V
10.000	4.8634	8.64	Q V
10.083	4.9234	8.71	Q V

10.167	4.9839	8.79	Q	V
10.250	5.0450	8.87	Q	V
10.333	5.1066	8.95	Q	V
10.417	5.1688	9.03	Q	V
10.500	5.2315	9.11	Q	V
10.583	5.2949	9.20	Q	V
10.667	5.3588	9.28	Q	V
10.750	5.4233	9.37	Q	V
10.833	5.4885	9.46	Q	V
10.917	5.5543	9.56	Q	V
11.000	5.6208	9.65	Q	V
11.083	5.6879	9.75	Q	V
11.167	5.7558	9.85	Q	V
11.250	5.8243	9.95	Q	V
11.333	5.8936	10.06	.Q	V
11.417	5.9636	10.17	.Q	V
11.500	6.0344	10.28	.Q	V
11.583	6.1060	10.39	.Q	V
11.667	6.1784	10.51	.Q	V
11.750	6.2516	10.63	.Q	V
11.833	6.3257	10.76	.Q	V
11.917	6.4007	10.89	.Q	V
12.000	6.4766	11.02	.Q	V
12.083	6.5534	11.15	.Q	V
12.167	6.6309	11.26	.Q	V
12.250	6.7090	11.34	.Q	V
12.333	6.7871	11.34	.Q	V
12.417	6.8653	11.35	.Q	V
12.500	6.9439	11.42	.Q	V
12.583	7.0232	11.52	.Q	V
12.667	7.1034	11.64	.Q	V
12.750	7.1844	11.77	.Q	V
12.833	7.2666	11.93	.Q	V
12.917	7.3498	12.09	.Q	V
13.000	7.4343	12.26	.Q	V
13.083	7.5200	12.45	.Q	V
13.167	7.6071	12.64	.Q	V
13.250	7.6955	12.85	.Q	V
13.333	7.7855	13.07	.Q	V
13.417	7.8771	13.29	.Q	V
13.500	7.9703	13.54	.Q	V
13.583	8.0653	13.79	.Q	V
13.667	8.1621	14.06	.Q	V
13.750	8.2609	14.34	.Q	V
13.833	8.3617	14.64	.Q	V.
13.917	8.4647	14.96	.Q	V.
14.000	8.5700	15.29	.Q	V.
14.083	8.6779	15.67	.Q	V.
14.167	8.7890	16.12	.Q	V.
14.250	8.9037	16.67	.Q	V.

14.333	9.0238	17.43	.Q	V.	.	.	.
14.417	9.1492	18.21	.Q	V.	.	.	.
14.500	9.2796	18.93	.Q	V	.	.	.
14.583	9.4146	19.61	.Q	V	.	.	.
14.667	9.5545	20.31	. Q	V	.	.	.
14.750	9.6993	21.02	. Q	V	.	.	.
14.833	9.8494	21.79	. Q	V	.	.	.
14.917	10.0050	22.60	. Q	V	.	.	.
15.000	10.1668	23.49	. Q	.V	.	.	.
15.083	10.3351	24.44	. Q	.V	.	.	.
15.167	10.5107	25.50	. Q	.V	.	.	.
15.250	10.6944	26.66	. Q	.V	.	.	.
15.333	10.8871	27.99	. Q	.V	.	.	.
15.417	11.0909	29.59	. Q	. V	.	.	.
15.500	11.3100	31.81	. Q	. V	.	.	.
15.583	11.5495	34.78	. Q	. V	.	.	.
15.667	11.8210	39.43	. Q	. V	.	.	.
15.750	12.1326	45.23	. Q	. V	.	.	.
15.833	12.4967	52.87	. Q	. V	.	.	.
15.917	12.9418	64.63	. Q	. V	.	.	.
16.000	13.5334	85.90	. Q	. V	.	.	.
16.083	14.3826	123.30	.	. Q	V	.	.
16.167	15.6278	180.81	.	.	V	Q	.
16.250	17.3286	246.95	.	.	V	Q	.
16.333	19.6289	334.00	.	.	.V	.	Q
16.417	21.8238	318.71	.	.	.	V	.Q
16.500	23.4962	242.83	.	.	.	QV	.
16.583	24.7643	184.12	.	.	Q	.	V
16.667	25.7836	148.01	.	.	Q	.	V
16.750	26.6312	123.08	.	.	Q	.	V
16.833	27.3563	105.27	.	.	Q	.	V.
16.917	27.9820	90.86	.	.	Q.	.	V
17.000	28.5340	80.14	.	.	Q	.	V
17.083	29.0308	72.14	.	.	Q	.	.V
17.167	29.4784	64.99	.	.	Q	.	.V
17.250	29.8771	57.89	.	.	Q	.	.V
17.333	30.2311	51.41	.	.	Q	.	.V
17.417	30.5566	47.25	.	.	Q	.	.V
17.500	30.8547	43.30	.	.	Q	.	.V
17.583	31.1300	39.97	. QV
17.667	31.3818	36.57	. QV
17.750	31.6136	33.65	. QV
17.833	31.8303	31.48	. QV
17.917	32.0284	28.76	. QV
18.000	32.2139	26.94	. QV
18.083	32.3826	24.50	. QV
18.167	32.5394	22.77	. QV
18.250	32.6847	21.10	. QV
18.333	32.8191	19.52	.QV
18.417	32.9503	19.05	.QV

18.500	33.0804	18.89	.Q	.	.	.	V	.
18.583	33.2078	18.50	.Q	.	.	.	V	.
18.667	33.3318	18.01	.Q	.	.	.	V	.
18.750	33.4516	17.39	.Q	.	.	.	V	.
18.833	33.5664	16.67	.Q	.	.	.	V	.
18.917	33.6746	15.70	.Q	.	.	.	V	.
19.000	33.7711	14.02	.Q	.	.	.	V	.
19.083	33.8514	11.66	.Q	.	.	.	V	.
19.167	33.9274	11.04	.Q	.	.	.	V	.
19.250	34.0008	10.66	.Q	.	.	.	V	.
19.333	34.0721	10.35	.Q	.	.	.	V	.
19.417	34.1416	10.10	.Q	.	.	.	V	.
19.500	34.2095	9.86	Q	.	.	.	V	.
19.583	34.2760	9.65	Q	.	.	.	V	.
19.667	34.3410	9.44	Q	.	.	.	V	.
19.750	34.4047	9.25	Q	.	.	.	V	.
19.833	34.4672	9.07	Q	.	.	.	V	.
19.917	34.5284	8.90	Q	.	.	.	V	.
20.000	34.5886	8.73	Q	.	.	.	V	.
20.083	34.6477	8.58	Q	.	.	.	V	.
20.167	34.7058	8.44	Q	.	.	.	V	.
20.250	34.7630	8.30	Q	.	.	.	V	.
20.333	34.8192	8.17	Q	.	.	.	V	.
20.417	34.8746	8.04	Q	.	.	.	V	.
20.500	34.9291	7.92	Q	.	.	.	V	.
20.583	34.9828	7.80	Q	.	.	.	V	.
20.667	35.0357	7.69	Q	.	.	.	V	.
20.750	35.0879	7.58	Q	.	.	.	V	.
20.833	35.1394	7.47	Q	.	.	.	V	.
20.917	35.1902	7.37	Q	.	.	.	V	.
21.000	35.2403	7.28	Q	.	.	.	V	.
21.083	35.2897	7.18	Q	.	.	.	V	.
21.167	35.3386	7.09	Q	.	.	.	V	.
21.250	35.3868	7.00	Q	.	.	.	V	.
21.333	35.4344	6.91	Q	.	.	.	V	.
21.417	35.4814	6.83	Q	.	.	.	V	.
21.500	35.5278	6.75	Q	.	.	.	V	.
21.583	35.5737	6.67	Q	.	.	.	V	.
21.667	35.6191	6.59	Q	.	.	.	V	.
21.750	35.6640	6.51	Q	.	.	.	V	.
21.833	35.7084	6.44	Q	.	.	.	V	.
21.917	35.7522	6.37	Q	.	.	.	V	.
22.000	35.7956	6.30	Q	.	.	.	V	.
22.083	35.8386	6.24	Q	.	.	.	V	.
22.167	35.8811	6.17	Q	.	.	.	V	.
22.250	35.9232	6.11	Q	.	.	.	V	.
22.333	35.9648	6.05	Q	.	.	.	V	.
22.417	36.0060	5.99	Q	.	.	.	V	.
22.500	36.0469	5.93	Q	.	.	.	V	.
22.583	36.0873	5.87	Q	.	.	.	V	.

22.667	36.1274	5.82	Q	.	.	.	V.
22.750	36.1670	5.76	Q	.	.	.	V.
22.833	36.2063	5.71	Q	.	.	.	V.
22.917	36.2453	5.66	Q	.	.	.	V.
23.000	36.2839	5.60	Q	.	.	.	V.
23.083	36.3221	5.55	Q	.	.	.	V.
23.167	36.3601	5.51	Q	.	.	.	V.
23.250	36.3977	5.46	Q	.	.	.	V.
23.333	36.4349	5.41	Q	.	.	.	V.
23.417	36.4719	5.37	Q	.	.	.	V.
23.500	36.5085	5.32	Q	.	.	.	V.
23.583	36.5449	5.28	Q	.	.	.	V.
23.667	36.5810	5.24	Q	.	.	.	V.
23.750	36.6167	5.19	Q	.	.	.	V.
23.833	36.6522	5.15	Q	.	.	.	V.
23.917	36.6874	5.11	Q	.	.	.	V.
24.000	36.7224	5.07	Q	.	.	.	V.
24.083	36.7567	4.98	Q	.	.	.	V.
24.167	36.7895	4.76	Q	.	.	.	V.
24.250	36.8195	4.36	Q	.	.	.	V.
24.333	36.8437	3.51	Q	.	.	.	V.
24.417	36.8622	2.68	Q	.	.	.	V.
24.500	36.8769	2.13	Q	.	.	.	V.
24.583	36.8891	1.77	Q	.	.	.	V.
24.667	36.8994	1.50	Q	.	.	.	V.
24.750	36.9082	1.28	Q	.	.	.	V.
24.833	36.9158	1.10	Q	.	.	.	V.
24.917	36.9224	0.96	Q	.	.	.	V.
25.000	36.9281	0.84	Q	.	.	.	V.
25.083	36.9332	0.73	Q	.	.	.	V.
25.167	36.9375	0.63	Q	.	.	.	V.
25.250	36.9413	0.55	Q	.	.	.	V.
25.333	36.9447	0.49	Q	.	.	.	V.
25.417	36.9477	0.43	Q	.	.	.	V.
25.500	36.9503	0.38	Q	.	.	.	V.
25.583	36.9525	0.33	Q	.	.	.	V.
25.667	36.9545	0.29	Q	.	.	.	V.
25.750	36.9562	0.25	Q	.	.	.	V.
25.833	36.9577	0.22	Q	.	.	.	V.
25.917	36.9590	0.19	Q	.	.	.	V.
26.000	36.9601	0.16	Q	.	.	.	V.
26.083	36.9611	0.14	Q	.	.	.	V.
26.167	36.9619	0.12	Q	.	.	.	V.
26.250	36.9627	0.10	Q	.	.	.	V.
26.333	36.9633	0.09	Q	.	.	.	V.
26.417	36.9639	0.08	Q	.	.	.	V.
26.500	36.9644	0.07	Q	.	.	.	V.
26.583	36.9648	0.06	Q	.	.	.	V.
26.667	36.9651	0.05	Q	.	.	.	V.
26.750	36.9653	0.03	Q	.	.	.	V.

26.833	36.9655	0.02	Q	.	.	.	V.
26.917	36.9655	0.01	Q	.	.	.	V

TIME DURATION(minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:
 (Note: 100% of Peak Flow Rate estimate assumed to have
 an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
=====	=====
0%	1615.0
10%	135.0
20%	70.0
30%	50.0
40%	35.0
50%	30.0
60%	20.0
70%	20.0
80%	10.0
90%	10.0

END OF FLOODSCx ROUTING ANALYSIS

F L O O D R O U T I N G A N A L Y S I S
USING COUNTY HYDROLOGY MANUAL OF SAN BERNARDINO(1986)
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Analysis prepared by:

***** DESCRIPTION OF STUDY *****

- * LOCKHART SOLAR *
 - * 100-YR STORM EVENT *
 - * AREA 2 PROPOSED CONDITIONS *
- *****

FILE NAME: LOPR2.DAT
TIME/DATE OF STUDY: 17:00 07/26/2021

FLOW PROCESS FROM NODE 200.00 TO NODE 201.00 IS CODE = 1

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS)<<<<<
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(UNIT-HYDROGRAPH ADDED TO STREAM #1)

WATERCOURSE LENGTH = 5520.000 FEET
 LENGTH FROM CONCENTRATION POINT TO CENTROID = 2488.000 FEET
 ELEVATION VARIATION ALONG WATERCOURSE = 19.000 FEET
 BASIN FACTOR = 0.035
 WATERSHED AREA = 218.070 ACRES
 BASEFLOW = 0.000 CFS/SQUARE-MILE
 WATERCOURSE "LAG" TIME = 0.370 HOURS
 CAUTION: LAG TIME IS LESS THAN 0.50 HOURS.
 THE 5-MINUTE PERIOD UH MODEL (USED IN THIS COMPUTER PROGRAM)
 MAY BE TOO LARGE FOR PEAK FLOW ESTIMATES.
 DESERT(UNDEVELOPED) S-GRAPH SELECTED
 MAXIMUM WATERSHED LOSS RATE(INCH/HOUR) = 0.348
 LOW LOSS FRACTION = 0.620
 HYDROGRAPH MODEL #1 SPECIFIED

SPECIFIED PEAK 5-MINUTES RAINFALL(INCH)= 0.33
 SPECIFIED PEAK 30-MINUTES RAINFALL(INCH)= 0.78

SPECIFIED PEAK 1-HOUR RAINFALL(INCH) = 1.05
 SPECIFIED PEAK 3-HOUR RAINFALL(INCH) = 1.50
 SPECIFIED PEAK 6-HOUR RAINFALL(INCH) = 1.85
 SPECIFIED PEAK 24-HOUR RAINFALL(INCH) = 2.91

PRECIPITATION DEPTH-AREA REDUCTION FACTORS:

5-MINUTE FACTOR = 0.990
 30-MINUTE FACTOR = 0.990
 1-HOUR FACTOR = 0.990
 3-HOUR FACTOR = 0.999
 6-HOUR FACTOR = 0.999
 24-HOUR FACTOR = 1.000

UNIT HYDROGRAPH TIME UNIT = 5.000 MINUTES
 UNIT INTERVAL PERCENTAGE OF LAG-TIME = 22.526

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UNIT HYDROGRAPH DETERMINATION

INTERVAL NUMBER	"S" GRAPH MEAN VALUES	UNIT HYDROGRAPH ORDINATES(CFS)
1	1.170	30.846
2	5.502	114.251
3	14.926	248.541
4	34.349	512.235
5	50.449	424.607
6	60.709	270.595
7	67.517	179.529
8	72.753	138.107
9	76.851	108.071
10	80.147	86.923
11	82.844	71.126
12	85.167	61.262
13	87.218	54.086
14	88.923	44.970
15	90.283	35.855
16	91.533	32.979
17	92.635	29.055
18	93.638	26.464
19	94.469	21.900
20	95.235	20.201
21	95.906	17.713
22	96.492	15.444
23	97.019	13.917
24	97.428	10.780
25	97.803	9.883

26	98.044	6.353
27	98.272	6.023
28	98.533	6.887
29	98.804	7.125
30	99.074	7.130
31	99.344	7.125
32	99.614	7.125
33	99.884	7.125
34	100.000	3.048

TOTAL SOIL-LOSS VOLUME(ACRE-FEET) = 27.1725
TOTAL STORM RUNOFF VOLUME(ACRE-FEET) = 25.6731

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2 4 - H O U R S T O R M
R U N O F F H Y D R O G R A P H

HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS(CFS)
(Note: Time indicated is at END of Each Unit Intervals)

TIME(HRS)	VOLUME(AF)	Q(CFS)	0.	75.0	150.0	225.0	300.0
0.083	0.0003	0.04	Q
0.167	0.0015	0.18	Q
0.250	0.0049	0.50	Q
0.333	0.0128	1.14	Q
0.417	0.0243	1.68	Q
0.500	0.0383	2.02	Q
0.583	0.0538	2.26	Q
0.667	0.0706	2.44	Q
0.750	0.0884	2.58	Q
0.833	0.1070	2.70	Q
0.917	0.1262	2.80	Q
1.000	0.1461	2.89	Q
1.083	0.1665	2.96	Q
1.167	0.1874	3.03	Q
1.250	0.2087	3.09	Q
1.333	0.2303	3.14	Q
1.417	0.2523	3.19	Q
1.500	0.2745	3.23	Q
1.583	0.2971	3.27	Q
1.667	0.3199	3.31	Q
1.750	0.3429	3.35	Q
1.833	0.3662	3.38	Q
1.917	0.3897	3.41	Q

2.000	0.4133	3.44	Q
2.083	0.4372	3.46	Q
2.167	0.4611	3.48	Q
2.250	0.4853	3.50	Q
2.333	0.5096	3.53	Q
2.417	0.5340	3.55	Q
2.500	0.5586	3.57	Q
2.583	0.5834	3.60	Q
2.667	0.6083	3.62	Q
2.750	0.6334	3.64	Q
2.833	0.6586	3.66	QV
2.917	0.6839	3.68	QV
3.000	0.7093	3.69	QV
3.083	0.7348	3.71	QV
3.167	0.7605	3.72	QV
3.250	0.7862	3.74	QV
3.333	0.8121	3.75	QV
3.417	0.8380	3.77	QV
3.500	0.8641	3.79	QV
3.583	0.8903	3.80	QV
3.667	0.9166	3.82	QV
3.750	0.9430	3.83	QV
3.833	0.9695	3.85	QV
3.917	0.9962	3.87	QV
4.000	1.0229	3.89	QV
4.083	1.0498	3.90	QV
4.167	1.0768	3.92	QV
4.250	1.1039	3.94	QV
4.333	1.1312	3.96	QV
4.417	1.1586	3.97	QV
4.500	1.1861	3.99	QV
4.583	1.2137	4.01	QV
4.667	1.2415	4.03	QV
4.750	1.2693	4.05	QV
4.833	1.2974	4.07	Q V
4.917	1.3255	4.09	Q V
5.000	1.3538	4.11	Q V
5.083	1.3823	4.13	Q V
5.167	1.4108	4.15	Q V
5.250	1.4396	4.17	Q V
5.333	1.4684	4.19	Q V
5.417	1.4974	4.21	Q V
5.500	1.5266	4.23	Q V
5.583	1.5559	4.25	Q V
5.667	1.5853	4.28	Q V
5.750	1.6149	4.30	Q V
5.833	1.6447	4.32	Q V
5.917	1.6746	4.34	Q V
6.000	1.7046	4.37	Q V
6.083	1.7349	4.39	Q V

6.167	1.7653	4.41	Q V
6.250	1.7958	4.44	Q V
6.333	1.8265	4.46	Q V
6.417	1.8574	4.49	Q V
6.500	1.8885	4.51	Q V
6.583	1.9197	4.54	Q V
6.667	1.9512	4.56	Q V
6.750	1.9827	4.59	Q V
6.833	2.0145	4.61	Q V
6.917	2.0465	4.64	Q V
7.000	2.0786	4.67	Q V
7.083	2.1110	4.69	Q V
7.167	2.1435	4.72	Q V
7.250	2.1762	4.75	Q V
7.333	2.2091	4.78	Q V
7.417	2.2422	4.81	Q V
7.500	2.2756	4.84	Q V
7.583	2.3091	4.87	Q V
7.667	2.3429	4.90	Q V
7.750	2.3768	4.93	Q V
7.833	2.4110	4.96	Q V
7.917	2.4454	4.99	Q V
8.000	2.4800	5.03	Q V
8.083	2.5149	5.06	Q V
8.167	2.5499	5.09	Q V
8.250	2.5853	5.13	Q V
8.333	2.6208	5.16	Q V
8.417	2.6566	5.20	Q V
8.500	2.6927	5.23	Q V
8.583	2.7290	5.27	Q V
8.667	2.7655	5.31	Q V
8.750	2.8023	5.35	Q V
8.833	2.8394	5.39	Q V
8.917	2.8768	5.42	Q V
9.000	2.9144	5.46	Q V
9.083	2.9523	5.51	Q V
9.167	2.9905	5.55	Q V
9.250	3.0290	5.59	Q V
9.333	3.0678	5.63	Q V
9.417	3.1069	5.68	Q V
9.500	3.1463	5.72	Q V
9.583	3.1860	5.77	Q V
9.667	3.2261	5.81	Q V
9.750	3.2665	5.86	Q V
9.833	3.3072	5.91	Q V
9.917	3.3482	5.96	Q V
10.000	3.3896	6.01	Q V
10.083	3.4314	6.06	Q V
10.167	3.4735	6.12	Q V
10.250	3.5160	6.17	Q V

10.333	3.5589	6.23	Q	V
10.417	3.6021	6.28	Q	V
10.500	3.6458	6.34	Q	V
10.583	3.6899	6.40	Q	V
10.667	3.7344	6.46	Q	V
10.750	3.7793	6.52	Q	V
10.833	3.8246	6.59	Q	V
10.917	3.8704	6.65	Q	V
11.000	3.9167	6.72	Q	V
11.083	3.9635	6.79	Q	V
11.167	4.0107	6.86	Q	V
11.250	4.0584	6.93	Q	V
11.333	4.1067	7.00	Q	V
11.417	4.1554	7.08	Q	V
11.500	4.2047	7.16	Q	V
11.583	4.2546	7.24	Q	V
11.667	4.3050	7.32	Q	V
11.750	4.3560	7.41	Q	V
11.833	4.4076	7.50	Q	V
11.917	4.4599	7.58	.Q	V
12.000	4.5127	7.68	.Q	V
12.083	4.5662	7.77	.Q	V
12.167	4.6202	7.84	.Q	V
12.250	4.6745	7.89	.Q	V
12.333	4.7288	7.87	.Q	V
12.417	4.7831	7.88	.Q	V
12.500	4.8377	7.93	.Q	V
12.583	4.8929	8.01	.Q	V
12.667	4.9486	8.09	.Q	V
12.750	5.0050	8.19	.Q	V
12.833	5.0622	8.30	.Q	V
12.917	5.1201	8.41	.Q	V
13.000	5.1789	8.54	.Q	V
13.083	5.2386	8.67	.Q	V
13.167	5.2992	8.81	.Q	V
13.250	5.3609	8.95	.Q	V
13.333	5.4236	9.11	.Q	V
13.417	5.4874	9.26	.Q	V
13.500	5.5524	9.44	.Q	V
13.583	5.6186	9.61	.Q	V
13.667	5.6861	9.81	.Q	V
13.750	5.7550	10.00	.Q	V
13.833	5.8254	10.22	.Q	V.
13.917	5.8973	10.44	.Q	V.
14.000	5.9709	10.68	.Q	V.
14.083	6.0462	10.94	.Q	V.
14.167	6.1238	11.26	.Q	V.
14.250	6.2041	11.66	.Q	V.
14.333	6.2881	12.20	.Q	V.
14.417	6.3757	12.72	.Q	V.

14.500	6.4667	13.21	.Q	V	.	.	.
14.583	6.5608	13.67	.Q	V	.	.	.
14.667	6.6583	14.16	.Q	V	.	.	.
14.750	6.7592	14.65	.Q	V	.	.	.
14.833	6.8638	15.19	. Q	V	.	.	.
14.917	6.9723	15.76	. Q	V	.	.	.
15.000	7.0852	16.39	. Q	.V	.	.	.
15.083	7.2027	17.06	. Q	.V	.	.	.
15.167	7.3254	17.82	. Q	.V	.	.	.
15.250	7.4537	18.64	. Q	.V	.	.	.
15.333	7.5886	19.58	. Q	.V	.	.	.
15.417	7.7314	20.74	. Q	. V	.	.	.
15.500	7.8856	22.38	. Q	. V	.	.	.
15.583	8.0554	24.67	. Q	. V	.	.	.
15.667	8.2501	28.27	. Q	. V	.	.	.
15.750	8.4736	32.45	. Q	. V	.	.	.
15.833	8.7364	38.16	. Q	. V	.	.	.
15.917	9.0622	47.32	. Q	. V	.	.	.
16.000	9.5014	63.77	. Q	. V	.	.	.
16.083	10.1346	91.94	.	. Q	. V	.	.
16.167	11.0734	136.31	.	.	. VQ	.	.
16.250	12.3740	188.85	.	.	. V.	. Q	.
16.333	14.0917	249.40	.	.	.V	.	. Q
16.417	15.5843	216.74 V	. Q
16.500	16.6971	161.58	.	.	.Q	. V	.
16.583	17.5425	122.74	.	.	. Q	. V	.
16.667	18.2262	99.28	.	.	. Q	. V	.
16.750	18.7936	82.39	.	. Q	.	. V.	.
16.833	19.2765	70.12	.	. Q.	.	. V	.
16.917	19.6953	60.81	.	. Q	.	. V	.
17.000	20.0668	53.95	.	. Q	.	.V	.
17.083	20.3996	48.32	.	. Q	.	.V	.
17.167	20.6943	42.79	.	. Q	.	. V	.
17.250	20.9546	37.79	.	. Q	.	. V	.
17.333	21.1922	34.51	.	. Q	.	. V	.
17.417	21.4084	31.39	.	. Q	.	. V	.
17.500	21.6067	28.79	. Q	.	.	. V	.
17.583	21.7862	26.06	. Q	.	.	. V	.
17.667	21.9522	24.11	. Q	.	.	. V	.
17.750	22.1048	22.16	. Q	.	.	. V	.
17.833	22.2449	20.34	. Q	.	.	. V	.
17.917	22.3742	18.77	. Q	.	.	. V	.
18.000	22.4912	17.00	. Q	.	.	. V	.
18.083	22.6001	15.81	. Q	.	.	. V	.
18.167	22.6987	14.32	.Q	.	.	. V	.
18.250	22.7935	13.75	.Q	.	.	. V	.
18.333	22.8874	13.64	.Q	.	.	. V	.
18.417	22.9798	13.41	.Q	.	.	. V	.
18.500	23.0695	13.03	.Q	.	.	. V	.
18.583	23.1560	12.56	.Q	.	.	. V	.

18.667	23.2384	11.97	.Q	.	.	.	V	.
18.750	23.3154	11.19	.Q	.	.	.	V	.
18.833	23.3811	9.53	.Q	.	.	.	V	.
18.917	23.4380	8.27	.Q	.	.	.	V	.
19.000	23.4922	7.87	.Q	.	.	.	V	.
19.083	23.5446	7.60	.Q	.	.	.	V	.
19.167	23.5954	7.39	Q	.	.	.	V	.
19.250	23.6450	7.20	Q	.	.	.	V	.
19.333	23.6934	7.03	Q	.	.	.	V	.
19.417	23.7407	6.87	Q	.	.	.	V	.
19.500	23.7870	6.72	Q	.	.	.	V	.
19.583	23.8324	6.58	Q	.	.	.	V	.
19.667	23.8768	6.45	Q	.	.	.	V	.
19.750	23.9203	6.32	Q	.	.	.	V	.
19.833	23.9631	6.21	Q	.	.	.	V	.
19.917	24.0051	6.10	Q	.	.	.	V	.
20.000	24.0463	5.99	Q	.	.	.	V	.
20.083	24.0869	5.89	Q	.	.	.	V	.
20.167	24.1267	5.79	Q	.	.	.	V	.
20.250	24.1660	5.70	Q	.	.	.	V	.
20.333	24.2046	5.61	Q	.	.	.	V	.
20.417	24.2426	5.52	Q	.	.	.	V	.
20.500	24.2801	5.44	Q	.	.	.	V	.
20.583	24.3170	5.36	Q	.	.	.	V	.
20.667	24.3535	5.29	Q	.	.	.	V	.
20.750	24.3894	5.21	Q	.	.	.	V	.
20.833	24.4248	5.14	Q	.	.	.	V	.
20.917	24.4597	5.07	Q	.	.	.	V	.
21.000	24.4942	5.01	Q	.	.	.	V	.
21.083	24.5282	4.94	Q	.	.	.	V	.
21.167	24.5618	4.88	Q	.	.	.	V	.
21.250	24.5950	4.82	Q	.	.	.	V	.
21.333	24.6277	4.76	Q	.	.	.	V	.
21.417	24.6601	4.70	Q	.	.	.	V	.
21.500	24.6921	4.65	Q	.	.	.	V	.
21.583	24.7237	4.59	Q	.	.	.	V	.
21.667	24.7550	4.54	Q	.	.	.	V	.
21.750	24.7859	4.49	Q	.	.	.	V	.
21.833	24.8165	4.44	Q	.	.	.	V	.
21.917	24.8467	4.39	Q	.	.	.	V	.
22.000	24.8766	4.34	Q	.	.	.	V	.
22.083	24.9063	4.30	Q	.	.	.	V	.
22.167	24.9356	4.25	Q	.	.	.	V	.
22.250	24.9646	4.21	Q	.	.	.	V	.
22.333	24.9933	4.17	Q	.	.	.	V	.
22.417	25.0217	4.13	Q	.	.	.	V	.
22.500	25.0499	4.09	Q	.	.	.	V.	.
22.583	25.0777	4.05	Q	.	.	.	V.	.
22.667	25.1054	4.01	Q	.	.	.	V.	.
22.750	25.1327	3.97	Q	.	.	.	V.	.

22.833	25.1599	3.94	Q	.	.	.	V.
22.917	25.1867	3.90	Q	.	.	.	V.
23.000	25.2134	3.87	Q	.	.	.	V.
23.083	25.2398	3.83	Q	.	.	.	V.
23.167	25.2659	3.80	Q	.	.	.	V.
23.250	25.2919	3.77	Q	.	.	.	V.
23.333	25.3176	3.74	Q	.	.	.	V.
23.417	25.3431	3.70	Q	.	.	.	V.
23.500	25.3684	3.67	Q	.	.	.	V.
23.583	25.3935	3.64	Q	.	.	.	V.
23.667	25.4184	3.61	Q	.	.	.	V.
23.750	25.4431	3.59	Q	.	.	.	V.
23.833	25.4676	3.56	Q	.	.	.	V.
23.917	25.4919	3.53	Q	.	.	.	V.
24.000	25.5160	3.50	Q	.	.	.	V.
24.083	25.5397	3.44	Q	.	.	.	V.
24.167	25.5622	3.27	Q	.	.	.	V.
24.250	25.5824	2.93	Q	.	.	.	V.
24.333	25.5980	2.27	Q	.	.	.	V.
24.417	25.6099	1.72	Q	.	.	.	V.
24.500	25.6193	1.37	Q	.	.	.	V.
24.583	25.6271	1.13	Q	.	.	.	V.
24.667	25.6336	0.95	Q	.	.	.	V.
24.750	25.6392	0.81	Q	.	.	.	V.
24.833	25.6440	0.69	Q	.	.	.	V.
24.917	25.6481	0.60	Q	.	.	.	V.
25.000	25.6516	0.52	Q	.	.	.	V.
25.083	25.6547	0.45	Q	.	.	.	V.
25.167	25.6574	0.39	Q	.	.	.	V.
25.250	25.6597	0.34	Q	.	.	.	V.
25.333	25.6617	0.29	Q	.	.	.	V.
25.417	25.6635	0.26	Q	.	.	.	V.
25.500	25.6650	0.22	Q	.	.	.	V.
25.583	25.6663	0.19	Q	.	.	.	V.
25.667	25.6675	0.16	Q	.	.	.	V.
25.750	25.6684	0.14	Q	.	.	.	V.
25.833	25.6693	0.12	Q	.	.	.	V.
25.917	25.6700	0.10	Q	.	.	.	V.
26.000	25.6706	0.09	Q	.	.	.	V.
26.083	25.6711	0.08	Q	.	.	.	V.
26.167	25.6716	0.07	Q	.	.	.	V.
26.250	25.6720	0.06	Q	.	.	.	V.
26.333	25.6723	0.05	Q	.	.	.	V.
26.417	25.6726	0.04	Q	.	.	.	V.
26.500	25.6728	0.03	Q	.	.	.	V.
26.583	25.6729	0.02	Q	.	.	.	V.
26.667	25.6730	0.01	Q	.	.	.	V.
26.750	25.6731	0.00	Q	.	.	.	V.

TIME DURATION(minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:

(Note: 100% of Peak Flow Rate estimate assumed to have an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
0%	1605.0
10%	120.0
20%	65.0
30%	45.0
40%	30.0
50%	25.0
60%	20.0
70%	15.0
80%	10.0
90%	5.0

END OF FLOODSCx ROUTING ANALYSIS

F L O O D R O U T I N G A N A L Y S I S
USING COUNTY HYDROLOGY MANUAL OF SAN BERNARDINO(1986)
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Analysis prepared by:

***** DESCRIPTION OF STUDY *****

- * LOCKHART SOLAR *
 - * 100-YR STORM EVENT *
 - * AREA 3 PROPOSED CONDITIONS *
- *****

FILE NAME: LOPR3.DAT
TIME/DATE OF STUDY: 17:03 07/26/2021

FLOW PROCESS FROM NODE 300.00 TO NODE 301.00 IS CODE = 1

>>>>SUBAREA RUNOFF (UNIT-HYDROGRAPH ANALYSIS)<<<<<
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(UNIT-HYDROGRAPH ADDED TO STREAM #1)

WATERCOURSE LENGTH = 8570.000 FEET
 LENGTH FROM CONCENTRATION POINT TO CENTROID = 4508.000 FEET
 ELEVATION VARIATION ALONG WATERCOURSE = 49.000 FEET
 BASIN FACTOR = 0.035
 WATERSHED AREA = 176.340 ACRES
 BASEFLOW = 0.000 CFS/SQUARE-MILE
 WATERCOURSE "LAG" TIME = 0.498 HOURS
 CAUTION: LAG TIME IS LESS THAN 0.50 HOURS.
 THE 5-MINUTE PERIOD UH MODEL (USED IN THIS COMPUTER PROGRAM)
 MAY BE TOO LARGE FOR PEAK FLOW ESTIMATES.
 DESERT(UNDEVELOPED) S-GRAPH SELECTED
 MAXIMUM WATERSHED LOSS RATE(INCH/HOUR) = 0.348
 LOW LOSS FRACTION = 0.620
 HYDROGRAPH MODEL #1 SPECIFIED

SPECIFIED PEAK 5-MINUTES RAINFALL(INCH)= 0.33
 SPECIFIED PEAK 30-MINUTES RAINFALL(INCH)= 0.78

SPECIFIED PEAK 1-HOUR RAINFALL(INCH) = 1.05
 SPECIFIED PEAK 3-HOUR RAINFALL(INCH) = 1.50
 SPECIFIED PEAK 6-HOUR RAINFALL(INCH) = 1.85
 SPECIFIED PEAK 24-HOUR RAINFALL(INCH) = 2.91

PRECIPITATION DEPTH-AREA REDUCTION FACTORS:

5-MINUTE FACTOR = 0.992
 30-MINUTE FACTOR = 0.992
 1-HOUR FACTOR = 0.992
 3-HOUR FACTOR = 0.999
 6-HOUR FACTOR = 0.999
 24-HOUR FACTOR = 1.000

UNIT HYDROGRAPH TIME UNIT = 5.000 MINUTES
 UNIT INTERVAL PERCENTAGE OF LAG-TIME = 16.744

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UNIT HYDROGRAPH DETERMINATION

INTERVAL NUMBER	"S" GRAPH MEAN VALUES	UNIT HYDROGRAPH ORDINATES(CFS)
1	0.780	16.630
2	3.392	55.702
3	7.808	94.182
4	16.237	179.758
5	31.276	320.724
6	44.847	289.415
7	54.506	205.980
8	61.351	145.989
9	66.456	108.863
10	70.648	89.399
11	74.102	73.676
12	77.046	62.781
13	79.528	52.926
14	81.675	45.779
15	83.521	39.376
16	85.215	36.134
17	86.778	33.332
18	88.141	29.055
19	89.286	24.427
20	90.279	21.174
21	91.222	20.105
22	92.080	18.304
23	92.872	16.881
24	93.613	15.801
25	94.248	13.552

26	94.817	12.142
27	95.385	12.115
28	95.872	10.383
29	96.307	9.279
30	96.742	9.261
31	97.093	7.483
32	97.394	6.428
33	97.694	6.398
34	97.909	4.592
35	98.077	3.572
36	98.245	3.582
37	98.434	4.040
38	98.635	4.279
39	98.836	4.289
40	99.037	4.284
41	99.238	4.284
42	99.438	4.284
43	99.639	4.284
44	99.840	4.284
45	100.000	3.408

TOTAL SOIL-LOSS VOLUME(ACRE-FEET) = 21.9600
TOTAL STORM RUNOFF VOLUME(ACRE-FEET) = 20.7765

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2 4 - H O U R S T O R M
R U N O F F H Y D R O G R A P H

HYDROGRAPH IN FIVE-MINUTE UNIT INTERVALS(CFS)
(Note: Time indicated is at END of Each Unit Intervals)

TIME(HRS)	VOLUME(AF)	Q(CFS)	0.	50.0	100.0	150.0	200.0
0.083	0.0001	0.02	Q
0.167	0.0008	0.09	Q
0.250	0.0022	0.21	Q
0.333	0.0052	0.44	Q
0.417	0.0110	0.84	Q
0.500	0.0193	1.21	Q
0.583	0.0294	1.47	Q
0.667	0.0408	1.66	Q
0.750	0.0532	1.80	Q
0.833	0.0665	1.92	Q
0.917	0.0804	2.02	Q
1.000	0.0949	2.10	Q

1.083	0.1099	2.18	Q
1.167	0.1253	2.24	Q
1.250	0.1412	2.30	Q
1.333	0.1574	2.36	Q
1.417	0.1740	2.41	Q
1.500	0.1909	2.45	Q
1.583	0.2080	2.49	Q
1.667	0.2254	2.53	Q
1.750	0.2431	2.56	Q
1.833	0.2609	2.59	Q
1.917	0.2790	2.63	Q
2.000	0.2973	2.66	Q
2.083	0.3158	2.68	Q
2.167	0.3345	2.71	Q
2.250	0.3533	2.73	Q
2.333	0.3723	2.76	Q
2.417	0.3914	2.78	Q
2.500	0.4107	2.80	Q
2.583	0.4302	2.82	Q
2.667	0.4498	2.84	Q
2.750	0.4695	2.86	Q
2.833	0.4893	2.88	Q
2.917	0.5093	2.90	Q
3.000	0.5293	2.91	QV
3.083	0.5495	2.93	QV
3.167	0.5698	2.95	QV
3.250	0.5902	2.96	QV
3.333	0.6107	2.98	QV
3.417	0.6314	3.00	QV
3.500	0.6522	3.02	QV
3.583	0.6731	3.04	QV
3.667	0.6941	3.05	QV
3.750	0.7153	3.07	QV
3.833	0.7365	3.09	QV
3.917	0.7579	3.10	QV
4.000	0.7793	3.11	QV
4.083	0.8009	3.13	QV
4.167	0.8225	3.14	QV
4.250	0.8442	3.15	QV
4.333	0.8660	3.17	QV
4.417	0.8880	3.18	QV
4.500	0.9100	3.20	QV
4.583	0.9321	3.21	QV
4.667	0.9543	3.23	QV
4.750	0.9767	3.24	QV
4.833	0.9991	3.26	QV
4.917	1.0216	3.27	QV
5.000	1.0443	3.29	Q V
5.083	1.0670	3.30	Q V
5.167	1.0899	3.32	Q V

5.250	1.1129	3.34	Q V
5.333	1.1360	3.35	Q V
5.417	1.1592	3.37	Q V
5.500	1.1825	3.39	Q V
5.583	1.2059	3.40	Q V
5.667	1.2295	3.42	Q V
5.750	1.2532	3.44	Q V
5.833	1.2770	3.46	Q V
5.917	1.3009	3.47	Q V
6.000	1.3249	3.49	Q V
6.083	1.3491	3.51	Q V
6.167	1.3734	3.53	Q V
6.250	1.3978	3.55	Q V
6.333	1.4224	3.57	Q V
6.417	1.4471	3.59	Q V
6.500	1.4719	3.60	Q V
6.583	1.4969	3.62	Q V
6.667	1.5220	3.64	Q V
6.750	1.5472	3.67	Q V
6.833	1.5726	3.69	Q V
6.917	1.5981	3.71	Q V
7.000	1.6238	3.73	Q V
7.083	1.6496	3.75	Q V
7.167	1.6756	3.77	Q V
7.250	1.7017	3.79	Q V
7.333	1.7280	3.82	Q V
7.417	1.7545	3.84	Q V
7.500	1.7811	3.86	Q V
7.583	1.8079	3.89	Q V
7.667	1.8348	3.91	Q V
7.750	1.8619	3.94	Q V
7.833	1.8892	3.96	Q V
7.917	1.9166	3.98	Q V
8.000	1.9442	4.01	Q V
8.083	1.9720	4.04	Q V
8.167	2.0000	4.06	Q V
8.250	2.0282	4.09	Q V
8.333	2.0565	4.12	Q V
8.417	2.0851	4.14	Q V
8.500	2.1138	4.17	Q V
8.583	2.1427	4.20	Q V
8.667	2.1719	4.23	Q V
8.750	2.2012	4.26	Q V
8.833	2.2307	4.29	Q V
8.917	2.2605	4.32	Q V
9.000	2.2905	4.35	Q V
9.083	2.3207	4.38	Q V
9.167	2.3511	4.42	Q V
9.250	2.3817	4.45	Q V
9.333	2.4126	4.48	Q V

9.417	2.4437	4.52	Q	V
9.500	2.4751	4.55	Q	V
9.583	2.5067	4.59	Q	V
9.667	2.5385	4.62	Q	V
9.750	2.5706	4.66	Q	V
9.833	2.6030	4.70	Q	V
9.917	2.6356	4.74	Q	V
10.000	2.6685	4.78	Q	V
10.083	2.7017	4.82	Q	V
10.167	2.7352	4.86	Q	V
10.250	2.7689	4.90	Q	V
10.333	2.8030	4.94	Q	V
10.417	2.8373	4.99	Q	V
10.500	2.8720	5.03	.Q	V
10.583	2.9070	5.08	.Q	V
10.667	2.9423	5.13	.Q	V
10.750	2.9779	5.17	.Q	V
10.833	3.0139	5.22	.Q	V
10.917	3.0502	5.27	.Q	V
11.000	3.0869	5.32	.Q	V
11.083	3.1239	5.38	.Q	V
11.167	3.1613	5.43	.Q	V
11.250	3.1991	5.49	.Q	V
11.333	3.2373	5.54	.Q	V
11.417	3.2759	5.60	.Q	V
11.500	3.3149	5.66	.Q	V
11.583	3.3544	5.73	.Q	V
11.667	3.3942	5.79	.Q	V
11.750	3.4346	5.85	.Q	V
11.833	3.4753	5.92	.Q	V
11.917	3.5166	5.99	.Q	V
12.000	3.5584	6.06	.Q	V
12.083	3.6006	6.13	.Q	V
12.167	3.6433	6.20	.Q	V
12.250	3.6863	6.25	.Q	V
12.333	3.7296	6.29	.Q	V
12.417	3.7730	6.30	.Q	V
12.500	3.8165	6.32	.Q	V
12.583	3.8603	6.36	.Q	V
12.667	3.9045	6.41	.Q	V
12.750	3.9491	6.48	.Q	V
12.833	3.9943	6.56	.Q	V
12.917	4.0401	6.64	.Q	V
13.000	4.0864	6.73	.Q	V
13.083	4.1334	6.83	.Q	V
13.167	4.1811	6.93	.Q	V
13.250	4.2296	7.03	.Q	V
13.333	4.2788	7.15	.Q	V
13.417	4.3288	7.27	.Q	V
13.500	4.3797	7.39	.Q	V

13.583	4.4316	7.52	.Q	V
13.667	4.4843	7.66	.Q	V
13.750	4.5381	7.81	.Q	V
13.833	4.5930	7.97	.Q	V
13.917	4.6490	8.13	.Q	V
14.000	4.7062	8.30	.Q	V.
14.083	4.7647	8.50	.Q	V.
14.167	4.8247	8.71	.Q	V.
14.250	4.8865	8.96	.Q	V.
14.333	4.9503	9.26	.Q	V.
14.417	5.0167	9.64	.Q	V.
14.500	5.0857	10.02	. Q	V.
14.583	5.1573	10.39	. Q	V.
14.667	5.2313	10.75	. Q	V
14.750	5.3079	11.13	. Q	V
14.833	5.3872	11.51	. Q	V
14.917	5.4694	11.93	. Q	V
15.000	5.5546	12.37	. Q	V
15.083	5.6432	12.86	. Q	V
15.167	5.7354	13.38	. Q	.V
15.250	5.8315	13.97	. Q	.V
15.333	5.9322	14.61	. Q	.V
15.417	6.0383	15.40	. Q	.V
15.500	6.1514	16.42	. Q	.V
15.583	6.2734	17.72	. Q	. V
15.667	6.4079	19.52	. Q	. V
15.750	6.5617	22.34	. Q	. V
15.833	6.7408	26.00	. Q	. V
15.917	6.9542	30.99	. Q	. V
16.000	7.2233	39.07	. Q	. V
16.083	7.6013	54.88	. Q	. V
16.167	8.1363	77.68	.	. Q
16.250	8.8420	102.47	.	.	. V	. Q	.	.
16.333	9.7728	135.15	.	.	. V	.	. Q	.
16.417	10.9351	168.78V	.	. Q
16.500	11.9990	154.47 V	.	. Q
16.583	12.8471	123.15 Q	.	.
16.667	13.5182	97.44	.	.	. Q.	. V	.	.
16.750	14.0644	79.31	.	.	. Q	.	. V	.
16.833	14.5297	67.56	.	.	. Q	.	. V	.
16.917	14.9325	58.49	.	.Q	.	.	. V	.
17.000	15.2879	51.61	.	. Q	.	.	. V.	.
17.083	15.6033	45.80	.	. Q.	.	.	. V	.
17.167	15.8869	41.17	.	. Q	.	.	. V	.
17.250	16.1434	37.24	.	. Q	.	.	.V	.
17.333	16.3795	34.29	.	. Q	.	.	.V	.
17.417	16.5970	31.57	.	. Q	.	.	.V	.
17.500	16.7944	28.67	.	. Q	.	.	. V	.
17.583	16.9732	25.96	.	. Q	.	.	. V	.
17.667	17.1374	23.85	.	. Q	.	.	. V	.

17.750	17.2917	22.40	.	Q	.	.	.	V	.
17.833	17.4359	20.94	.	Q	.	.	.	V	.
17.917	17.5713	19.65	.	Q	.	.	.	V	.
18.000	17.6984	18.46	.	Q	.	.	.	V	.
18.083	17.8163	17.12	.	Q	.	.	.	V	.
18.167	17.9270	16.07	.	Q	.	.	.	V	.
18.250	18.0326	15.34	.	Q	.	.	.	V	.
18.333	18.1313	14.33	.	Q	.	.	.	V	.
18.417	18.2245	13.53	.	Q	.	.	.	V	.
18.500	18.3138	12.96	.	Q	.	.	.	V	.
18.583	18.3966	12.03	.	Q	.	.	.	V	.
18.667	18.4744	11.29	.	Q	.	.	.	V	.
18.750	18.5486	10.78	.	Q	.	.	.	V	.
18.833	18.6172	9.96	.	.Q	.	.	.	V	.
18.917	18.6821	9.42	.	.Q	.	.	.	V	.
19.000	18.7455	9.21	.	.Q	.	.	.	V	.
19.083	18.8085	9.15	.	.Q	.	.	.	V	.
19.167	18.8707	9.03	.	.Q	.	.	.	V	.
19.250	18.9317	8.85	.	.Q	.	.	.	V	.
19.333	18.9913	8.67	.	.Q	.	.	.	V	.
19.417	19.0496	8.46	.	.Q	.	.	.	V	.
19.500	19.1061	8.20	.	.Q	.	.	.	V	.
19.583	19.1605	7.90	.	.Q	.	.	.	V	.
19.667	19.2121	7.49	.	.Q	.	.	.	V	.
19.750	19.2590	6.81	.	.Q	.	.	.	V	.
19.833	19.2981	5.68	.	.Q	.	.	.	V	.
19.917	19.3353	5.40	.	.Q	.	.	.	V	.
20.000	19.3713	5.23	.	.Q	.	.	.	V	.
20.083	19.4064	5.10	.	.Q	.	.	.	V	.
20.167	19.4408	4.99	Q	V	.
20.250	19.4745	4.89	Q	V	.
20.333	19.5076	4.80	Q	V	.
20.417	19.5400	4.71	Q	V	.
20.500	19.5719	4.63	Q	V	.
20.583	19.6032	4.55	Q	V	.
20.667	19.6341	4.47	Q	V	.
20.750	19.6644	4.40	Q	V	.
20.833	19.6942	4.34	Q	V	.
20.917	19.7237	4.27	Q	V	.
21.000	19.7527	4.21	Q	V	.
21.083	19.7813	4.15	Q	V	.
21.167	19.8095	4.10	Q	V	.
21.250	19.8373	4.04	Q	V	.
21.333	19.8648	3.99	Q	V	.
21.417	19.8919	3.94	Q	V	.
21.500	19.9187	3.89	Q	V	.
21.583	19.9451	3.84	Q	V	.
21.667	19.9712	3.79	Q	V	.
21.750	19.9971	3.75	Q	V	.
21.833	20.0226	3.71	Q	V	.

21.917	20.0478	3.66	Q	.	.	.	V .
22.000	20.0728	3.62	Q	.	.	.	V .
22.083	20.0974	3.58	Q	.	.	.	V .
22.167	20.1218	3.54	Q	.	.	.	V .
22.250	20.1460	3.50	Q	.	.	.	V .
22.333	20.1698	3.47	Q	.	.	.	V .
22.417	20.1935	3.43	Q	.	.	.	V .
22.500	20.2169	3.40	Q	.	.	.	V .
22.583	20.2400	3.36	Q	.	.	.	V .
22.667	20.2629	3.33	Q	.	.	.	V .
22.750	20.2857	3.30	Q	.	.	.	V .
22.833	20.3081	3.26	Q	.	.	.	V .
22.917	20.3304	3.23	Q	.	.	.	V .
23.000	20.3525	3.20	Q	.	.	.	V .
23.083	20.3743	3.17	Q	.	.	.	V .
23.167	20.3960	3.15	Q	.	.	.	V .
23.250	20.4175	3.12	Q	.	.	.	V .
23.333	20.4388	3.09	Q	.	.	.	V .
23.417	20.4598	3.06	Q	.	.	.	V .
23.500	20.4808	3.04	Q	.	.	.	V .
23.583	20.5015	3.01	Q	.	.	.	V .
23.667	20.5221	2.99	Q	.	.	.	V .
23.750	20.5425	2.96	Q	.	.	.	V .
23.833	20.5627	2.94	Q	.	.	.	V .
23.917	20.5827	2.91	Q	.	.	.	V .
24.000	20.6027	2.89	Q	.	.	.	V .
24.083	20.6223	2.85	Q	.	.	.	V .
24.167	20.6412	2.75	Q	.	.	.	V .
24.250	20.6592	2.62	Q	.	.	.	V .
24.333	20.6756	2.37	Q	.	.	.	V .
24.417	20.6890	1.95	Q	.	.	.	V .
24.500	20.6998	1.57	Q	.	.	.	V .
24.583	20.7088	1.30	Q	.	.	.	V .
24.667	20.7164	1.11	Q	.	.	.	V .
24.750	20.7231	0.96	Q	.	.	.	V .
24.833	20.7289	0.84	Q	.	.	.	V .
24.917	20.7340	0.75	Q	.	.	.	V .
25.000	20.7386	0.66	Q	.	.	.	V .
25.083	20.7426	0.59	Q	.	.	.	V .
25.167	20.7463	0.53	Q	.	.	.	V .
25.250	20.7495	0.47	Q	.	.	.	V .
25.333	20.7525	0.43	Q	.	.	.	V .
25.417	20.7551	0.38	Q	.	.	.	V .
25.500	20.7574	0.34	Q	.	.	.	V .
25.583	20.7595	0.31	Q	.	.	.	V .
25.667	20.7615	0.28	Q	.	.	.	V .
25.750	20.7632	0.25	Q	.	.	.	V .
25.833	20.7648	0.23	Q	.	.	.	V .
25.917	20.7662	0.20	Q	.	.	.	V .
26.000	20.7674	0.18	Q	.	.	.	V .

26.083	20.7685	0.16	Q	.	.	.	V.
26.167	20.7695	0.15	Q	.	.	.	V.
26.250	20.7704	0.13	Q	.	.	.	V.
26.333	20.7713	0.12	Q	.	.	.	V.
26.417	20.7720	0.10	Q	.	.	.	V.
26.500	20.7726	0.09	Q	.	.	.	V.
26.583	20.7732	0.08	Q	.	.	.	V.
26.667	20.7737	0.07	Q	.	.	.	V.
26.750	20.7741	0.06	Q	.	.	.	V.
26.833	20.7745	0.06	Q	.	.	.	V.
26.917	20.7749	0.05	Q	.	.	.	V.
27.000	20.7752	0.05	Q	.	.	.	V.
27.083	20.7755	0.04	Q	.	.	.	V.
27.167	20.7758	0.04	Q	.	.	.	V.
27.250	20.7760	0.03	Q	.	.	.	V.
27.333	20.7762	0.03	Q	.	.	.	V.
27.417	20.7763	0.02	Q	.	.	.	V.
27.500	20.7764	0.02	Q	.	.	.	V.
27.583	20.7765	0.01	Q	.	.	.	V.

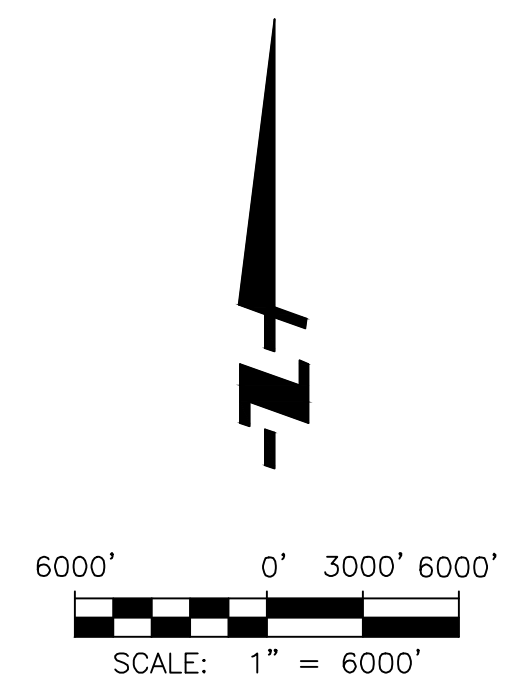
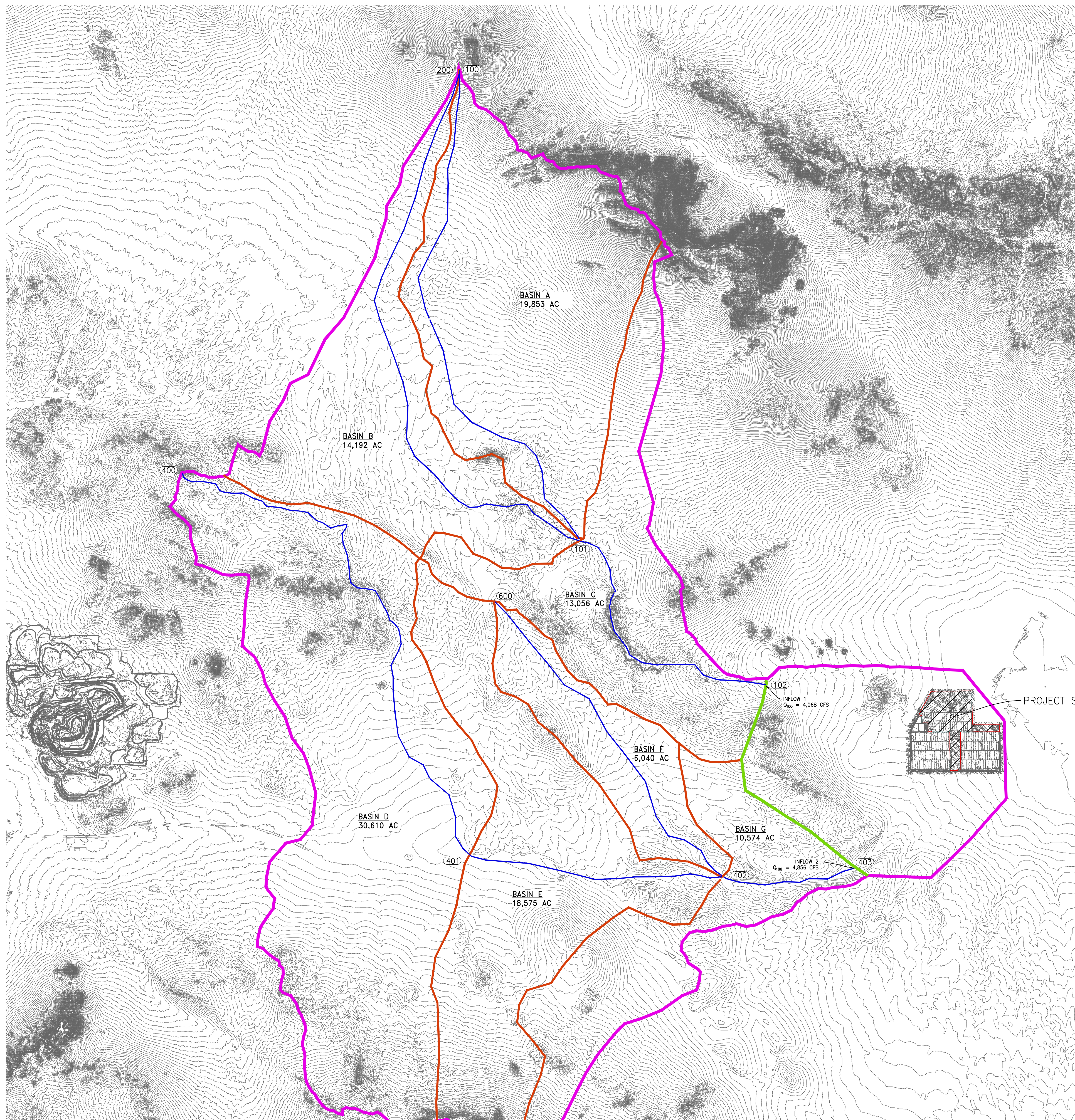
TIME DURATION(minutes) OF PERCENTILES OF ESTIMATED PEAK FLOW RATE:
 (Note: 100% of Peak Flow Rate estimate assumed to have
 an instantaneous time duration)

Percentile of Estimated Peak Flow Rate	Duration (minutes)
=====	=====
0%	1655.0
10%	155.0
20%	85.0
30%	60.0
40%	45.0
50%	30.0
60%	25.0
70%	20.0
80%	15.0
90%	10.0

=====

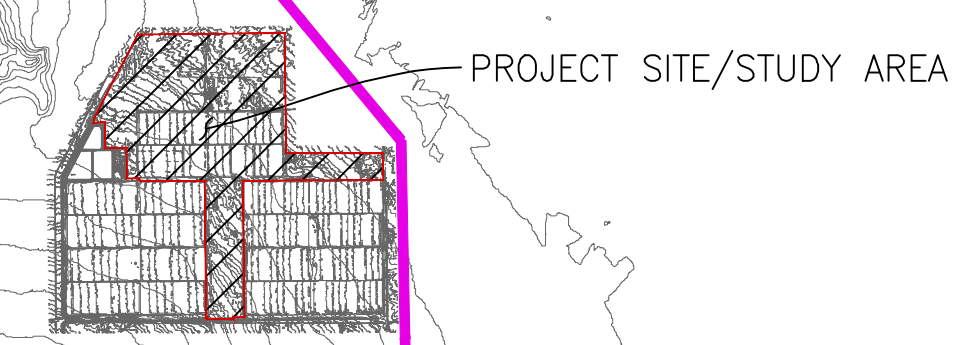
END OF FLOODSCx ROUTING ANALYSIS

APPENDIX 4
Offsite Hydrology Map



LEGEND

- BASIN BOUNDARY —
- SUBBASIN BOUNDARY —
- FLO-2D STUDY BOUNDARY —
- MAIN FLOW PATH —
- PROJECT BOUNDARY —
- STUDY AREA
- NODE NUMBER (101)



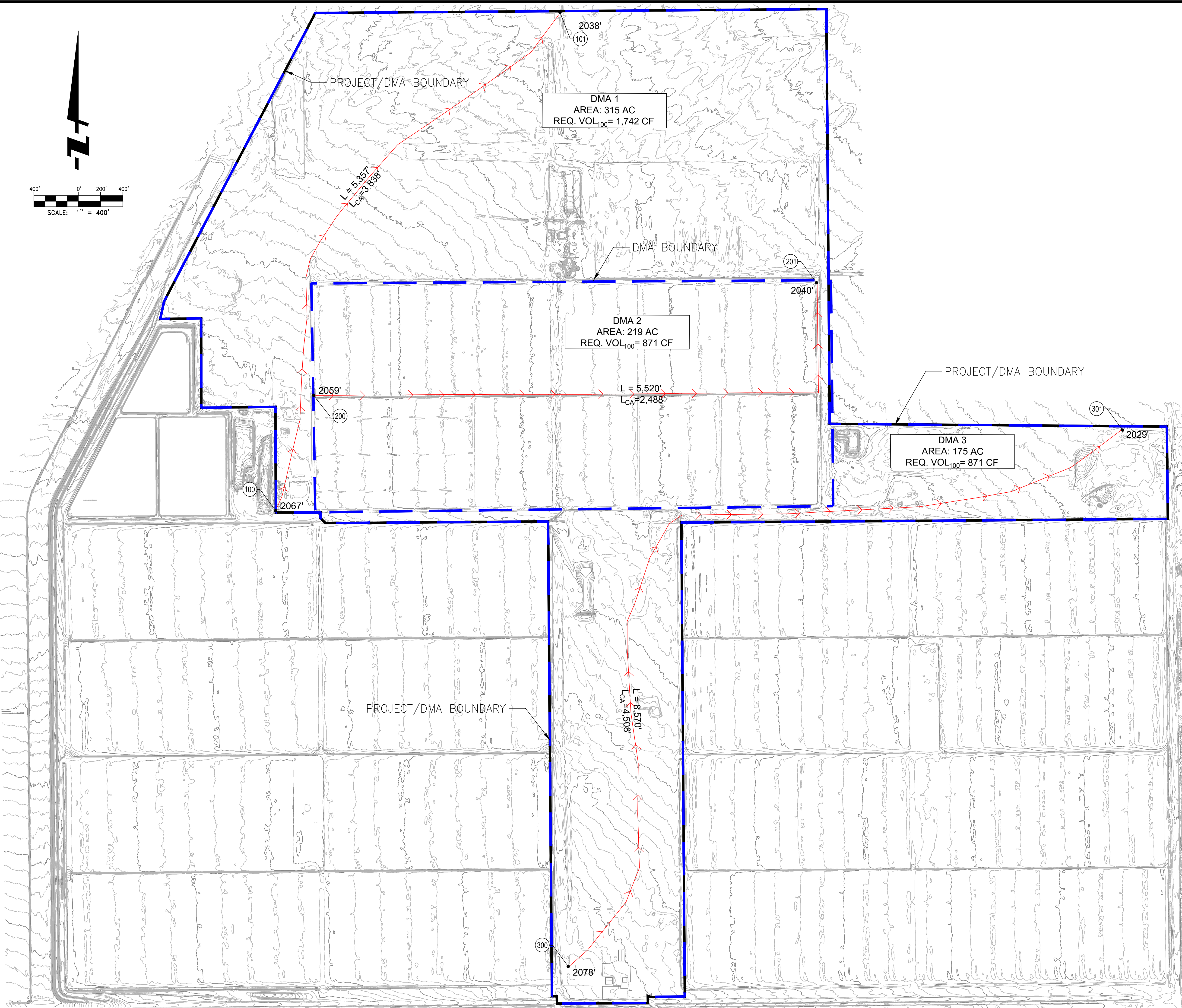
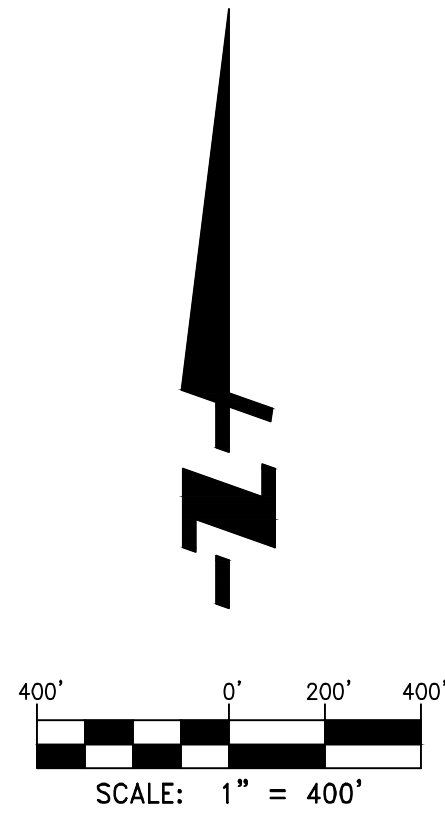
**OFFSITE HYDROLOGY
100-YR STORM EVENT**

LOCKHART SOLAR PV II
 SAN BERNARDINO COUNTY, CALIFORNIA
 PROJECT NUMBER: 1859-003
 SCALE: 1"=6000'
 DATE: SEPTEMBER 2021
SHEET 1 OF 1



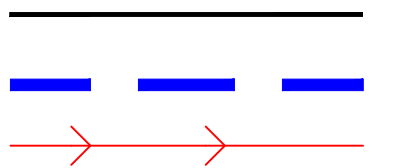
F:\Projects\1859\003_Support_Files\Report\Hydrology\HDR-CDFS.dwg (9/21/2021 4:11 PM) Plotted by: Patrick Chen

APPENDIX 5
Onsite Hydrology Map



LEGEND

- PROJECT BOUNDARY
- DMA BOUNDARY
- FLOW PATH



ONSITE HYDROLOGY

LOCKHART SOLAR PV II

SAN BERNARDINO COUNTY, CALIFORNIA
PROJECT NUMBER: 1859-003

DATE: SEPTEMBER 2021

FUSCOE
ENGINEERING
6390 Greenwich Drive, Suite 170
San Diego, California 92122
tel 858.554.1500 • fax 858.597.0335
www.fuscoe.com

F:\Projects\1859\003_Support Files\Reports\Hydrology\1859-003 - Hydrology Mapping (9/21/2021 4:23 PM).Pctd by: Patrick Chen

APPENDIX 6
San Bernardino County Hydrology Manual

Residential Landscaping (Lawn, Shrubs, etc.) - The pervious portions of commercial establishments, single and multiple family dwellings, trailer parks and schools where the predominant land cover is lawn, shrubbery and trees.

Row Crops - Lettuce, tomatoes, beets, tulips or any field crop planted in rows far enough apart that most of the soil surface is exposed to rainfall impact throughout the growing season. At plowing, planting and harvest times it is equivalent to fallow.

Small Grain - Wheat, oats, barley, flax, etc. planted in rows close enough that the soil surface is not exposed except during planting and shortly thereafter.

Legumes - Alfalfa, sweetclover, timothy, etc. and combinations are either planted in close rows or broadcast.

Fallow - Fallow land is land plowed but not yet seeded or tilled.

Woodland - grass - Areas with an open cover of broadleaf or coniferous trees usually live oak and pines, with the intervening ground space occupied by annual grasses or weeds. The trees may occur singly or in small clumps. Canopy density, the amount of ground surface shaded at high noon, is from 20 to 50 percent.

Woodland - Areas on which coniferous or broadleaf trees predominate. The canopy density is at least 50 percent. Open areas may have a cover of annual or perennial grasses or of brush. Herbaceous plant cover under the trees is usually sparse because of leaf or needle litter accumulation.

Chaparral - Land on which the principal vegetation consists of evergreen shrubs with broad, hard, stiff leaves such as manzanita, ceanothus and scrub oak. The brush cover is usually dense or moderately dense. Diffusely branched evergreen shrubs with fine needle-like leaves, such as chamise and redchank, with dense high growth are also included in this soil cover.

Annual Grass - Land on which the principal vegetation consists of annual grasses and weeds such as annual bromes, wild barley, soft chess, ryegrass and filaree.

Irrigated Pasture - Irrigated land planted to perennial grasses and legumes for production of forage and which is cultivated only to establish or renew the stand of plants. Dry land pasture is considered as annual grass.

Meadow - Land areas with seasonally high water table, locally called cienegas. Principal vegetation consists of sod-forming grasses interspersed with other plants.

Orchard (Deciduous) - Land planted to such deciduous trees as apples, apricots, pears, walnuts, and almonds.

Orchard (Evergreen) - Land planted to evergreen trees which include citrus and avocados and coniferous plantings.

Turf - Golf courses, parks and similar lands where the predominant cover is irrigated mowed close-grown turf grass. Parks in which trees are dense may be classified as woodland.

SAN BERNARDINO COUNTY
HYDROLOGY MANUAL

SCS
COVER TYPE
DESCRIPTIONS

POOR: Heavily grazed or regularly burned areas. Less than 50 percent of the ground surface is protected by plant cover or brush and tree canopy.

FAIR: Moderate cover with 50 percent to 75 percent of the ground surface protected by vegetation.

GOOD: Heavy or dense cover with more than 75 percent of the ground surface protected by vegetation.

In most cases, watershed existing conditions cover type and quality can be readily determined by a field review of a watershed. In ultimate planned open spaces, the soil cover condition shall be considered as "good." Figure C-3 provides the CN values for various types and quality of ground cover. Impervious areas shall be assigned a CN of 98. It is noted that for ultimately developed conditions, the CN for urban landscaping (turf) is provided in Figure C-3.

C.4. WATERSHED DEVELOPMENT CONDITIONS

Ultimate development of the watershed should normally be assumed since watershed urbanization is reasonably likely within the expected life of most hydraulic facilities. Long range master plans for the County and incorporated cities should be reviewed to insure that reasonable land use assumptions are made for the ultimate development of the watershed. A field review shall also be made to confirm existing use and drainage patterns. Particular attention shall be paid to existing and proposed landscape practices, as it is common in some areas to use ornamental gravels underlain by impervious plastic materials in place of lawns and shrubs. Appropriate actual impervious percentages can then be selected from Figure C-4. It should be noted that the recommended values from these figures are for average conditions and, therefore, some adjustment for particular applications may be required.

Runoff Index Numbers of Hydrologic Soil-Cover Complexes For Pervious Areas-AMC II

Cover Type (3)	Quality of Cover (2)	Soil Group			
		A	B	C	D
<u>NATURAL COVERS -</u>					
Barren (Rockland, eroded and graded land)		78	86	91	93
Chaparrel, Broadleaf (Manzonita, ceanothus and scrub oak)	Poor	53	70	80	85
	Fair	40	63	75	81
	Good	31	57	71	78
Chaparrel, Narrowleaf (Chamise and redshank)	Poor	71	82	88	91
	Fair	55	72	81	86
Grass, Annual or Perennial	Poor	67	78	86	89
	Fair	50	69	79	84
	Good	38	61	74	80
Meadows or Cienegas (Areas with seasonally high water table, principal vegetation is sod forming grass)	Poor	63	77	85	88
	Fair	51	70	80	84
	Good	30	58	71	78
Open Brush (Soft wood shrubs - buckwheat, sage, etc.)	Poor	62	76	84	88
	Fair	46	66	77	83
	Good	41	63	75	81
Woodland (Coniferous or broadleaf trees predominate. Canopy density is at least 50 percent.)	Poor	45	66	77	83
	Fair	36	60	73	79
	Good	25	55	70	77
Woodland, Grass (Coniferous or broadleaf trees with canopy density from 20 to 50 percent)	Poor	57	73	82	86
	Fair	44	65	77	82
	Good	33	58	72	79
<u>URBAN COVERS -</u>					
Residential or Commercial Landscaping (Lawn, shrubs, etc.)	Good	32	56	69	75
Turf (Irrigated and mowed grass)	Poor	58	74	83	87
	Fair	44	65	77	82
	Good	33	58	72	79
<u>AGRICULTURAL COVERS -</u>					
Fallow (Land plowed but not tilled or seeded)		77	86	91	94

SAN BERNARDINO COUNTY
HYDROLOGY MANUAL

CURVE

NUMBERS
FOR
PERVIOUS AREAS

Runoff Index Numbers of Hydrologic Soil-Cover Complexes For Pervious Areas-AMC II

Cover Type (3)	Quality of Cover (2)	Soil Group			
		A	B	C	D
AGRICULTURAL COVERS (Continued)					
Legumes, Close Seeded (Alfalfa, sweetclover, timothy, etc.)	Poor	66	77	85	89
	Good	58	72	81	85
Orchards, Evergreen (Citrus, avocados, etc.)	Poor	57	73	82	86
	Fair	44	65	77	82
	Good	33	58	72	79
Pasture, Dryland (Annual grasses)	Poor	68	79	86	89
	Fair	49	69	79	84
	Good	39	61	74	80
Pasture, Irrigated (Legumes and perennial grass)	Poor	58	74	83	87
	Fair	44	65	77	82
	Good	33	58	72	79
Row Crops (Field crops - tomatoes, sugar beets, etc.)	Poor	72	81	88	91
	Good	67	78	85	89
Small grain (Wheat, oats, barley, etc.)	Poor	65	76	84	88
	Good	63	75	83	87

Notes:

1. All runoff index (RI) numbers are for Antecedent Moisture Condition (AMC) II.
2. Quality of cover definitions:

Poor-Heavily grazed or regularly burned areas. Less than 50 percent of the ground surface is protected by plant cover or brush and tree canopy.

Fair-Moderate cover with 50 percent to 75 percent of the ground surface protected.

Good-Heavy or dense cover with more than 75 percent of the ground surface protected.
3. See Figure C-2 for definition of cover types.

SAN BERNARDINO COUNTY
HYDROLOGY MANUAL

NUMBERS
FOR
PERVIOUS AREAS

TABLE C.1. CURVE NUMBER RELATIONSHIPS

CN for AMC <u>Condition II</u>	Corresponding CN for AMC Condition	
	<u>I</u>	<u>III</u>
100	100	100
95	87	99
90	78	98
85	70	97
80	63	94
75	57	91
70	51	87
65	45	83
60	40	79
55	35	75
50	31	70
45	27	65
40	23	60
35	19	55
30	15	50
25	12	45
20	9	39
15	7	33
10	4	26
5	2	17
0	0	0

C.6. ESTIMATION OF LOSS RATES

In estimating loss rates for design hydrology, a watershed curve number (CN) is determined for each soil-cover complex within the watershed using Figure C-3. The working range of CN values is between 0 and 98, where a low CN indicates low runoff potential (high infiltration), and a high CN indicates high runoff potential (low infiltration). Selection of a CN takes into account the major factors affecting loss rates on pervious surfaces including the hydrologic soil group, cover type and quality, and antecedent moisture condition (AMC).

Also included in the CN selection are the effects of "initial abstraction" (Ia) which represents the combined effects of other effective rainfall losses including depression storage, vegetation interception, evaporation, and transpiration, among other factors.

C.6.1. Estimation of Initial Abstraction (Ia)

The initial abstraction (Ia) for an area is a function of land use, treatment, and condition; interception; infiltration; depression storage; and antecedent soil moisture. An estimate for Ia is given by the SCS as

$$Ia = 0.2S \quad (C.1)$$

where S is an estimate of total soil capacity given by

$$S = \frac{1000}{CN} - 10 \quad (C.2)$$

where CN is the area curve number.

C.6.2. Estimation of Storm Runoff Yield

Given the CN for a subarea A_j , the corresponding 24-hour storm runoff yield fraction, Y_j , is estimated by

$$Y_j = \frac{(P_{24} - Ia)^2}{(P_{24} - Ia + S)P_{24}} \quad (C.3)$$

where

- Y_j = 24-hour storm runoff yield fraction for subarea A_j
- P_{24} = 24-hour storm rainfall
- Ia = initial abstraction from (C.1)
- S = see (C.2)

It is noted that should Ia be greater than P_{24} in (C.3), then Y_j is defined to be zero. In this manual, the notation Y and Y_j will represent the runoff yield fraction, rather than the volume of runoff.

If the area under study contains several (say m) CN designations, then the yield, Y, for the total area must represent the net effect of the several curve

numbers. By weighting each of the subarea yield values according to the respective areas,

$$Y = (Y_1 A_1 + \dots + Y_m A_m) / (A_1 + A_2 + \dots + A_m) \quad (C.4)$$

where each Y_j follows from (C.3).

C.6.3. Low Loss Rate, F^*

In design storm runoff hydrograph studies, the following formula is used to estimate that portion of rainfall to be attributed to watershed losses:

$$\bar{Y} = 1 - Y \quad (C.5)$$

where

\bar{Y}	=	catchment low loss fraction
Y	=	catchment 24-hour storm runoff yield fraction computed from (C.4)

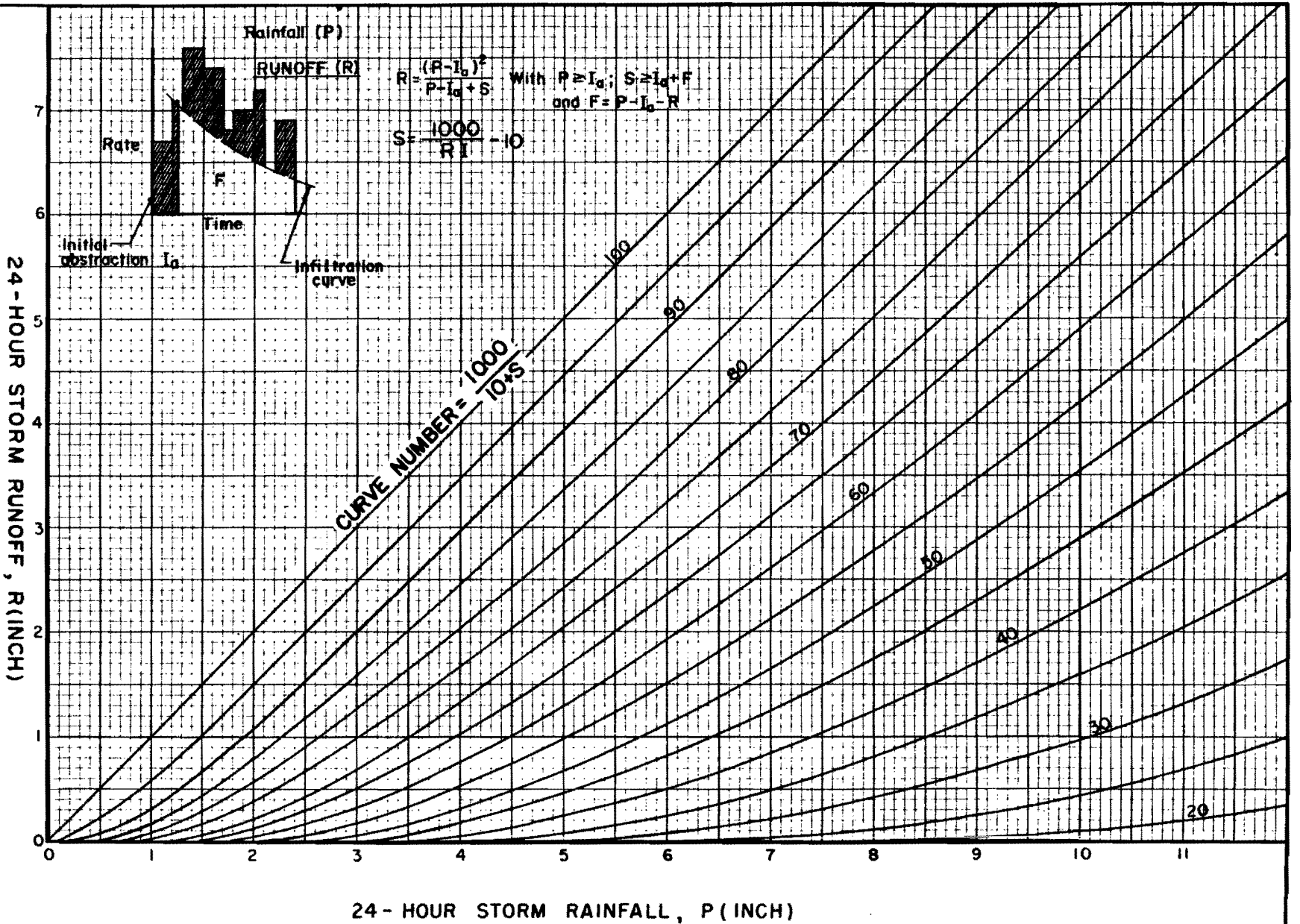
Using the low loss fraction, \bar{Y} , the corresponding low loss rate, F^* , is given by

$$F^* = \bar{Y} \cdot I \quad (C.6)$$

where I is the rainfall intensity and F^* has units of inches/hour. Use of F^* enables the design storm 24-hour storm runoff yield to approximate the yield values obtained from the CN approach (see Figure C-5).

C.6.4. Infiltration Rates

Soil infiltration rates have been estimated for each of the soil groups by laboratory studies and measurements. These measurements show that an initially dry soil will have an associated infiltration rate which essentially decreases with time as the soil becomes wetted. As the soil is subjected to continual heavy rainfall, this infiltration rate approaches a minimum (usually within about 30 minutes) which represents the infiltration capacity of the soil.



When sufficient stream gauge information is available, infiltration rates for unit hydrograph hydrology can be estimated from a study of rainfall-runoff relationships of major storms. Where such data is not available, infiltration rates for pervious areas as a function of CN can be estimated using Figures C-3 and C-6. Loss rates for pervious areas estimated from the Figure C-6 curves are generally consistent with values developed from rainfall-runoff reconstitution studies in San Bernardino County watersheds.

C.6.5. Estimation of Catchment Maximum Loss Rates, F_m

The infiltration rate selected from Figure C-6 applies to the pervious area fraction of the watershed. The infiltration rate assumed for an impervious surface is 0.0 inch/hour. The maximum loss rate, F_m , for a catchment is therefore given by

$$F_m = a_p F_p \quad (C.7)$$

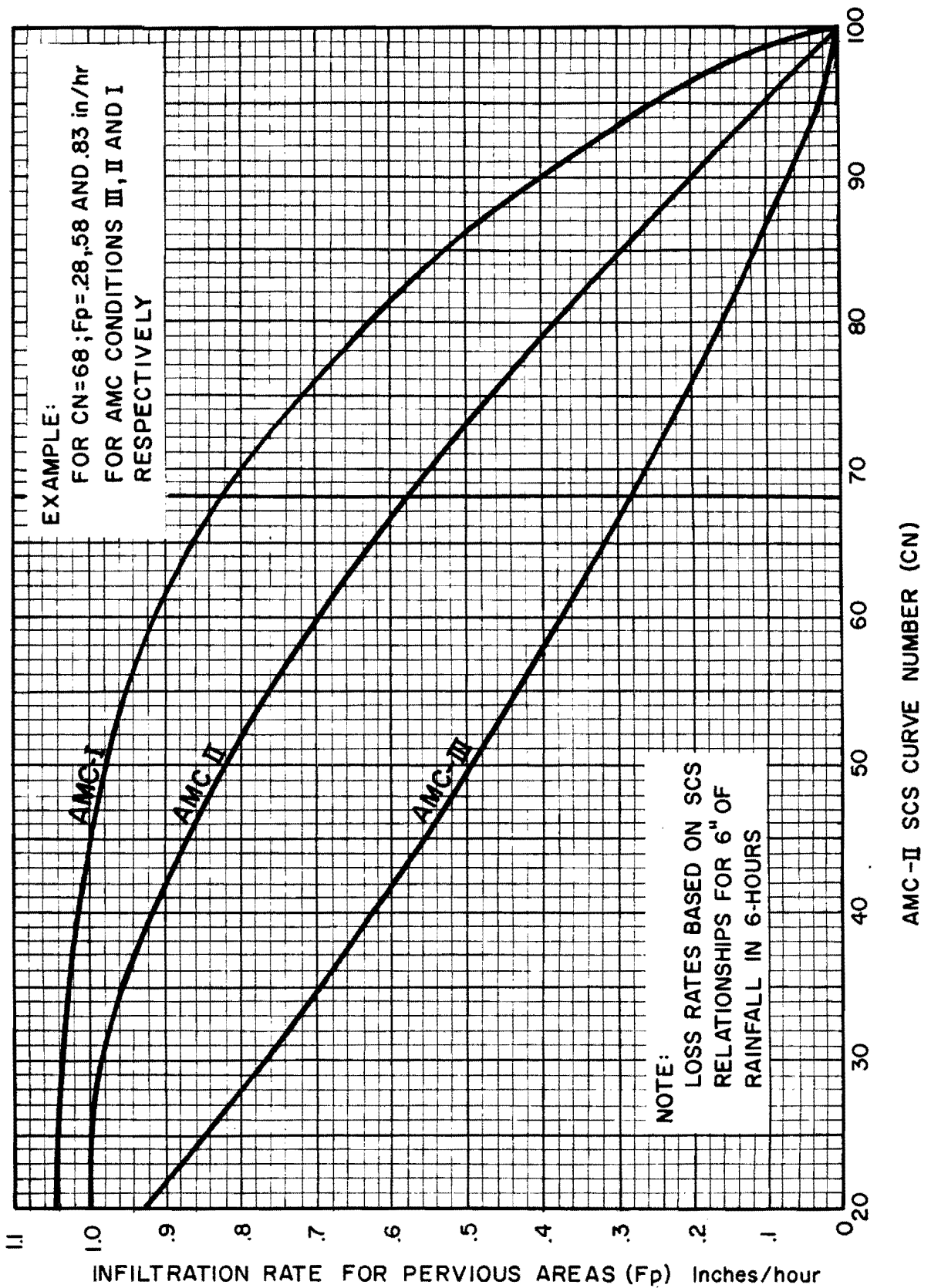
where a_p is the pervious area fraction, and F_p is the infiltration rate for the pervious area.

Should a catchment contain several F_p values, the composite F_m value is determined as a simple area average of the several F_m values. Table C.2 provides F_m values for a wide range of cover types and soil groups.

C.6.6. Design Storm Loss Rates

In design storm runoff hydrograph studies, a 24-hour duration storm pattern is used to develop the time distribution of effective rainfall over the watershed. The effective rainfall quantities are determined by subtracting the watershed losses from the design storm rainfall.

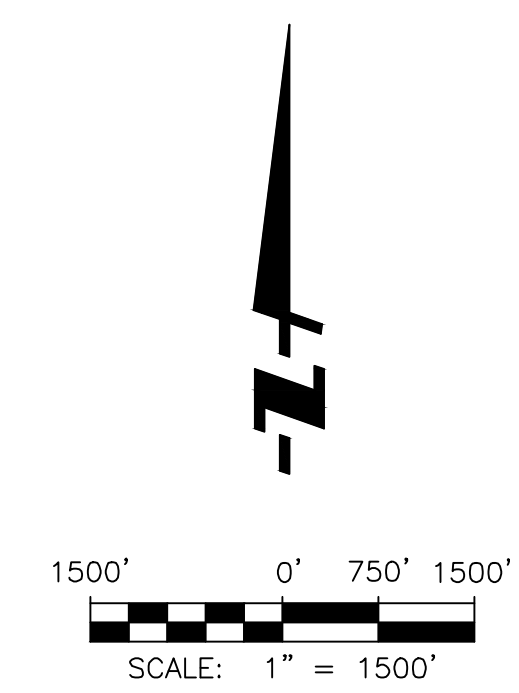
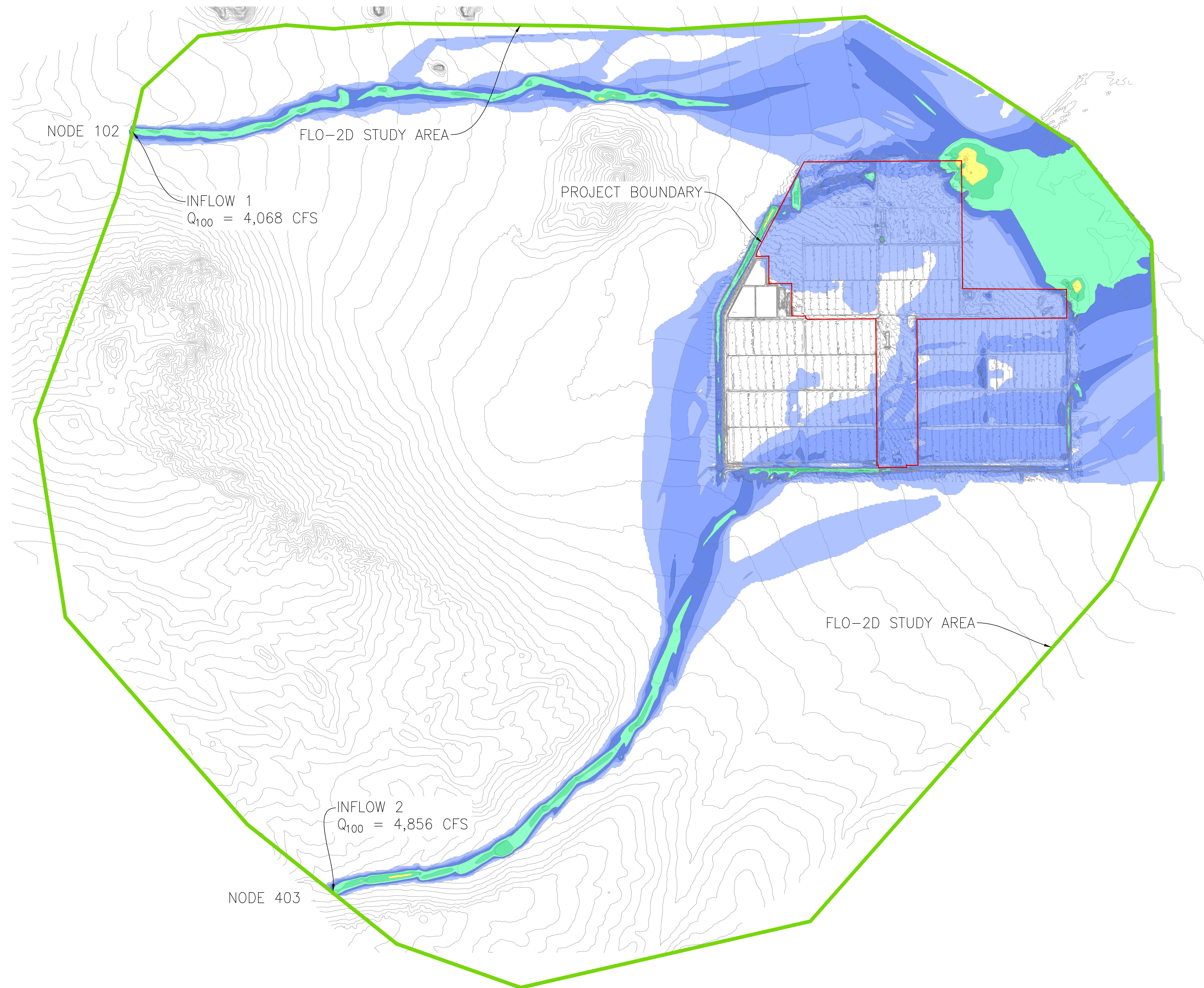
The loss rate used for a particular catchment is a combination of the maximum loss rate F_m and the low loss rate F^* . F^* is used as the loss rate unless F^* exceeds F_m , in which case F_m is used as the loss rate. That is, F_m serves as the maximum loss rate. Typically in 100-year storm studies, F^* serves as the loss rate for the entire storm pattern except for the most



**SAN BERNARDINO COUNTY
HYDROLOGY MANUAL**

**INFILTRATION RATE FOR
PERVIOUS AREAS VERSUS
SCS CURVE NUMBERS**

APPENDIX 7
Flow Depth Results



MAX FLOW DEPTH (FT)

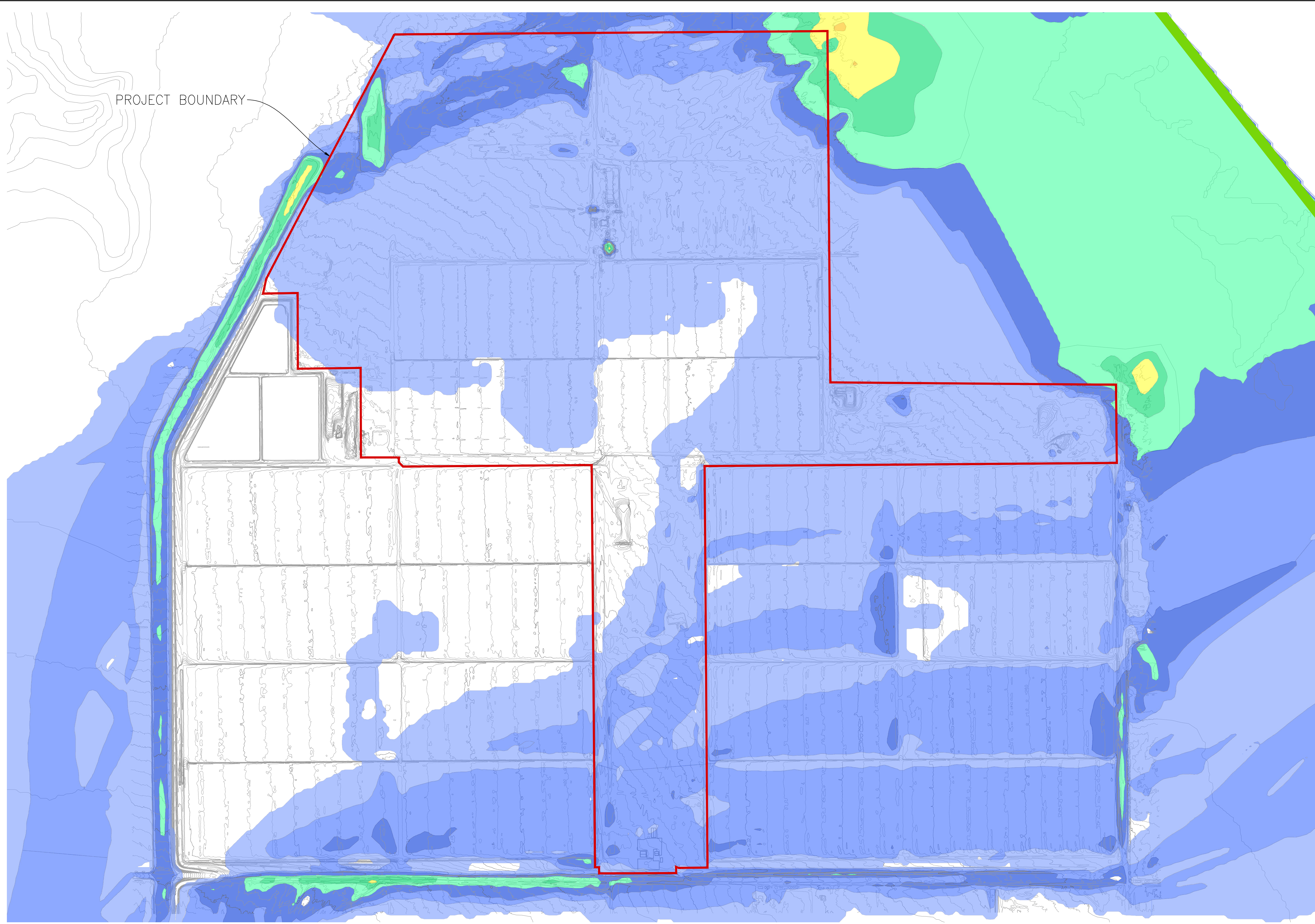
0.1 FT - 0.5 FT
0.5 FT - 1.0 FT
1.0 FT - 2.0 FT
2.0 FT - 3.0 FT
3.0 FT - 4.0 FT
4.0 FT - 5.0 FT
5.0 FT - 6.0 FT
6.0 FT - MAX

MAXIMUM FLOW DEPTH EXHIBIT (OVERALL)
 100-YR STORM EVENT
 LOCKHART SOLAR PV II
 SAN BERNARDINO COUNTY, CALIFORNIA
 PROJECT NUMBER: 1859-003
 DATE: SEPTEMBER 2021
 SHEET 1 OF 2

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 San Diego, California 92122
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 www.fuscoec.com

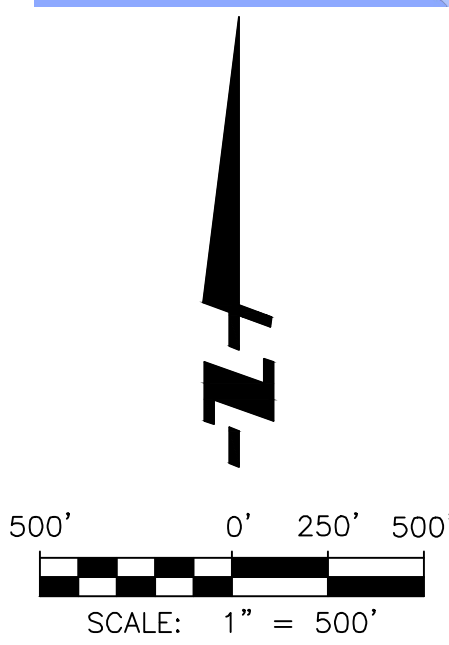
C:\PROJECTS\1859\003_SUPPORT_FILES\REPORTS\REPORTS\1859-003-MAX-DEPTH.PDF (09-21-2021 4:32:10PM) Printed by: PAVAN, SHAM



PROJECT BOUNDARY

MAX FLOW DEPTH (FT)

- 0.1 FT - 0.5 FT
- 0.5 FT - 1.0 FT
- 1.0 FT - 2.0 FT
- 2.0 FT - 3.0 FT
- 3.0 FT - 4.0 FT
- 4.0 FT - 5.0 FT
- 5.0 FT - 6.0 FT
- 6.0 FT - MAX



MAXIMUM FLOW DEPTH EXHIBIT

100-YR STORM EVENT

LOCKHART SOLAR PV II

SAN BERNARDINO COUNTY, CALIFORNIA

PROJECT NUMBER: 1859-003

DATE: SEPTEMBER 2021

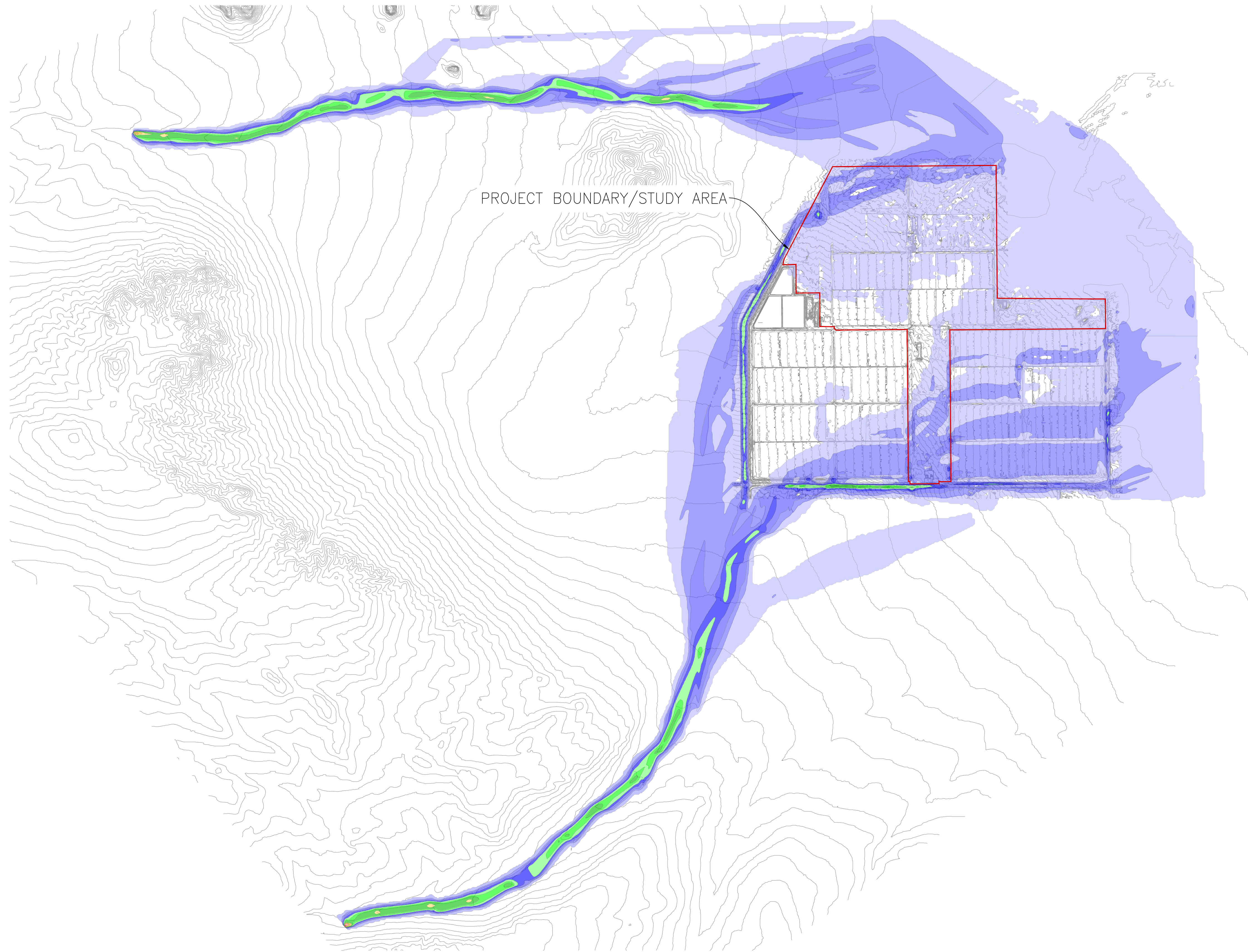
SHEET 2 OF 2

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www.fuscoe.com

C:\PROJECTS\1859\003_SUPPORT_FILES\REPORTS\REPORTS\1859-003_MAX_DEPTH.PNG (09-21-2021 4:22:58PM) Plotted by: PFW@cs.com

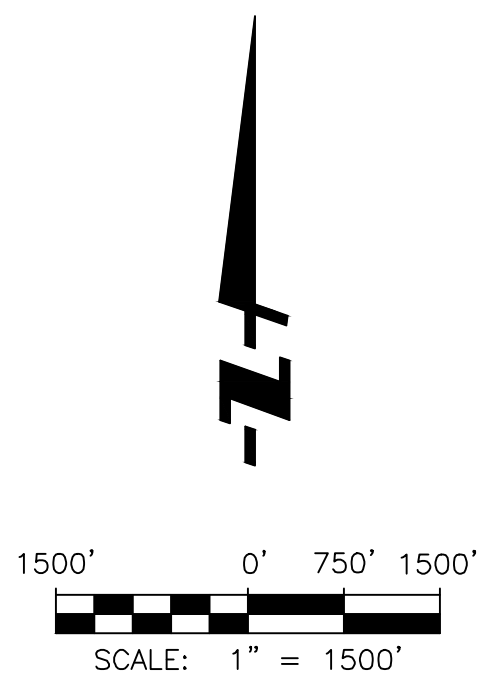
APPENDIX 8
Flow Velocity Results



PROJECT BOUNDARY/STUDY AREA

MAX FLOW DEPTH (FPS)

0.0 FPS - 1.0 FPS
1.0 FPS - 2.0 FPS
2.0 FPS - 3.0 FPS
3.0 FPS - 4.0 FPS
4.0 FPS - 5.0 FPS
5.0 FPS - 6.0 FPS
6.0 FPS - 8.0 FPS
8.0 FPS - 10.0 FPS
10.0 FPS - 12.0 FPS
12.0 FPS - 15.0 FPS
15.0 FPS - 20.0 FPS
20.0 FPS - MAX



MAXIMUM VELOCITY EXHIBIT (OVERALL)

100-YR STORM EVENT

LOCKHART SOLAR PV II

SAN BERNARDINO COUNTY, CALIFORNIA

PROJECT NUMBER: 1859-003

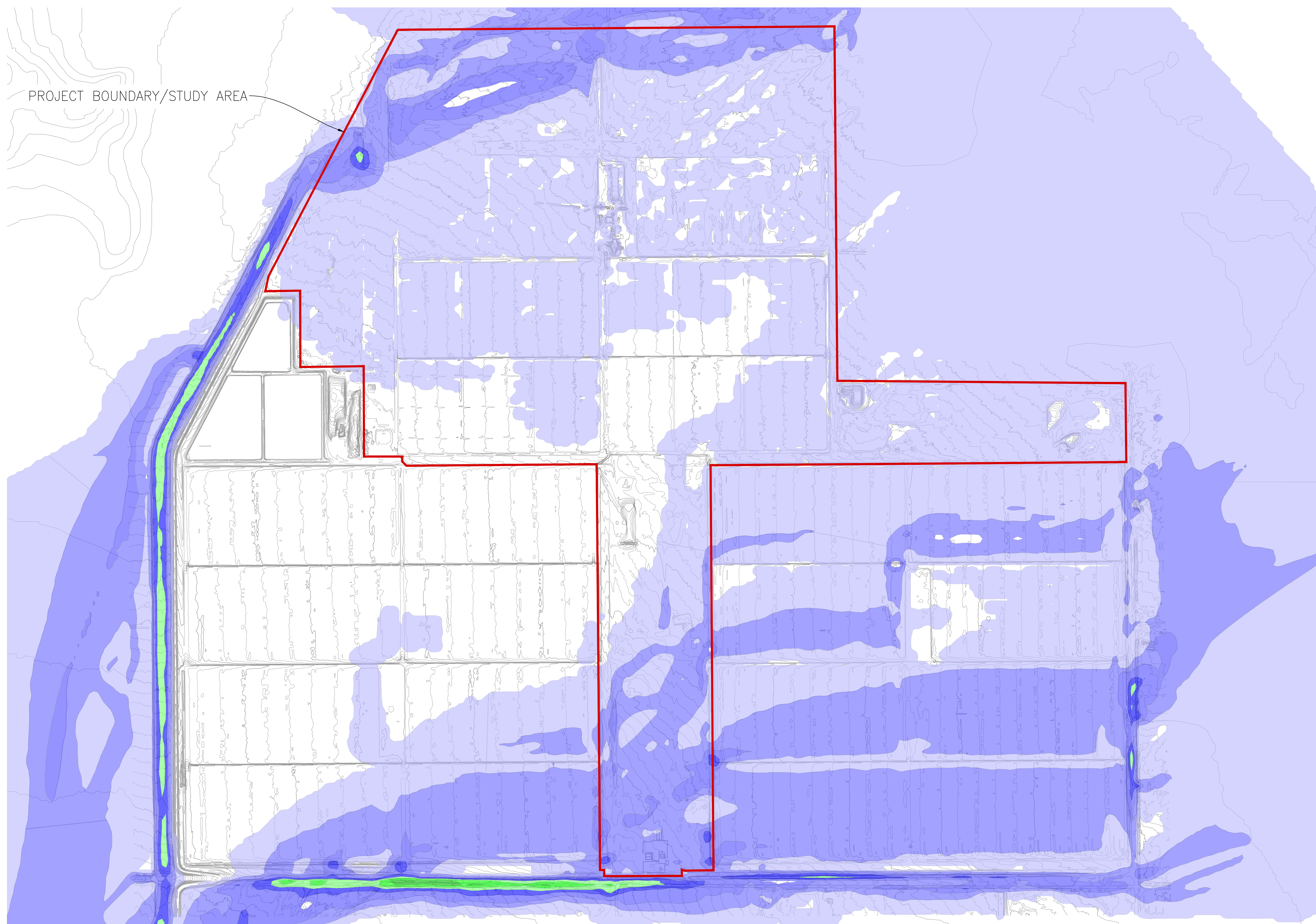
DATE: SEPTEMBER 2021

SHEET 1 OF 2

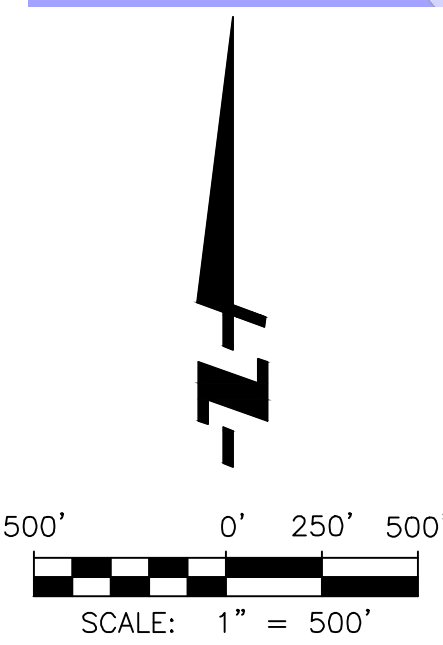
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PROJECT BOUNDARY/STUDY AREA



MAX FLOW DEPTH (FPS)	
0.0 FPS - 1.0 FPS	Lightest Blue
1.0 FPS - 2.0 FPS	Light Blue
2.0 FPS - 3.0 FPS	Medium Light Blue
3.0 FPS - 4.0 FPS	Medium Blue
4.0 FPS - 5.0 FPS	Light Green
5.0 FPS - 6.0 FPS	Light Green
6.0 FPS - 8.0 FPS	Green
8.0 FPS - 10.0 FPS	Yellow-Green
10.0 FPS - 12.0 FPS	Yellow
12.0 FPS - 15.0 FPS	Orange
15.0 FPS - 20.0 FPS	Red-Orange
20.0 FPS - MAX	Red



MAXIMUM VELOCITY EXHIBIT

100-YR STORM EVENT

LOCKHART SOLAR PV II

SAN BERNARDINO COUNTY, CALIFORNIA

PROJECT NUMBER: 1859-003

DATE: SEPTEMBER 2021

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C:\PROJECTS\1859-003_SUPPORT FILES\REPORTS\REPORTS\1859-003_MAX_VELOCITY_EXHIBIT (09-21-2021 4:28:28PM) Plotted by: Patrick Chen

APPENDIX 9
Precipitation Frequency Data



NOAA Atlas 14, Volume 6, Version 2
Location name: Hinkley, California, USA*
Latitude: 35.044°, Longitude: -117.3489°
Elevation: 2048.76 ft**



* source: ESRI Maps
 ** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Tryppaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

[PF_tabular](#) | [PF_graphical](#) | [Maps_&_aerials](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.076 (0.062-0.094)	0.108 (0.088-0.133)	0.151 (0.123-0.187)	0.188 (0.152-0.234)	0.240 (0.188-0.309)	0.281 (0.216-0.370)	0.326 (0.244-0.438)	0.373 (0.272-0.515)	0.439 (0.309-0.632)	0.493 (0.335-0.733)
10-min	0.109 (0.089-0.135)	0.154 (0.126-0.191)	0.216 (0.176-0.268)	0.269 (0.217-0.336)	0.343 (0.269-0.443)	0.403 (0.310-0.530)	0.467 (0.350-0.628)	0.534 (0.390-0.739)	0.630 (0.442-0.906)	0.707 (0.481-1.05)
15-min	0.132 (0.108-0.163)	0.187 (0.152-0.231)	0.262 (0.213-0.324)	0.325 (0.263-0.406)	0.415 (0.325-0.535)	0.488 (0.375-0.642)	0.564 (0.423-0.760)	0.646 (0.472-0.893)	0.762 (0.535-1.10)	0.855 (0.581-1.27)
30-min	0.181 (0.148-0.224)	0.257 (0.210-0.317)	0.360 (0.293-0.446)	0.447 (0.361-0.559)	0.571 (0.448-0.737)	0.671 (0.515-0.882)	0.776 (0.583-1.05)	0.889 (0.649-1.23)	1.05 (0.736-1.51)	1.18 (0.800-1.75)
60-min	0.246 (0.201-0.304)	0.348 (0.284-0.430)	0.488 (0.397-0.605)	0.606 (0.490-0.757)	0.774 (0.607-0.998)	0.910 (0.699-1.20)	1.05 (0.790-1.42)	1.21 (0.880-1.67)	1.42 (0.998-2.04)	1.60 (1.08-2.37)
2-hr	0.339 (0.277-0.418)	0.461 (0.377-0.571)	0.630 (0.514-0.781)	0.774 (0.626-0.967)	0.979 (0.767-1.26)	1.15 (0.879-1.51)	1.32 (0.990-1.78)	1.51 (1.10-2.08)	1.77 (1.25-2.55)	1.99 (1.35-2.95)
3-hr	0.395 (0.323-0.488)	0.533 (0.436-0.659)	0.723 (0.589-0.896)	0.885 (0.715-1.11)	1.12 (0.874-1.44)	1.30 (1.00-1.71)	1.50 (1.13-2.02)	1.71 (1.25-2.37)	2.01 (1.41-2.89)	2.25 (1.53-3.34)
6-hr	0.494 (0.404-0.611)	0.665 (0.543-0.822)	0.898 (0.732-1.11)	1.10 (0.887-1.37)	1.38 (1.08-1.78)	1.61 (1.24-2.11)	1.85 (1.39-2.49)	2.10 (1.54-2.91)	2.46 (1.73-3.54)	2.75 (1.87-4.09)
12-hr	0.574 (0.470-0.709)	0.791 (0.646-0.978)	1.09 (0.886-1.35)	1.34 (1.08-1.67)	1.68 (1.32-2.17)	1.96 (1.51-2.58)	2.25 (1.69-3.03)	2.55 (1.87-3.53)	2.98 (2.09-4.28)	3.31 (2.25-4.92)
24-hr	0.701 (0.623-0.806)	0.999 (0.886-1.15)	1.40 (1.24-1.61)	1.73 (1.52-2.01)	2.18 (1.85-2.63)	2.54 (2.11-3.12)	2.91 (2.36-3.66)	3.29 (2.59-4.26)	3.82 (2.88-5.15)	4.22 (3.08-5.91)
2-day	0.809 (0.718-0.929)	1.17 (1.03-1.34)	1.64 (1.45-1.90)	2.03 (1.78-2.37)	2.56 (2.17-3.08)	2.97 (2.47-3.65)	3.39 (2.75-4.27)	3.82 (3.01-4.95)	4.40 (3.32-5.94)	4.85 (3.54-6.78)
3-day	0.863 (0.766-0.992)	1.25 (1.11-1.44)	1.77 (1.56-2.04)	2.19 (1.92-2.55)	2.75 (2.33-3.31)	3.19 (2.64-3.91)	3.62 (2.94-4.56)	4.07 (3.21-5.28)	4.68 (3.53-6.32)	5.14 (3.75-7.19)
4-day	0.906 (0.804-1.04)	1.32 (1.17-1.52)	1.87 (1.65-2.15)	2.31 (2.02-2.68)	2.90 (2.46-3.49)	3.35 (2.78-4.12)	3.81 (3.08-4.79)	4.27 (3.36-5.53)	4.89 (3.70-6.61)	5.37 (3.91-7.51)
7-day	0.967 (0.859-1.11)	1.41 (1.25-1.62)	2.00 (1.77-2.30)	2.47 (2.17-2.87)	3.11 (2.63-3.74)	3.59 (2.98-4.41)	4.06 (3.29-5.12)	4.55 (3.58-5.89)	5.19 (3.92-7.01)	5.67 (4.14-7.93)
10-day	1.01 (0.894-1.16)	1.47 (1.31-1.69)	2.09 (1.85-2.41)	2.59 (2.27-3.02)	3.27 (2.77-3.93)	3.78 (3.14-4.64)	4.28 (3.47-5.40)	4.80 (3.78-6.21)	5.47 (4.14-7.40)	5.98 (4.36-8.36)
20-day	1.14 (1.01-1.31)	1.69 (1.50-1.95)	2.44 (2.16-2.82)	3.05 (2.68-3.55)	3.90 (3.30-4.69)	4.54 (3.77-5.58)	5.19 (4.20-6.54)	5.85 (4.61-7.58)	6.70 (5.06-9.06)	7.34 (5.36-10.3)
30-day	1.28 (1.14-1.47)	1.91 (1.70-2.20)	2.79 (2.47-3.22)	3.52 (3.09-4.09)	4.54 (3.85-5.46)	5.32 (4.42-6.54)	6.11 (4.95-7.70)	6.91 (5.45-8.96)	7.97 (6.02-10.8)	8.75 (6.39-12.2)
45-day	1.47 (1.31-1.69)	2.22 (1.97-2.55)	3.26 (2.88-3.76)	4.14 (3.63-4.82)	5.39 (4.57-6.49)	6.38 (5.30-7.84)	7.38 (5.98-9.30)	8.40 (6.62-10.9)	9.76 (7.37-13.2)	10.8 (7.86-15.1)
60-day	1.60 (1.42-1.84)	2.42 (2.14-2.78)	3.58 (3.17-4.13)	4.58 (4.02-5.33)	6.00 (5.09-7.23)	7.15 (5.93-8.78)	8.32 (6.74-10.5)	9.51 (7.49-12.3)	11.1 (8.40-15.0)	12.3 (9.00-17.3)

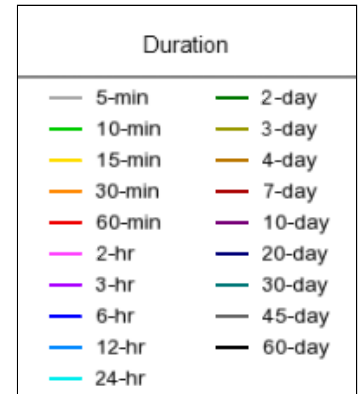
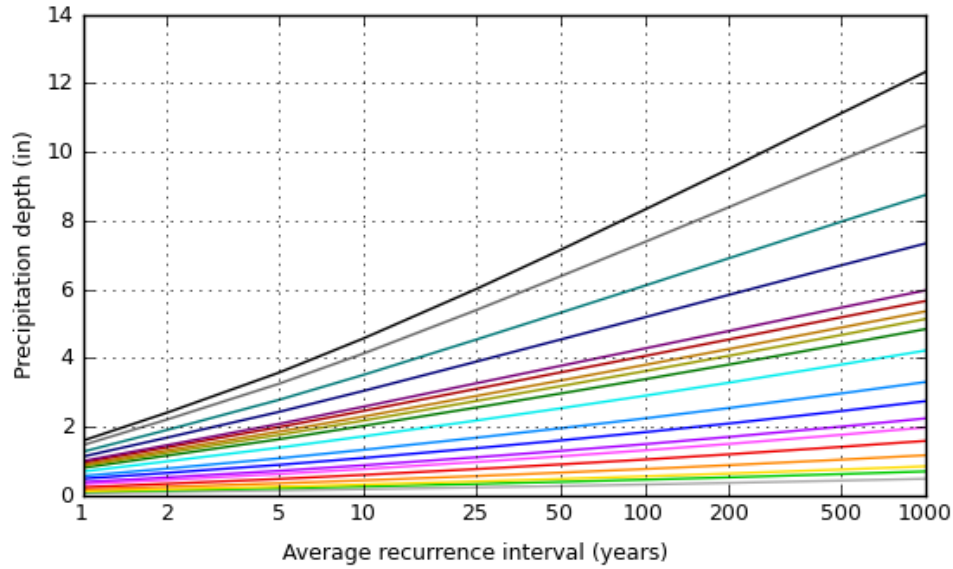
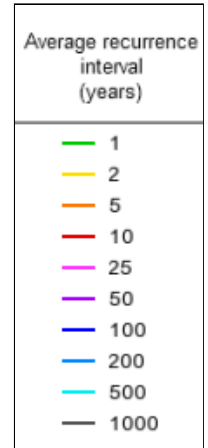
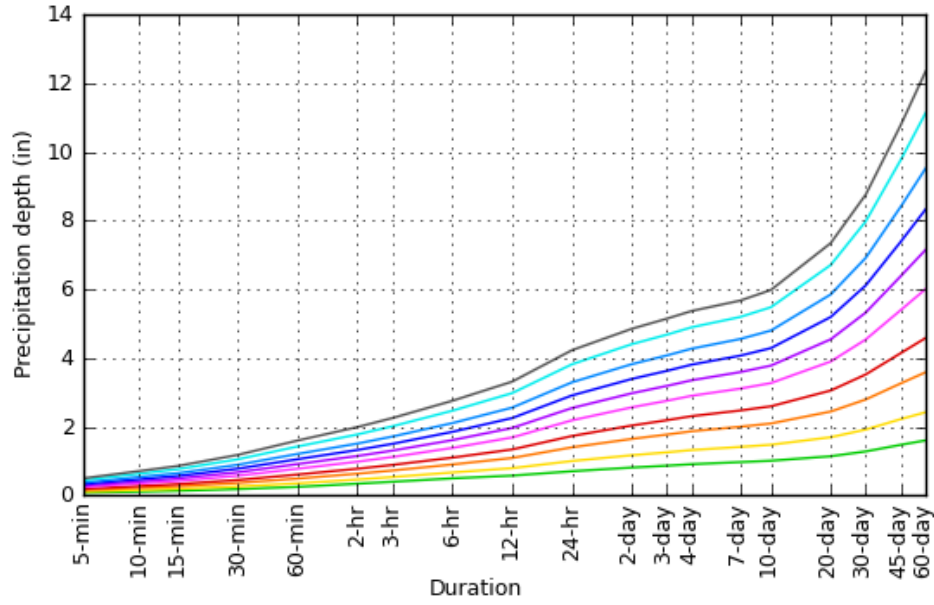
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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PF graphical

PDS-based depth-duration-frequency (DDF) curves

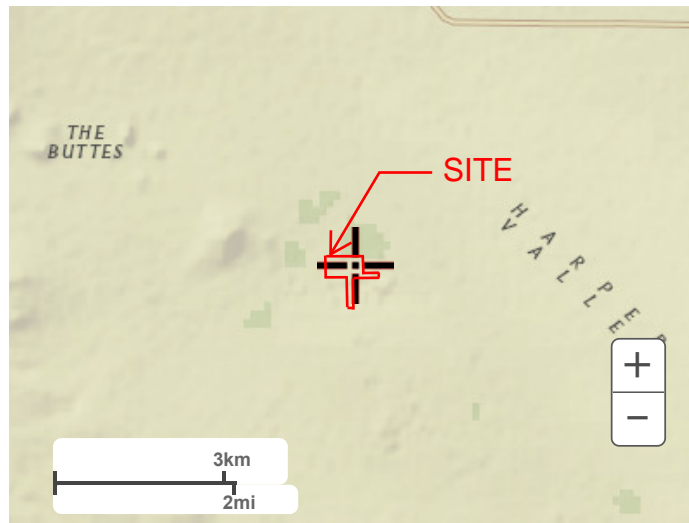
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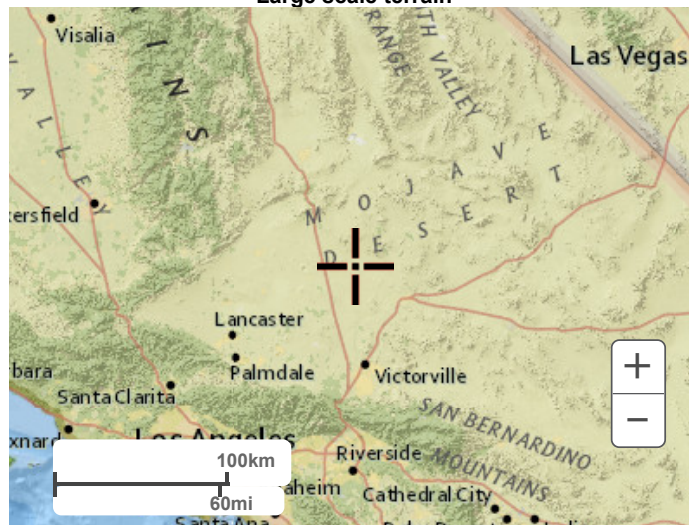
[Back to Top](#)

Maps & aerials

Small scale terrain



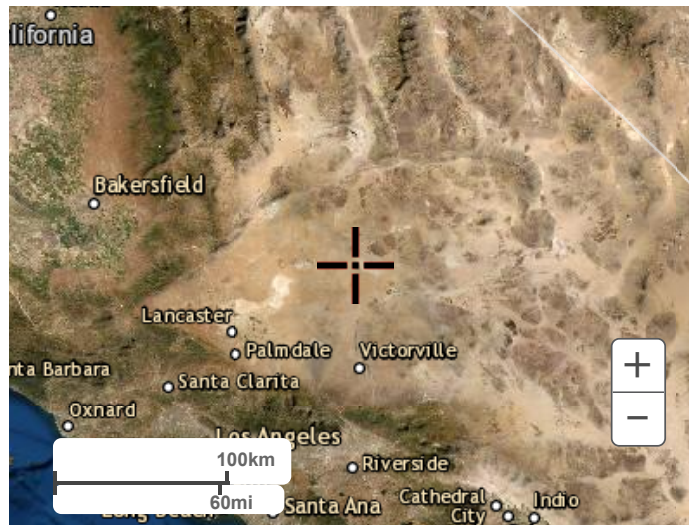
Large scale terrain



Large scale map



Large scale aerial



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Questions?: HDSC.Questions@noaa.gov

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APPENDIX 10
Hydrologic Soil Group



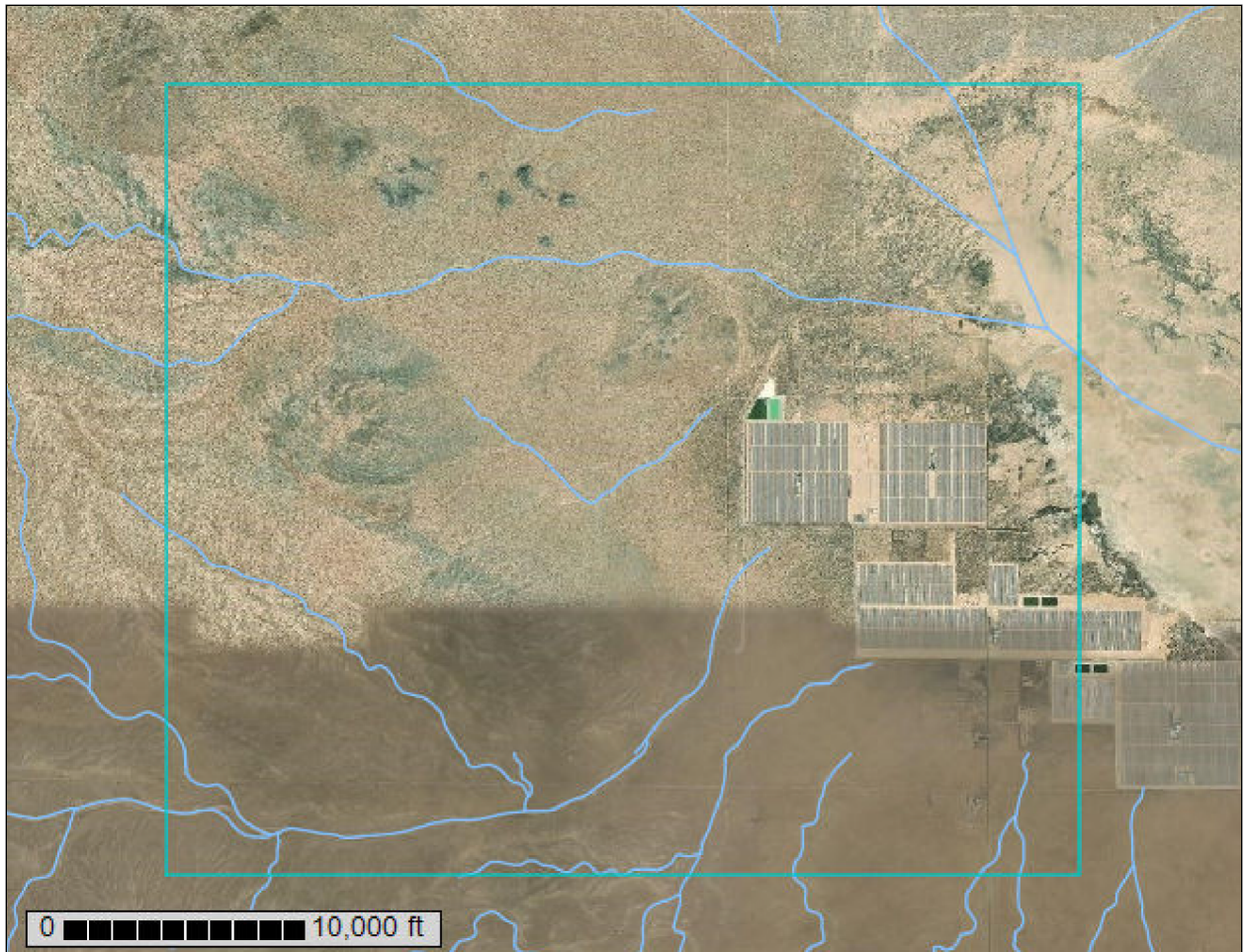
United States
Department of
Agriculture

NRCS

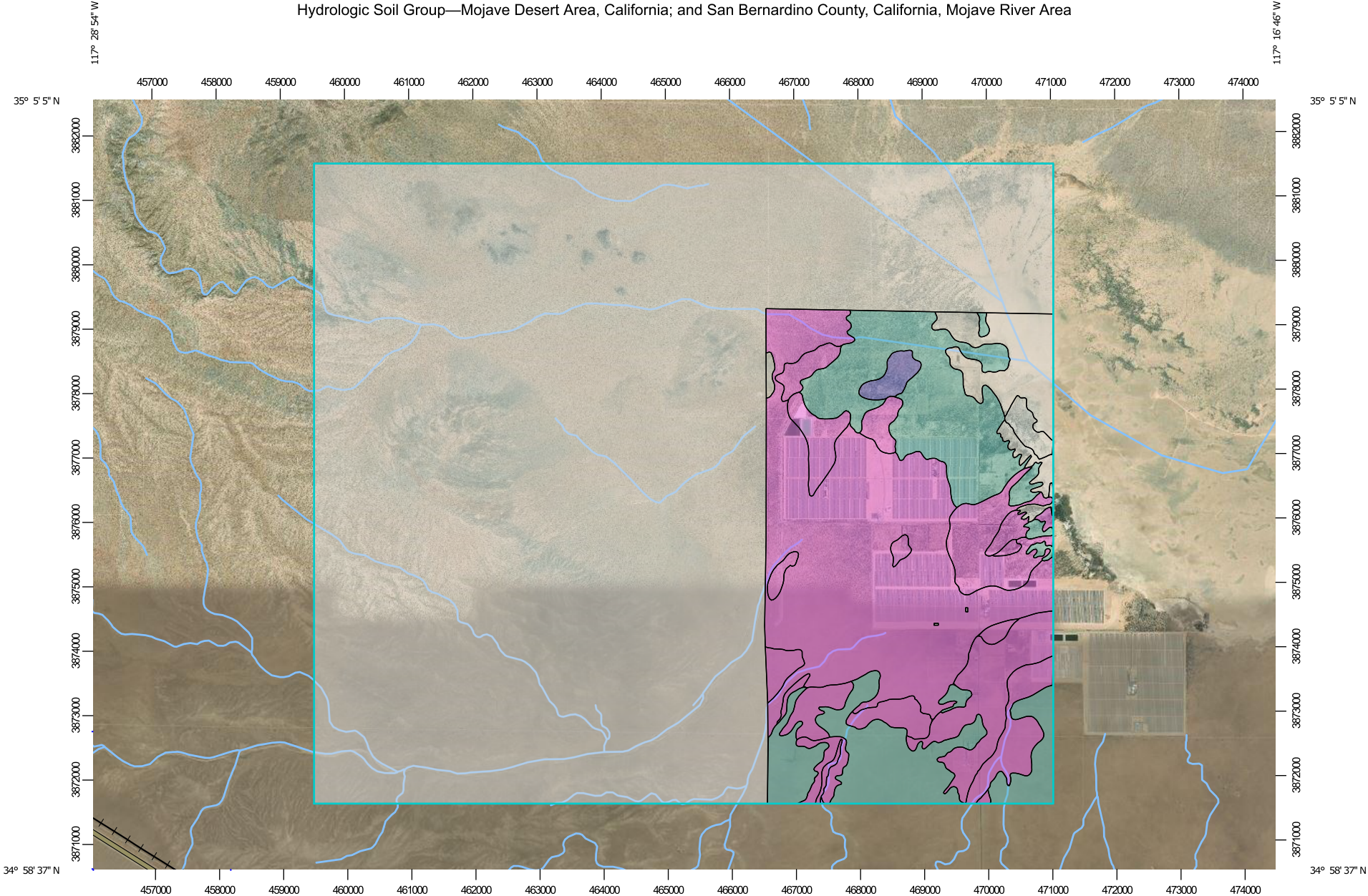
Natural
Resources
Conservation
Service

A product of the National
Cooperative Soil Survey,
a joint effort of the United
States Department of
Agriculture and other
Federal agencies, State
agencies including the
Agricultural Experiment
Stations, and local
participants

Custom Soil Resource Report for Mojave Desert Area, California; and San Bernardino County, California, Mojave River Area



Hydrologic Soil Group—Mojave Desert Area, California; and San Bernardino County, California, Mojave River Area



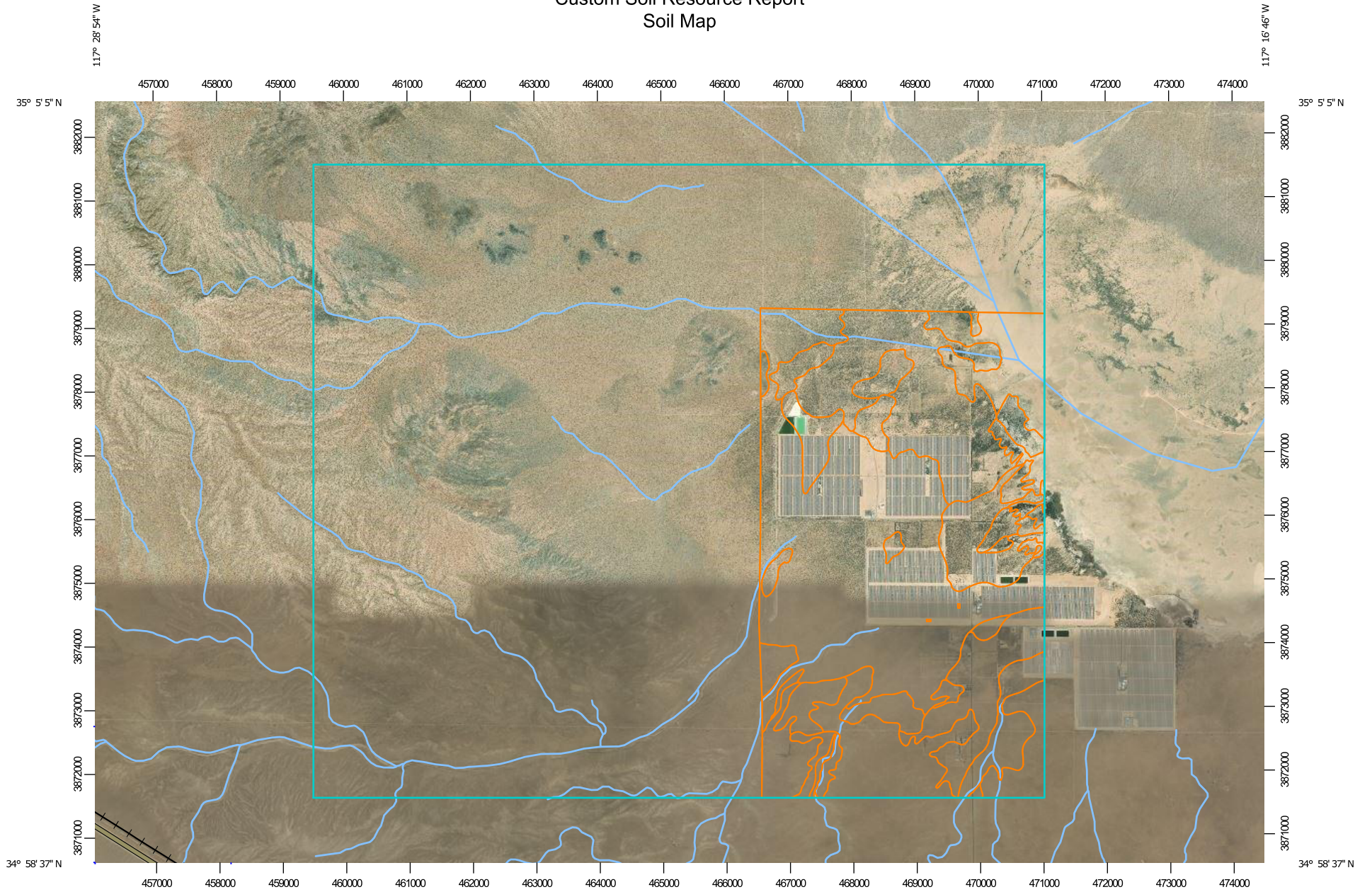
Map Scale: 1:84,300 if printed on A landscape (11" x 8.5") sheet.

0 1000 2000 4000 6000 Meters

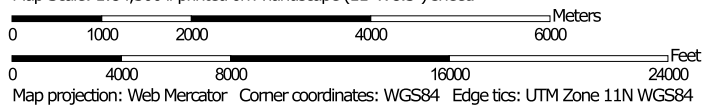
0 4000 8000 16000 24000 Feet

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 11N WGS84

Custom Soil Resource Report Soil Map




Map Scale: 1:84,300 if printed on A landscape (11" x 8.5") sheet.



MAP LEGEND

Area of Interest (AOI)









 Area of Interest (AOI)

Soils

Soil Rating Polygons





 A
 A/D
 B
 B/D
 C
 C/D
 D
 Not rated or not available

Soil Rating Lines

 A
 A/D
 B
 B/D
 C
 C/D
 D
 Not rated or not available

Soil Rating Points





 A
 A/D
 B
 B/D

 C
 C/D
 D
 Not rated or not available

Water Features

 Streams and Canals

Transportation

 Rails
 Interstate Highways
 US Routes
 Major Roads
 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Mojave Desert Area, California
 Survey Area Data: Version 18, Jun 8, 2020

Soil Survey Area: San Bernardino County, California, Mojave River Area
 Survey Area Data: Version 12, May 27, 2020

Your area of interest (AOI) includes more than one soil survey area. These survey areas may have been mapped at different scales, with a different land use in mind, at different times, or at different levels of detail. This may result in map unit symbols, soil properties, and interpretations that do not completely agree across soil survey area boundaries.

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Dec 31, 2009—Jul 8, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
NOTCOM	No Digital Data Available		19,938.3	70.1%
Subtotals for Soil Survey Area			19,938.3	70.1%
Totals for Area of Interest			28,425.7	100.0%

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
112	CAJON SAND, 0 TO 2 PERCENT SLOPES	A	3,452.3	12.1%
113	CAJON SAND, 2 TO 9 PERCENT SLOPES	A	319.1	1.1%
117	CAJON LOAMY SAND, LOAMY SUBSTRATUM, 0 TO 2 PERCENT SLOPES	A	1,204.6	4.2%
137	KIMBERLINA LOAMY FINE SAND, COOL, 0 TO 2 PERCENT SLOPES	A	318.2	1.1%
152	NOROB-HALLORAN COMPLEX, 0 TO 5 PERCENT SLOPES*	C	2,398.9	8.4%
156	PLAYAS		570.4	2.0%
158	ROCK OUTCROP-LITHIC TORRIORTHENTS COMPLEX, 15 TO 50 PERCENT SLOPES*		19.3	0.1%
170	VICTORVILLE VARIANT SAND	B	96.0	0.3%
178	WATER		108.6	0.4%
Subtotals for Soil Survey Area			8,487.5	29.9%
Totals for Area of Interest			28,425.7	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher