

December 2018

HYDROLOGY STUDY

“Glenelder”

Vesting Tentative Tract Map No. 82159

16234 Folger Street

City of Hacienda Heights | County of Los Angeles

Prepared For:

LENNAR

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WO#: 3916-28X HydroStdy-GlenelderTT82159

HYDROLOGY REPORT

FOR

**“GLENELDER” -
VESTING TENTATIVE TRACT
MAP NO. 82159
16234 FOLGER STREET**

**CITY OF HACIENDA HEIGHTS
COUNTY OF LOS ANGELES**

Prepared Date: 5/20/2019



PREPARED UNDER THE SUPERVISION OF:

A handwritten signature in black ink, appearing to read "Jianhua Guan".

5/20/2019

Jianhua “Gary” Guan, R.C.E. 64519, Exp. 06/30/19 Date:

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SECTION 1
INTRODUCTION & DISCUSSION

A. INTRODUCTION

The proposed residential developments for Glenelder – Vesting Tentative Tract Map (VTTM) No. 82159 is located at 16234 Folger Street, in the City of Hacienda Heights, County of Los Angeles. The property is located on the south side of Folger Street; east side of Hinnen Avenue; west side of Glenelder and north side of Denley. The general location can be found from the vicinity map.

The site is presently an Elementary School that consists of classroom buildings, surface parking, playground and open spaces areas. The overall property is 10 acres (net) in size and is a generally Trapezoid-shaped parcel. Surrounding land uses include primarily residential land uses on all sides, including Folger Street to the north, Glenelder Avenue to the east, Denley Street to the south and Hinnen Avenue to the west.

LENNAR proposes Vesting Tentative Tract Map No. 82159 for the development of 86 detached condominiums, common open space, parking, private drives, curb, gutter, sidewalk and storm drain improvements, retaining wall, wet and dry utilities and related infrastructure improvements. There will be improvements to Glenelder Avenue, Folger Street and Hinnen Avenue.

The subject property is zoned H9 Residential (0-9 du/ac).

B. DRAINAGE PATTERNS

The overall property is approximately 10 (net) acres in size and the site is currently an Elementary School that consists of classroom buildings, surface parking, playground and open spaces areas.

Stormwater and surface water onsite generally flow from south/southeast to north/northwest (i.e., from the Glenelder Avenue/Shadybend Drive intersection towards Folger Street/Hinnen Avenue intersection). Runoff from the site will be conveyed to an existing storm drain system (Hacienda Heights Project No. 1273 – Line “C”) in Folger Street. The existing storm drain plans can be found in Reference Section 6.

During the proposed condition, the site will be graded to follow the existing condition drainage pattern. The stormwater runoffs will be collected by the proposed storm drain systems and connect to an existing storm drain in Folger Street.

C. STUDY PURPOSE

The purpose of this study is to analyze pre-project and post-project hydrology of the project site to determine the peak flow rates of storm runoff and to compare with the allowable flows as well as to analyze the negative impacts, if any, due to the project developments.

D. HYDROLOGIC INFORMATION

2-year, 5-year, 10-year, 25-year and 50-year storms were analyzed for the project site. The project site encompasses the No.17 soil group. The 50-year 24-hour isohyet is approximately 6.4 inches. The project falls into DPA zone 7. The 85th Percentile, 24-hr Rainfall is approximately 1.1 inches. The reference Los Angeles County Hydrology Map GIS information can be found in Section I.

The area weighted average of 36% of impervious percentage was applied for the existing condition. The school site has 6.2 acres of vacant land with 1% of imperviousness and 3.7 acres with school site with 82% of imperviousness. There are 6.9 acres of off-site single family residential areas with 42% of imperviousness; refer to Land Use Exhibit 1 for details.

The area weighted average of 50% of impervious percentage was applied for the proposed condition. The project area (Vacant and school in the existing condition) has a 55% of imperviousness with Duplexes, Triplexes, etc residential land use. The off-site areas remain the same with the same acreage and 42% of imperviousness.

E. METHODOLOGY

The methodology described in the Los Angeles County Department of Public Works (LACDPW) Hydrology Manual dated January 2006, was used to compute storm run-off from the project site. The LACDPW HydroCalc computer program was used to compute subarea time of concentration (TC), Peak Flow Rates and Runoff Volume. The hydrology calculations are included in Section 2 and Section 3 of this report, for existing (pre-project) and proposed (post-project) conditions.

F. HYDROLOGY CALCULATION RESULTS

The overall studied area including the off-site areas is approximately 16.8 acres in size. The runoffs from the studied area are conveyed to an existing storm drain system (Hacienda Heights Project No. 1273 – Line "C") in Folger Street.

The summary of the hydrology study results and the comparisons between the existing and proposed conditions can be found in the following tables.

As indicated from the summary table, the peak flow rates slightly increase due to the land use changes from school and vacant areas (36% imperviousness) to residential (55% imperviousness). Based on the allowable flow info referenced in Section 5, the allowable flow is about 1.70 cfs/acre for the existing storm drain system. The total allowable flow is about 28.56 cfs with 16.8 acres of the studied area while the total area produces the 25-year storm of 28.29 cfs which is less than the allowable flows.

The existing condition hydrology calculations and map can be found in Section 2 and the proposed condition can be found in Section 3.

Hydrology Summary Table
 Glenelder - VTTM 82159
 City of Hacienda Heights, County Of Los Angeles

Drainage Area	Existing Condition (1)						Proposed Condition (2)						Differences (3)=(2)-(1)					
	Area	2-yr Storm	5-yr Storm	10-yr Storm	25-yr Storm	50--yr Storm	Area	2-yr Storm	5-yr Storm	10-yr Storm	25-yr Storm	50--yr Storm	Area	2-yr Storm	5-yr Storm	10-yr Storm	25-yr Storm	50--yr Storm
	(acre)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(acre)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(acre)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)
Overall	16.8	7.21	14.30	20.18	28.00	33.45	16.8	7.74	15.12	20.58	28.29	33.67	0	0.53	0.82	0.40	0.29	0.22
Total	16.8	7.21	14.30	20.18	28.00	33.45	16.8	7.74	15.12	20.58	28.29	33.67	0	0.53	0.82	0.40	0.29	0.22

G. LID/WATER QUALITY

The project will be required to comply with the newly adopted MS4 Permit. This will require all filtration water quality devices to be sized to 1.5 times the design storm. The design storm is determined using the 0.75 inch storm or the 85th percentile storm, whichever is greater. The 85th Percentile, 24-hr Rainfall is approximately 1.1 inches per Los Angeles County Hydrology Map GIS information. The 0.75 inch storm and the 85th percentile storm calculations can be found in Section 4 by applying the LACDPW HydroCalc computer program and the 85th percentile storm (1.1 inches) was selected as the project design storm.

There are 5 Modular Wetland Systems (MWS) provided for the project. The water quality peak flows are interpolated based on the tributary areas. Along the public roadway frontier, the low flow lines are provided to collect the water quality flows from the project site and send to the proposed MWS for treatments. The water quality flows from the public roadways and off-site areas are not treated by the proposed private Modular Wetland Systems (MWS).

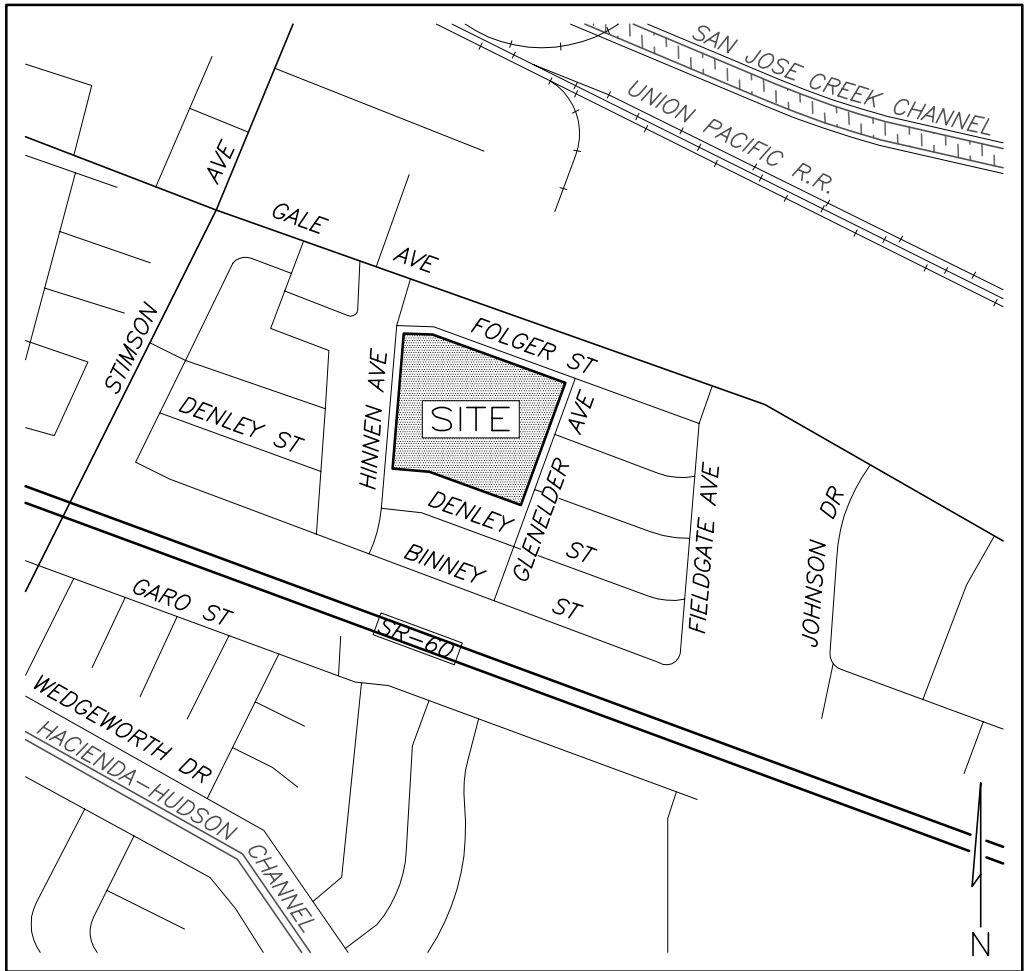
The detailed Low Impact Development (LID) can be found from the separate LID report and preliminary sizing for the MWS systems can be found in Section 4.

F. PRELIMINARY HYDRAULIC UPDATES FOR PROJECT 1273 – LINE “C” , CATCH BASIN AND STREET CAPACITY CALC’S

There are new connections to the existing storm drain systems (Project 1273 – Line “C”) along Folger Street and there are hydraulic impacts to the existing storm drain systems. The preliminary hydraulic analysis were performed by applying the calculated flow rates and conceptual connections. Detailed hydraulic calculations can be found in Section 5. The hydraulic calculation results indicated that the proposed HGLs are still below the existing ground and meet the storm drain design requirements.

The street capacities along Hinnen Avenue and Glenelder Avenue were performed by applying the FlowMaster program due to the additional tributary areas from the project site. The catch basin sizing calculations were also performed for Catch Basin #4 (for Glenelder street flows) and Catch Basin #6 (for Hinnen street flows). The detailed calculations can be found in Section 5. The calculation results indicated that the street and catch basins have enough capacity to convey the proposed flows.

Overall, it is concluded that there will have no adverse impacts to the existing drainage systems due to the project developments.



VICINITY MAP

N.T.S.

34° 07' 30"

AZUSA 1-HI.31

-118° 00' 00"

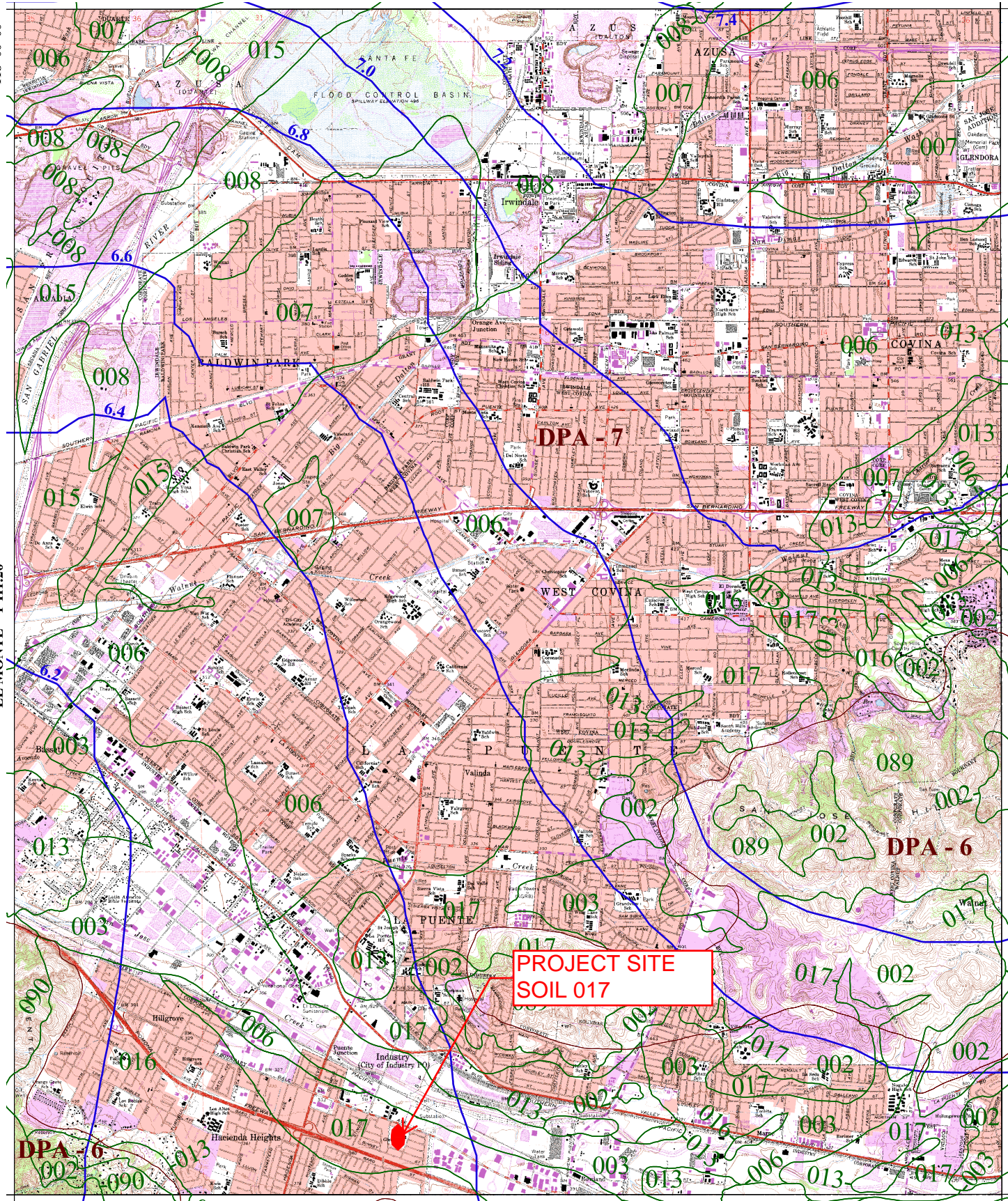
EL MONTE 1-HI.20

SAN DIMAS 1-HI.22

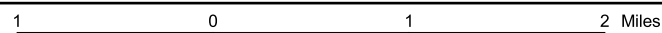
-117° 52' 30"

LA HABRA 1-HI.11

34° 00' 00"



- 016 SOIL CLASSIFICATION AREA
- 7.2 INCHES OF RAINFALL
- DPA - 6** DEBRIS POTENTIAL AREA



25-YEAR 24-HOUR ISOHYET REDUCTION FACTOR: 0.878
 10-YEAR 24-HOUR ISOHYET REDUCTION FACTOR: 0.714

BALDWIN PARK 50-YEAR 24-HOUR ISOHYET

1-HI.21



Hydrology Map

LAYERS

- 50yr Two Tenths (Rainfall)
- DPA Zones
- Soils 2004
- Final 85th Percentile, 24-hr Rainfall
- Final 95th Percentile, 24-hr Rainfall
- 1-year, 1-hour Rainfall Intensity

SEARCH

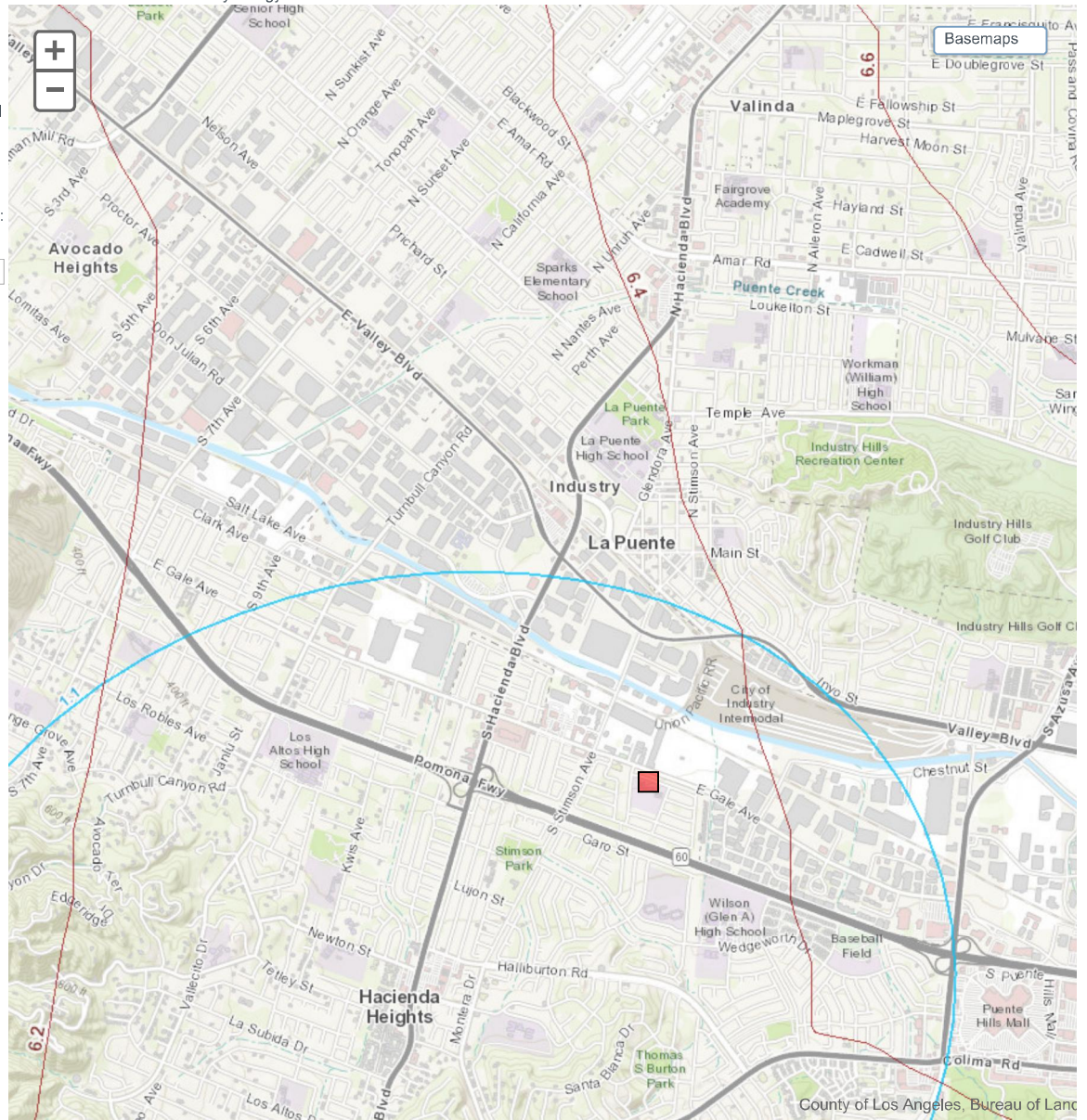
Enter Address, Cross Street, or Parcel No.:
 (ex: 900 S. Fremont Ave., Fremont@Valley,
 5342005904)

16234 Folger St. hacienda heights

Search

Address Search Results:

16234 Folger St hacienda heights



Map Tips
 Hold SHIFT to drag a zoom location box.
 Hold SHIFT + CTRL to drag a zoom out location box.

SECTION 2

EXISTING CONDITION

HYDROLOGY CALCULATIONS AND MAP

Peak Flow Hydrologic Analysis

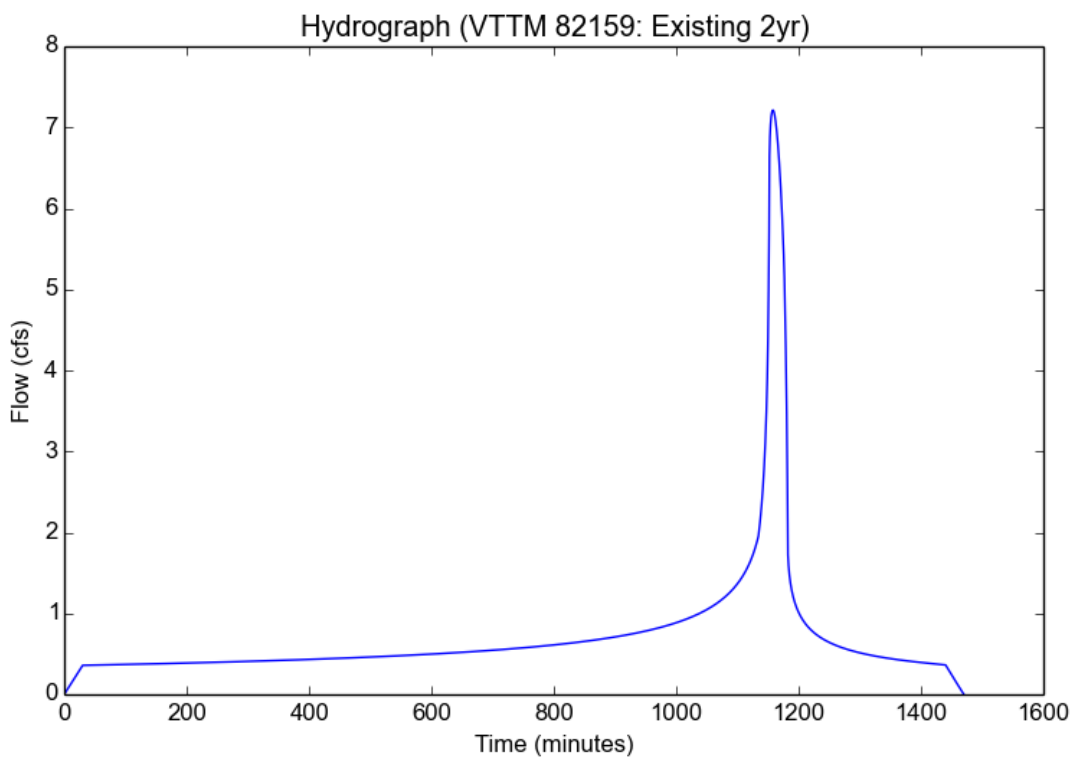
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Version: HydroCalc 1.0.3

Input Parameters

Project Name	VTTM 82159
Subarea ID	Existing 2yr
Area (ac)	16.8
Flow Path Length (ft)	1835.0
Flow Path Slope (vft/hft)	0.013
50-yr Rainfall Depth (in)	6.4
Percent Impervious	0.36
Soil Type	17
Design Storm Frequency	2-yr
Fire Factor	0
LID	False

Output Results

Modeled (2-yr) Rainfall Depth (in)	2.4768
Peak Intensity (in/hr)	0.6366
Undeveloped Runoff Coefficient (Cu)	0.5476
Developed Runoff Coefficient (Cd)	0.6744
Time of Concentration (min)	30.0
Clear Peak Flow Rate (cfs)	7.213
Burned Peak Flow Rate (cfs)	7.213
24-Hr Clear Runoff Volume (ac-ft)	1.4508
24-Hr Clear Runoff Volume (cu-ft)	63198.1452



Peak Flow Hydrologic Analysis

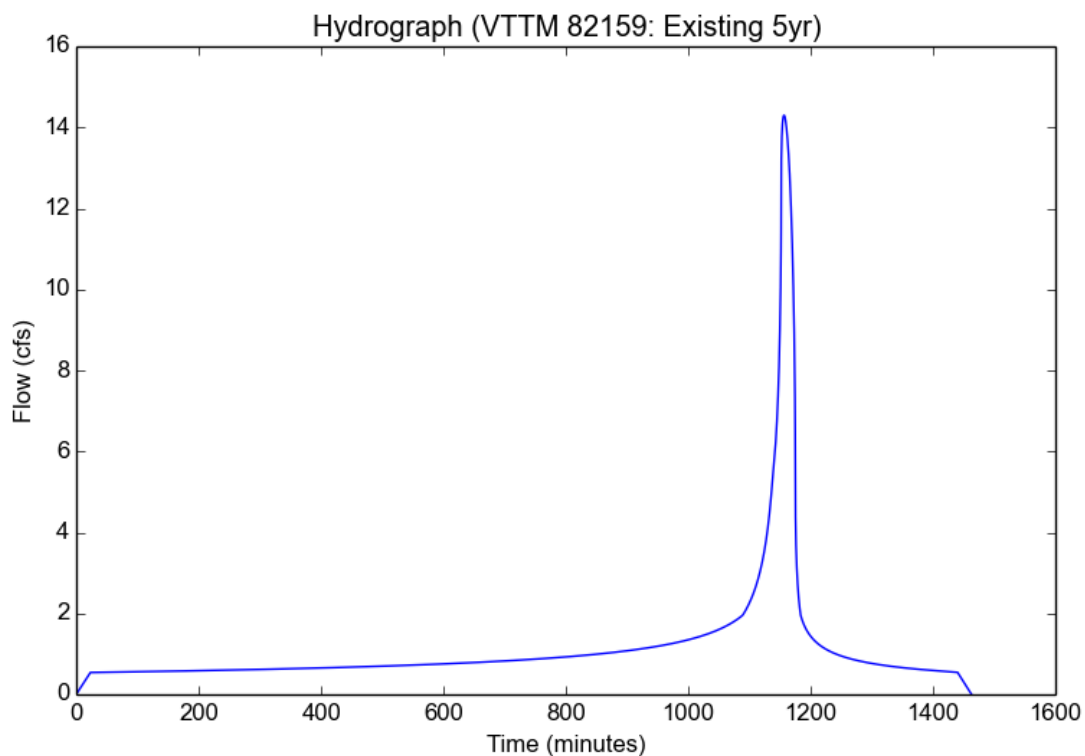
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Version: HydroCalc 1.0.3

Input Parameters

Project Name	VTTM 82159
Subarea ID	Existing 5yr
Area (ac)	16.8
Flow Path Length (ft)	1835.0
Flow Path Slope (vft/hft)	0.013
50-yr Rainfall Depth (in)	6.4
Percent Impervious	0.36
Soil Type	17
Design Storm Frequency	5-yr
Fire Factor	0
LID	False

Output Results

Modeled (5-yr) Rainfall Depth (in)	3.7376
Peak Intensity (in/hr)	1.0884
Undeveloped Runoff Coefficient (Cu)	0.7157
Developed Runoff Coefficient (Cd)	0.782
Time of Concentration (min)	23.0
Clear Peak Flow Rate (cfs)	14.2999
Burned Peak Flow Rate (cfs)	14.2999
24-Hr Clear Runoff Volume (ac-ft)	2.3156
24-Hr Clear Runoff Volume (cu-ft)	100867.2544



Peak Flow Hydrologic Analysis

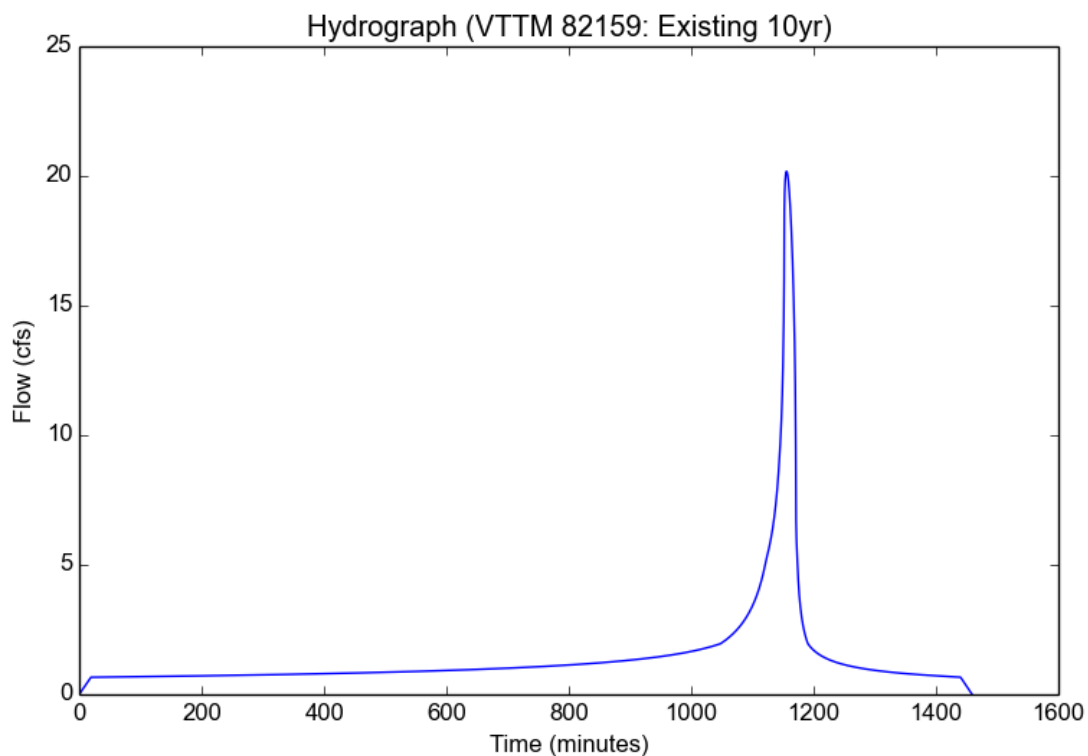
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Version: HydroCalc 1.0.3

Input Parameters

Project Name	VTTM 82159
Subarea ID	Existing 10yr
Area (ac)	16.8
Flow Path Length (ft)	1835.0
Flow Path Slope (vft/hft)	0.013
50-yr Rainfall Depth (in)	6.4
Percent Impervious	0.36
Soil Type	17
Design Storm Frequency	10-yr
Fire Factor	0
LID	False

Output Results

Modeled (10-yr) Rainfall Depth (in)	4.5696
Peak Intensity (in/hr)	1.4557
Undeveloped Runoff Coefficient (Cu)	0.7831
Developed Runoff Coefficient (Cd)	0.8252
Time of Concentration (min)	19.0
Clear Peak Flow Rate (cfs)	20.1804
Burned Peak Flow Rate (cfs)	20.1804
24-Hr Clear Runoff Volume (ac-ft)	2.9287
24-Hr Clear Runoff Volume (cu-ft)	127572.4969



Peak Flow Hydrologic Analysis

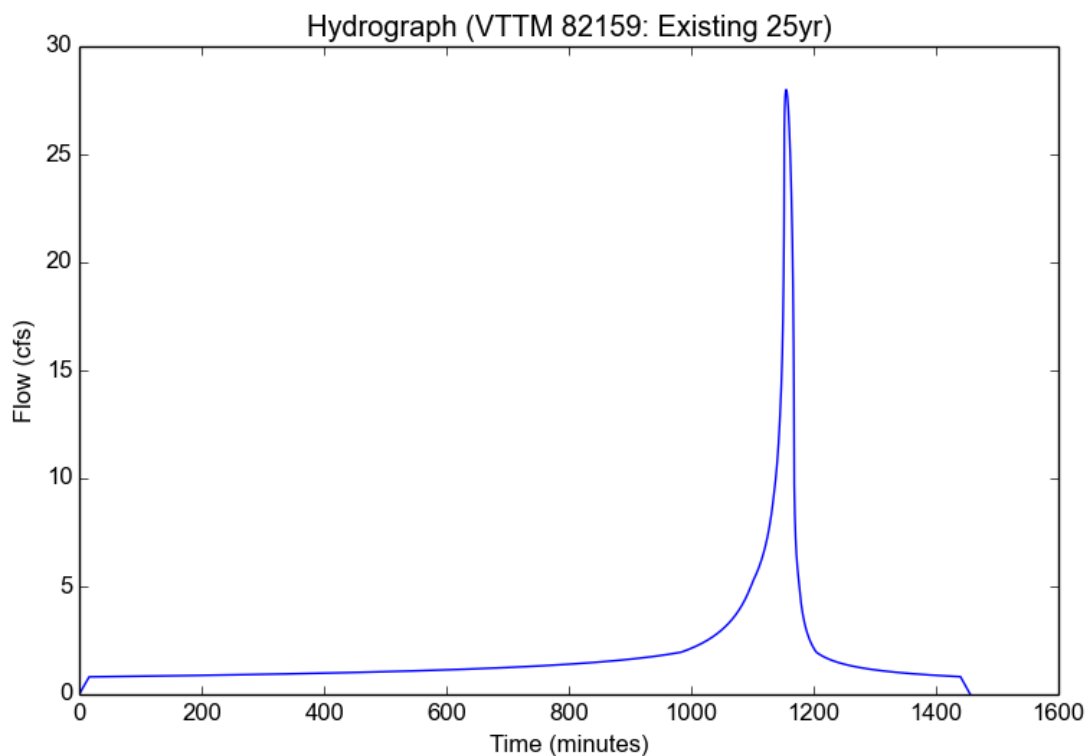
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Version: HydroCalc 1.0.3

Input Parameters

Project Name	VTTM 82159
Subarea ID	Existing 25yr
Area (ac)	16.8
Flow Path Length (ft)	1835.0
Flow Path Slope (vft/hft)	0.013
50-yr Rainfall Depth (in)	6.4
Percent Impervious	0.36
Soil Type	17
Design Storm Frequency	25-yr
Fire Factor	0
LID	False

Output Results

Modeled (25-yr) Rainfall Depth (in)	5.6192
Peak Intensity (in/hr)	1.9407
Undeveloped Runoff Coefficient (Cu)	0.8355
Developed Runoff Coefficient (Cd)	0.8587
Time of Concentration (min)	16.0
Clear Peak Flow Rate (cfs)	27.9973
Burned Peak Flow Rate (cfs)	27.9973
24-Hr Clear Runoff Volume (ac-ft)	3.7567
24-Hr Clear Runoff Volume (cu-ft)	163642.5477



Peak Flow Hydrologic Analysis

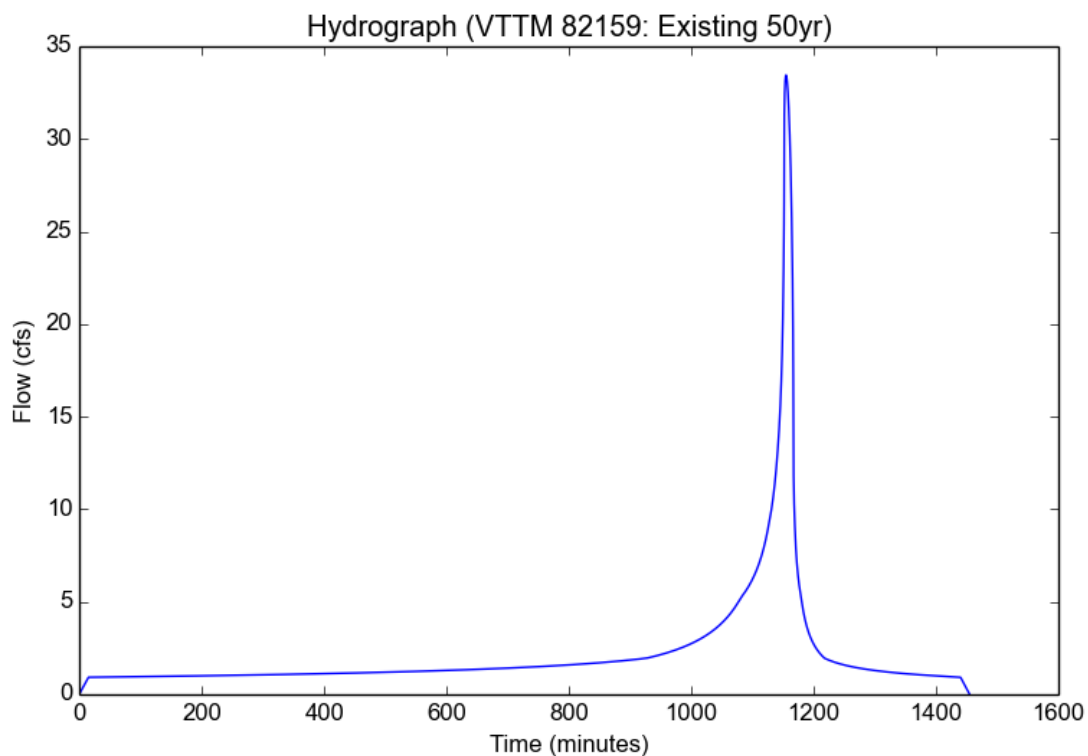
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Version: HydroCalc 1.0.3

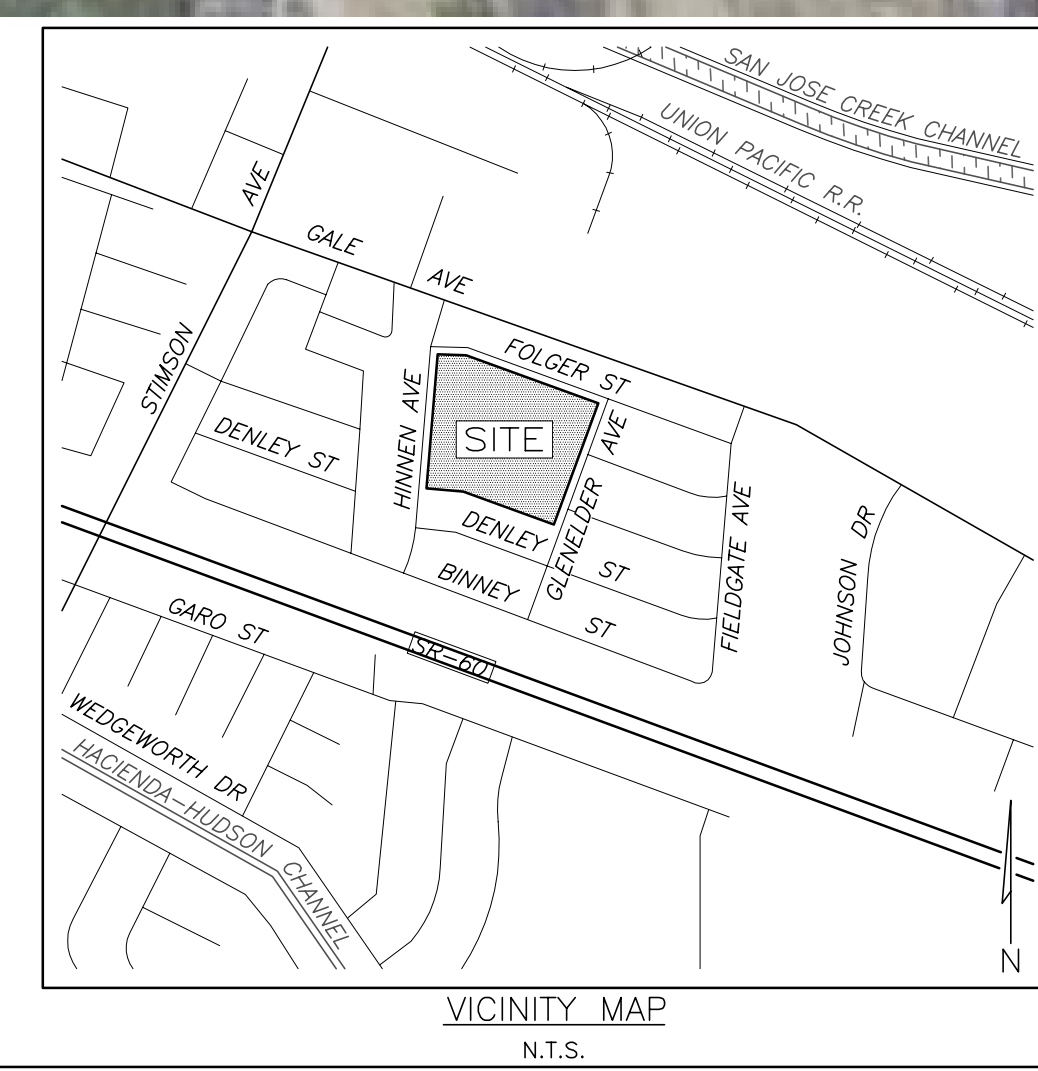
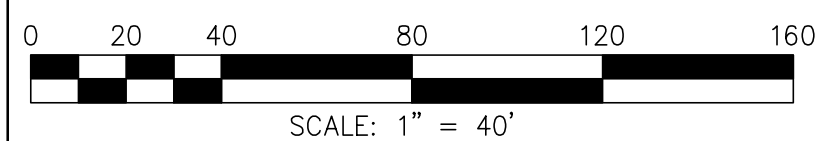
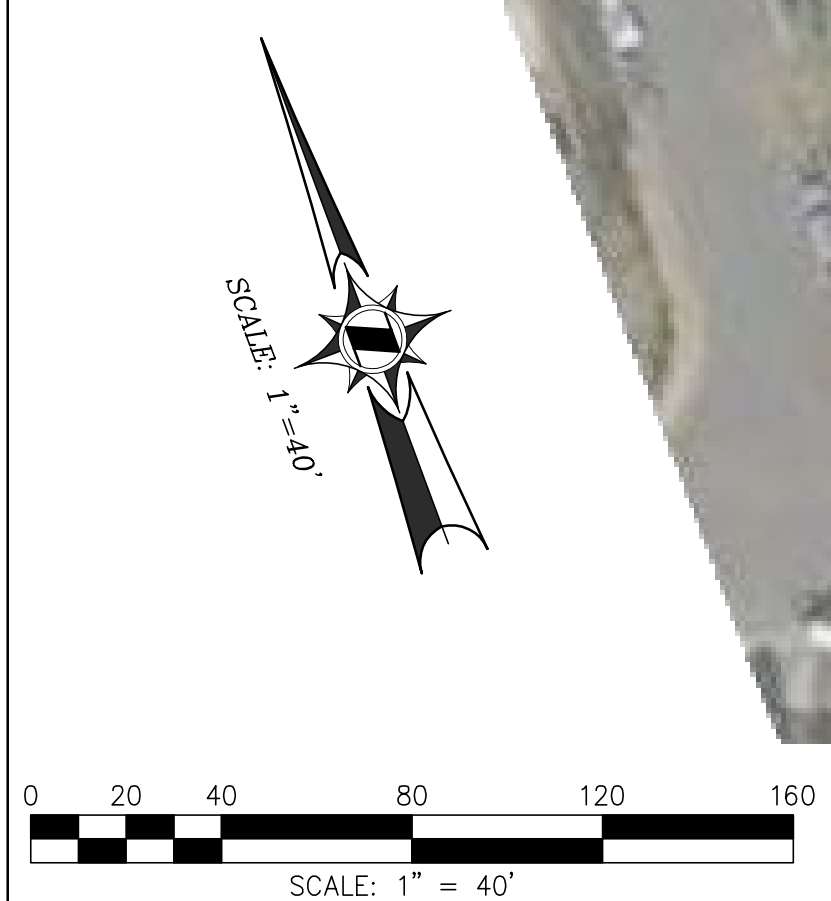
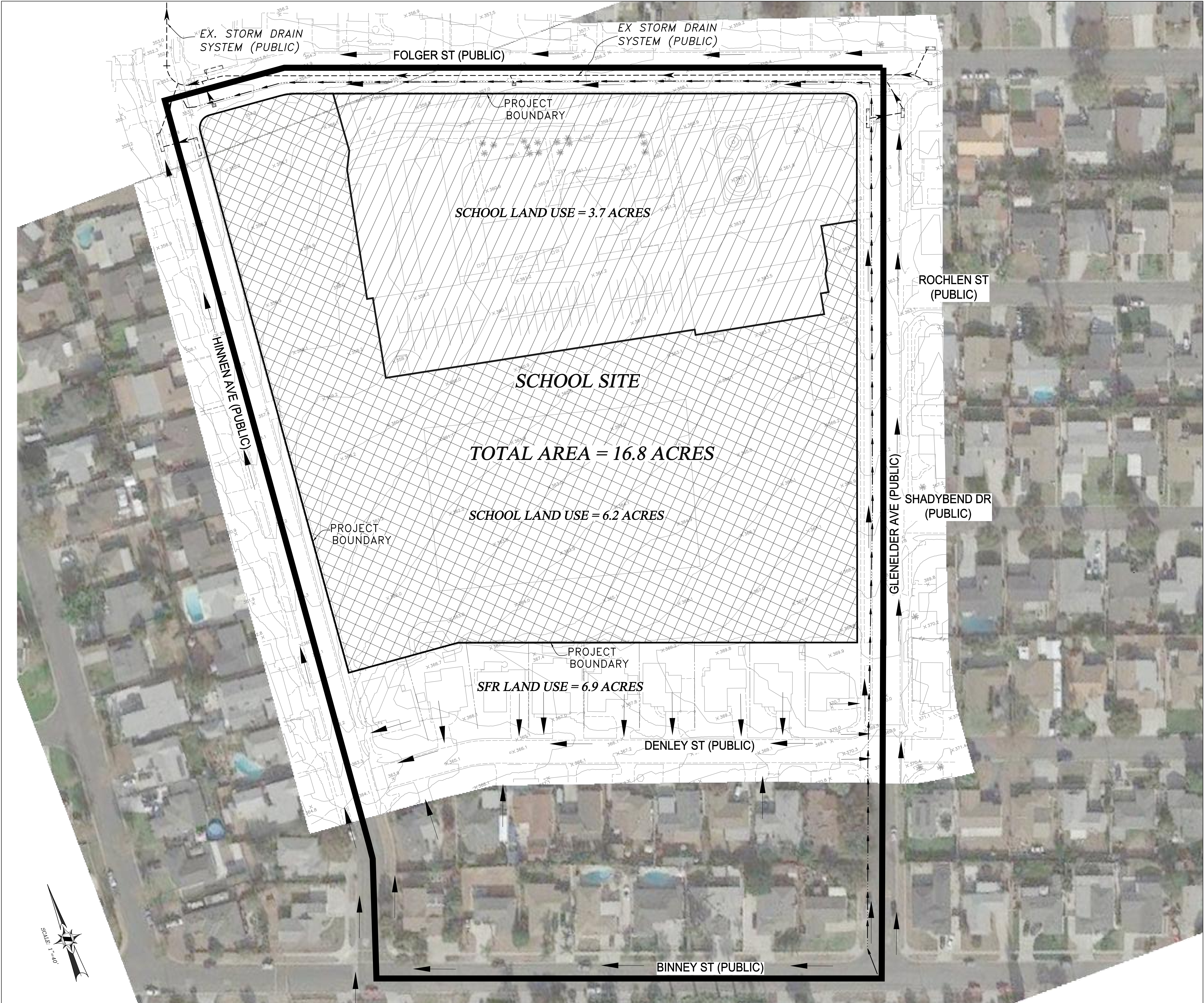
Input Parameters

Project Name	VTTM 82159
Subarea ID	Existing 50yr
Area (ac)	16.8
Flow Path Length (ft)	1835.0
Flow Path Slope (vft/hft)	0.013
50-yr Rainfall Depth (in)	6.4
Percent Impervious	0.36
Soil Type	17
Design Storm Frequency	50-yr
Fire Factor	0
LID	False




Output Results

Modeled (50-yr) Rainfall Depth (in)	6.4
Peak Intensity (in/hr)	2.2784
Undeveloped Runoff Coefficient (Cu)	0.8592
Developed Runoff Coefficient (Cd)	0.8739
Time of Concentration (min)	15.0
Clear Peak Flow Rate (cfs)	33.4511
Burned Peak Flow Rate (cfs)	33.4511
24-Hr Clear Runoff Volume (ac-ft)	4.412
24-Hr Clear Runoff Volume (cu-ft)	192188.1044





LEGEND

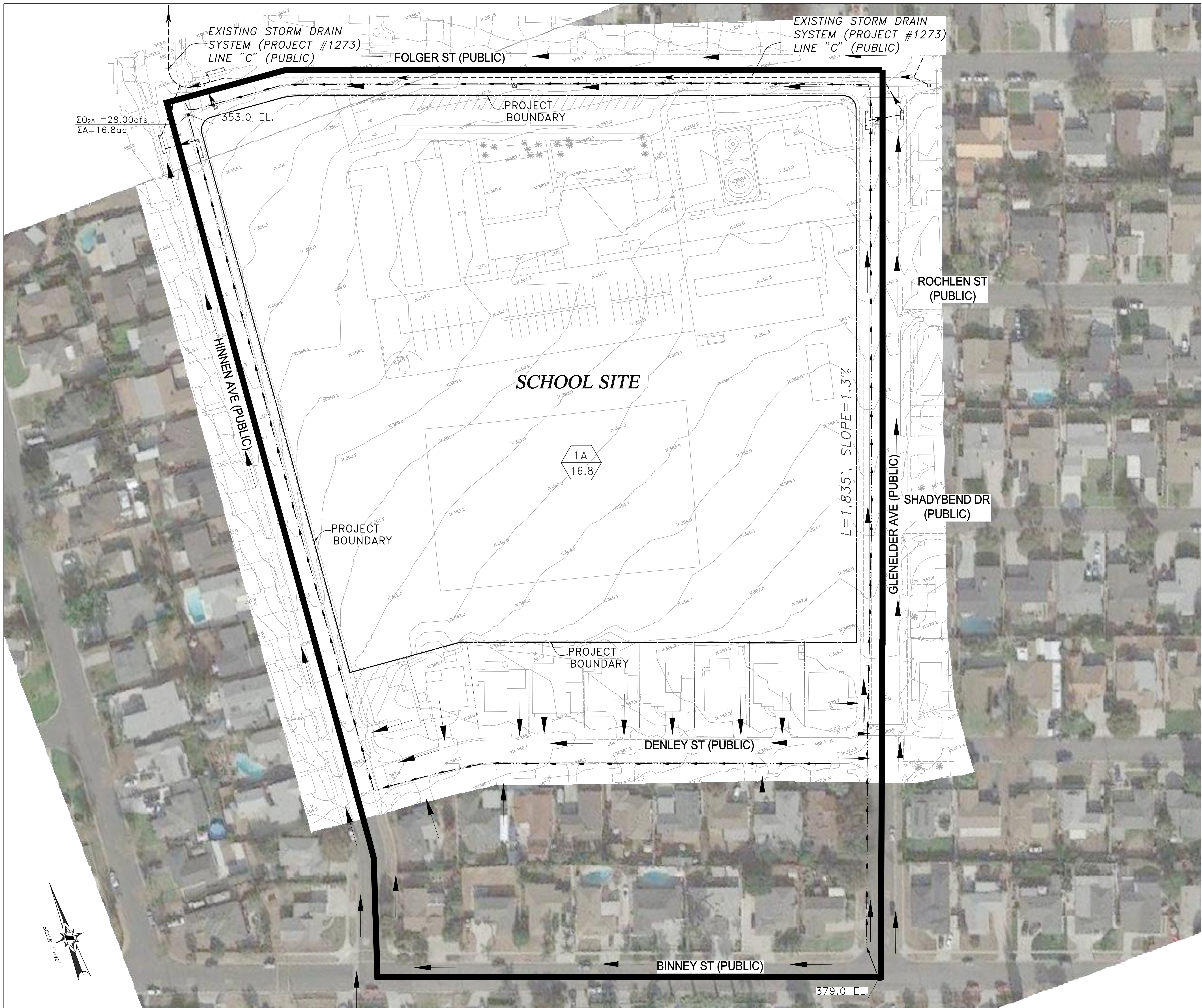
-  DRAINAGE BOUNDARY
-  VACANT (AREA=6.2 ACRES, 1% IMPERVIOUSNESS)
-  ELEMENTARY SCHOOL (AREA=3.7 ACRES, 82% IMPERVIOUSNESS)

THE REST OFF-SITE AREAS WITHIN THE STUDIED BOUNDARY (SFR , AREA=6.9 ACRES, 42% IMPERVIOUSNESS)
 EXISTING CONDITION:
 THE AREA WEIGHTED AVERAGE IMPERVIOUS PERCENTAGE=
 $(6.2 \times 1 + 3.7 \times 82 + 6.9 \times 42) / 16.8 = 36\%$

PROPOSED CONDITION:
 THE PROJECT AREA (VACANT AND ELEMENTARY SCHOOL ABOVE) HAS A 55% IMPERVIOUSNESS WITH DUPLEXES RESIDENTIAL LAND USE
 THE REST OFF-SITE AREAS WITHIN THE STUDIED BOUNDARY (SFR , AREA=6.9 ACRES, 42% IMPERVIOUSNESS)
 THE AREA WEIGHTED AVERAGE IMPERVIOUS PERCENTAGE=
 $(6.2 \times 55 + 3.7 \times 82 + 6.9 \times 42) / 16.8 = 50\%$

THOMAS GUIDE PAGE: 678
 GRID: D2

EXHIBIT 1
LAND USE EXHIBIT
VESTING TENTATIVE TRACT MAP 082159
16234 FOLGER STREET, HACIENDA HEIGHTS, CA 91745



$\Sigma Q_{25} = 28.00\text{cfs}$
 $\Sigma A = 16.8\text{ac}$

EXISTING STORM DRAIN SYSTEM (PROJECT #1273) LINE "C" (PUBLIC)

FOLGER ST (PUBLIC)

EXISTING STORM DRAIN SYSTEM (PROJECT #1273) LINE "C" (PUBLIC)

HINNEN AVE (PUBLIC)

SCHOOL SITE

1A
16.8

L=1,835', SLOPE=1.3%

ROCHLEN ST (PUBLIC)

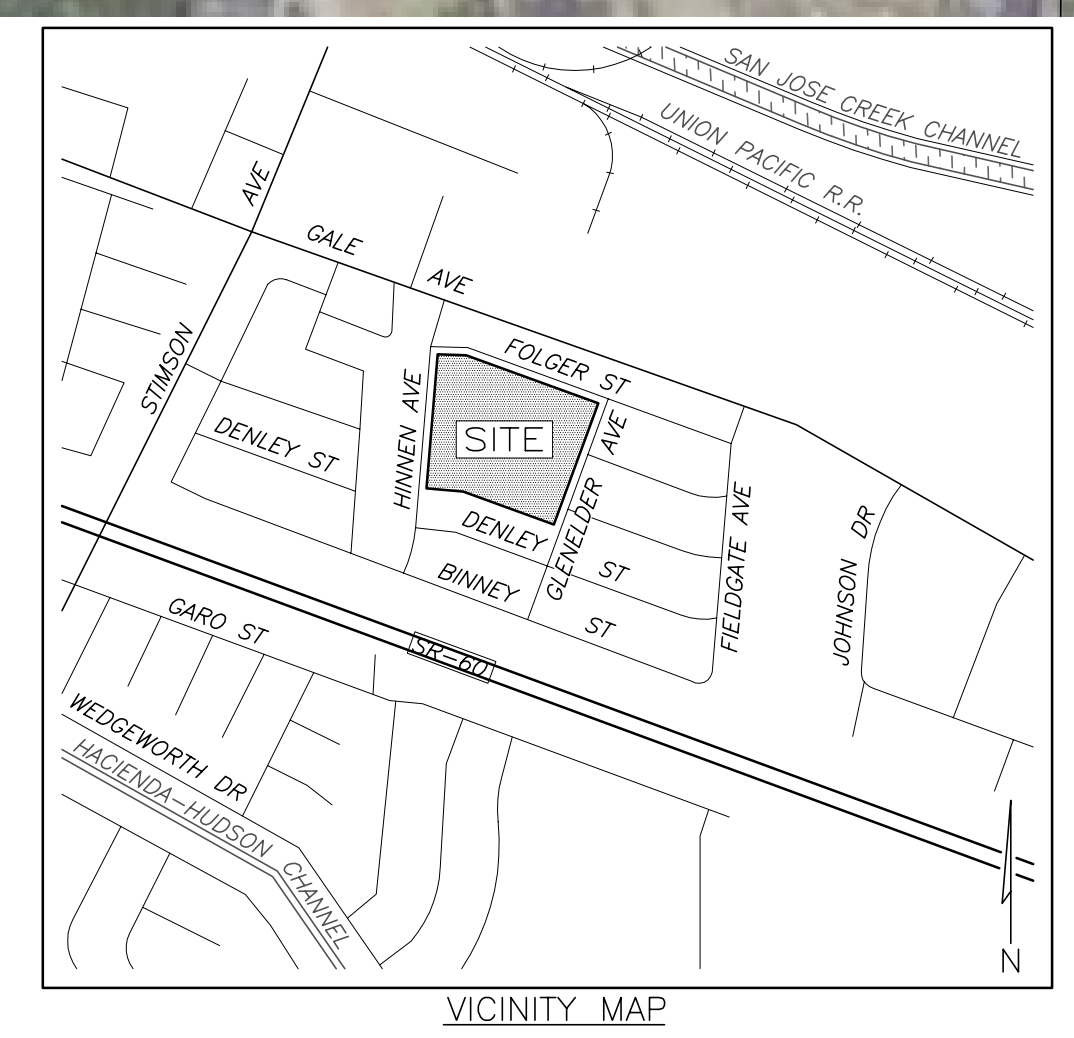
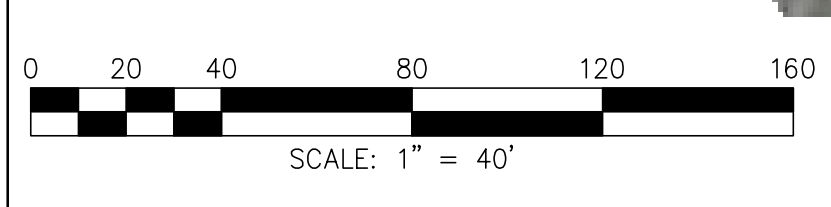
SHADYBEND DR (PUBLIC)

GLENELDER AVE (PUBLIC)

DENLEY ST (PUBLIC)

BINNEY ST (PUBLIC)

379.0 EL.



HYDROLOGIC INFORMATION

- 7 DPA ZONE
- 017 SOIL GROUP
- 6.4" 50-YEAR 24-HOUR ISOHYET
- 36 AREA WEIGHTED AVERAGE % IMPERVIOUSNESS (SEE LAND USE EXHIBIT 1)
- 1 BURN FACTOR
- 1 BULKING FACTOR

LEGEND

- DRAINAGE BOUNDARY
- AREA DESIGNATION FOR AREA "A" AREA ACREAGE (IN ACRES)
- FLOW LINE
- $\Sigma Q_{25} = 30.0\text{cfs}$
 $\Sigma A = 10.0\text{ac}$ SUMMATION 25-YR FLOW RATE TOTAL DRAINAGE AREA IN ACRES

NOTE:

- NOT WITHIN COUNTY ADOPTED FLOODWAY
- NOT WITHIN FEMA FLOOD ZONE A
- THE DRAINAGE FACILITIES TO BE MAINTAINED BY LACDPW UNLESS OTHERWISE NOTED.

THOMAS GUIDE PAGE: 678
 GRID: D2

EXHIBIT 2
EXISTING CONDITION HYDROLOGY MAP
VESTING TENTATIVE TRACT MAP 082159
16234 FOLGER STREET, HACIENDA HEIGHTS, CA 91745

SECTION 3

PROPOSED CONDITION HYDROLOGY CALCULATIONS AND MAP

Peak Flow Hydrologic Analysis

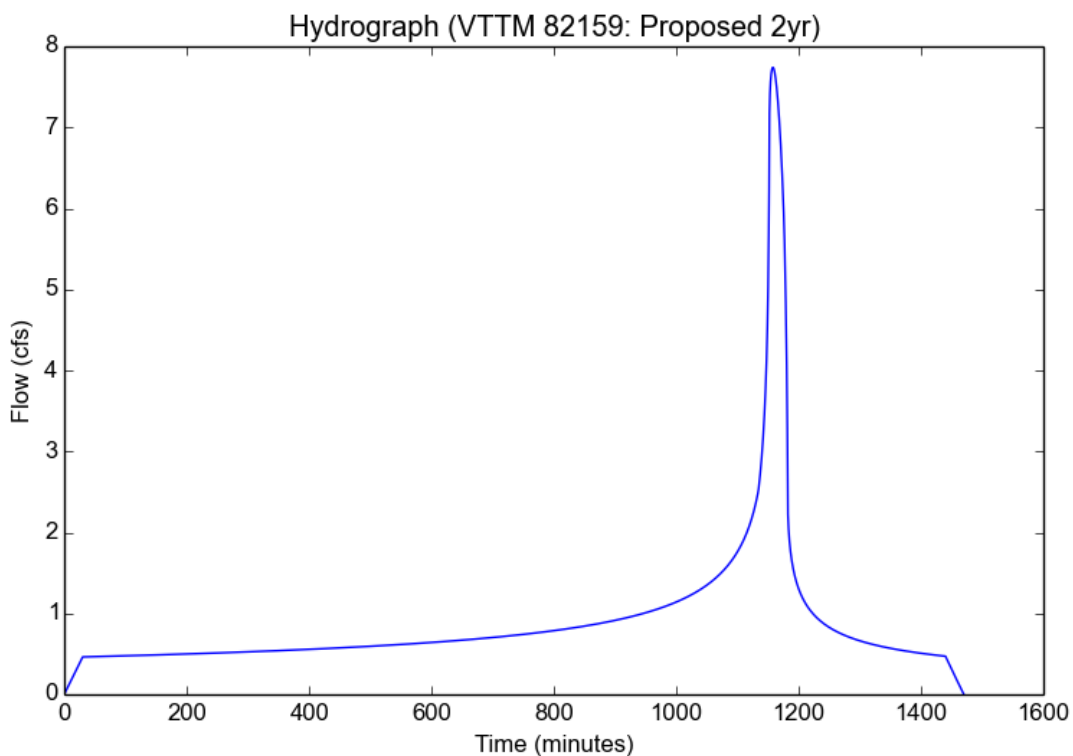
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Version: HydroCalc 1.0.3

Input Parameters

Project Name	VTTM 82159
Subarea ID	Proposed 2yr
Area (ac)	16.8
Flow Path Length (ft)	1835.0
Flow Path Slope (vft/hft)	0.013
50-yr Rainfall Depth (in)	6.4
Percent Impervious	0.5
Soil Type	17
Design Storm Frequency	2-yr
Fire Factor	0
LID	False

Output Results

Modeled (2-yr) Rainfall Depth (in)	2.4768
Peak Intensity (in/hr)	0.6366
Undeveloped Runoff Coefficient (Cu)	0.5476
Developed Runoff Coefficient (Cd)	0.7238
Time of Concentration (min)	30.0
Clear Peak Flow Rate (cfs)	7.7407
Burned Peak Flow Rate (cfs)	7.7407
24-Hr Clear Runoff Volume (ac-ft)	1.8105
24-Hr Clear Runoff Volume (cu-ft)	78865.1472



Peak Flow Hydrologic Analysis

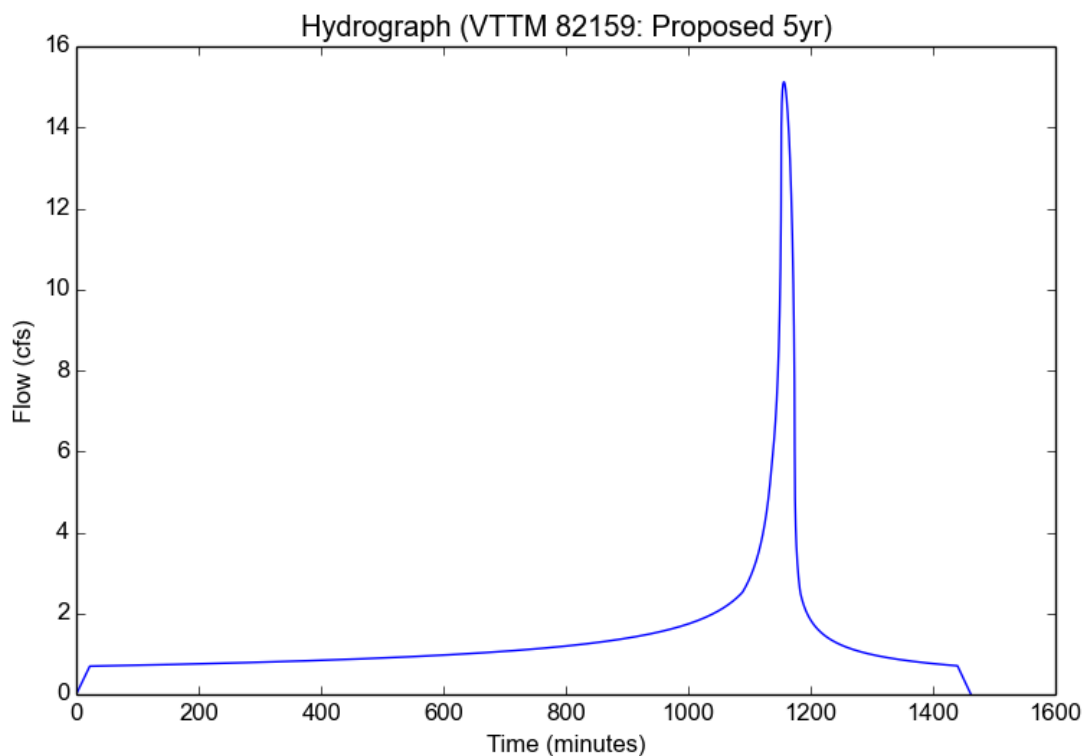
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Version: HydroCalc 1.0.3

Input Parameters

Project Name	VTTM 82159
Subarea ID	Proposed 5yr
Area (ac)	16.8
Flow Path Length (ft)	1835.0
Flow Path Slope (vft/hft)	0.013
50-yr Rainfall Depth (in)	6.4
Percent Impervious	0.5
Soil Type	17
Design Storm Frequency	5-yr
Fire Factor	0
LID	False

Output Results

Modeled (5-yr) Rainfall Depth (in)	3.7376
Peak Intensity (in/hr)	1.1114
Undeveloped Runoff Coefficient (Cu)	0.7199
Developed Runoff Coefficient (Cd)	0.8099
Time of Concentration (min)	22.0
Clear Peak Flow Rate (cfs)	15.123
Burned Peak Flow Rate (cfs)	15.123
24-Hr Clear Runoff Volume (ac-ft)	2.8308
24-Hr Clear Runoff Volume (cu-ft)	123308.3227



Peak Flow Hydrologic Analysis

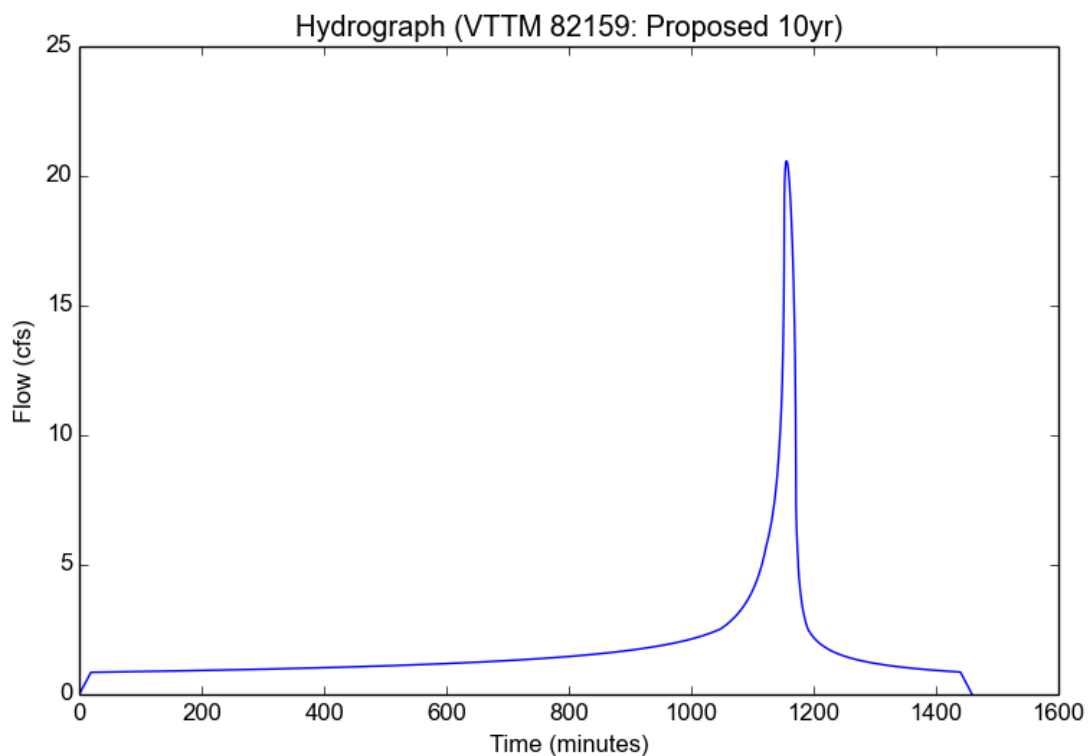
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Version: HydroCalc 1.0.3

Input Parameters

Project Name	VTTM 82159
Subarea ID	Proposed 10yr
Area (ac)	16.8
Flow Path Length (ft)	1835.0
Flow Path Slope (vft/hft)	0.013
50-yr Rainfall Depth (in)	6.4
Percent Impervious	0.5
Soil Type	17
Design Storm Frequency	10-yr
Fire Factor	0
LID	False

Output Results

Modeled (10-yr) Rainfall Depth (in)	4.5696
Peak Intensity (in/hr)	1.4557
Undeveloped Runoff Coefficient (Cu)	0.7831
Developed Runoff Coefficient (Cd)	0.8415
Time of Concentration (min)	19.0
Clear Peak Flow Rate (cfs)	20.5808
Burned Peak Flow Rate (cfs)	20.5808
24-Hr Clear Runoff Volume (ac-ft)	3.5371
24-Hr Clear Runoff Volume (cu-ft)	154076.4892



Peak Flow Hydrologic Analysis

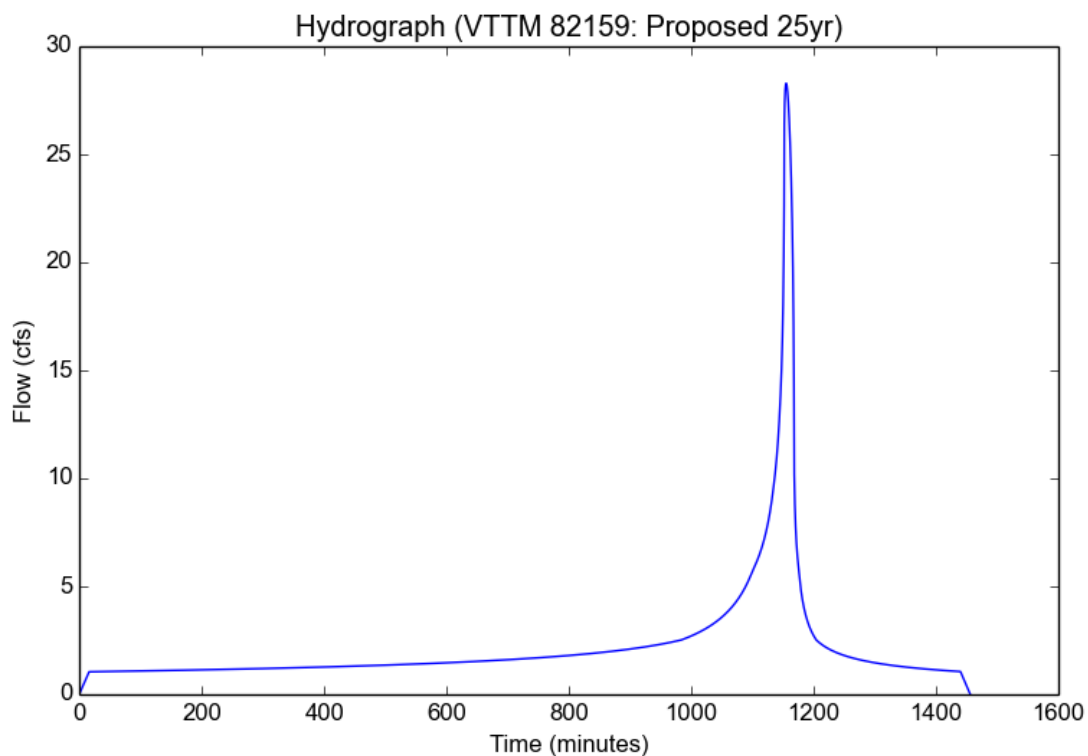
File location: C:/SD/VTTM 082159/Updated/VTTM 82159 - Proposed - 25yr.pdf
Version: HydroCalc 1.0.3

Input Parameters

Project Name	VTTM 82159
Subarea ID	Proposed 25yr
Area (ac)	16.8
Flow Path Length (ft)	1835.0
Flow Path Slope (vft/hft)	0.013
50-yr Rainfall Depth (in)	6.4
Percent Impervious	0.5
Soil Type	17
Design Storm Frequency	25-yr
Fire Factor	0
LID	False

Output Results

Modeled (25-yr) Rainfall Depth (in)	5.6192
Peak Intensity (in/hr)	1.9407
Undeveloped Runoff Coefficient (Cu)	0.8355
Developed Runoff Coefficient (Cd)	0.8677
Time of Concentration (min)	16.0
Clear Peak Flow Rate (cfs)	28.2917
Burned Peak Flow Rate (cfs)	28.2917
24-Hr Clear Runoff Volume (ac-ft)	4.4709
24-Hr Clear Runoff Volume (cu-ft)	194753.7713



Peak Flow Hydrologic Analysis

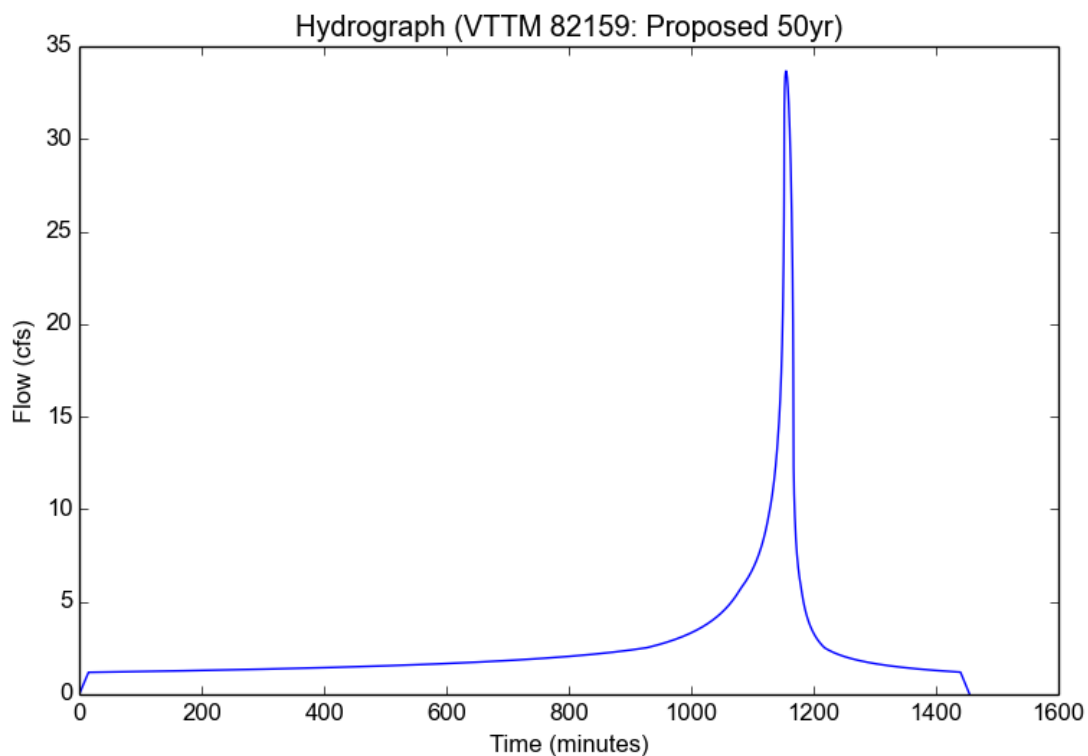
File location: C:/SD/VTTM 082159/Updated/VTTM 82159 - Proposed - 50yr.pdf
Version: HydroCalc 1.0.3

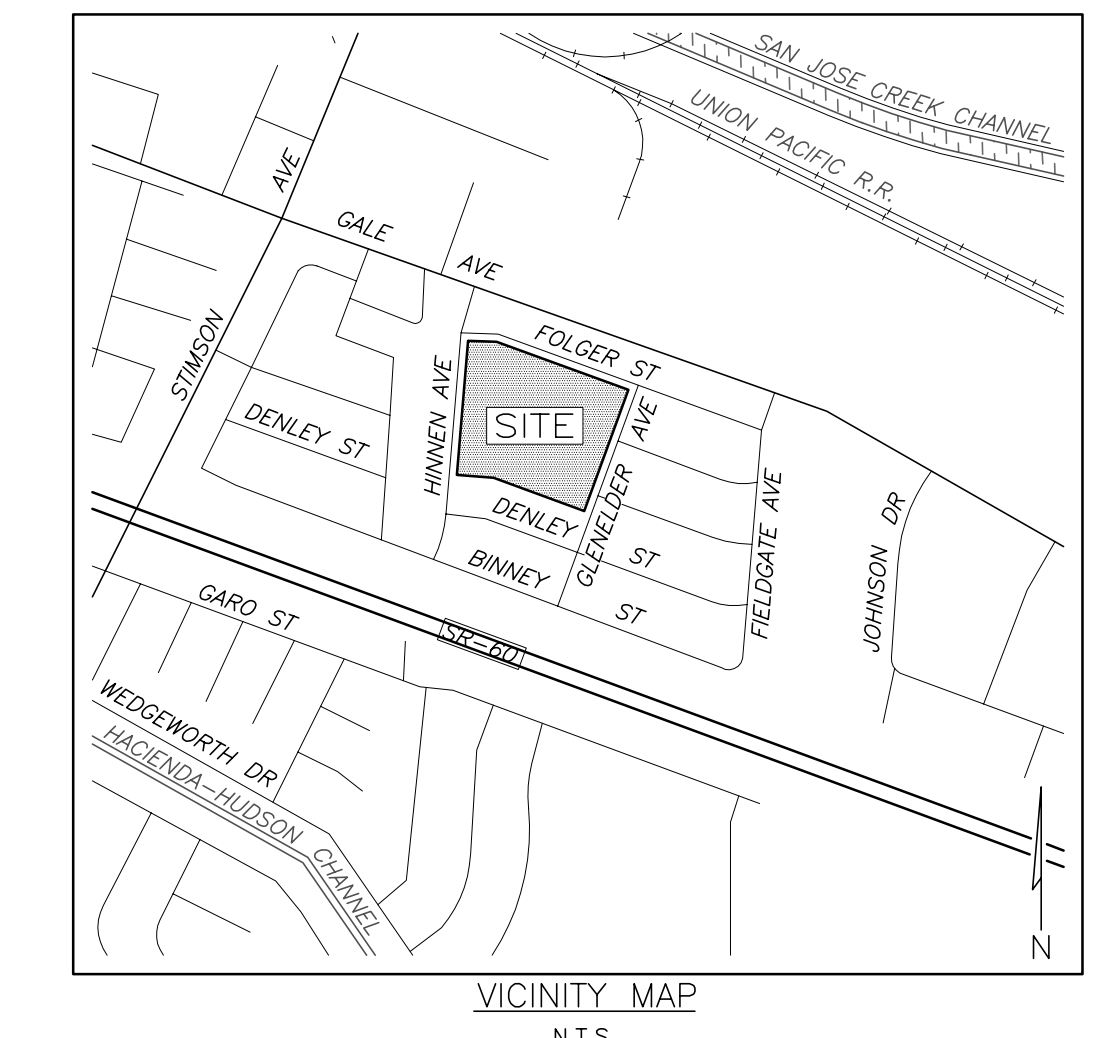
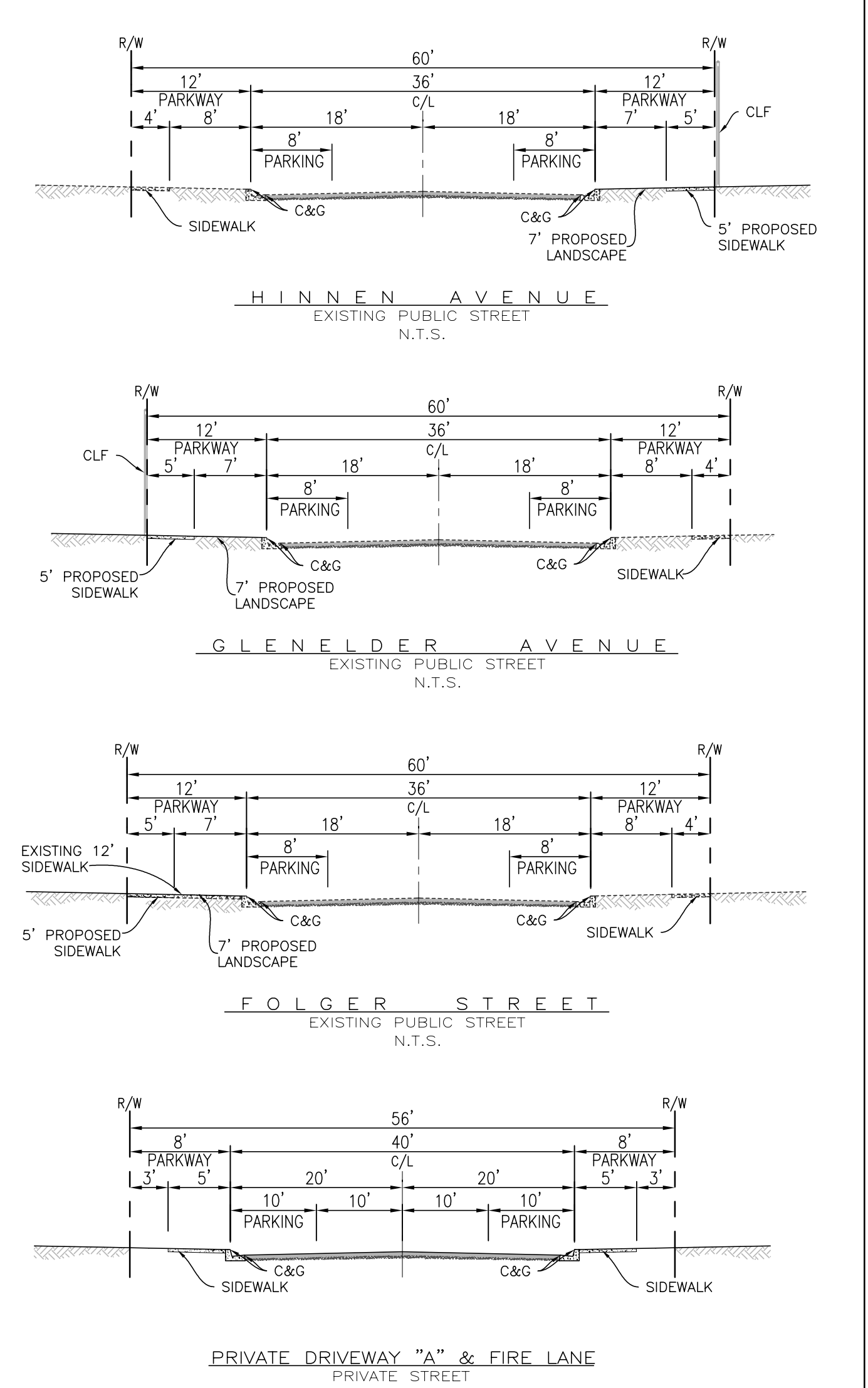
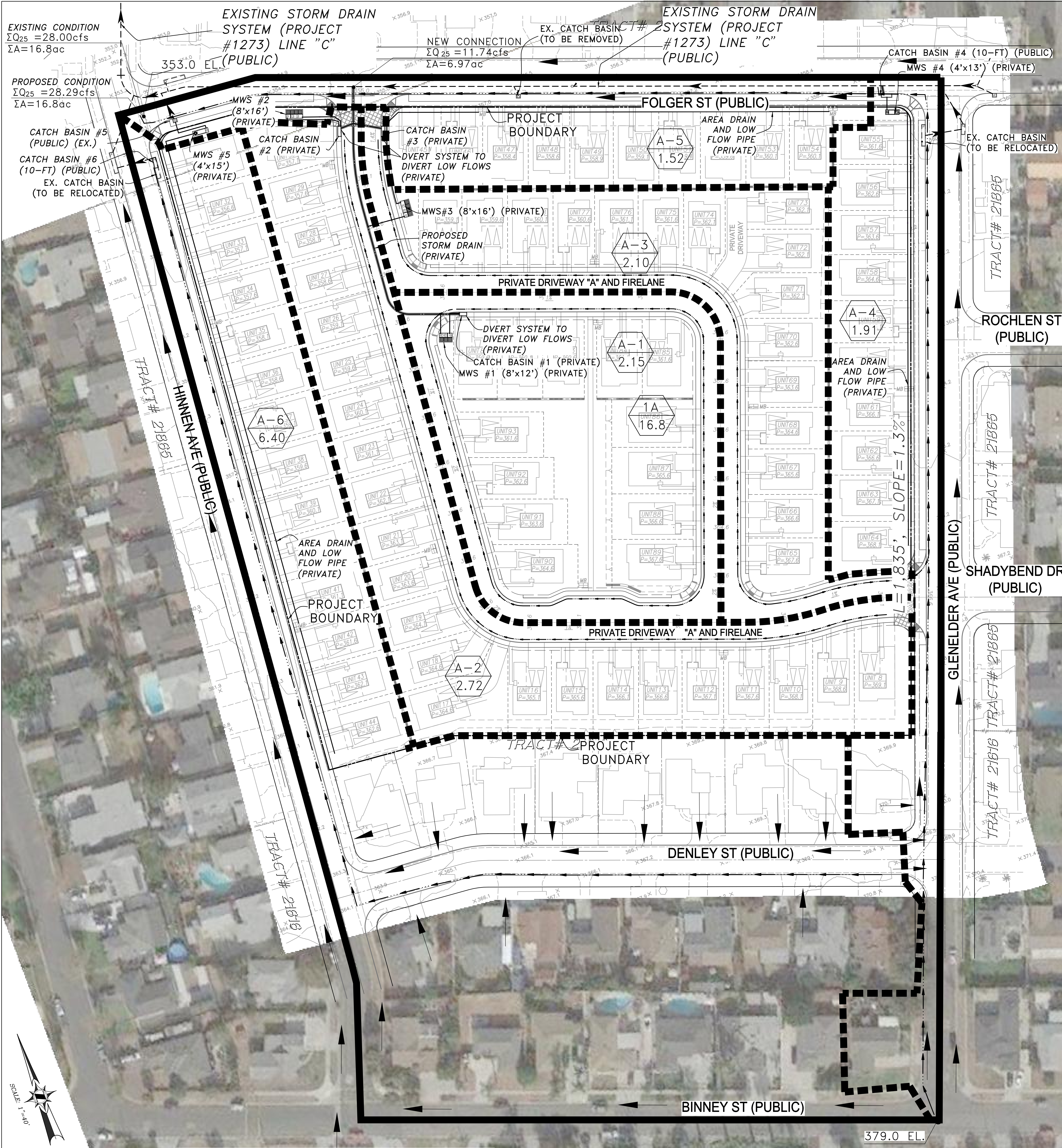
Input Parameters

Project Name	VTTM 82159
Subarea ID	Proposed 50yr
Area (ac)	16.8
Flow Path Length (ft)	1835.0
Flow Path Slope (vft/hft)	0.013
50-yr Rainfall Depth (in)	6.4
Percent Impervious	0.5
Soil Type	17
Design Storm Frequency	50-yr
Fire Factor	0
LID	False

Output Results

Modeled (50-yr) Rainfall Depth (in)	6.4
Peak Intensity (in/hr)	2.2784
Undeveloped Runoff Coefficient (Cu)	0.8592
Developed Runoff Coefficient (Cd)	0.8796
Time of Concentration (min)	15.0
Clear Peak Flow Rate (cfs)	33.6696
Burned Peak Flow Rate (cfs)	33.6696
24-Hr Clear Runoff Volume (ac-ft)	5.1963
24-Hr Clear Runoff Volume (cu-ft)	226351.9731





HYDROLOGIC INFORMATION

7	DPA ZONE
017	SOIL GROUP
6.4"	50-YEAR 24-HOUR ISOHYET
50	AREA WEIGHTED AVERAGE % IMPERVIOUSNESS
1	BURN FACTOR
1	BULKING FACTOR
1.1"	85TH PERCENTILE STORM
PROJECT DESIGN STORM	85TH PERCENTILE STORM
PERCENT OF DESIGN STORM RETAINED ON-SITE	0%

APPORTION OF POST-DEVELOPMENT FLOW RATES PER SUBAREA

SUBAREA	AREA (ACRES)	10-YR FLOW RATE (CFS)	25-YR FLOW RATE (CFS)	50-YR FLOW RATE (CFS)	CATCH BASIN NUMBER
A-1	2.15	2.63	3.62	4.31	#1
A-2	2.72	3.33	4.58	5.45	#2
A-3	2.10	2.57	3.54	4.21	#3
A-4	1.91	2.34	3.22	3.83	#4
A-5	1.52	1.86	2.56	3.05	#5
A-6	6.40	7.84	10.78	12.83	#6
OVERALL	16.80	20.58	28.29	33.67	

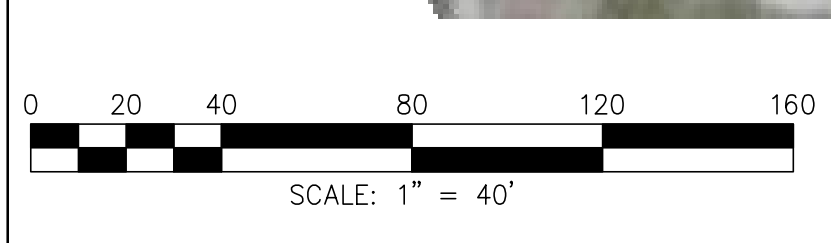
ALLOWABLE FLOW RATES

PER LA COUNTY DEPARTMENT OF PUBLIC WORKS DESIGN DIVISION - HYDRAULIC ANALYSIS UNIT, THE Q ALLOWABLE FOR PROJECT 1273 - LINE "C" IS 1.7 CFS/ACRE.

SUBAREA AREA: 16.8 AC
 SUBAREA Q ALLOWABLE: 28.56 CFS
 UPDATED Q25: 28.29 CFS
 THE PROJECT DESIGN FLOWS ARE LESS THAN THE ALLOWABLE DISCHARGE.

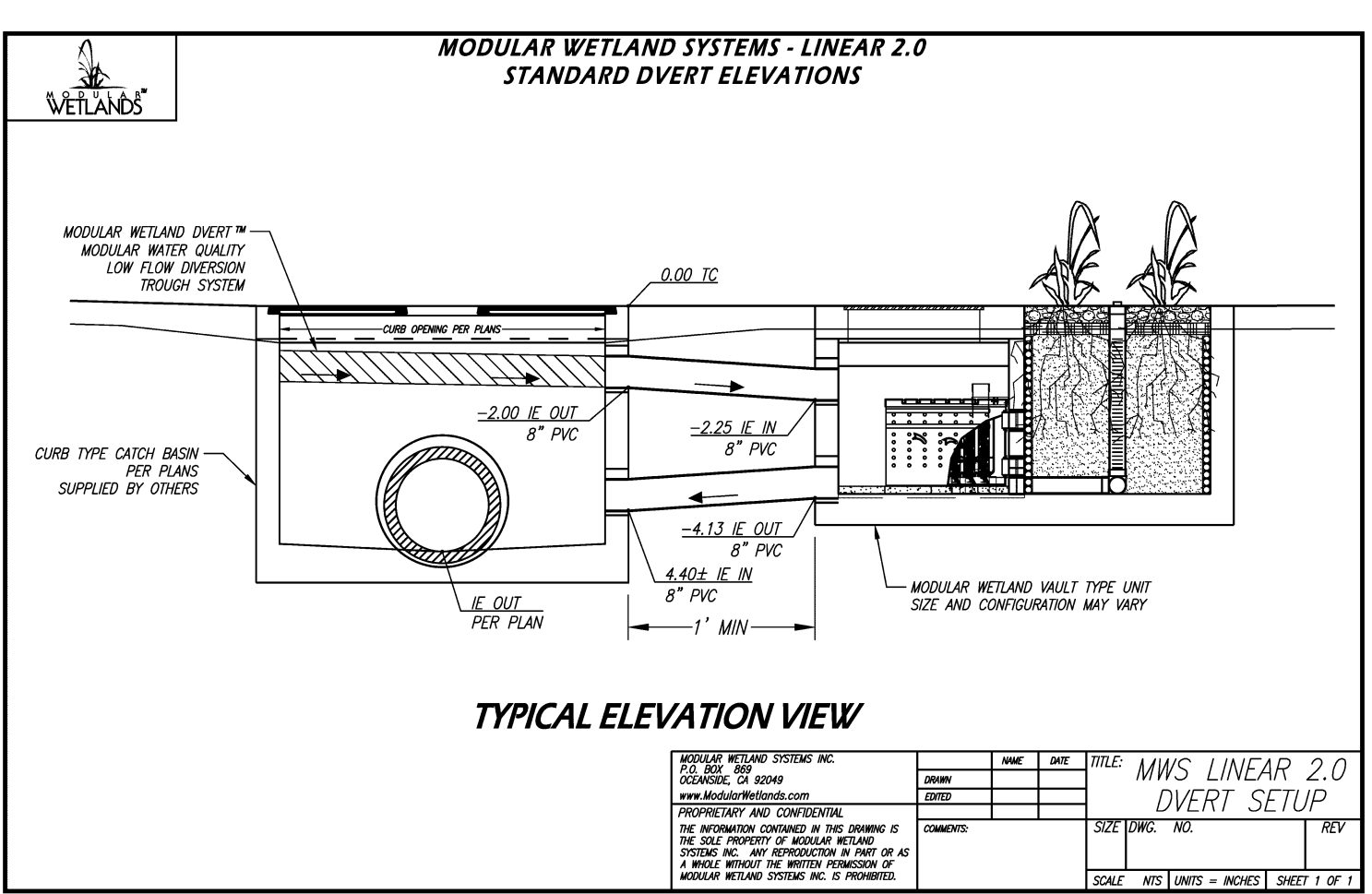
LEGEND

- DRAINAGE BOUNDARY
- - - SUB-AREA DRAINAGE BOUNDARY
- 1A 1.0 AREA DESIGNATION FOR AREA "A" AREA ACREAGE (IN ACRES)
- A-1 1.0 SUB-AREA DESIGNATION AREA ACREAGE (IN ACRES)
- FLOW LINE
- ΣQ₂₅=30.0cfs ΣA=10.0ac SUMMATION 25-YR FLOW RATE TOTAL DRAINAGE AREA IN ACRES



PREPARED FOR:
LENNAR
 15131 ALTON PARKWAY, SUITE 365
 IRVINE, CA 92618
 (949) 349-8000

PREPARED BY:
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 IRVINE, INC.
 PLANNING • ENGINEERING • SURVEYING
 Two Hughes • Irvine, CA 92618 • P.O. BOX 520-1010 • P.O. BOX 520-0759



APPORTION OF POST-DEVELOPMENT WATER QUALITY RATES PER SUBAREA

SUBAREA	AREA (ACRES)	VOLUME (CF)		WQ FLOW RATE (CFS)		MWS UNITS NUMBER	MWS UNITS SIZE	NOTE
		PER CALC'S	1.5 TIMES	PER CALC'S	1.5 TIMES			
A-1	2.15	4,257	6,386	0.20	0.30	#1	MWS-8-12	
A-2	2.72	5,386	8,079	0.25	0.38	#2	MWS-8-16	
A-3	2.95**	5,841	8,079	0.28	0.41	#3	MWS-8-16**	LOW FLOW PIPE PROVIDED FOR PROJECT AREA ONLY, NO PUBLIC ROAD LOW FLOWS
A-4	0.92	1,822	2,733	0.09	0.13	#4	MWS-4-13	
A-5	1.11	2,198	3,297	0.10	0.16	#5	MWS-4-15	
A-6	1.11	2,198	3,297	0.10	0.16	#5	MWS-4-15	
OVERALL	9.85	19,504	29,256	0.92	1.38			

NOTE:
 MWS #3 INCLUDES AREA "A-3" AND PROJECT PORTIONS OF AREA "A-5"

NOTE:

- NOT WITHIN COUNTY ADOPTED FLOODWAY
- NOT WITHIN FEMA FLOOD ZONE A
- THE DRAINAGE FACILITIES TO BE MAINTAINED BY LACDPW UNLESS OTHERWISE NOTED.

SECTION 4

LID AND MWS SIZING CALCUALTIONS

Peak Flow Hydrologic Analysis

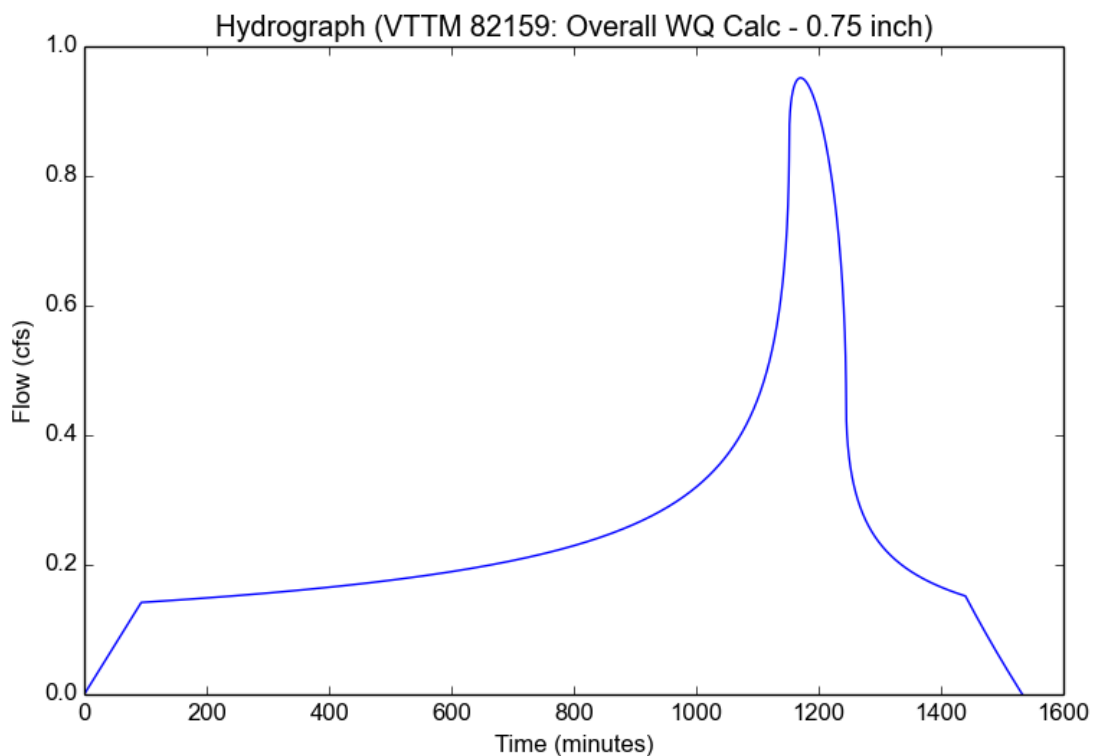
File location: C:/SD/VTTM 082159/Updated/VTTM 82159 - Overall WQ Calc - 0.75 inch.pdf
Version: HydroCalc 1.0.3

Input Parameters

Project Name	VTTM 82159
Subarea ID	Overall WQ Calc - 0.75 inch
Area (ac)	16.8
Flow Path Length (ft)	1835.0
Flow Path Slope (vft/hft)	0.013
0.75-inch Rainfall Depth (in)	0.75
Percent Impervious	0.5
Soil Type	17
Design Storm Frequency	0.75 inch storm
Fire Factor	0
LID	True

Output Results

Modeled (0.75 inch storm) Rainfall Depth (in)	0.75
Peak Intensity (in/hr)	0.1133
Undeveloped Runoff Coefficient (Cu)	0.1
Developed Runoff Coefficient (Cd)	0.5
Time of Concentration (min)	93.0
Clear Peak Flow Rate (cfs)	0.9514
Burned Peak Flow Rate (cfs)	0.9514
24-Hr Clear Runoff Volume (ac-ft)	0.5207
24-Hr Clear Runoff Volume (cu-ft)	22682.6096



Peak Flow Hydrologic Analysis

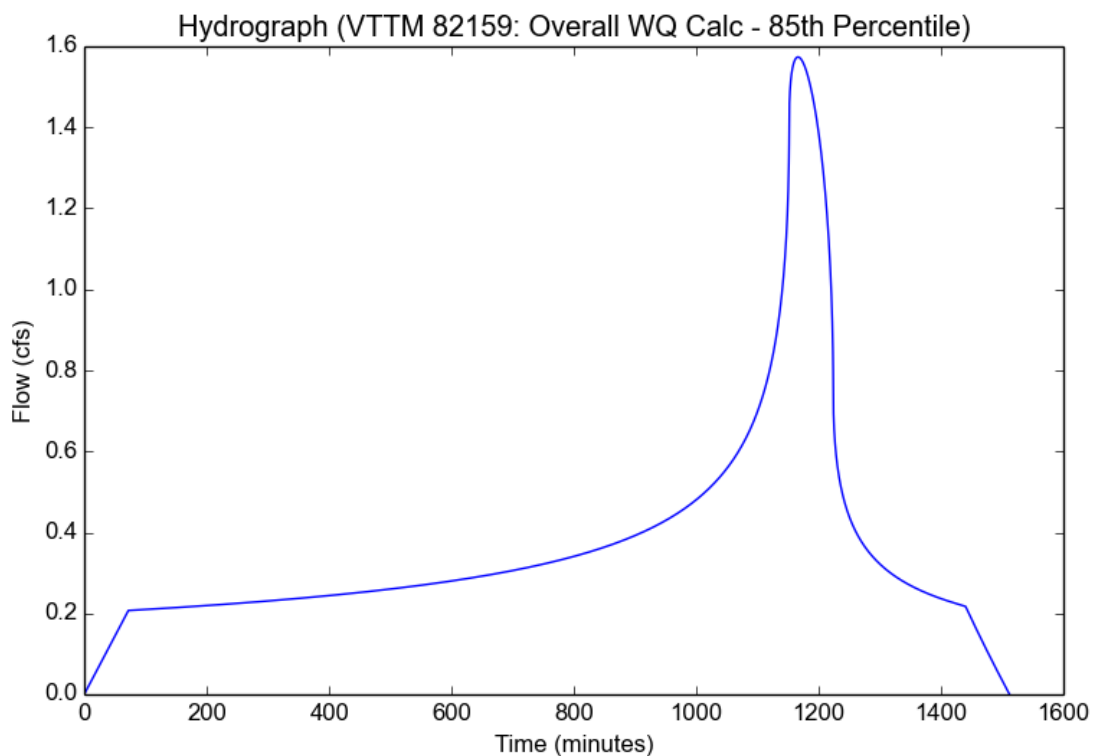
File location: C:/SD/VTTM 082159/Updated/VTTM 82159 - Overall WQ Calc - 85th Percentile.pdf
Version: HydroCalc 1.0.3

Input Parameters

Project Name	VTTM 82159
Subarea ID	Overall WQ Calc - 85th Percentile
Area (ac)	16.8
Flow Path Length (ft)	1835.0
Flow Path Slope (vft/hft)	0.013
85th Percentile Rainfall Depth (in)	1.1
Percent Impervious	0.5
Soil Type	17
Design Storm Frequency	85th percentile storm
Fire Factor	0
LID	True

Output Results

Modeled (85th percentile storm) Rainfall Depth (in)	1.1
Peak Intensity (in/hr)	0.1874
Undeveloped Runoff Coefficient (Cu)	0.1
Developed Runoff Coefficient (Cd)	0.5
Time of Concentration (min)	72.0
Clear Peak Flow Rate (cfs)	1.5738
Burned Peak Flow Rate (cfs)	1.5738
24-Hr Clear Runoff Volume (ac-ft)	0.7637
24-Hr Clear Runoff Volume (cu-ft)	33266.2618





LEGEND

- PROJECT LIMITS
- NAP NOT A PART
- DRAINAGE MANAGEMENT AREA (DMA) LIMITS
- (DMA1 1.81) DMA DESIGNATION AND ACREAGE
- EXISTING STORM DRAIN & FLOW
- PROJECT STORM DRAIN & FLOW
- FLOW DIRECTION (ONSITE)
- FLOW DIRECTION (OFFSITE)
- EXISTING CATCH BASIN
- PROJECT CATCH BASIN W/ BMPS
STORM DRAIN STENCILING (SD-13)
FULL TRASH CAPTURE INLET SCREEN
- PROJECT WALKWAYS, DRIVEWAYS AND PRIVATE DRIVEWAY
- COMMON OPEN SPACE AREAS
- PRIVATE OPEN SPACE AREAS
(FRONT, SIDE, AND REAR WITHIN BOUNDARY AREA OF DWELLING.)
- PROJECT DISCHARGE POINT
- PROPRIETARY BIOFILTRATION
MODULAR WETLAND SYSTEM (OR APPROVED EQUAL)
- AREA DRAIN AND LOW FLOW PIPE
- DWELLING AREA LIMITS

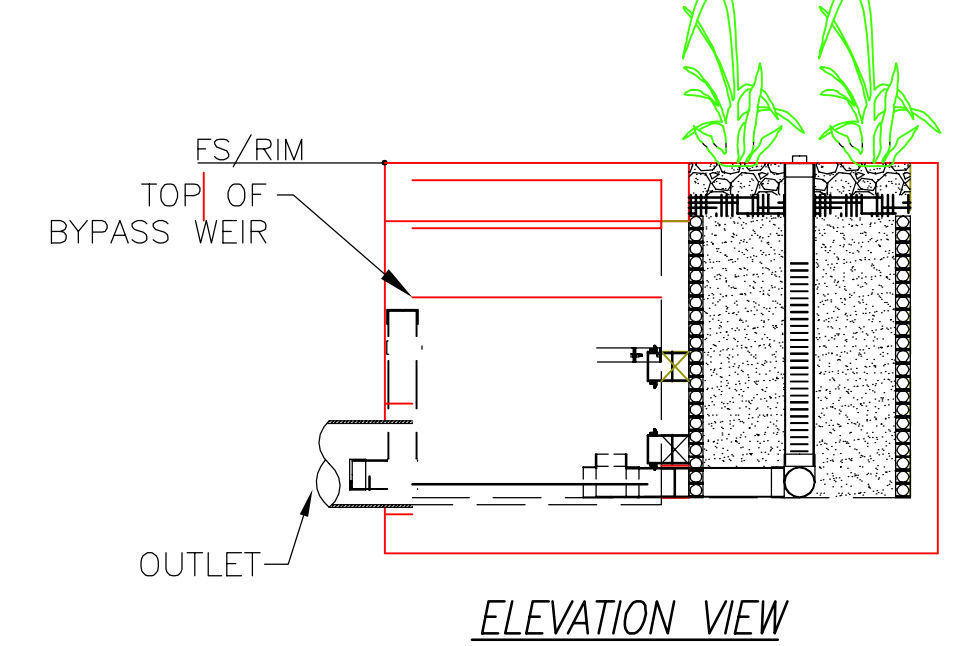
DMA TABLE

DMA	AREA (ACRES)	VOLUME (CF)		WQ FLOW RATE (CFS)		MWS UNITS NUMBER	MWS UNITS SIZE	NOTE
		PER CALC'S	1.5 TIMES	PER CALC'S	1.5 TIMES			
1	2.15	4,257	6,386	0.20	0.30	#1	MWS-8-12	
2	2.72	5,386	8,079	0.25	0.38	#2	MWS-8-16	LOW FLOW PIPE PROVIDED FOR PROJECT AREA ONLY, NO PUBLIC ROAD LOW FLOWS
3	2.95	5,841	8,079	0.28	0.41	#3	MWS-8-16	
4	0.92	1,822	2,733	0.09	0.13	#4	MWS-4-13	
5	1.11	2,198	3,297	0.10	0.16	#5	MWS-4-15	
OVERALL	9.85	19,504	29,256	0.92	1.38			

PROPRIETARY BIOFILTRATION (MWS)

TYPICAL SECTION DETAIL

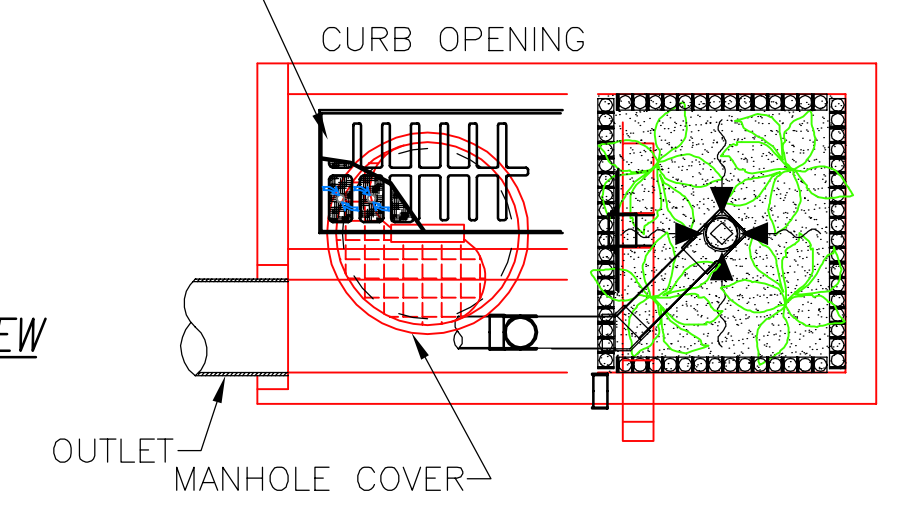
N.T.S.



ELEVATION VIEW

BioMediaGREEN CARTRIDGE
MEDIA FILTER
35 SQUARE FEET MEDIA
SURFACE AREA

PLAN VIEW

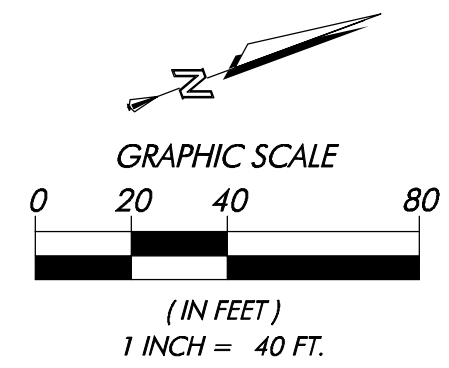


LOW IMPACT DEVELOPMENT (LID) PLAN - SITE PLAN

APPLICANT: **LENNAR**
15131 ALTON PARKWAY, SUITE 365
IRVINE, CA 92618
(949) 349-8100

PREPARED BY: **HUNSAKER & ASSOCIATES**
IRVINE, INC.
PLANNING • ENGINEERING • SURVEYING
Three Hughes • Irvine, CA 92618 • PH: (949) 583-1010 • FX: (949) 583-0759

"VESTING TENTATIVE TRACT MAP NO. 082159"
16234 FOLGER STREET
CITY OF HACIENDA HEIGHTS
LOS ANGELES COUNTY, CA



SECTION 5

**PRELIMINARY HYDRAULIC UPDATES FOR
PROJECT 1273 - LINE "C", CATCH BASIN
AND STREET CAPACITY CALC'S**

ELEMENT NO	10	IS A JUNCTION	1235.000	342.890	5				.013			.000	.000	.000	0
		U/S DATA	STATION	INVERT	SECT	LAT-1	LAT-2	N	Q3	Q4	INVERT-3	INVERT-4	PHI 3	PHI 4	
			1235.001	342.891	5	1	0	.013	2.550	.000	343.200	.000	60.000	.000	
											RADIUS	ANGLE			
											.000	.000			

W S P G W

WATER SURFACE PROFILE - ELEMENT CARD LISTING

ELEMENT NO	11	IS A REACH	*	*	*										
		U/S DATA	STATION	INVERT	SECT			N			RADIUS	ANGLE	ANG PT	MAN H	
			1241.870	343.190	5			.013			.000	.000	.000	0	

ELEMENT NO	12	IS A JUNCTION	*	*	*										
		U/S DATA	STATION	INVERT	SECT	LAT-1	LAT-2	N	Q3	Q4	INVERT-3	INVERT-4	PHI 3	PHI 4	
			1246.530	343.940	4	1	0	.013	.001	.000	344.500	.000	90.000	.000	
											RADIUS	ANGLE			
											.000	.000			

ELEMENT NO	13	IS A REACH	*	*	*										
		U/S DATA	STATION	INVERT	SECT			N			RADIUS	ANGLE	ANG PT	MAN H	
			1261.380	344.440	4			.013			.000	.000	.000	0	

ELEMENT NO	14	IS A REACH	*	*	*										
		U/S DATA	STATION	INVERT	SECT			N			RADIUS	ANGLE	ANG PT	MAN H	
			1273.430	344.840	4			.013			44.990	15.346	.000	0	

ELEMENT NO	15	IS A REACH	*	*	*										
		U/S DATA	STATION	INVERT	SECT			N			RADIUS	ANGLE	ANG PT	MAN H	
			1285.000	345.230	4			.013			.000	.000	.000	0	

ELEMENT NO	16	IS A REACH	*	*	*										
		U/S DATA	STATION	INVERT	SECT			N			RADIUS	ANGLE	ANG PT	MAN H	
			1440.400	346.470	4			.013			.000	.000	.000	0	

ELEMENT NO	17	IS A JUNCTION	*	*	*										
		U/S DATA	STATION	INVERT	SECT	LAT-1	LAT-2	N	Q3	Q4	INVERT-3	INVERT-4	PHI 3	PHI 4	
			1440.401	346.471	4	2	0	.013	11.740	.000	347.000	.000	90.000	.000	
											RADIUS	ANGLE			
											.000	.000			

ELEMENT NO	18	IS A REACH	*	*	*										
		U/S DATA	STATION	INVERT	SECT			N			RADIUS	ANGLE	ANG PT	MAN H	
			1657.070	348.210	4			.013			.000	.000	.000	0	

ELEMENT NO	19	IS A JUNCTION	*	*	*										
		U/S DATA	STATION	INVERT	SECT	LAT-1	LAT-2	N	Q3	Q4	INVERT-3	INVERT-4	PHI 3	PHI 4	
			1661.731	348.250	4	0	0	.013	.000	.000	.000	.000	.000	.000	
											RADIUS	ANGLE			
											.000	.000			

W S P G W

WATER SURFACE PROFILE - ELEMENT CARD LISTING

ELEMENT NO	20	IS A REACH	*	*	*										
		U/S DATA	STATION	INVERT	SECT			N			RADIUS	ANGLE	ANG PT	MAN H	
			1770.000	349.110	4			.013			.000	.000	.000	0	

ELEMENT NO	21	IS A REACH	*	*	*										
		U/S DATA	STATION	INVERT	SECT			N			RADIUS	ANGLE	ANG PT	MAN H	
			1975.130	350.400	4			.013			.000	.000	.000	0	

ELEMENT NO	22	IS A JUNCTION	*	*	*										
		U/S DATA	STATION	INVERT	SECT	LAT-1	LAT-2	N	Q3	Q4	INVERT-3	INVERT-4	PHI 3	PHI 4	

		1975.131	350.401	4	1	0	.013	3.220	.000	344.500	.000	80.000	.000
										RADIUS	ANGLE		
										.000	.000		
ELEMENT NO	23 IS A REACH	*	*	*									
	U/S DATA	STATION	INVERT	SECT			N			RADIUS	ANGLE	ANG PT	MAN H
		2026.000	350.730	4			.013			.000	.000	.000	0
ELEMENT NO	24 IS A JUNCTION	*	*	*	*	*		*		*		*	
	U/S DATA	STATION	INVERT	SECT	LAT-1	LAT-2	N	Q3	Q4	INVERT-3	INVERT-4	PHI 3	PHI 4
		2033.000	350.770	4	3	0	.013	29.780	.000	350.800	.000	60.000	.000
										RADIUS	ANGLE		
										.000	.000		
ELEMENT NO	25 IS A REACH	*	*	*									
	U/S DATA	STATION	INVERT	SECT			N			RADIUS	ANGLE	ANG PT	MAN H
		2068.200	351.000	4			.013			.000	.000	.000	0
ELEMENT NO	26 IS A SYSTEM HEADWORKS			*				*					
	U/S DATA	STATION	INVERT	SECT						W S ELEV			
		2068.200	351.000	4						351.000			

Hacienda Heights Project No. 1273
 Updated Hydraulic Calculations for Line C

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope				SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch	
1130.000	342.020	5.180	347.200	77.00	8.00	.99	348.19	.00	2.74	.00	3.500	.000	.00	1 .0
59.000	.0024				.0059	.35	5.18	.00	3.50	.013	.00	.00		PIPE
1189.000	342.160	5.386	347.546	77.00	8.00	.99	348.54	.00	2.74	.00	3.500	.000	.00	1 .0
TRANS STR	.0022				.0059	.02	5.39	.00		.013	.00	.00		PIPE
1192.200	342.167	5.397	347.564	77.00	7.70	.92	348.49	.00	3.58	2.00	5.000	2.000	.00	0 .0
6.000	.0022				.0082	.05	5.40	.61	5.00	.014	.00	.00		BOX
1198.200	342.180	5.434	347.614	77.00	7.70	.92	348.53	.00	3.58	2.00	5.000	2.000	.00	0 .0
TRANS STR	.0020				.0082	.08	5.43	.61		.014	.00	.00		BOX
1208.200	342.200	5.496	347.696	77.00	7.70	.92	348.62	.00	3.58	2.00	5.000	2.000	.00	0 .0
JUNCT STR	.0055				.0042	.01	.00	.61		.013	.00	.00		BOX
1210.000	342.210	6.091	348.301	59.29	6.16	.59	348.89	.00	2.41	.00	3.500	.000	.00	1 .0
4.200	.0024				.0035	.01	.00	.00	3.50	.013	.00	.00		PIPE
1214.200	342.220	6.136	348.356	59.29	6.16	.59	348.95	.00	2.41	.00	3.500	.000	.00	1 .0
16.170	.0322				.0035	.06	.00	.00	1.38	.013	.00	.00		PIPE
1230.370	342.740	5.752	348.492	59.29	6.16	.59	349.08	.00	2.41	.00	3.500	.000	.00	1 .0
4.630	.0324				.0035	.02	5.75	.00	1.38	.013	.00	.00		PIPE
1235.000	342.890	5.618	348.508	59.29	6.16	.59	349.10	.00	2.41	.00	3.500	.000	.00	1 .0
JUNCT STR	1.0000				.0033	.00	5.62	.00		.013	.00	.00		PIPE

Hacienda Heights Project No. 1273
 Updated Hydraulic Calculations for Line C

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt/ or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1235.001	342.891	5.711	348.602	56.74	5.90	.54	349.14	.00	2.36	.00	3.500	.000	.00	1 .0
6.869	.0435					.0032	.02	5.71	.00	1.24	.013	.00	.00	PIPE
1241.870	343.190	5.433	348.623	56.74	5.90	.54	349.16	.00	2.36	.00	3.500	.000	.00	1 .0
JUNCT STR	.1609					.0073	.03	5.43	.00		.013	.00	.00	PIPE
1246.530	343.940	3.890	347.830	56.74	9.55	1.42	349.25	.00	2.44	.00	2.750	.000	.00	1 .0
14.850	.0337					.0115	.17	3.89	.00	1.51	.013	.00	.00	PIPE
1261.380	344.440	3.561	348.001	56.74	9.55	1.42	349.42	.00	2.44	.00	2.750	.000	.00	1 .0
12.050	.0332					.0115	.14	.00	.00	1.52	.013	.00	.00	PIPE
1273.430	344.840	3.416	348.256	56.74	9.55	1.42	349.67	.00	2.44	.00	2.750	.000	.00	1 .0
11.570	.0337					.0115	.13	3.42	.00	1.51	.013	.00	.00	PIPE
1285.000	345.230	3.160	348.390	56.74	9.55	1.42	349.81	.00	2.44	.00	2.750	.000	.00	1 .0
155.400	.0080					.0115	1.79	3.16	.00	2.75	.013	.00	.00	PIPE
1440.400	346.470	3.708	350.178	56.74	9.55	1.42	351.60	.00	2.44	.00	2.750	.000	.00	1 .0
JUNCT STR	1.0313					.0094	.00	3.71	.00		.013	.00	.00	PIPE
1440.401	346.471	4.759	351.230	45.00	7.58	.89	352.12	.00	2.22	.00	2.750	.000	.00	1 .0
216.669	.0080					.0072	1.57	4.76	.00	2.14	.013	.00	.00	PIPE
1657.070	348.210	4.588	352.798	45.00	7.58	.89	353.69	.00	2.22	.00	2.750	.000	.00	1 .0
JUNCT STR	.0086					.0072	.03	4.59	.00		.013	.00	.00	PIPE

Street Capacity Calc's for Hinnen Avenue

Project Description

Friction Method Manning Formula
 Solve For Normal Depth

Input Data

Channel Slope 0.01500 ft/ft
 Discharge 3.22 ft³/s
 Section Definitions

Station (ft)	Elevation (ft)
0+00	100.67
0+00	100.00
0+02	100.13
0+18	100.43

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00, 100.67)	(0+18, 100.43)	0.015

Options

Current Roughness weighted Method Pavlovskii's Method
 Open Channel Weighting Method Pavlovskii's Method
 Closed Channel Weighting Method Pavlovskii's Method

Results

Normal Depth 0.29 ft
 Elevation Range 100.00 to 100.67 ft
 Flow Area 1.18 ft²
 Wetted Perimeter 10.99 ft
 Hydraulic Radius 0.11 ft
 Top Width 10.69 ft
 Normal Depth 0.29 ft
 Critical Depth 0.33 ft
 Critical Slope 0.00676 ft/ft

Street Capacity Calc's for Hinnen Avenue

Results

Velocity	2.74	ft/s
Velocity Head	0.12	ft
Specific Energy	0.41	ft
Froude Number	1.45	
Flow Type	Supercritical	

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.29	ft
Critical Depth	0.33	ft
Channel Slope	0.01500	ft/ft
Critical Slope	0.00676	ft/ft

Cross Section for Hinnen Avenue

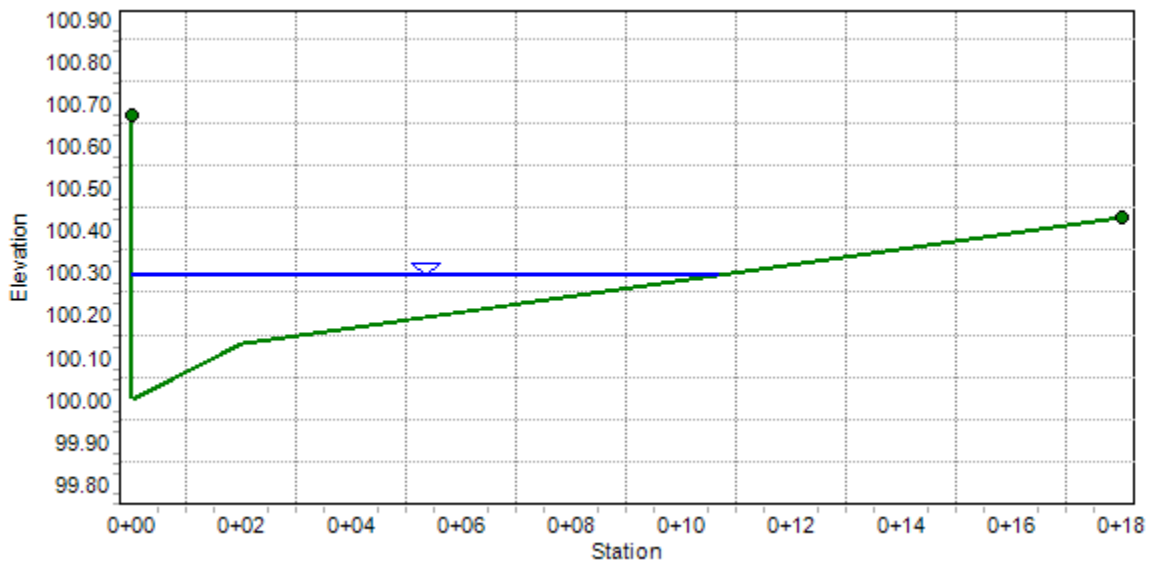
Project Description

Friction Method	Manning Formula
Solve For	Normal Depth

Input Data

Channel Slope	0.01500	ft/ft
Normal Depth	0.29	ft
Discharge	3.22	ft ³ /s

Cross Section Image



Worksheet for Glenelder Avenue Street Capacity Calc's

Project Description

Friction Method Manning Formula
 Solve For Normal Depth

Input Data

Channel Slope 0.01200 ft/ft
 Discharge 10.78 ft³/s
 Section Definitions

Station (ft)	Elevation (ft)
0+00	100.67
0+00	100.00
0+02	100.13
0+18	100.43

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00, 100.67)	(0+18, 100.43)	0.015

Options

Current Roughness weighted Method Pavlovskii's Method
 Open Channel Weighting Method Pavlovskii's Method
 Closed Channel Weighting Method Pavlovskii's Method

Results

Normal Depth 0.43 ft
 Elevation Range 100.00 to 100.67 ft
 Flow Area 3.20 ft²
 Wetted Perimeter 18.44 ft
 Hydraulic Radius 0.17 ft
 Top Width 18.00 ft
 Normal Depth 0.43 ft
 Critical Depth 0.48 ft
 Critical Slope 0.00562 ft/ft

Worksheet for Glenelder Avenue Street Capacity Calc's

Results

Velocity	3.37	ft/s
Velocity Head	0.18	ft
Specific Energy	0.61	ft
Froude Number	1.41	
Flow Type	Supercritical	

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.43	ft
Critical Depth	0.48	ft
Channel Slope	0.01200	ft/ft
Critical Slope	0.00562	ft/ft

Cross Section for Glenelder Avenue

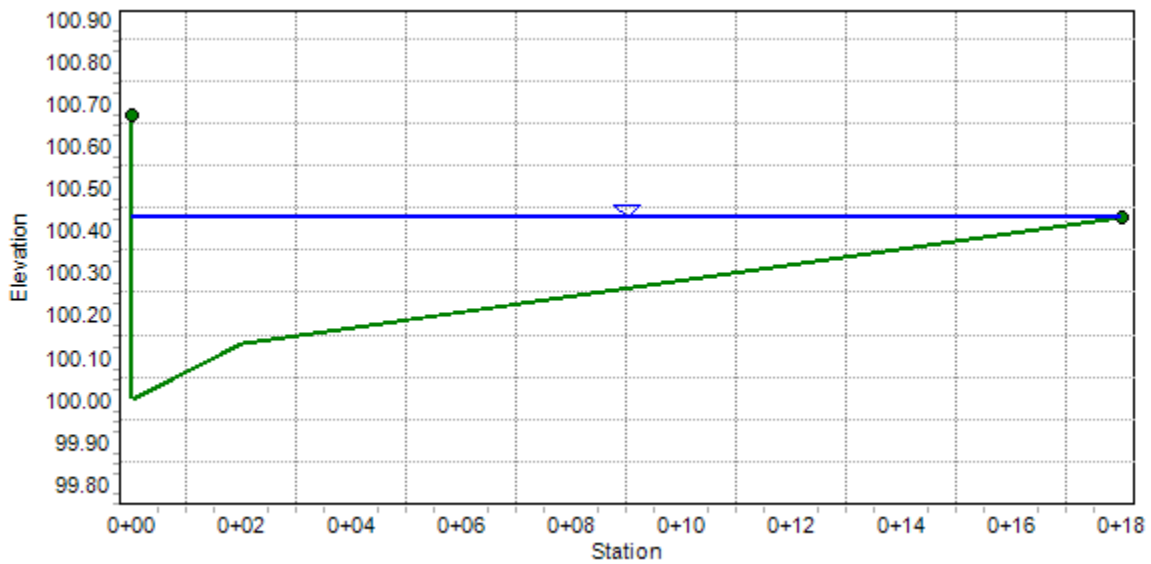
Project Description

Friction Method Manning Formula
Solve For Normal Depth

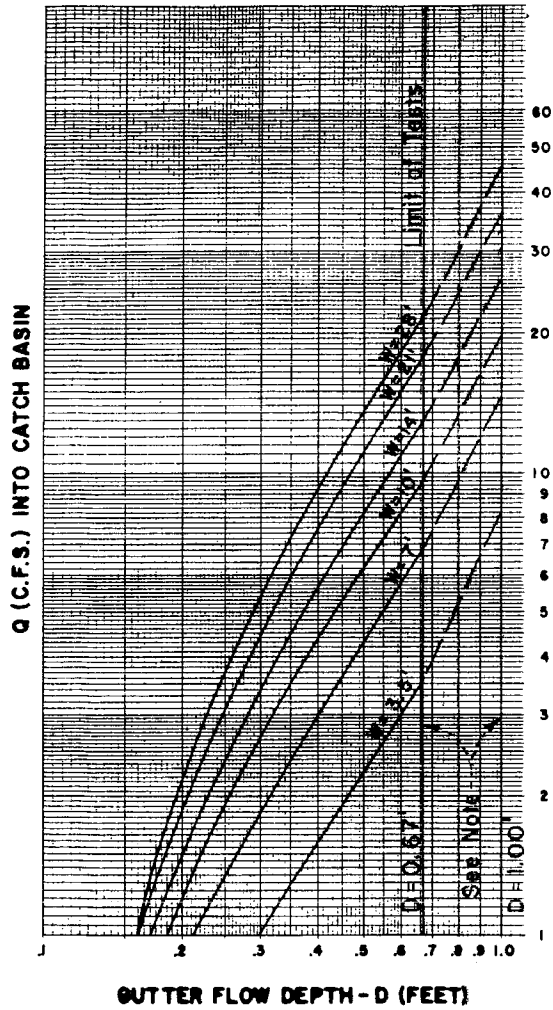
Input Data

Channel Slope 0.01200 ft/ft
Normal Depth 0.43 ft
Discharge 10.78 ft³/s

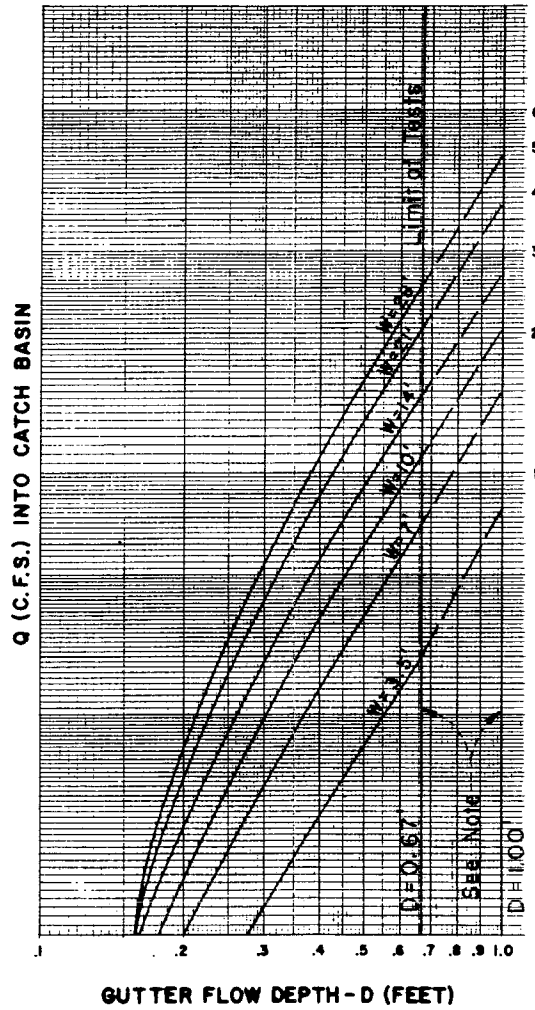
Cross Section Image



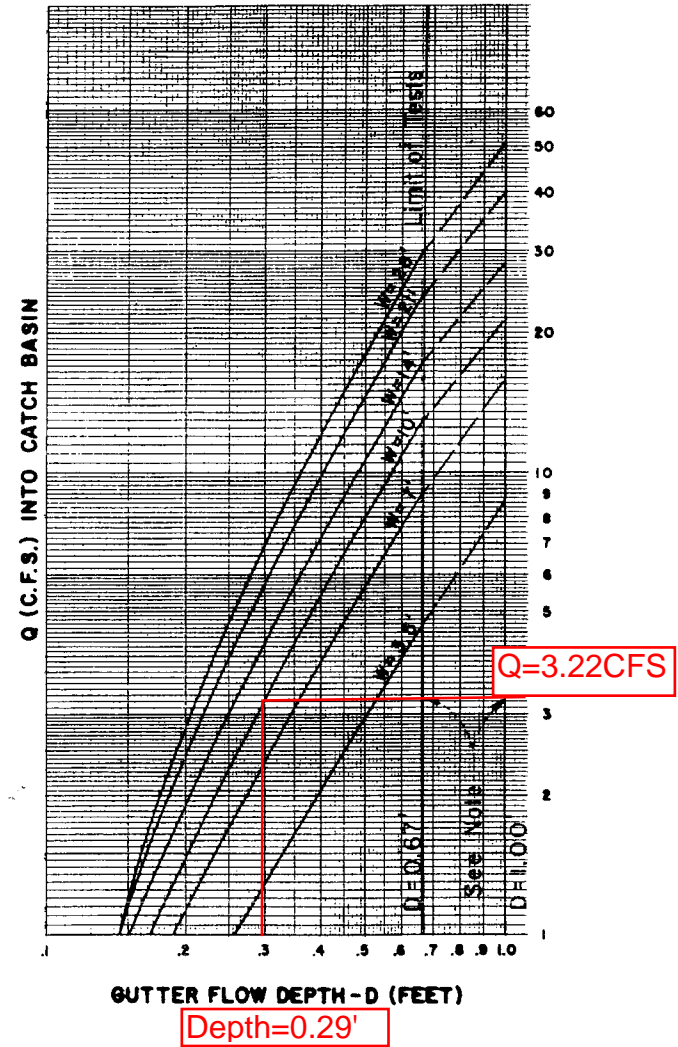
1" GUTTER DEPRESSION



2" GUTTER DEPRESSION



4" GUTTER DEPRESSION



NOTE: Curves between D=0.67' and 1.0' are not from model test data and will be revised in the future when additional model test data are available.

CURB OPENING CATCH BASIN CAPACITIES

STREET SLOPE = .01

D-10B

Catch Basin #4
along Folger Street

Hunsaker & Associates Irvine, Inc.

DATE: 05/28/19
BY: G. Guan

C.B. #6 Along Hinnen Avenue
Curb Opening (Sump)

Given: (a) discharge Q = 10.8 c.f.s.
(b) 6" curb

Solution:
H (depth at opening) = 0.50 '
 $L = Q/3.087H^{3/2}$ = 9.88 '

Use:
L = 10.00 '

Then:
H = 0.4959 '

SECTION 6
REFERENCES

- **ALLOWABLE FLOWS FROM LACDPW**



Office Use Only

Sent Initials: _____

Fax Email Other: _____

Date: _____ Time: _____

**LOS ANGELES COUNTY
DEPARTMENT OF PUBLIC WORKS
DESIGN DIVISION – HYDRAULIC ANALYSIS UNIT**

INFORMATION REQUEST SUMMARY

INFORMATION REQUESTED BY

*Requester's Name: Gary Guan
 Company: Hunsaker & Associates
 *Phone Number: 949-768-2591 Fax Number: _____
 *Email: gguan@hunsaker.com

Method of Contact: Walk-in Phone Fax Email Prelim. Mtg. Date: _____

Intended Use: VTTM hydrology analysis

Proposed Project Type: Convert the school to detached condominiums Acreage Involved: 16.8 acres

*Will information be used in any litigation? YES NO
 Case Info. Name: _____ No: _____ Location: _____

INFORMATION REQUESTED (Attach Assessor Map)

LACFCD Facility: Name: Project No. 1273
 Unit: DWG: 470-1273-D Line: C Station: 1200 to 21+00
 City: Hacienda Heights

*Street/Cross-street: Hinnen Ave/Folger St

*Thomas Guide: Page: 678 Grid: D2 Site Map/Plans Submitted

Info. Requested:
 Allowable Q
 Hydrology Data, As Built Drawings
 Hydraulic Grade Line

***Required Information. See Page 2 of 2 for Instructions.**

BELOW SECTION TO BE COMPLETED BY THE HYDRAULIC ANALYSIS UNIT

INFORMATION PROVIDED:

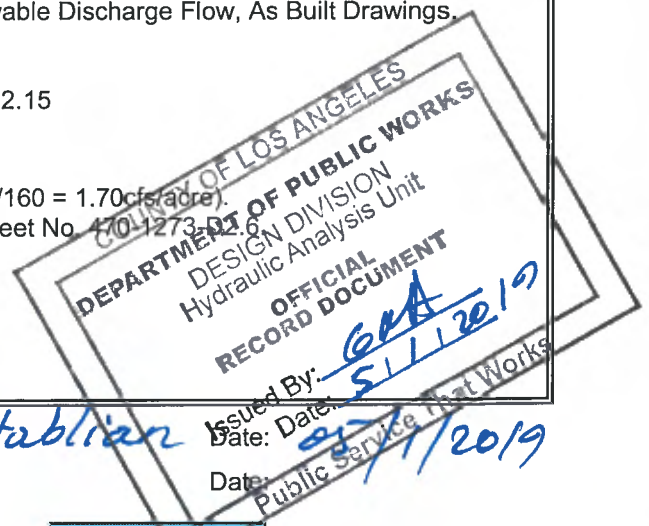
Project 1273 Line "C" Hydrology Data, Drainage Map, and allowable Discharge Flow, As Built Drawings.

REFERENCES SEARCHED:

Project 1273 Files and As Built Drawing No. 470-1273-D2.1 to D2.15

COMMENTS, ETC:

- 1- Subarea No. 34C allowable discharge flow ($Q = 51/21 \times 112/160 = 1.70$ cfs/acre).
- 2- Hydraulic Grade Line shown on As Built Plan and Profile Sheet No. 470-1273-D2.6

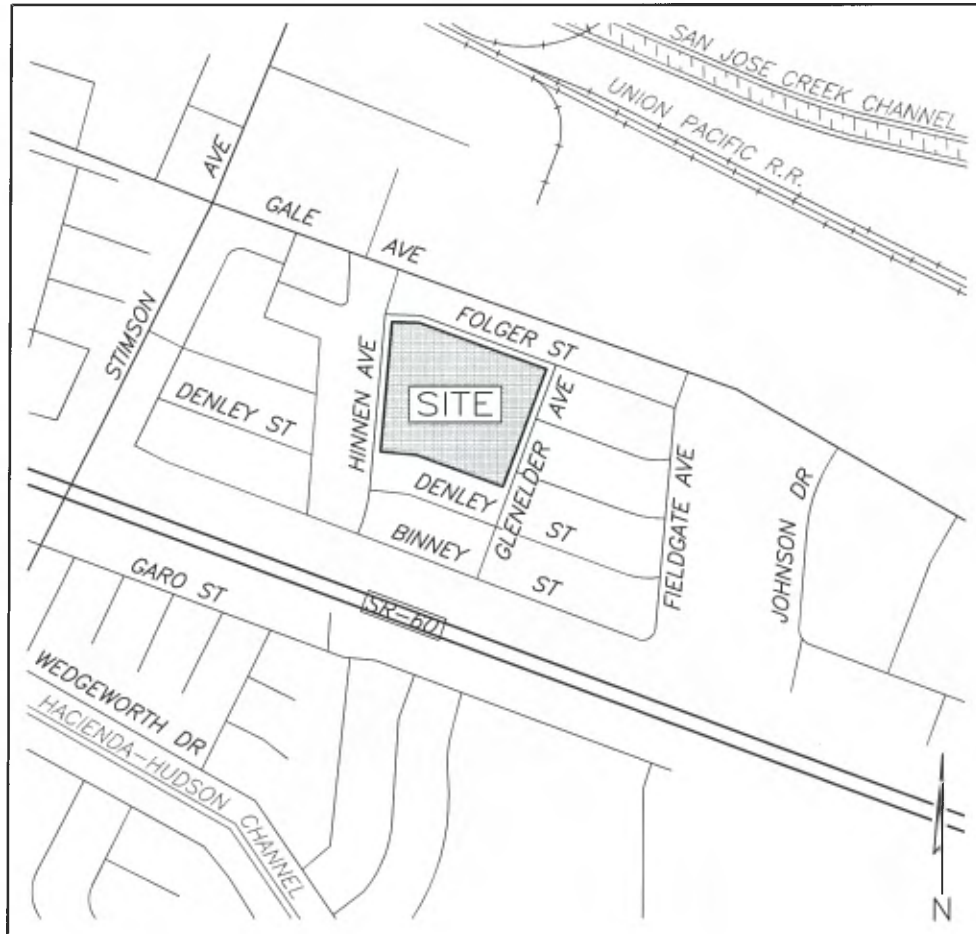


INFORMATION PROVIDED BY: George K. Aintablian

INFORMATION REVIEWED BY:

Print

Save a Copy



VICINITY MAP

N.T.S.



REACH	Q (CFS)
LINE A	
25A-31A	740
31A-RD 51	860
LINE B	
28B-LN. A	140
LINE C	
33C-34C	65 45.0
34C-35C	40 77.0
35C-PR.527	160 112.0
LINE D	
36D-37D	90
37D-38D	110
38D-39D	130
39D-40D	160
40D-PR.527	190

COUNTY OF LOS ANGELES
 DEPARTMENT OF PUBLIC WORKS
 DESIGN DIVISION
 Hydraulic Analysis Unit
 OFFICIAL
 RECORD DOCUMENT
 Issued By: *GKA*
 Date: *5/1/2019*
 Public Service That Works



LEGEND

- WATERSHED BOUNDARY
- ==== PROJECT DRAIN
- EXISTING DRAIN
- SUBAREA AND COLLECTION POINT
- - - PROPOSED DRAIN

LA HABRA & BALDWIN PARK QUARTERS

PREPARED D.A.L.	LOS ANGELES COUNTY FLOOD CONTROL DISTRICT
DATE 11-24-71	1970 BOND ISSUE PROJECT #1273
SCALE 1"=2000'	10-YR. FREQUENCY Q's

GKA

SHEET NO.	INDEX TO DRAWINGS
1	LOCATION MAP AND INDEX TO DRAWINGS
2	STANDARD DRAWINGS, GENERAL NOTE AND ABBREVIATIONS
3	PLAN AND PROFILE—LINE "A"
4	PLAN AND PROFILE—LINE "B"
5T06	PLAN AND PROFILE—LINE "C"
7T09	PLAN AND PROFILE—LINE "D"
10	CROSS SECTIONS—LINE "A"
11	CROSS SECTIONS—LINE "B"
12T013	STRUCTURAL DETAILS—LINE "A"
14	SINGLE BOXES, SCHEDULE AND DETAILS
15	RESURFACING MAP AND LOG OF BORINGS—LINES A, B, C, & D



LOCATION MAP
SCALE 1"=1000'

LEGEND	
	Proposed Storm Drain
	Existing Storm Drain
	Proposed Storm Drain
	Other Drain
	City Boundary

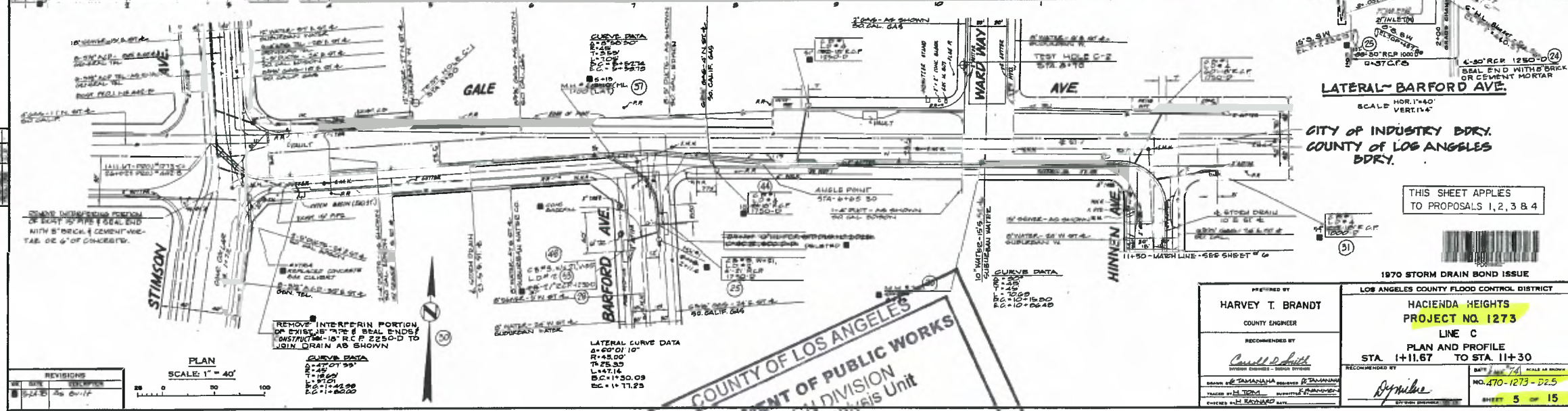
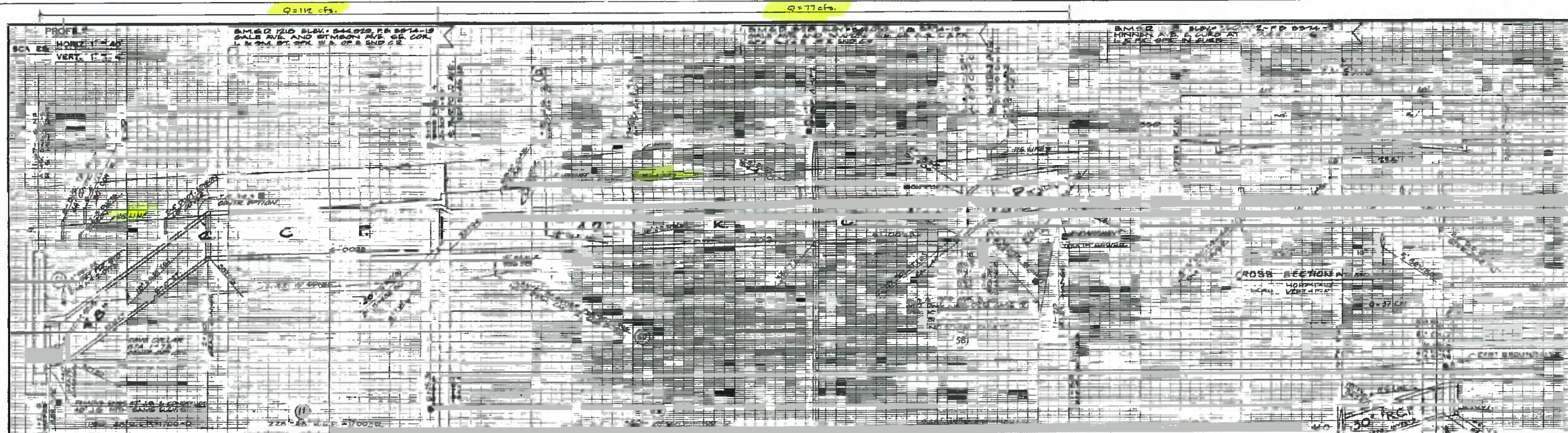
COUNTY OF LOS ANGELES
DEPARTMENT OF PUBLIC WORKS
 DESIGN DIVISION
 Hydraulic Analysis Unit
OFFICIAL RECORD DOCUMENT
 Issued By: *GKA*
 Date: *5/1/2019*
Public Service That Works

AS BUILT DRAWINGS

REVISIONS	
NO.	DESCRIPTION

SANITATION DIVISION COUNTY OF LOS ANGELES <i>Richard E. ...</i>	APPROVED AS TO CITY OF INDUSTRY CITY ENGINEER DEPUTY <i>...</i> DATE: <i>10/1/18</i>	PREPARED BY HARVEY T. BRAND COUNTY ENGINEER	LOS ANGELES COUNTY FLOOD CONTROL DISTRICT HACIENDA HEIGHTS PROJECT NO. 1273 LINES A-D
		RECOMMENDED BY <i>...</i> DATE: <i>...</i>	LOCATION MAP AND INDEX TO DRAWINGS SHEET 1 OF 15

AS BUILT DRAWING



REV.	DATE	DESCRIPTION
1	5-21-73	AS SHOWN

PLAN
SCALE: 1" = 40'

REMOVE INTERFERING PORTION OF EXISTING 18" R.C.P. 2250-D TO JOIN DRAIN AS SHOWN

REMOVE INTERFERING PORTION OF EXISTING 18" R.C.P. 2250-D TO JOIN DRAIN AS SHOWN

REMOVE INTERFERING PORTION OF EXISTING 18" R.C.P. 2250-D TO JOIN DRAIN AS SHOWN

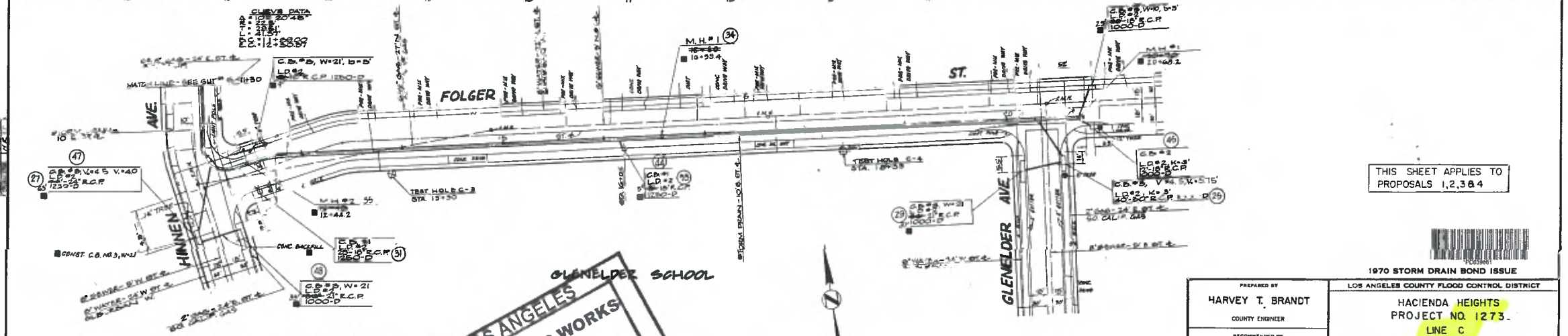
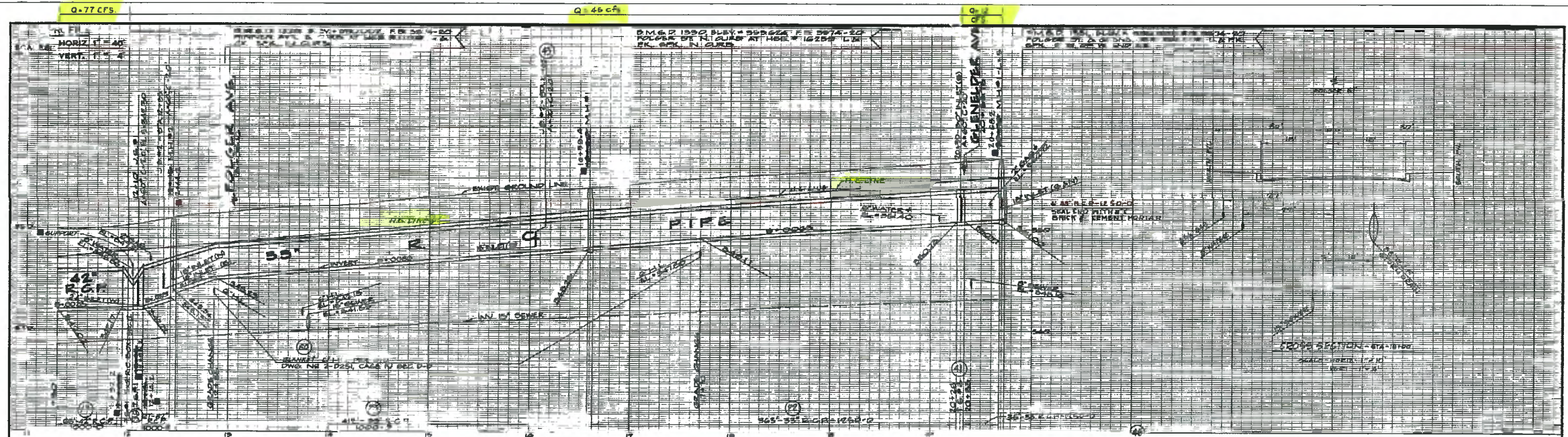
LATERAL CURVE DATA
 Δ=60°01'10"
 R=45.00'
 L=25.30'
 L=47.16'
 B.C. = 11.30.09
 E.C. = 11.11.25

COUNTY OF LOS ANGELES
 DEPARTMENT OF PUBLIC WORKS
 DESIGN DIVISION
 Hydraulic Analysis Unit
 OFFICIAL RECORD DOCUMENT
 Issued By: *OKA*
 Date: *5/1/2019*
 Public Service That Works

PREPARED BY
 HARVEY T. BRANDT
 COUNTY ENGINEER
 RECOMMENDED BY
Carol Smith
 DESIGNER

1970 STORM DRAIN BOND ISSUE
 LOS ANGELES COUNTY FLOOD CONTROL DISTRICT
 HACIENDA HEIGHTS
 PROJECT NO. 1273
 LINE C
 PLAN AND PROFILE
 STA. 11+1.67 TO STA. 11+30
 SHEET 5 OF 15

"AS BUILT" DRAWING



NO.	DATE	REVISION
1	5/1/74	AS BUILT

PLAN
SCALE: 1" = 40'

COUNTY OF LOS ANGELES
 DEPARTMENT OF PUBLIC WORKS
 DESIGN DIVISION
 Hydraulic Analysis Unit
 OFFICIAL
 RECORD DOCUMENT
 Issued By: GKA
 Date: 5/1/2019
 Public Service That Works

THIS SHEET APPLIES TO
PROPOSALS 1,2,3&4

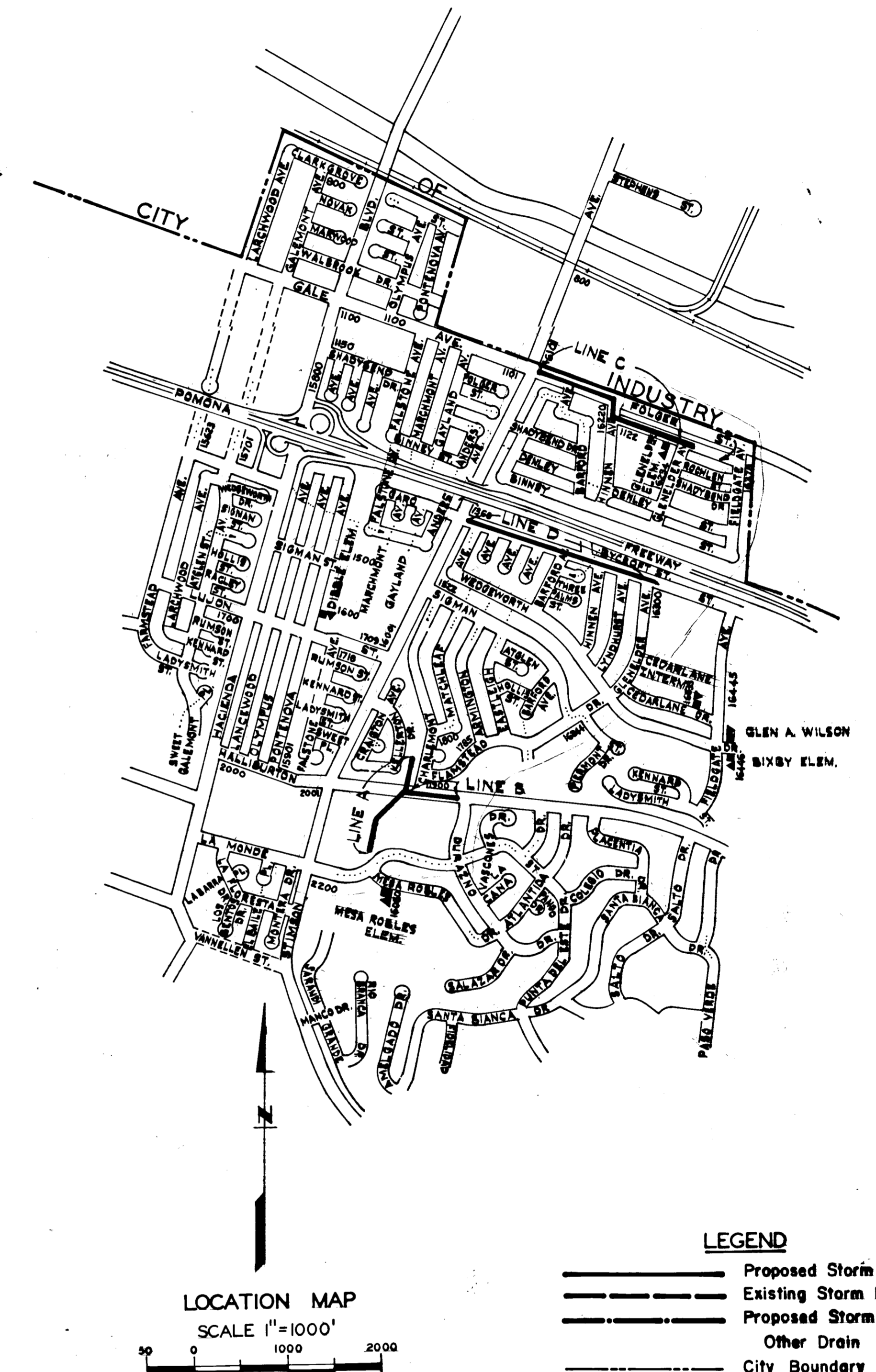


1970 STORM DRAIN BOND ISSUE	
LOS ANGELES COUNTY FLOOD CONTROL DISTRICT	
PREPARED BY HARVEY T. BRANDT COUNTY ENGINEER	HACIENDA HEIGHTS PROJECT NO. 1273
RECOMMENDED BY <i>Carl Schell</i> COUNTY ENGINEER	LINE C PLAN AND PROFILE STA. 11+30 TO STA. 20+70
DATE: <u>5/1/74</u>	SHEET 6 OF 15

"AS BUILT" DRAWING

- **EXISTING STORM DRAIN PLANS**

SHEET NO.	INDEX TO DRAWINGS
1	LOCATION MAP AND INDEX TO DRAWINGS
2	STANDARD DRAWINGS, GENERAL NOTE AND ABBREVIATIONS
3	PLAN AND PROFILE—LINE "A"
4	PLAN AND PROFILE—LINE "B"
5T06	PLAN AND PROFILE—LINE "C"
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10	CROSS SECTIONS—LINE "A"
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12T013	STRUCTURAL DETAILS—LINE "A"
14	SINGLE BOXES, SCHEDULE AND DETAILS
15	RESURFACING MAP AND LOG OF BORINGS—LINES A, B, C, & D



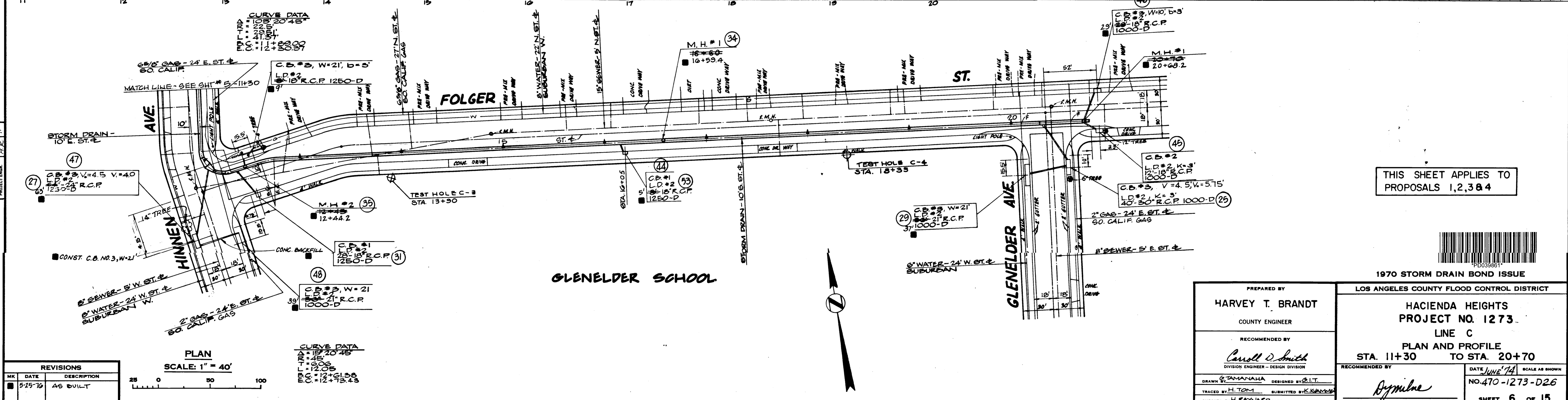
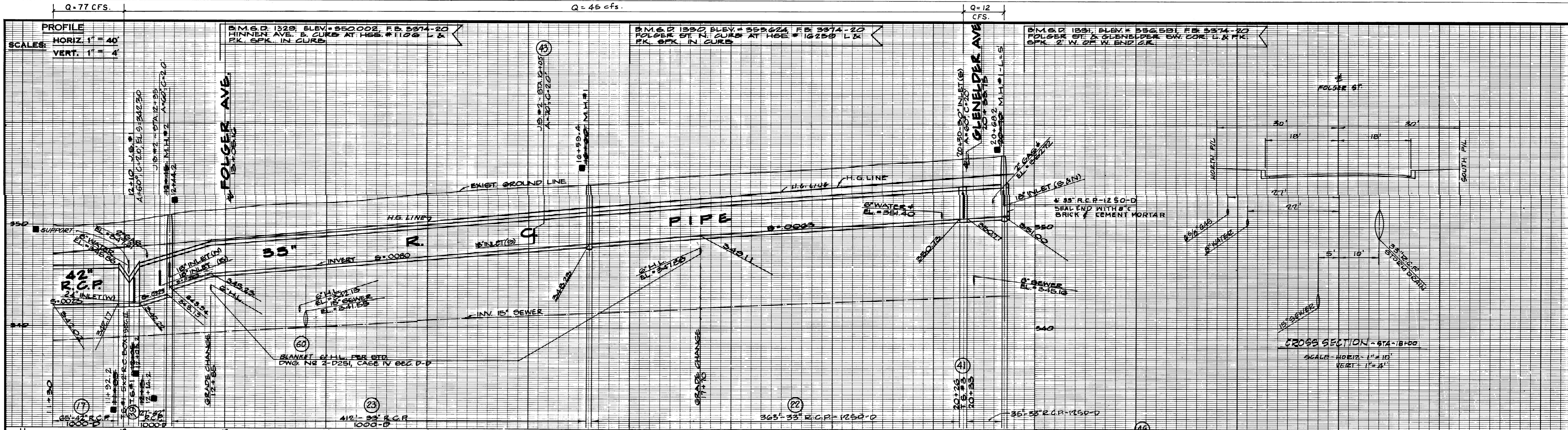
AS BUILT DRAWINGS

1970 STORM DRAIN BOND ISSUE

REVISIONS		
MK	DATE	DESCRIPTION

SANITATION DIVISION COUNTY OF LOS ANGELES <i>Richard E. Kulan</i> 4-17-73	APPROVED AS TO CITY OF INDUSTRY <i>George O. Roche</i> CITY ENGINEER DEPUTY DATE: 10/17/73	PREPARED BY HARVEY T. BRAND COUNTY ENGINEER	LOS ANGELES COUNTY FLOOD CONTROL DISTRICT HACIENDA HEIGHTS PROJECT NO. 1273 LINES A-D
		RECOMMENDED BY <i>Conell D. Smith</i> DIVISION ENGINEER - DESIGN DIVISION	RECOMMENDED BY <i>Symche</i> DIVISION ENGINEER - DES. DIV.
DRAWN BY: <i>E. MOORE</i> DESIGNED BY: <i>A. MALABARA</i> TRACED BY: <i>HEATHMAN</i> SUBMITTED BY: <i>K. KYRANAKIS</i> CHECKED BY: <i>HEATHMAN</i> DATE:		DATE: <i>10-17-73</i> SCALE AS SHOWN NO. 170-1273-D2.1 SHEET 1 OF 15	

"AS BUILT" DRAWING



THIS SHEET APPLIES TO PROPOSALS 1,2,3 & 4



1970 STORM DRAIN BOND ISSUE

PREPARED BY HARVEY T. BRANDT COUNTY ENGINEER	LOS ANGELES COUNTY FLOOD CONTROL DISTRICT HACIENDA HEIGHTS PROJECT NO. 1273 LINE C PLAN AND PROFILE STA. 11+30 TO STA. 20+70
RECOMMENDED BY <i>Conrad D. Smith</i> DIVISION ENGINEER - DESIGN DIVISION	RECOMMENDED BY <i>Rymilne</i> DIVISION ENGINEER (DESIGN)
DRAWN BY T. TAMANAWA TRACED BY H. TOM	DATE June 74 SCALE AS SHOWN NO. 470-1273-D26 SHEET 6 OF 15

NO.	DATE	DESCRIPTION
1	5-25-76	AS BUILT

PLAN SCALE: 1" = 40'

CURVE DATA
 Δ = 120° 48'
 R = 400'
 L = 1200'
 E.C. = 12+51.58
 P.C. = 12+73.43

"AS BUILT" DRAWING

December 2018

HYDROLOGY STUDY

“Glenelder”

Vesting Tentative Tract Map No. 82159

16234 Folger Street

City of Hacienda Heights | County of Los Angeles



HUNSAKER & ASSOCIATES IRVINE, INC.



HYDROLOGY STUDY

"Glenelder" | Vesting Tentative Tract Map No. 82159
16234 Folger Street
City of Hacienda Heights | County of Los Angeles



HUNSAKER
&
ASSOCIATES
IRVINE, INC.

December
2018