



COFFEY ENGINEERING, INC.

Drainage Report

**1615 Ocean Front
San Diego, CA 92104**

APN 539-143-01, 539-143-02

(PTS No. 612237)

Prepared for:

The City of San Diego



September 3, 2021

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1. Existing Conditions

In existing conditions, this project site is a residential lot with existing house. The majority of storm water runoff generated from this site (house, carport, front yard hardscape and landscape areas) currently discharges to the alley at the front of the property, where it flows northeasterly to an existing public storm drain system at the intersection of Cable Street and Niagara Avenue, before ultimately discharging to the Pacific Ocean.

See Appendix A- Drainage Map A.

2. Proposed Project

In proposed conditions, this project site will remain a residential lot and include a large addition to the house. A new carport will be constructed, along with a covered deck and surrounding hardscape. A below grade car lift will also be installed. The flow patterns will remain relatively the same as pre-construction conditions. Storm water runoff coming into contact with the rooftop and deck cover will reach landscape drains in the front yard and rear courtyard and be routed to a below-ground sump pump system. Sump flows will be pumped to a city-standard D-27 sidewalk underdrain at the northeast corner of the site, which will discharge the flows to the street through 2-3" diameter PVC outlet pipes. The walkway and parking area/lift will sheet flow into Ocean Front Street.

The rear yard bluff will remain untouched, and replicate the pre-construction drainage pattern.

See Appendix A- Drainage Map B.

3. Purpose and Scope of Report

This report will evaluate the proposed drainage pipe system and flow rate discharge to the street.

4. Method of Calculations

The Rational Method, as defined by *City of San Diego Drainage Design Manual 2017*, will be used to calculate storm water flow rates. Where noted, the following calculations were used to determine flow properties:

Rainfall Characteristics

$Q = C * I * A$, where

Q = Flow rate (ft³/sec)

C = Runoff coefficient

(Runoff coefficient per *City of San Diego Drainage Design Manual 2017* reproduced in Appendix C. Soil type D determined from the *Soil Hydrologic Groups* map from the County of San Diego Hydrology Manual reproduced in Appendix C also.)

I = Rainfall intensity (in/hr.)

A = Area (acres)

5. **Results and Conclusions:**

Based on the calculations, the site will feature a slightly larger discharge to Ocean Front Street in proposed conditions, from 0.17 cfs to 0.24 cfs. Consequently, there is less runoff directed to the bluffs, from 0.25 cfs to 0.18 cfs. There are no anticipated impacts to adjacent properties as all storm water runoff from the habitable area discharges directly to the frontage street.

In pre-construction conditions, there is a 3” outlet located at the northeast corner of the property. Since it is unknown exactly how much runoff directly reaches this outlet pipe, at max velocity the pipe is expected to discharge to the street at 2.43 FPS.

The proposed D-27 sidewalk underdrain contains 2-3” diameter outlet pipes discharging to Ocean Front Street at a velocity of 2.43 FPS. Therefore, there is no increase or decrease of velocity expected to Ocean Front Street.

The final flow calculations will be submitted during the ministerial review.

6. **Clean Water Act (CWA) Compliance**

The proposed project is exempt from permitting under Federal Clean Water Act section 401 or 404 because it does not directly discharge into navigable waters of the United States.

7. Declaration of Responsible Charge

I hereby declare that I am the Civil Engineer of work for this project, that I have exercised responsible charge over the design of the project as defined in section 6703 of the business and professions code, and that the design is consistent with current design.

I understand that the check of project drawings and specifications by the City of San Diego is confined to a review only and does not relieve me, as Engineer of Work, of my responsibilities for project design.

Michael C. Kinnear
RCE 76785
Exp. 12-31-22

Date



Appendix A – Drainage Map

GRADING TABULATIONS	
TOTAL AMOUNT OF SITE TO BE GRADED: 0 S.F.*	% OF TOTAL SITE: 0%
AMOUNT OF CUT: 0 CUBIC YARDS	MAXIMUM DEPTH OF CUT: 0 FEET
AMOUNT OF FILL: 0 CUBIC YARDS	MAXIMUM DEPTH OF FILL: 0 FEET
MAXIMUM HEIGHT OF FILL SLOPE(S): 0 FEET	SLOPE RATIO: N/A
MAXIMUM HEIGHT OF CUT SLOPE(S): 0 FEET	SLOPE RATIO: N/A
AMOUNT OF IMPORT/EXPORT SOIL: 0 CUBIC YARDS	MAXIMUM HEIGHT: N/A FEET
RETAINING/ CRIB WALLS: LENGTH 0 FEET	

*WHEN INCLUDING BUILDING FOOTPRINT EXCAVATION AREA:
TOTAL AMOUNT OF SITE TO BE GRADED: 1,825 S.F.
% OF TOTAL SITE: 24%
AMOUNT OF CUT: 420 CUBIC YARDS
MAXIMUM DEPTH OF CUT: 11 FEET
AMOUNT OF EXPORT SOIL: 420 CUBIC YARDS

DEVELOPMENT PROJECT CATEGORY

STANDARD

CONSTRUCTION SITE PRIORITY

LOW

STORM WATER DOCUMENTATION

WPCP REQUIRED

PROJECT PROPERTY AREA

LOT AREA = 7,478 SQ FT (0.17 ACRES)
TOTAL DISTURBANCE AREA = 3,743 SQ FT
EXISTING AMOUNT OF PERVIOUS AREA = 3,909 SQ FT
EXISTING AMOUNT OF IMPERVIOUS AREA = 3,569 SQ FT
PROPOSED AMOUNT OF PERVIOUS AREA = 5,161 SQ FT
PROPOSED AMOUNT OF IMPERVIOUS AREA = 2,308 SQ FT
IMPERVIOUS % INCREASE = -17%

STORM WATER NOTES

- ALL PROPOSED DRAINAGE SHALL MAINTAIN EXISTING FLOW PATTERNS TOWARD THE STREET AND TOWARDS THE ALLEY.
- THIS PROJECT WILL NOT DISCHARGE ANY INCREASE IN STORM WATER RUN-OFF ONTO THE COASTAL BLUFF.

DRAINAGE NOTES

- ALL MAIN DRAIN LINES SHOWN TO BE 6" PVC @ 1% MINIMUM SLOPE UNLESS OTHERWISE NOTED.
- ALL CATCH BASIN LEADS TO BE 4" PVC @ 2% MINIMUM SLOPE UNLESS OTHERWISE NOTED.
- HARDSCAPE GRADES TO BE 1% MINIMUM TO DRAINS AND AWAY FROM STRUCTURE.
- SOFTSCAPE GRADES TO BE 2% MINIMUM TO DRAINS (1% WHERE FLOW IS CONCENTRATED) AND 2% MINIMUM AWAY FROM STRUCTURE.
- SOIL COVER ABOVE DRAIN LINES SHALL BE 12" MINIMUM UNLESS OTHERWISE NOTED.
- NOTIFY CIVIL ENGINEER IF ANY NON-DRAINING SUMP CONDITIONS BECOME APPARENT DURING CONSTRUCTION.

LEGEND

DESCRIPTION	STD DWG	SYMBOL
PROPERTY LINE		N45°45'45"W
STREET CENTERLINE		90
(E) CONTOUR		+100.00
(E) SPOT ELEVATION		W
(E) FENCE TO REMAIN		S
(E) WATER LINE		W
(E) SEWER LINE		S
(E) WATER SERVICE		W
(E) SEWER SERVICE		S
(E) FIRE HYDRANT		Star symbol
EXISTING TREE		Tree symbol
PVT SPOT ELEVATION		100.00
PVT PVC DRAIN LINE (PER PLAN)	SDR-35 SCH 40	--- ---
ROOF DRAIN		o DS
PVT FREESTANDING WALL		--- ---
PVT BUILDING FOOTPRINT		--- ---
PVT FENCE (MAT'L PER ARCHITECT)		--- ---
PVT PAVERS		--- ---
PVT DECK		--- ---
BUILDING FOOTPRINT		--- ---
BUILDING STEP (ELEVATION CHANGE)		--- ---
BUILDING OVERHANG		--- ---

LEGAL DESCRIPTION

LOT 9 & 10 IN BLOCK 66 OF OCEAN BEACH, IN THE CITY OF SAN DIEGO, COUNTY OF SAN DIEGO, STATE OF CALIFORNIA, ACCORDING TO MAP THEREOF NO. 279 FILED IN THE OFFICE OF THE COUNTY RECORDER OF SAN DIEGO, MAY 28, 1987.

APN: 448-161-02

BENCHMARK

CABLE STREET AND DEL MAR AVENUE
N 2103, E 16916, EL=39.021, MSL, SBP
AS PUBLISHED IN THE CITY OF SAN DIEGO VERTICAL CONTROL BENCHMARK, OCTOBER 2011.

SOURCE OF TOPOGRAPHY

FIELD TOPO BY:
BERGREN & ASSOCIATES
6046 CORNERSTONE COURT, #116
SAN DIEGO, 92121
DATED: FEBRUARY 2019

EASEMENTS

THIS PLAN WAS PREPARED WITHOUT THE BENEFIT OF A TITLE REPORT. EASEMENTS MAY BE PRESENT ON THE SUBJECT PROPERTY.

ABBREVIATIONS

AC	ASPHALTIC CONCRETE	HP	HIGH POINT
BRW	BOTTOM OF RETAINING WALL GRADE	IE	INVERT ELEVATION
BX	BOX	MIN	MINIMUM
CB	CATCH BASIN	PA	PLANTING AREA
C/L	CENTERLINE	PCC	PORTLAND CEMENT CONCRETE
CONC	CONCRETE	P.I.P.	PROTECT IN PLACE
D	DRAIN	PNL	PANEL
EB	ELECTRIC BOX	PP	POWER POLE
EL	ELEVATION	(P), PR	PROPOSED
ELEC	ELECTRIC	PVC	POLYVINYLCHLORIDE
ELEV	ELEVATOR	ROW	RIGHT OF WAY
EQ	EQUIPMENT	RP	REDUCED PRESSURE
(E), EX	EXISTING	S, SWR	SEWER
FH	FIRE HYDRANT	SLB	STREET LIGHT BOX
FF	FINISH FLOOR	STD	STANDARD
FL	FLOWLINE	TC	TOP OF CURB
FS	FINISH SURFACE/SLAB	TE	TRASH ENCLOSURE
GM	GAS METER	TSB	TRAFFIC SIGNAL BOX
GV	GAS VALVE	TRW	TOP OF RETAINING WALL GRADE
GP	GUARD POST	U.N.O.	UNLESS NOTED OTHERWISE
		WM	WATER METER

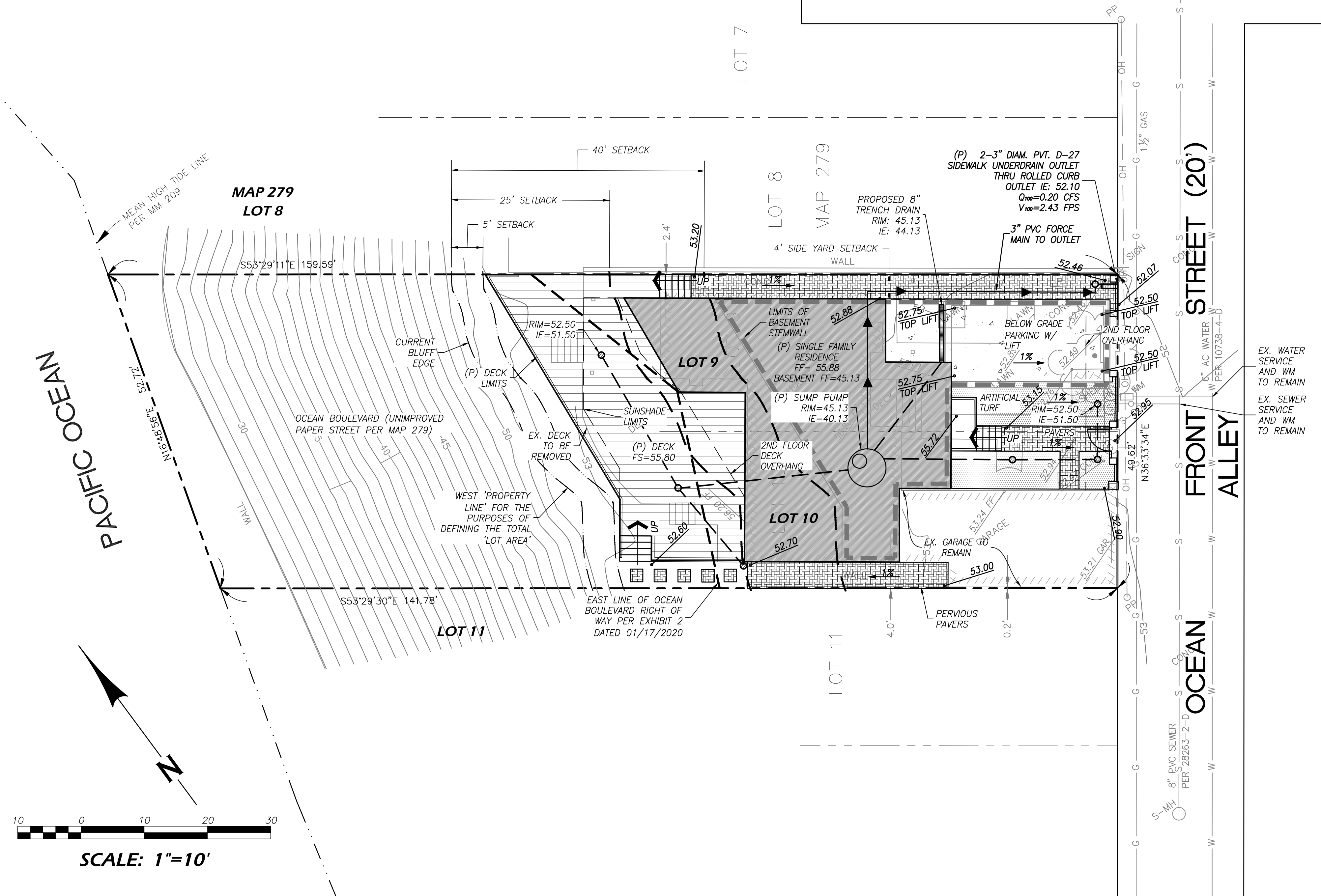
UTILITY NOTE

- THE LOCATIONS OF UTILITIES, IF ANY, SHOWN ON THIS PLAN ARE GENERATED FROM RECORDS PROVIDED BY UTILITY/GOVERNING AGENCIES AND/OR FIELD DATA COLLECTED DURING THE SURVEY. THE PLOTTING OF UTILITIES ON THIS PLAN DOES NOT CONSTITUTE A GUARANTEE OF THEIR LOCATION, DEPTH, SIZE, OR TYPE.

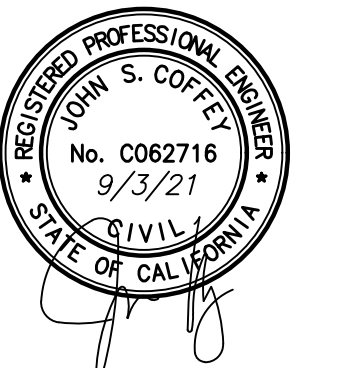
OCEAN FRONT STREET (20')

FRONT STREET (20')

OCEAN FRONT ALLEY



COFFEY ENGINEERING, INC.
9666 BUSINESSPARK AVENUE, SUITE 210, SAN DIEGO, CA 92131 PH: (619)831-0111 FAX: (619)831-0179



OCEAN FRONT RESIDENCE
Remodel & Addition
1615 Ocean Front Street
San Diego, CA 92107

DRAWN BY:	DK / MR
CHECKED BY:	JC
ORIGINAL	9/3/21
REVISION 1	
REVISION 2	
REVISION 3	
REVISION 4	
REVISION 5	

GRADING and DRAINAGE PLAN

SCALE: 1" = 10'

C.1

SHT 1 OF 1 SHTS

LEGEND

DESCRIPTION	SYMBOL
DIRECTION OF FLOW	→ →
SITE BASIN LIMIT	—
DIRECTION OF FLOW	→ →
IMPERVIOUS AREA	▨

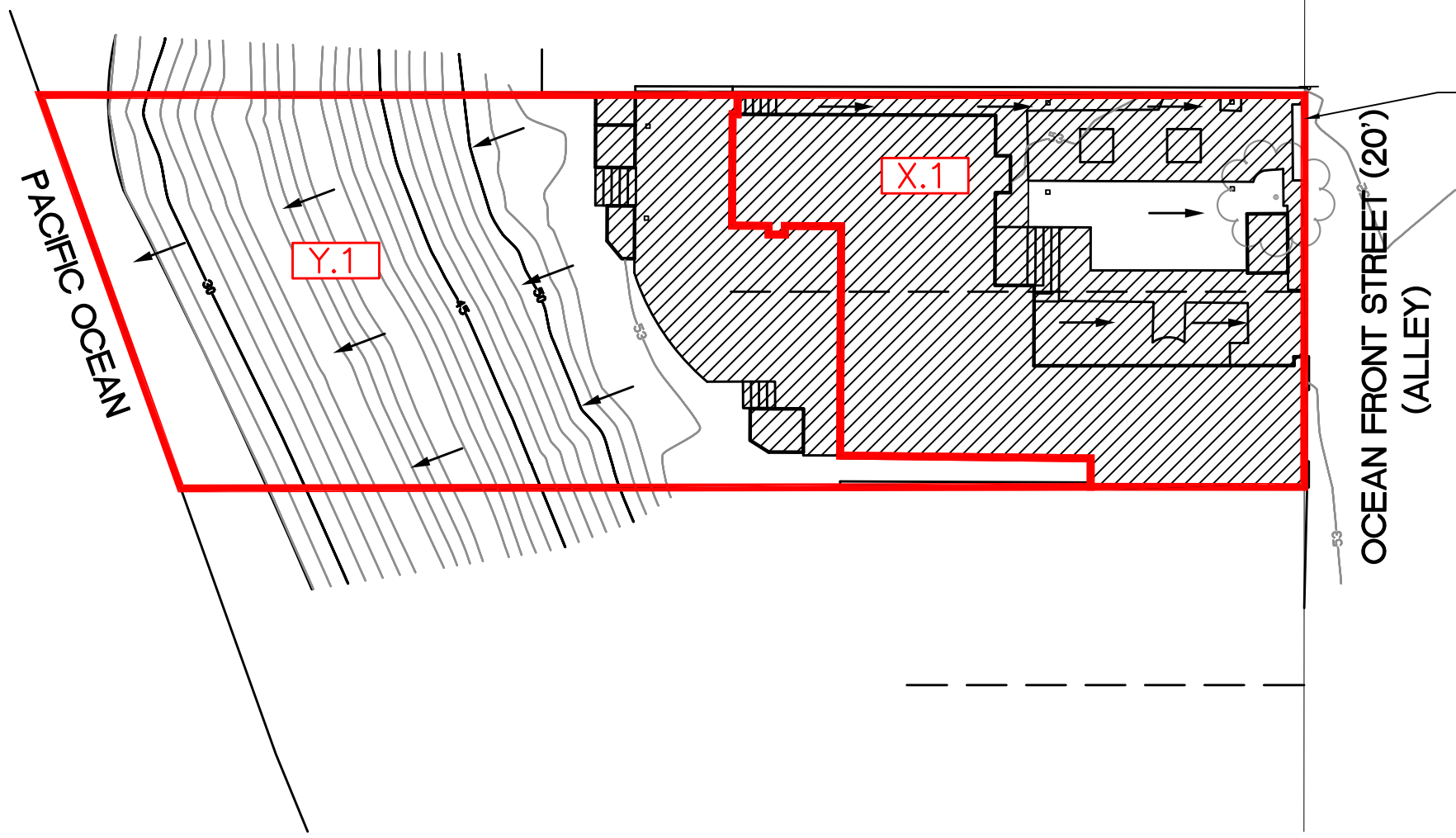
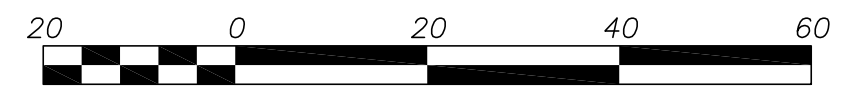
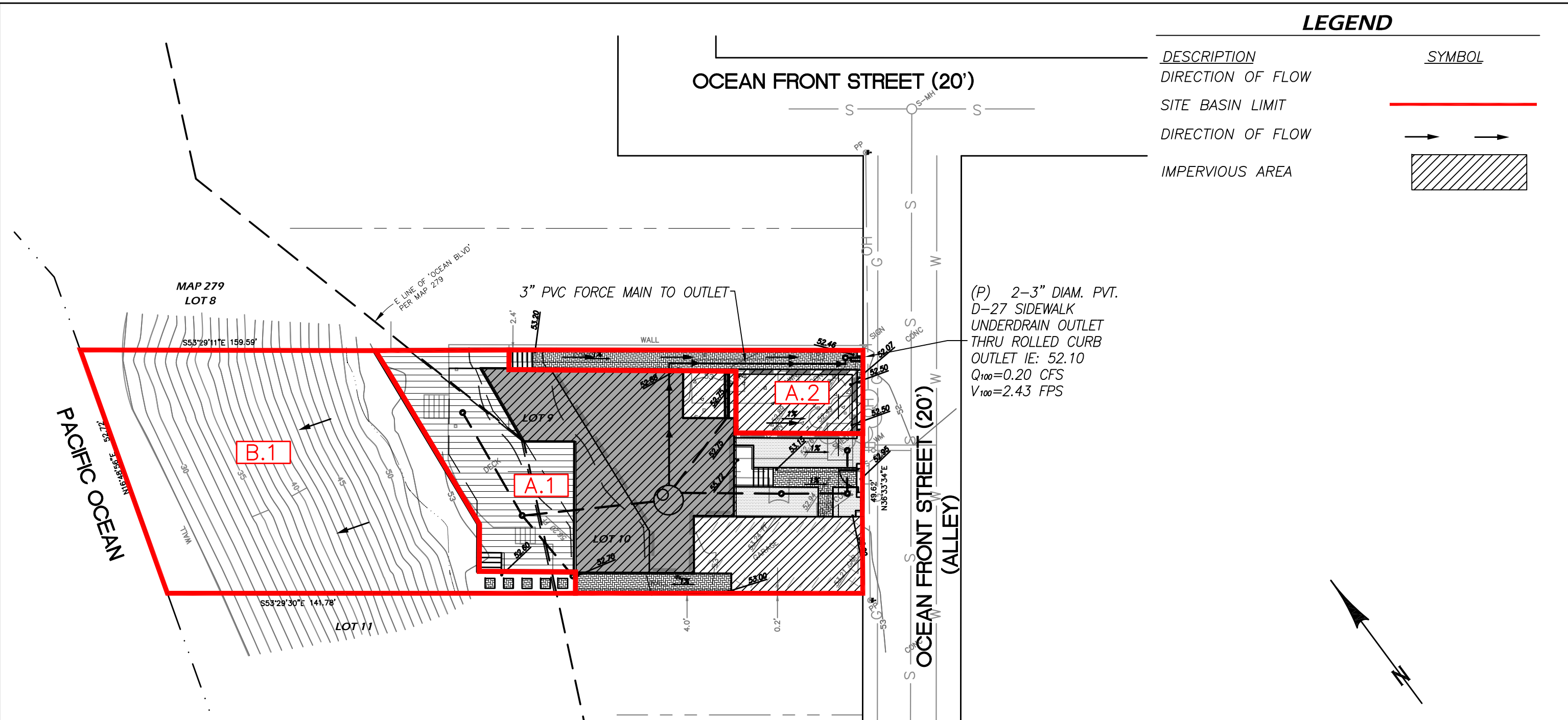


Table A - Pre Construction Flow Conditions						Hydraulics of Existing Structures	
Flow ID (Basin)	Runoff Coefficient, C	Summary				Flow ID (Basin)	Flow Description
		(5 min minimum) Total time-of-concentration, T _c (min)	Rainfall Intensity, I (in/hr)	Basin Area, A (acres)	Q (cfs)		
X.1	0.55	5.00	4.40	0.07	0.17	X.1	Discharge to Ocean Front Street
Y.1	0.55	5.00	4.40	0.10	0.25	Y.1	Bluff to Pacific Ocean
Sum =					0.42		

*1615 Ocean Front
San Diego CA 92107
DRAINAGE MAP 'A'
EXISTING CONDITIONS*

LEGEND

DESCRIPTION	SYMBOL
DIRECTION OF FLOW	
SITE BASIN LIMIT	
DIRECTION OF FLOW	
IMPERVIOUS AREA	



Flow ID (Basin)	Runoff Coefficient, C	(5 min minimum) Total time-of-concentration, T_c (min)	Rainfall Intensity, I (in/hr)	Basin Area, A (acres)	Q (cfs)	Flow ID (Basin)	Flow Description
A.1	0.55	5.00	4.40	0.08	0.20	A.1	Sump pump to Ocean Front Street
A.2	0.55	5.00	4.40	0.01	0.04	A.2	Sheet flow to Ocean Front Street
B.1	0.55	5.00	4.40	0.08	0.18	B.1	Bluff to Pacific Ocean
Sum =					0.42		

*1615 Ocean Front
 San Diego CA 92107
 DRAINAGE MAP 'B'
 PROPOSED CONDITIONS*

Appendix B –Calculation/Evaluations

100 Year Storm

Table A - Pre Construction Flow Conditions						Hydraulics of Existing Structures	
Summary							
Flow ID (Basin)	Runoff Coefficient, C	(5 min minimum) Total time-of-concentration, T _c (min)	Rainfall Intensity, I (in/hr)	Basin Area, A (acres)	Q (cfs)	Flow ID (Basin)	Flow Description
X.1	0.55	5.00	4.40	0.07	0.17	X.1	Discharge to Ocean Front Street
Y.1	0.55	5.00	4.40	0.10	0.25	Y.1	Bluff to Pacific Ocean
Sum =					0.42		

Table B - Post Construction Flow Conditions						Hydraulics of Proposed Structures	
Summary							
Flow ID (Basin)	Runoff Coefficient, C	(5 min minimum) Total time-of-concentration, T _c (min)	Rainfall Intensity, I (in/hr)	Basin Area, A (acres)	Q (cfs)	Flow ID (Basin)	Flow Description
A.1	0.55	5.00	4.40	0.08	0.20	A.1	Sump pump to Ocean Front Street
A.2	0.55	5.00	4.40	0.01	0.04	A.2	Sheet flow to Ocean Front Street
B.1	0.55	5.00	4.40	0.08	0.18	B.1	Bluff to Pacific Ocean
Sum =					0.42		

Weighted Runoff Coefficients					
Pre-Construction Conditions					
Drainage Basin ID (Type)	Impervious Area (SF)	Pervious Area (SF)	Basin Area (SF)	Total (AC)	C-Value
X.1	2,712	306	3,018	0.07	0.55
Y.1	857	3,603	4,460	0.10	0.55
Total	3,569	3,909	7,478	0.17	

Post-Construction Conditions					
Drainage Basin ID (Type)	Impervious Area (SF)	Pervious Area (SF)	Basin Area (SF)	Total (AC)	C-Value
A.1	1,972	1,562	3,534	0.08	0.55
A.2	336	294	630	0.01	0.55
B.1	0	3,305	3,314	0.08	0.55
Total	2,308	5,161	7,478	0.17	

Table A-1. Runoff Coefficients for Rational Method

Land Use	Runoff Coefficient (C)
	Soil Type ⁽¹⁾
Residential:	
Single Family	0.55
Multi-Units	0.70
Mobile Homes	0.65
Rural (lots greater than 1/2 acre)	0.45
Commercial ⁽²⁾	
80% Impervious	0.85
Industrial ⁽²⁾	
90% Impervious	0.95

Note:

⁽¹⁾ Type D soil to be used for all areas.

⁽²⁾ Where actual conditions deviate significantly from the tabulated imperviousness values of 80% or 90%, the values given for coefficient C, may be revised by multiplying 80% or 90% by the ratio of actual imperviousness to the tabulated imperviousness. However, in case shall the final coefficient be less than 0.50. For example: Consider commercial property on D soil.

Actual imperviousness	=	50%
Tabulated imperviousness	=	80%
Revised C	=	(50/80) x 0.85 = 0.53

Pipe Flow

Radius	Arc Length	
0.125	0.545415391	
Theta (Deg)	Theta (Rad)	Perimeter
250	4.36332313	0.075960105

2-3" PVC Discharging to Ocean Front St. (A.1) - 3" Closed Conduit - PVC @ 1%

	Manning N
Diameter (inches)	1. Closed Conduit - PVC
3	0.011
Slope (FT/FT)	Q (CFS)
0.0100	0.100653814030
Depth (feet)	Depth/Diameter
0.19670	78.6788%
Velocity (ft/sec)	Velocity Head (feet)
2.429502155	0.091653427
Area	
0.041429811	

Appendix C –Reference Tables & Figures (City of San Diego Drainage Manual 2017)

APPENDIX A: RATIONAL METHOD AND MODIFIED RATIONAL METHOD

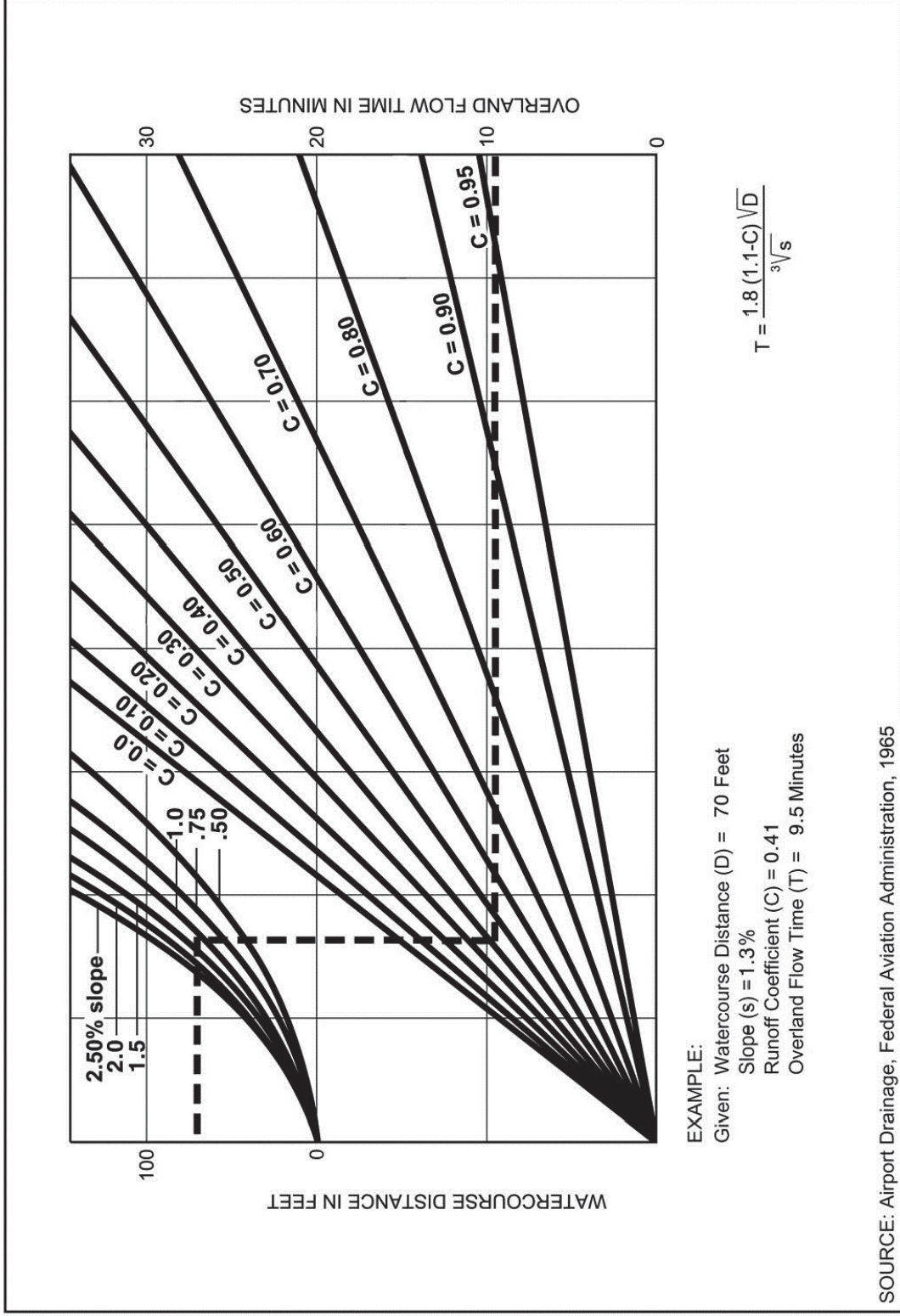


Figure A-4. Rational Formula - Overland Time of Flow Nomograph

Note: Use formula for watercourse distances in excess of 100 feet.



APPENDIX A: RATIONAL METHOD AND MODIFIED RATIONAL METHOD

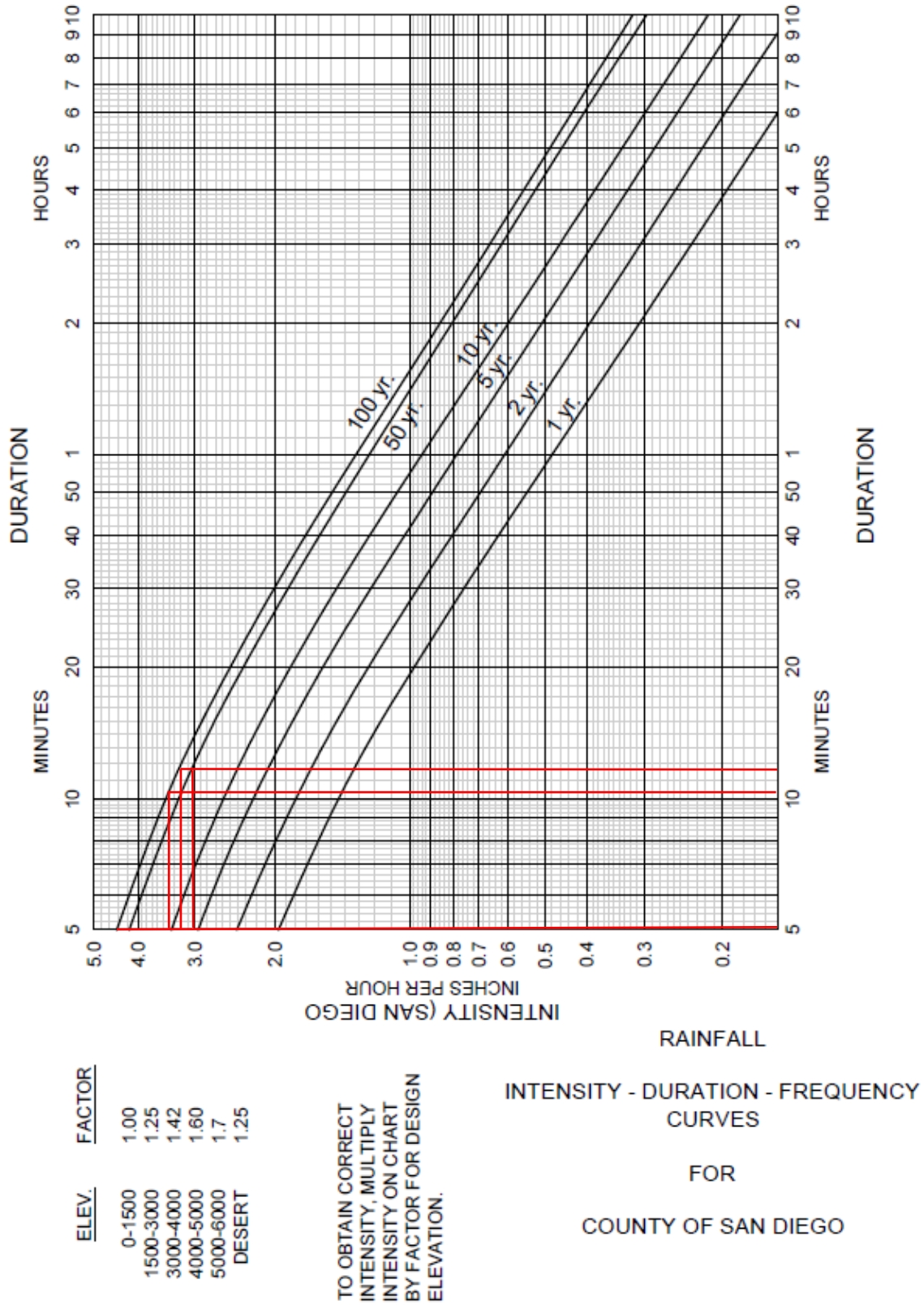


Figure A-1. Intensity-Duration-Frequency Design Chart



APPENDIX A: RATIONAL METHOD AND MODIFIED RATIONAL METHOD

Table A-1. Runoff Coefficients for Rational Method

Land Use	Runoff Coefficient (C)
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90% Impervious	0.95

Note:

⁽¹⁾ Type D soil to be used for all areas.

⁽²⁾ Where actual conditions deviate significantly from the tabulated imperviousness values of 80% or 90%, the values given for coefficient C, may be revised by multiplying 80% or 90% by the ratio of actual imperviousness to the tabulated imperviousness. However, in case shall the final coefficient be less than 0.50. For example: Consider commercial property on D soil.

$$\begin{array}{lcl}
 \text{Actual imperviousness} & = & 50\% \\
 \text{Tabulated imperviousness} & = & 80\% \\
 \text{Revised C} & = & (50/80) \times 0.85 = 0.53
 \end{array}$$

The values in Table A-1 are typical for urban areas. However, if the basin contains rural or agricultural land use, parks, golf courses, or other types of nonurban land use that are expected to be permanent, the appropriate value should be selected based upon the soil and cover and approved by the City.

A.1.3. Rainfall Intensity

The rainfall intensity (I) is the rainfall in inches per hour (in/hr.) for a duration equal to the T_c for a selected storm frequency. Once a particular storm frequency has been selected for design and a T_c calculated for the drainage area, the rainfall intensity can be determined from the Intensity-Duration-Frequency Design Chart (Figure A-1).

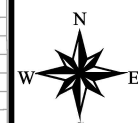
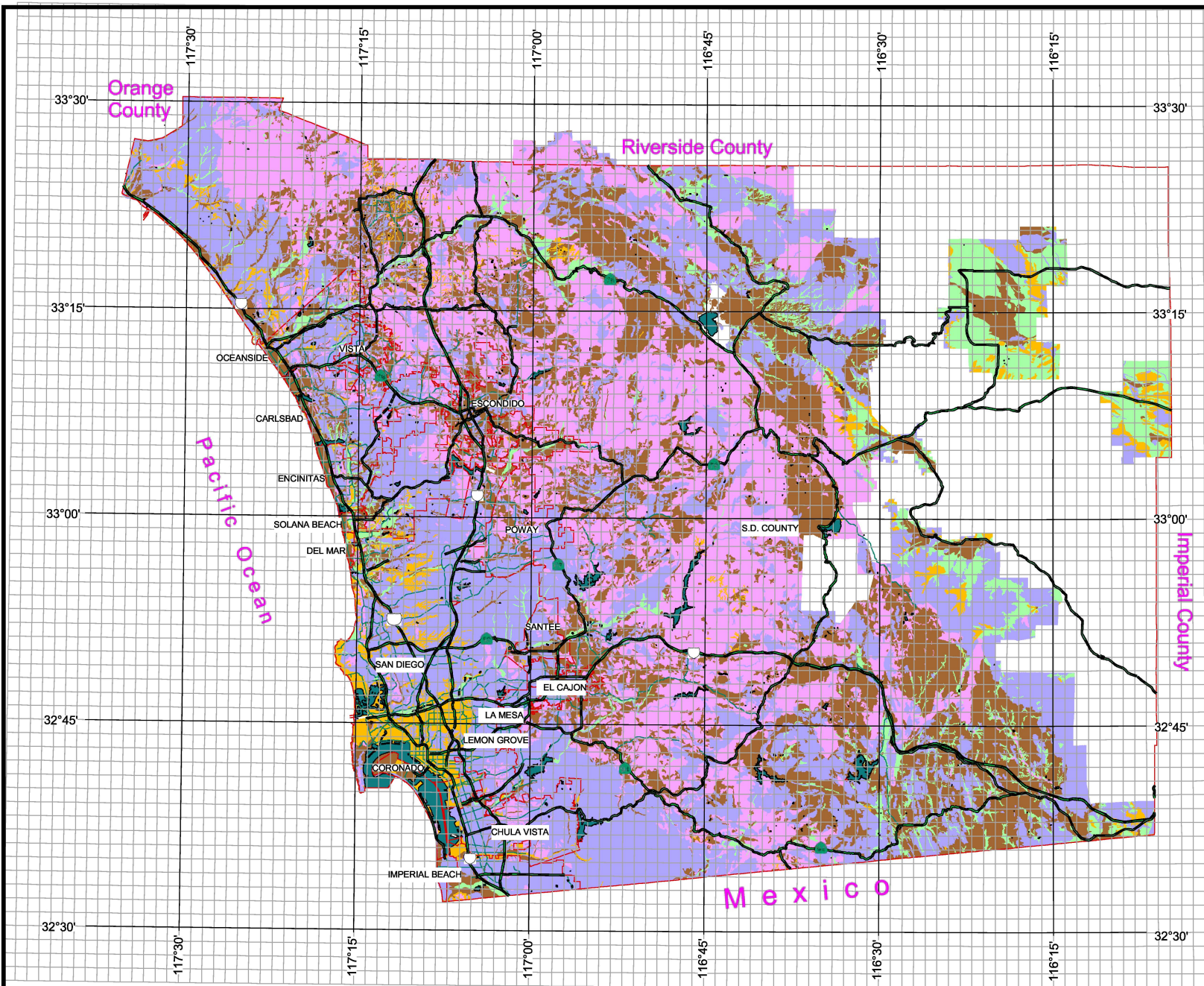
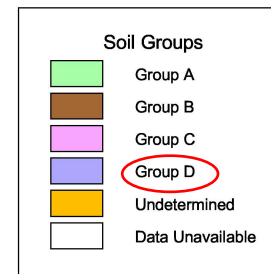


County of San Diego Hydrology Manual



Soil Hydrologic Groups

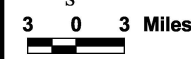
Legend



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Appendix C –Reference Tables & Figures (City of San Diego Drainage Manual 2017)