
Appendix M-1

Sewer Area Study

SEWER AREA STUDY

WILEY CANYON

APNs: 2825-012-007, -010, -011, -901
Santa Clarita, CA

SAS20-00003



Prepared For:
SHERIDAN-EBBERT DEVELOPMENT/
ROYAL CLARK DEVELOPMENT COMPANY
13120 Telfair Avenue
Sylmar, CA 91342
(818) 364-7505

Prepared By:
Alliance Land Planning & Engineering, Inc.
2248 Faraday Ave.
Carlsbad, CA 92008
(760) 431-9896

AUGUST 10, 2020

Prepared Under the Direction of:

A handwritten signature in black ink, appearing to read "Craig M. Whitteker".

Craig M. Whitteker

R.C.E. No. 51929

Date

8/10/20



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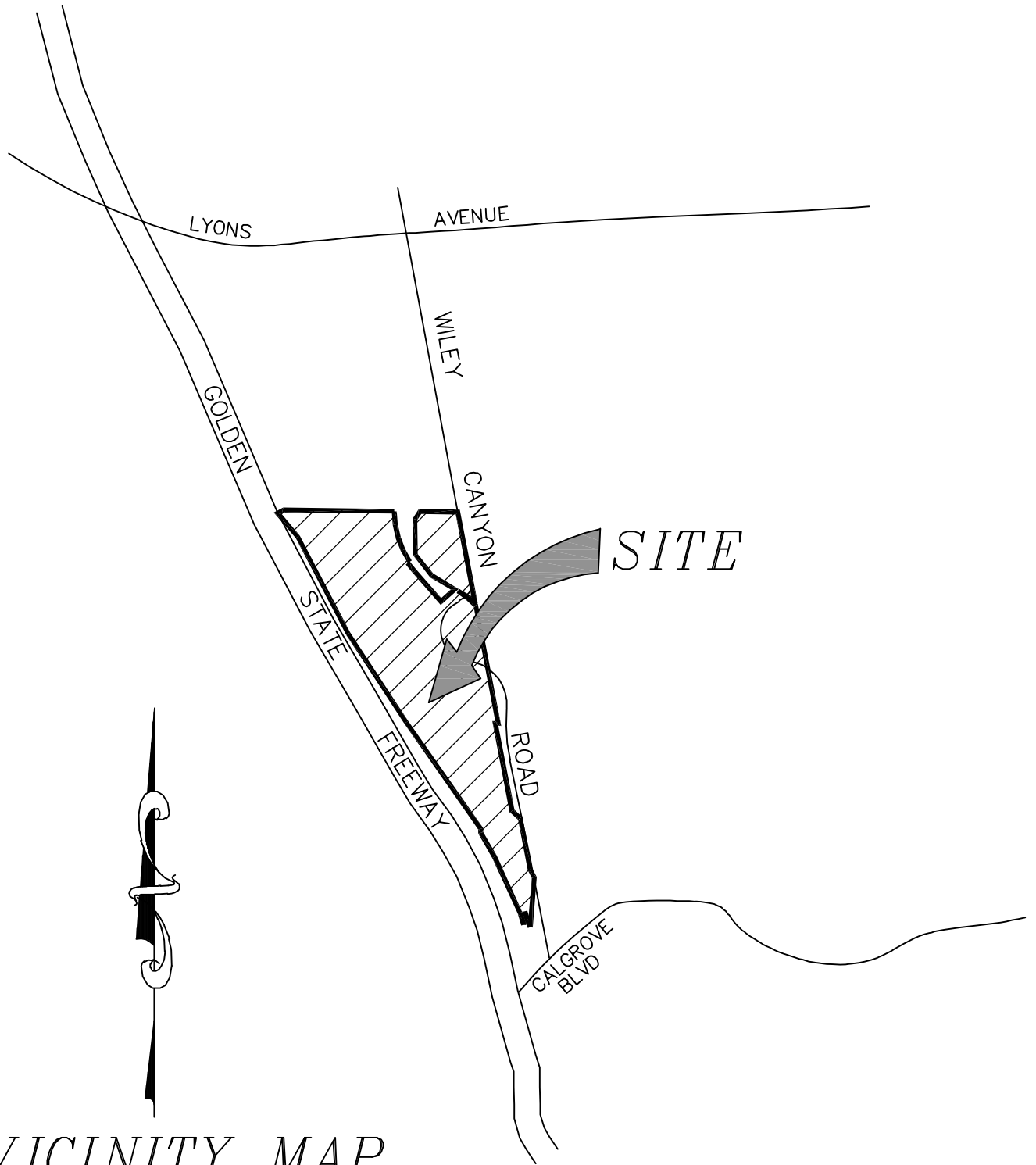
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Exhibit 1 - Sewer Area Study Map (Existing, Proposed and Future Condition)



VICINITY MAP
NOT TO SCALE

INTRODUCTION

The proposed project site lies within the jurisdiction of the City of Santa Clarita. This Sewer Area Study has been prepared by Alliance Land Planning & Engineering, Inc. for Sheridan-Ebbert Development/Royal Clark Development Company located at 13120 Telfair Avenue, Sylmar, CA 91342. This study is being prepared at the request of the City of Santa Clarita Engineering Department to evaluate the capacity of the existing sewer systems that will serve this proposed development. The purpose of this study is to evaluate the capacity of the existing off-site downstream sewer sections from our development to the Los Angeles County Sanitation District maintained trunk sewer and determine if the existing sewer facilities can adequately serve the proposed development.

References used in the preparation of this study: LA County Sewer Maintenance Division Maps, LA County Department of Public Works As-builts, Sanitary Sewer Drawings (PC 7549, PC 7599 and PC 8132), City of Santa Clarita Zoning Maps, City of Santa Clarita standards, approved off-site Sewer Area Study for TTM No. 43896 and sewer flow test data performed in June 2020.

This study evaluates the downstream off-site facilities and includes all tributary flows to the existing sewer system from the existing, proposed and future developments within the overall tributary areas. This study will show and determine the potential impacts of our proposed development.

SITE AND PROJECT DESCRIPTION

The Wiley Canyon project encompasses 31.8 acres and the proposed project is mixed use with a combination of commercial and residential land uses.

AREA ID	LAND USE	AREA OR NO. UNITS
1	Commercial	7.0 AC
2	Multi-family Residential	145 Units
3	Multi-family Residential	230 Units

The development lies halfway between Wabuska Street and Calgrove Avenue directly adjacent to the east side of Golden State Freeway 5 in Santa Clarita. The property information is APN's 2825-012-007, -010, -011 and -901. C.S.M.D. Index No. N-1258, N-1259, N-1297 and N-1298. Thomas Guide 4640 D1-D2, E-E2, F1-F3, 4550 E7, F7 and G7. See Exhibit 1 for project location and surrounding areas.

The following area study has been prepared to show that the capacity is adequate for the existing downstream sewer segments from the proposed development site to the Los Angeles County Sanitation District (LACSD) trunk sewer (24" line within diversion structure) located at approximately 225 feet south of Wiley Canyon Road and Orchard Village Road intersection at manhole #781.

The developed areas of Wiley Canyon flows into an existing manhole #49 with an existing 10"-15" downstream sewer lines (PC 7599), existing 12"-18" sewer lines (PC 7549) and then to existing 24" La County Sanitary District trunk sewer line (within diversion structure at manhole #781); see the Sewer Area Study Exhibit (Exhibit 1, Appendix G) within the pocket of this report for the layout of the existing and proposed sewer systems.

DESCRIPTION OF EXISTING SEWER SYSTEM

The existing downstream sewer system consists of approximately 9,600 linear feet of gravity sewer pipe ranging in size from 10" to 18" before connecting to the 24" Sanitation District trunk line (within diversion structure at manhole #781).

Analysis of the existing sewer system begins at MH #49, located in Wiley Canyon Road approximately 840 feet south of Wiley Canyon Road and Wabuska Street intersection and terminates at the Sanitation District trunk line located at approximately 240 feet south of Wiley Canyon Road and Orchard Village Road intersection at manhole #781.

SEWER CAPACITY ANALYSIS

The sewer capacity analysis performed for this project includes calculating the proposed flow due to The Wiley Canyon development. See Table 1 for capacity calculations for each Planning Area. See Appendix A for Kutter Formula Calculations for each capacity and pipe size. See Sewer Area Study Map in the pocket of this report for sewer layout and capacity calculations.

The analysis also includes the percentage full of the pipes within the system. For the basis of this study all proposed sewer lines are calculated using a 1.0% slope. The percentage full for each pipe segment is shown on the Sewer Area Study Map in the pocket of this report.

The total flowrate generated by Wiley Canyon project is 0.480 cfs and enters the existing 10" sewer (PC 7599) at manhole #49, this pipe segment from MH #49 to #48 will be at 29.1% full (see Appendix A for calculation and flow capacity table within exhibit 1, Appendix G).

Only one segment (segment 41) of the existing 18" sewer line is over 75% full (84.4%) based on theoretical flow calculations. A flow test was performed at three locations, upstream MH 28, upstream MH 780 and existing 18" trunk line manhole northeast of MH 781, see flow test results within Appendix D of this report. Due to the very low flowrate at the existing 18" trunk line manhole, further investigation was performed and found that there is a diversion structure at MH 781 which directed all upstream flow to an existing 24" CDS trunk line. Based on the flow test data (actual flowrate of 2.500 cfs) at the upstream MH 28 and accumulated flow from upstream tributary areas, total Q for segment 41 is now 6.107 cfs which is currently 39.9% full for this segment before entering the diversion structure.

CONCLUSION

The Wiley Canyon project generates a total flowrate of 0.480 cfs which enters the existing 10"-15" sewer lines (PC 7599), existing 12"-18" sewer lines (PC 7549) and then to existing 24" LA County Sanitary District trunk sewer line (diversion structure). The pipe sizing shown on the Sewer Area Study Map is adequate to meet the City of Santa Clarita standards for a maximum of 50% full for the existing 10" to 12" downstream pipe segments and meet standards for a maximum of 75% full for the existing 15" to 18" downstream pipe segments. Therefore, it can be concluded that the existing downstream sewer system is of adequate size and capacity to accept the proposed flow from this project.

LAND USE/ SEWER GENERATION TABLE

AREA I.D.	LAND USE	AREA OR NO. UNITS	PEAK Q COEF.	PEAK Q
1	COMMERCIAL	7.0 AC	0.015 cfs/AC	0.105 cfs
2	MULTI-FAMILY RESIDENTIAL	145 UNITS	0.001 cfs/AC	0.145 cfs
3	MULTI-FAMILY RESIDENTIAL	230 UNITS	0.001 cfs/AC	0.230 cfs

FLOW CAPACITY FOR PROPOSED DEVELOPMENT

BASIN ID	PROPOSED PIPE SIZE (in)	SLOPE (%)	FLOWRATE, Q* (cfs)	CAPACITY, Q* (cfs)	Q/Q* (% FULL) 12" ≤ PIPE	Q/Q* (% FULL) 15" ≥ PIPE
1	8	1.00	0.105	1.212	8.7	N/A
2	8	1.00	0.145	1.212	12.0	N/A
3	8	1.00	0.230	1.212	19.0	N/A
P1	8	1.00	0.480	1.212	36.9	N/A

(EXISTING, PROPOSED AND FUTURE DEVELOPMENT)
FLOW CAPACITY FOR ONSITE / DOWNSTREAM SEWERS
WILEY CANYON ROAD TO EXISTING 24" DIVERSION TRUNK SEWER AT MH #781

SEGMENT	MH TO MH	EXISTING PIPE SIZE (in)	SLOPE (%)	FLOWRATE, Q (cfs)	CAPACITY, Q* (cfs)	Q/Q* (% FULL) 12" < PIPE	Q/Q* (% FULL) 15" > PIPE	TRIBUTARY AREA	CALCULATED FLOW (cfs)	EXIST. PIPE ADEQUATE	NOTES
①	49-48	10	1.80	0.876	3.012	29.1	N/A	A1-A3, ONSITE P1	0.876	YES	
②	48-47	10	6.32	0.878	5.649	15.5	N/A	A3	0.002	YES	
③	47-43	10	1.00	0.881	2.243	39.3	N/A	A3	0.003	YES	
④	43-42	12	0.40	0.884	2.341	37.8	N/A	A3	0.004	YES	
⑤	42-36	12	0.40	0.889	2.341	38.0	N/A	A3	0.005	YES	
⑥	36-35	15	0.40	0.978	4.321	N/A	22.6	A3-A4	0.089	YES	
⑦	35-34	15	0.40	0.982	4.321	N/A	22.7	A3	0.004	YES	
⑧	34-33	15	0.40	0.983	4.321	N/A	22.7	A6	0.001	YES	
⑨	33-32	15	0.40	1.083	4.321	N/A	25.1	A5-A6	0.100	YES	
⑩	32-428	15	0.40	1.088	4.321	N/A	25.2	A6	0.005	YES	
⑪	428-427	12	2.56	1.092	5.946	18.4	N/A	A6	0.004	YES	
⑫	427-426	15	0.56	1.891	5.118	N/A	36.9	A7-A12	0.799	YES	
⑬	426-425	15	1.24	1.891	7.628	N/A	24.8			YES	
⑭	425-424	15	0.56	1.891	5.118	N/A	36.9			YES	
⑮	424-423	15	0.72	2.100	5.807	N/A	36.2	A13-A15	0.209	YES	
⑯	423-362	18	0.60	2.100	8.720	N/A	24.1			YES	
⑰	362-363	15	0.60	2.254	5.299	N/A	42.5	A17-A18	0.154	YES	
⑱	363-380	15	0.60	2.273	5.299	N/A	42.9	A18	0.019	YES	
⑲	380-381	15	0.60	2.277	5.299	N/A	43.0	A18	0.004	YES	
⑳	381-392	15	0.60	2.281	5.299	N/A	43.0	A18	0.004	YES	
㉑	393-394	12	2.00	2.286	5.255	43.5	N/A	A18	0.005	YES	
㉒	394-398	18	0.24	2.588	5.495	N/A	47.1	A18-A21	0.302	YES	
㉓	398-389	18	0.24	2.593	5.495	N/A	47.2	A21	0.005	YES	
㉔	389-387	18	0.24	2.621	5.495	N/A	47.7	A21	0.028	YES	
㉕	387-374	18	0.24	2.665	5.495	N/A	48.5	A21	0.044	YES	
㉖	374-372	18	0.24	2.726	5.495	N/A	49.6	A21	0.061	YES	
㉗	372-344	18	0.24	2.786	5.495	N/A	50.7	A21	0.060	YES	
㉘	344-345	18	0.76	2.845	9.819	N/A	29.0	A21	0.059	YES	
㉙	345-346	18	0.68	2.847	9.286	N/A	30.7	A21	0.002	YES	
㉚	346-349	18	0.40	2.862	7.111	N/A	40.2	A21	0.015	YES	
㉛	349-310	18	0.64	2.944	9.007	N/A	32.7	A22-A23	0.082	YES	
㉜	310-312	15	1.04	2.996	6.984	N/A	42.9	A23-A24	0.052	YES	
㉝	312-319	18	0.60	3.027	8.720	N/A	34.7	A24	0.031	YES	
㉞	319-320	18	0.64	3.054	9.007	N/A	33.9	A24	0.027	YES	
㉟	320-326	18	0.40	3.200	7.111	N/A	45.0	A24-A27	0.146	YES	
㊱	326-327	18	0.40	3.225	7.111	N/A	45.4	A24	0.025	YES	
㊲	327-328	18	0.40	3.227	7.111	N/A	45.4	A24	0.002	YES	
㊳	328-784	18	1.12	3.584	11.927	N/A	30.0	A28-A32	0.357	YES	
㊴	784-783	18	0.60	3.584	8.720	N/A	41.1			YES	
㊵	783-782	18	0.60	3.584	8.720	N/A	41.1			YES	
㊶	782-781 DIV. STRUC.	18	1.84	12.902 6.107**	15.295	N/A	84.4 39.9**	A33-A34	9.318 2.523**	NO YES**	ACTUAL FLOWRATE OF 2,500 cfs AT UPSTREAM MH #28 PER FLOW TEST PERFORMED IN JUNE 2020

* NOTE: MAXIMUM % FULL TO BE 50% FOR 12" PIPE OR SMALLER AND 75% FOR 15" PIPE OR LARGER.

** NOTE: ACUTAL FLOWREATE OF 2,500 cfs AT UPSTREAM MH #28 PER FLOW TEST PERFORMED IN JUNE 2020.

“WILL SERVE”



SANITATION DISTRICTS OF LOS ANGELES COUNTY

Converting Waste Into Resources

Robert C. Ferrante

Chief Engineer and General Manager

1955 Workman Mill Road, Whittier, CA 90601-1400
Mailing Address: P.O. Box 4998, Whittier, CA 90607-4998
(562) 699-7411 • www.lacsd.org

October 28, 2019

Ref. DOC 5335146

Ms. Ashley Holland, Project Coordinator
Alliance Land Planning & Engineering
2248 Faraday Avenue
Carlsbad, CA 92008

Dear Ms. Holland:

Will Serve Letter for Wiley Canyon Project

The Santa Clarita Valley Sanitation District (District) received your will serve letter request for the subject project on September 26, 2019. We offer the following comments regarding sewerage service:

1. The project area is outside the jurisdictional boundaries of the District and will require annexation into the District before sewerage service can be provided to the proposed development. For a copy of the District's Annexation Information and Processing Fee sheets, go to www.lacsd.org, Wastewater & Sewer Systems, Will Serve Program, and click on the appropriate link. For more specific information regarding the annexation procedure and fees, please contact Ms. Donna Curry at (562) 908-4288, extension 2708.
2. The wastewater flow originating from the proposed project will discharge to a local sewer line, which is not maintained by the District, for conveyance to either or both the District's Valencia Trunk Sewer, located in Orcharge Village Road east of Wiley Canyon Road, or the District No. 32 Main Trunk Sewer, located in a private right of way northeast of the intersection of Wiley Canyon Road and Orcharge Village Road. The District's 24-inch diameter Valencia Trunk Sewer has a capacity of 9.4 million gallons per day (mgd) and conveyed a peak flow of 1.9 mgd when last measured in 2018. The District's 18-inch diameter District No. 32 Main Trunk Sewer has a capacity of 3.3 mgd and conveyed a peak flow of 0.1 mgd when last measured in 2018.
3. The District operates two water reclamation plants (WRPs), the Saugus WRP and the Valencia WRP, which provide wastewater treatment in the Santa Clarita Valley. These facilities are interconnected to form a regional treatment system known as the Santa Clarita Valley Joint Sewerage System (SCVJSS). The SCVJSS has a capacity of 28.1 mgd and currently produces an average recycled water flow of 19.6 mgd.
4. The expected average wastewater flow from the project, described in the request as 150 assisted living units, 415 multi-family residential units, 10,000 square feet of commercial space and a 60,000 square-foot office building, is 100,105 gallons per day. For a copy of the District's average wastewater generation factors, go to www.lacsd.org, Wastewater & Sewer Systems, click on Will Serve Program, and click on the Table 1. Loadings for Each Class of Land Use link.

5. The District is empowered by the California Health and Safety Code to charge a fee for the privilege of connecting (directly or indirectly) to the District's Sewerage System for increasing the strength or quantity of wastewater discharged from connected facilities. This connection fee is a capital facilities fee that is imposed in an amount sufficient to construct an incremental expansion of the Sewerage System to accommodate the proposed project. Payment of a connection fee will be required before a permit to connect to the sewer is issued. For more information and a copy of the Connection Fee Information Sheet, go to www.lacsd.org, Wastewater & Sewer Systems, click on Will Serve Program, and search for the appropriate link. In determining the impact to the Sewerage System and applicable connection fees, the Districts' Chief Engineer and General Manager will determine the user category (e.g. Condominium, Single Family home, etc.) that best represents the actual or anticipated use of the parcel or facilities on the parcel. For more specific information regarding the connection fee application procedure and fees, please contact the Connection Fee Counter at (562) 908-4288, extension 2727.

6. In order for the District to conform to the requirements of the Federal Clean Air Act (CAA), the capacities of District wastewater treatment facilities are based on the regional growth forecast adopted by the Southern California Association of Governments (SCAG). Specific policies included in the development of the SCAG regional growth forecast are incorporated into clean air plans, which are prepared by the South Coast and Antelope Valley Air Quality Management Districts in order to improve air quality in the South Coast and Mojave Desert Air Basins as mandated by the CCA. All expansions of District facilities must be sized and service phased in a manner that will be consistent with the SCAG regional growth forecast for the counties of Los Angeles, Orange, San Bernardino, Riverside, Ventura, and Imperial. The available capacity of District treatment facilities will, therefore, be limited to levels associated with the approved growth identified by SCAG. As such, this letter does not constitute a guarantee of wastewater service, but is to advise you that the District intends to provide this service up to the levels that are legally permitted and to inform you of the currently existing capacity and any proposed expansion of District facilities.

If you have any questions, please contact the undersigned at (562) 908-4288, extension 2717.

Very truly yours,



Adriana Raza
Customer Service Specialist
Facilities Planning Department

AR:ar

cc: D. Curry
A. Schmidt
A. Howard

Appendix A

Flow Coefficients and Capacity Requirements



City of Santa Clarita
Engineering Services Division

SEWAGE FLOW COEFFICIENTS

ZONING		DESCRIPTION	COEFFICIENT
			(cfs/gross acreage)
Residential	RE	Residential Estate – large custom single family homes on uniquely configured lots	0.00075
	RVL	Residential Very Low Density - 1 DU/AC	0.001
	RL	Residential Low Density – 2.2 DU/AC	0.0015
	RS	Residential Suburban - 5 DU/AC	0.005
	RM	Residential Moderate – 11 DU/AC	0.012
	RMH	Residential Medium High – 20 DU/AC	0.015
	RH	Residential High – 28 DU/AC	0.023
<p><i>The above coefficients shall be used for undeveloped land, land that is not entitled, and apartment complexes. For developed land, and for entitled residential developments (except apartment complexes), a value of 0.001 cfs/dwelling unit shall be used in lieu of the above coefficients.</i></p>			
Agricultural	A	Agricultural - 1 single family home/ legal lot	0.0002
Mixed-Use	MU	existing zone + 16 dwelling units per acre	existing zone coefficient + 0.016
Open Space	OS	Open Space - Natural / Unimproved	0
		Open Space - Parks	0.0002
		Community Rooms	0.0005
		Community Pool Facilities	0.001
Commercial	CTC	Commercial Town Center	0.015
	CC	Community Commercial	
	CN	Commercial Neighborhood	
	CO	Commercial Office	
Industrial	VSR	Visitor Serving/Resort	0.021
	BP	Business Park	
	IC	Industrial Commercial	
	I	Industrial	
SP 3: Newhall Specific Plan	UG1	Urban General 1	0.005
	UG2	Urban General 2	0.012
	UC	Urban Center	0.015
	COR	Corridor	0.021
	CD	Creative District	0.021
	OS	Open Space	0
			(gal/student)
Schools	Elementary & Junior High Schools		25
	High School		37.5
	University & College		50
	College with dormitories		212.5

Estimated Average Daily Sewage Flows for Various Occupancies

Occupancy	Abbreviation	*Average daily flow	
Apartment Buildings:			
Bachelor or Single dwelling units	Apt	100	gal/D.U. → 150
1 bedroom dwelling units	Apt	150	gal/D.U. → 200
2 bedroom dwelling units	Apt	200	gal/D.U. → 250
3 bedroom or more dwelling units	Apt	250	gal/D.U. → use 300 GPD per SMD
Auditoriums, churches, etc.	Aud	5	gal/seat
Automobile parking	P	25	gal/1000 sq ft gross floor area
Bars, cocktails lounges, etc.	Bar	20	gal/seat
Commercial Shops & Stores	CS	100	gal/1000 sq ft gross floor area
Hospitals (surgical)	HS	500	gal/bed
Hospitals (convalescent)	HC	85	gal/bed
Hotels	H	150	gal/room
Medical Buildings	MB	300	gal/1000 sq ft gross floor area
Motels	M	150	gal/unit
Office Buildings	Off	200	gal/1000 sq ft gross floor area
Restaurants, cafeterias, etc.	R	50	gal/seat
Schools:			
Elementary or Jr. High	S	10	gal/student
High Schools	HS	15	gal/student
Universities or Colleges	U	20	gal/student
College Dormitories	CD	85	gal/student

*Multiply the average daily flow by 2.5 to obtain the peak flow

Zoning Coefficients

Zone	Coefficient (cfs/Acre)
Agriculture -----	0.001
Residential ⁺ :	
R-1 -----	0.004
R-2 -----	0.008
R-3 -----	0.012
R-4 -----	0.016*
Commercial:	
C-1 through C-4 -----	0.015*
Heavy Industrial:	
M1 through M-4 -----	0.021*

*Individual building, commercial or industrial plant capacities shall be the determining factor when they exceed the coefficients shown

+ Use 0.001 (cfs/unit) for condominiums only

Appendix B

Zoning and Land Use Data

City of Santa Clarita Zoning Descriptions

Chapter 17.31

ZONING DESIGNATION PURPOSE

Sections:

[17.31.010](#) Purpose.

[17.31.020](#) Consistency with the Zoning Map.

17.31.010 Purpose.

The non-urban, urban residential, commercial, industrial, mixed use, open space, public/institutional, specific plan, and corridor plan zones are established to achieve the following purposes:

- A. To reserve appropriate areas for the continuation of existing farms and ranches, residential living at a broad range of dwelling unit densities; for office uses, retail stores, service establishments, and wholesale businesses, offering commodities and services required by residents of the City and its surrounding market area; for industrial uses and the protection of these areas from intrusion by dwellings and other inharmonious uses consistent with the Santa Clarita General Plan and with sound standards to preserve public health, safety and welfare.
- B. To minimize traffic congestion and to avoid the overloading of public services and utilities by preventing the construction of buildings of excessive bulk or number in relation to the land area around them.
- C. To facilitate the provision of utility services and other public facilities commensurate with anticipated population, dwelling unit densities and service requirements.
- D. To promote high standards for site planning, architecture and landscape design for development within the City while preserving the City's historical and natural resources such as oak trees, river areas and ridgelines.
- E. To protect residential and commercial uses from noise, odor, dust, smoke, light intrusion, truck traffic and other objectionable influences and to prevent fire, explosion, radiation, and other hazards incidental to certain industrial activities.
- F. To ensure adequate light, air, privacy and open space for each dwelling and to provide sufficient open space around commercial and industrial structures to protect them from hazard and to minimize the impact of commercial and industrial operations on nearby residential zones.
- G. To encourage commercial and industrial uses to concentrate for the convenience of the public and for a more mutually beneficial relationship. (Ord. 13-8 § 4 (Exh. A), 6/11/13)

17.31.020 Consistency with the Zoning Map.

The zoning designations contained within this code shall correspond and be consistent with the zoning map as approved by Council ordinance. (Ord. 13-8 § 4 (Exh. A), 6/11/13)



CITY OF SANTA CLARITA
Zoning Map
 Last Update: September 2015

Urban Residential

- UR1 - 2.5 du/acre
- UR2 - 5.0 du/acre
- UR3 - 11.0 du/acre
- UR4 - 18.0 du/acre
- UR5 - 25.0 du/acre
- UR6 - 30.0 du/acre

Non-Urban Residential

- NR1 - 0.75 du/acre
- NR2 - 1.5 du/acre
- NR3 - 2.25 du/acre
- NR4 - 3.0 du/acre
- NR5 - 3.75 du/acre

Commercial

- CC - Community Commercial
- CH - Neighborhood Commercial
- CR - Regional Commercial

Open Space

- OS-NE - Open Space - Natural Forest
- OS-A - Open Space - Agriculture
- OS - Open Space
- OS-UM - Open Space - Survey of Land Management

Mixed Use

- CU - Cluster Plan
- MX-C - Mixed Use - Center
- MX-GL - Mixed Use - Neighborhood

Industrial

- BP - Business Park
- I - Industrial

Other

- PS - Public/Subsidiary

Specific Plan

- SP

Boundaries

- City Boundary
- Zoning Boundary

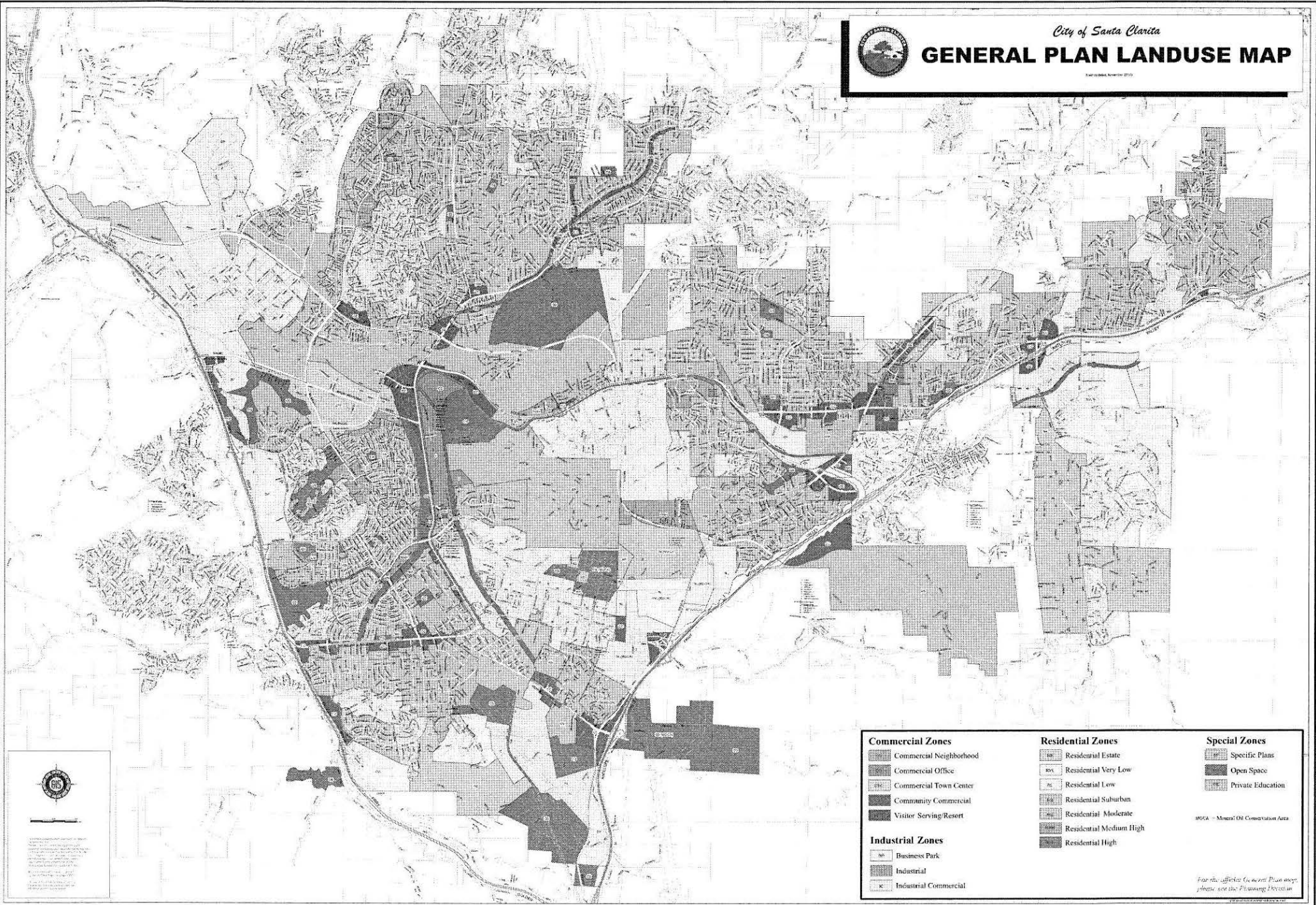
Map prepared by the City of Santa Clarita Planning Department. All rights reserved. No part of this map may be reproduced without the written permission of the City of Santa Clarita. Santa Clarita, CA 91350. 2015-09-01.



City of Santa Clarita

GENERAL PLAN LANDUSE MAP

February, November 2010




The City of Santa Clarita is committed to providing a high quality of life for its residents. This map is a key component of the City's General Plan, which guides the City's long-term development. The map shows the City's land use zones, which are designed to meet the needs of the community while protecting the environment and promoting economic growth.

Commercial Zones	Residential Zones	Special Zones
Commercial Neighborhood	Residential Estate	Specific Plans
Commercial Office	Residential Very Low	Open Space
Commercial Town Center	Residential Low	Private Education
Community Commercial	Residential Suburban	
Visitor Serving/Resort	Residential Moderate	
	Residential Medium High	
	Residential High	
Industrial Zones		
Business Park		
Industrial		
Industrial Commercial		

MCGA - Mineral Oil Conservation Area

For the official General Plan map, please visit the Planning Director's website.

Appendix C

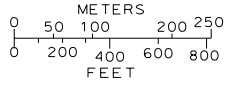
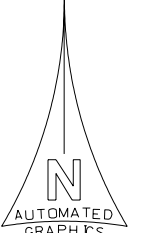
SMD Maps

SEE SHT. NO. N-1257

A-42
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THIS MAP IS INTENDED FOR USE ONLY AS OPERATIONS MAP BY LOS ANGELES COUNTY SEWER MAINTENANCE DISTRICTS. LOS ANGELES COUNTY EXPRESSLY DISCLAIMS ANY LIABILITY FOR ANY INACCURACIES WHICH MAY BE PRESENT IN THIS MAP.



LEGEND

- ○ ○ ○ ○ CLAY SEWERS MAINTAINED BY S.M.D. 8" UNLESS OTHERWISE NOTED
- ● ● ● ● PLASTIC SEWERS
- — — — — CONCRETE SEWERS
- — — — — CLAY SEWERS, LINED
- — — — — CEMENT SEWERS, LINED
- — — — — FORCE MAINS
- - - - - SEWERS NOT MAINTAINED BY SMD
- - - - - TRUNK SEWERS
- - - - - CITY BOUNDARY
- STANDARD MANHOLE
- △ DROP MANHOLE
- SHALLOW MANHOLE
- ◇ TRAP MANHOLE
- ⊕ WEIR MANHOLE
- C.D. ● CLEANOUT
- L.H. ● LAMP HOLE
- PUMP STATION

TOTAL MH'S THIS MAP: 649

SEE SHT. NO. 1219

SEE SHT. NO. N-1259

1

2

3

A

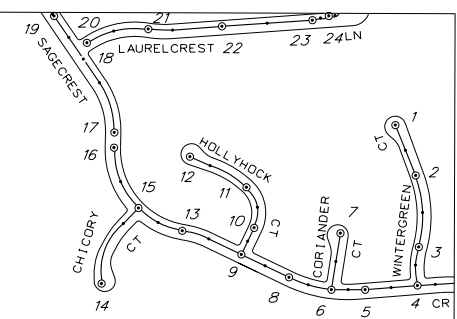
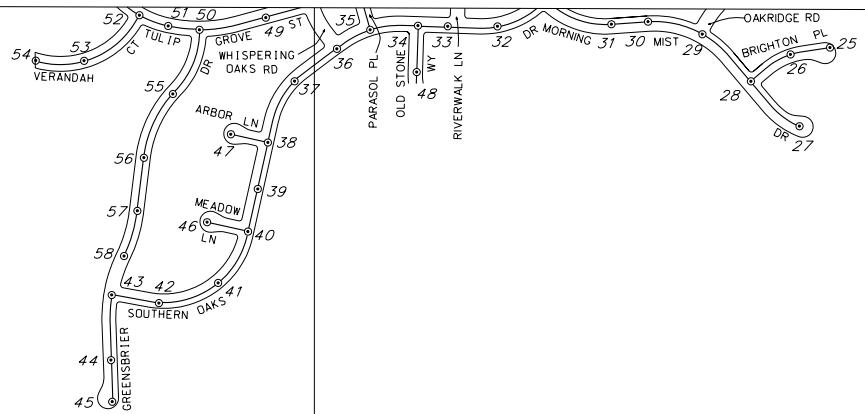
B

C

D

MAP REV
11-09-06
DATA BASE REV
05-16-88

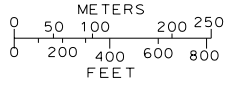
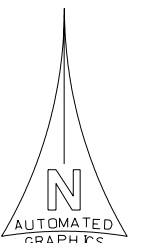
SEE SHT. NO. N-1258



THIS MAP IS INTENDED FOR USE ONLY AS OPERATIONS MAP BY LOS ANGELES COUNTY SEWER MAINTENANCE DISTRICTS. LOS ANGELES COUNTY EXPRESSLY DISCLAIMS ANY LIABILITY FOR ANY INACCURACIES WHICH MAY BE PRESENT IN THIS MAP.

LOS ANGELES COUNTY

SEE SHT. NO. N-1298



LEGEND

- CLAY SEWERS MAINTAINED BY SMD, 8" UNLESS OTHERWISE NOTED
- PLASTIC SEWERS
- CONCRETE SEWERS
- CLAY SEWERS, LINED
- CEMENT SEWERS, LINED
- FORCE MAINS
- - - SEWERS NOT MAINTAINED BY SMD
- - - TRUNK SEWERS
- - - CITY BOUNDARY
- STANDARD MANHOLE
- △ DROP MANHOLE
- SHALLOW MANHOLE
- ◇ TRAP MANHOLE
- ⊕ WEIR MANHOLE
- C.D. ● CLEANOUT
- L.H. ● LAMP HOLE
- PUMP STATION

TOTAL MH'S THIS MAP: 58

1220

E 4,107,000

N 4,241,500

1220

1

2

3

A

B

C

D

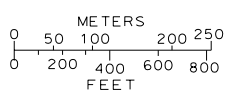
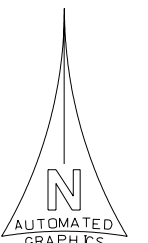
MAP REV
01-18-06
DATA BASE REV
08-01-88

SEE SHT. NO. N-1297

A-25
A-26
A-41
A-42



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LEGEND

- CLAY SEWERS MAINTAINED BY SMD, 8" UNLESS OTHERWISE NOTED
- PLASTIC SEWERS
- CONCRETE SEWERS
- CLAY SEWERS, LINED
- CEMENT SEWERS, LINED
- FORCE MAINS
- - - SEWERS NOT MAINTAINED BY SMD
- - - TRUNK SEWERS
- - - CITY BOUNDARY
- STANDARD MANHOLE
- △ DROP MANHOLE
- SHALLOW MANHOLE
- ◇ TRAP MANHOLE
- ⊕ WEIR MANHOLE
- C.O. CLEANOUT
- L.H. LAMP HOLE
- PUMP STATION

TOTAL MH'S THIS MAP: 416

SEE SHT. NO. N-1259

SEE SHT. NO. N-1337

E 4.117.000

N 4.241.500

1299

SEC 10

MAP REV 05-07-07
DATA BASE REV 05-20-93

Appendix D

Sewer Capacity Calculations

Worksheet for ONSITE 8" PIPE P1

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.01000	ft/ft
Normal Depth	0.620	ft
Diameter	0.667	ft

Results

Discharge	1.212	ft ³ /s
Flow Area	0.34	ft ²
Wetted Perimeter	1.74	ft
Hydraulic Radius	0.195	ft
Top Width	0.34	ft
Critical Depth	0.521	ft
Percent Full	93.0	%
Critical Slope	0.01259	ft/ft
Velocity	3.58	ft/s
Velocity Head	0.20	ft
Specific Energy	0.82	ft
Froude Number	0.63	
Maximum Discharge	1.21	ft ³ /s
Discharge Full	1.11	ft ³ /s
Slope Full	0.01194	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	92.954	%
Downstream Velocity	Infinity	ft/s

Worksheet for ONSITE 8" PIPE P1

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	0.620	ft
Critical Depth	0.521	ft
Channel Slope	0.01000	ft/ft
Critical Slope	0.01259	ft/ft

Worksheet for SEGMENT 1

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.01800	ft/ft
Normal Depth	0.775	ft
Diameter	0.833	ft

Results

Discharge	3.012	ft ³ /s
Flow Area	0.53	ft ²
Wetted Perimeter	2.17	ft
Hydraulic Radius	0.243	ft
Top Width	0.42	ft
Critical Depth	0.752	ft
Percent Full	93.0	%
Critical Slope	0.01821	ft/ft
Velocity	5.70	ft/s
Velocity Head	0.51	ft
Specific Energy	1.28	ft
Froude Number	0.90	
Maximum Discharge	3.01	ft ³ /s
Discharge Full	2.76	ft ³ /s
Slope Full	0.02145	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.037	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENT 1

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	0.775	ft
Critical Depth	0.752	ft
Channel Slope	0.01800	ft/ft
Critical Slope	0.01821	ft/ft

Worksheet for SEGMENT 2

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.06320	ft/ft
Normal Depth	0.775	ft
Diameter	0.833	ft

Results

Discharge	5.649	ft ³ /s
Flow Area	0.53	ft ²
Wetted Perimeter	2.17	ft
Hydraulic Radius	0.243	ft
Top Width	0.42	ft
Critical Depth	0.825	ft
Percent Full	93.0	%
Critical Slope	0.06855	ft/ft
Velocity	10.69	ft/s
Velocity Head	1.78	ft
Specific Energy	2.55	ft
Froude Number	1.69	
Maximum Discharge	5.65	ft ³ /s
Discharge Full	5.17	ft ³ /s
Slope Full	0.07545	ft/ft
Flow Type	SuperCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.037	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENT 2

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	0.775	ft
Critical Depth	0.825	ft
Channel Slope	0.06320	ft/ft
Critical Slope	0.06855	ft/ft

Worksheet for SEGMENT 3

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.01000	ft/ft
Normal Depth	0.775	ft
Diameter	0.833	ft

Results

Discharge	2.243	ft ³ /s
Flow Area	0.53	ft ²
Wetted Perimeter	2.17	ft
Hydraulic Radius	0.243	ft
Top Width	0.42	ft
Critical Depth	0.669	ft
Percent Full	93.0	%
Critical Slope	0.01187	ft/ft
Velocity	4.25	ft/s
Velocity Head	0.28	ft
Specific Energy	1.06	ft
Froude Number	0.67	
Maximum Discharge	2.24	ft ³ /s
Discharge Full	2.05	ft ³ /s
Slope Full	0.01190	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.037	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENT 3

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	0.775	ft
Critical Depth	0.669	ft
Channel Slope	0.01000	ft/ft
Critical Slope	0.01187	ft/ft

Worksheet for SEGMENTS 4-5

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.00400	ft/ft
Normal Depth	0.930	ft
Diameter	1.000	ft

Results

Discharge	2.341	ft ³ /s
Flow Area	0.76	ft ²
Wetted Perimeter	2.61	ft
Hydraulic Radius	0.292	ft
Top Width	0.51	ft
Critical Depth	0.655	ft
Percent Full	93.0	%
Critical Slope	0.00789	ft/ft
Velocity	3.08	ft/s
Velocity Head	0.15	ft
Specific Energy	1.08	ft
Froude Number	0.44	
Maximum Discharge	2.34	ft ³ /s
Discharge Full	2.15	ft ³ /s
Slope Full	0.00475	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.000	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENTS 4-5

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	0.930	ft
Critical Depth	0.655	ft
Channel Slope	0.00400	ft/ft
Critical Slope	0.00789	ft/ft

Worksheet for SEGMENTS 6-10

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.00400	ft/ft
Normal Depth	1.163	ft
Diameter	1.250	ft

Results

Discharge	4.321	ft ³ /s
Flow Area	1.19	ft ²
Wetted Perimeter	3.26	ft
Hydraulic Radius	0.365	ft
Top Width	0.64	ft
Critical Depth	0.842	ft
Percent Full	93.0	%
Critical Slope	0.00729	ft/ft
Velocity	3.63	ft/s
Velocity Head	0.20	ft
Specific Energy	1.37	ft
Froude Number	0.47	
Maximum Discharge	4.32	ft ³ /s
Discharge Full	3.97	ft ³ /s
Slope Full	0.00473	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.040	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENTS 6-10

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	1.163	ft
Critical Depth	0.842	ft
Channel Slope	0.00400	ft/ft
Critical Slope	0.00729	ft/ft

Worksheet for SEGMENT 11

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.02560	ft/ft
Normal Depth	0.930	ft
Diameter	1.000	ft

Results

Discharge	5.946	ft ³ /s
Flow Area	0.76	ft ²
Wetted Perimeter	2.61	ft
Hydraulic Radius	0.292	ft
Top Width	0.51	ft
Critical Depth	0.954	ft
Percent Full	93.0	%
Critical Slope	0.02581	ft/ft
Velocity	7.81	ft/s
Velocity Head	0.95	ft
Specific Energy	1.88	ft
Froude Number	1.13	
Maximum Discharge	5.95	ft ³ /s
Discharge Full	5.45	ft ³ /s
Slope Full	0.03034	ft/ft
Flow Type	SuperCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.000	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENT 11

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	0.930	ft
Critical Depth	0.954	ft
Channel Slope	0.02560	ft/ft
Critical Slope	0.02581	ft/ft

Worksheet for SEGMENTS 12, 14

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.00560	ft/ft
Normal Depth	1.163	ft
Diameter	1.250	ft

Results

Discharge	5.118	ft ³ /s
Flow Area	1.19	ft ²
Wetted Perimeter	3.26	ft
Hydraulic Radius	0.365	ft
Top Width	0.64	ft
Critical Depth	0.917	ft
Percent Full	93.0	%
Critical Slope	0.00815	ft/ft
Velocity	4.30	ft/s
Velocity Head	0.29	ft
Specific Energy	1.45	ft
Froude Number	0.55	
Maximum Discharge	5.12	ft ³ /s
Discharge Full	4.70	ft ³ /s
Slope Full	0.00662	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.040	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENTS 12, 14

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	1.163	ft
Critical Depth	0.917	ft
Channel Slope	0.00560	ft/ft
Critical Slope	0.00815	ft/ft

Worksheet for SEGMENT 13

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.01240	ft/ft
Normal Depth	1.163	ft
Diameter	1.250	ft

Results

Discharge	7.628	ft ³ /s
Flow Area	1.19	ft ²
Wetted Perimeter	3.26	ft
Hydraulic Radius	0.365	ft
Top Width	0.64	ft
Critical Depth	1.096	ft
Percent Full	93.0	%
Critical Slope	0.01289	ft/ft
Velocity	6.41	ft/s
Velocity Head	0.64	ft
Specific Energy	1.80	ft
Froude Number	0.83	
Maximum Discharge	7.63	ft ³ /s
Discharge Full	7.01	ft ³ /s
Slope Full	0.01466	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.040	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENT 13

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	1.163	ft
Critical Depth	1.096	ft
Channel Slope	0.01240	ft/ft
Critical Slope	0.01289	ft/ft

Worksheet for SEGMENT 15

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.00720	ft/ft
Normal Depth	1.163	ft
Diameter	1.250	ft

Results

Discharge	5.807	ft ³ /s
Flow Area	1.19	ft ²
Wetted Perimeter	3.26	ft
Hydraulic Radius	0.365	ft
Top Width	0.64	ft
Critical Depth	0.975	ft
Percent Full	93.0	%
Critical Slope	0.00911	ft/ft
Velocity	4.88	ft/s
Velocity Head	0.37	ft
Specific Energy	1.53	ft
Froude Number	0.63	
Maximum Discharge	5.81	ft ³ /s
Discharge Full	5.34	ft ³ /s
Slope Full	0.00851	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.040	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENT 15

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	1.163	ft
Critical Depth	0.975	ft
Channel Slope	0.00720	ft/ft
Critical Slope	0.00911	ft/ft

Worksheet for SEGMENTS 16, 33, 39-40

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.00600	ft/ft
Normal Depth	1.395	ft
Diameter	1.500	ft

Results

Discharge	8.720	ft ³ /s
Flow Area	1.71	ft ²
Wetted Perimeter	3.91	ft
Hydraulic Radius	0.438	ft
Top Width	0.77	ft
Critical Depth	1.143	ft
Percent Full	93.0	%
Critical Slope	0.00800	ft/ft
Velocity	5.09	ft/s
Velocity Head	0.40	ft
Specific Energy	1.80	ft
Froude Number	0.60	
Maximum Discharge	8.72	ft ³ /s
Discharge Full	8.03	ft ³ /s
Slope Full	0.00707	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.000	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENTS 16, 33, 39-40

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	1.395	ft
Critical Depth	1.143	ft
Channel Slope	0.00600	ft/ft
Critical Slope	0.00800	ft/ft

Worksheet for SEGMENTS 17-20

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.00600	ft/ft
Normal Depth	1.163	ft
Diameter	1.250	ft

Results

Discharge	5.299	ft ³ /s
Flow Area	1.19	ft ²
Wetted Perimeter	3.26	ft
Hydraulic Radius	0.365	ft
Top Width	0.64	ft
Critical Depth	0.933	ft
Percent Full	93.0	%
Critical Slope	0.00838	ft/ft
Velocity	4.45	ft/s
Velocity Head	0.31	ft
Specific Energy	1.47	ft
Froude Number	0.57	
Maximum Discharge	5.30	ft ³ /s
Discharge Full	4.87	ft ³ /s
Slope Full	0.00710	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.040	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENTS 17-20

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	1.163	ft
Critical Depth	0.933	ft
Channel Slope	0.00600	ft/ft
Critical Slope	0.00838	ft/ft

Worksheet for SEGMENT 21

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.02000	ft/ft
Normal Depth	0.930	ft
Diameter	1.000	ft

Results

Discharge	5.255	ft ³ /s
Flow Area	0.76	ft ²
Wetted Perimeter	2.61	ft
Hydraulic Radius	0.292	ft
Top Width	0.51	ft
Critical Depth	0.929	ft
Percent Full	93.0	%
Critical Slope	0.02002	ft/ft
Velocity	6.90	ft/s
Velocity Head	0.74	ft
Specific Energy	1.67	ft
Froude Number	1.00	
Maximum Discharge	5.25	ft ³ /s
Discharge Full	4.82	ft ³ /s
Slope Full	0.02381	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.000	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENT 21

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	0.930	ft
Critical Depth	0.929	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.02002	ft/ft

Worksheet for SEGMENTS 22-27

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.00240	ft/ft
Normal Depth	1.395	ft
Diameter	1.500	ft

Results

Discharge	5.495	ft ³ /s
Flow Area	1.71	ft ²
Wetted Perimeter	3.91	ft
Hydraulic Radius	0.438	ft
Top Width	0.77	ft
Critical Depth	0.904	ft
Percent Full	93.0	%
Critical Slope	0.00606	ft/ft
Velocity	3.21	ft/s
Velocity Head	0.16	ft
Specific Energy	1.55	ft
Froude Number	0.38	
Maximum Discharge	5.49	ft ³ /s
Discharge Full	5.06	ft ³ /s
Slope Full	0.00282	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.000	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENTS 22-27

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	1.395	ft
Critical Depth	0.904	ft
Channel Slope	0.00240	ft/ft
Critical Slope	0.00606	ft/ft

Worksheet for SEGMENT 28

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.00760	ft/ft
Normal Depth	1.395	ft
Diameter	1.500	ft

Results

Discharge	9.819	ft ³ /s
Flow Area	1.71	ft ²
Wetted Perimeter	3.91	ft
Hydraulic Radius	0.438	ft
Top Width	0.77	ft
Critical Depth	1.209	ft
Percent Full	93.0	%
Critical Slope	0.00902	ft/ft
Velocity	5.73	ft/s
Velocity Head	0.51	ft
Specific Energy	1.91	ft
Froude Number	0.68	
Maximum Discharge	9.82	ft ³ /s
Discharge Full	9.04	ft ³ /s
Slope Full	0.00896	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.000	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENT 28

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	1.395	ft
Critical Depth	1.209	ft
Channel Slope	0.00760	ft/ft
Critical Slope	0.00902	ft/ft

Worksheet for SEGMENT 29

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.00680	ft/ft
Normal Depth	1.395	ft
Diameter	1.500	ft

Results

Discharge	9.286	ft ³ /s
Flow Area	1.71	ft ²
Wetted Perimeter	3.91	ft
Hydraulic Radius	0.438	ft
Top Width	0.77	ft
Critical Depth	1.178	ft
Percent Full	93.0	%
Critical Slope	0.00849	ft/ft
Velocity	5.42	ft/s
Velocity Head	0.46	ft
Specific Energy	1.85	ft
Froude Number	0.64	
Maximum Discharge	9.29	ft ³ /s
Discharge Full	8.55	ft ³ /s
Slope Full	0.00801	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.000	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENT 29

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	1.395	ft
Critical Depth	1.178	ft
Channel Slope	0.00680	ft/ft
Critical Slope	0.00849	ft/ft

Worksheet for SEGMENTS 30, 35-37

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.00400	ft/ft
Normal Depth	1.395	ft
Diameter	1.500	ft

Results

Discharge	7.111	ft ³ /s
Flow Area	1.71	ft ²
Wetted Perimeter	3.91	ft
Hydraulic Radius	0.438	ft
Top Width	0.77	ft
Critical Depth	1.033	ft
Percent Full	93.0	%
Critical Slope	0.00687	ft/ft
Velocity	4.15	ft/s
Velocity Head	0.27	ft
Specific Energy	1.66	ft
Froude Number	0.49	
Maximum Discharge	7.11	ft ³ /s
Discharge Full	6.55	ft ³ /s
Slope Full	0.00471	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.000	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENTS 30, 35-37

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	1.395	ft
Critical Depth	1.033	ft
Channel Slope	0.00400	ft/ft
Critical Slope	0.00687	ft/ft

Worksheet for SEGMENTS 31, 34

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.00640	ft/ft
Normal Depth	1.395	ft
Diameter	1.500	ft

Results

Discharge	9.007	ft ³ /s
Flow Area	1.71	ft ²
Wetted Perimeter	3.91	ft
Hydraulic Radius	0.438	ft
Top Width	0.77	ft
Critical Depth	1.161	ft
Percent Full	93.0	%
Critical Slope	0.00824	ft/ft
Velocity	5.26	ft/s
Velocity Head	0.43	ft
Specific Energy	1.82	ft
Froude Number	0.62	
Maximum Discharge	9.01	ft ³ /s
Discharge Full	8.29	ft ³ /s
Slope Full	0.00754	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.000	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENTS 31, 34

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	1.395	ft
Critical Depth	1.161	ft
Channel Slope	0.00640	ft/ft
Critical Slope	0.00824	ft/ft

Worksheet for SEGMENT 32

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.01040	ft/ft
Normal Depth	1.163	ft
Diameter	1.250	ft

Results

Discharge	6.984	ft ³ /s
Flow Area	1.19	ft ²
Wetted Perimeter	3.26	ft
Hydraulic Radius	0.365	ft
Top Width	0.64	ft
Critical Depth	1.059	ft
Percent Full	93.0	%
Critical Slope	0.01132	ft/ft
Velocity	5.87	ft/s
Velocity Head	0.54	ft
Specific Energy	1.70	ft
Froude Number	0.76	
Maximum Discharge	6.98	ft ³ /s
Discharge Full	6.42	ft ³ /s
Slope Full	0.01231	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.040	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENT 32

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	1.163	ft
Critical Depth	1.059	ft
Channel Slope	0.01040	ft/ft
Critical Slope	0.01132	ft/ft

Worksheet for SEGMENT 38

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.01120	ft/ft
Normal Depth	1.395	ft
Diameter	1.500	ft

Results

Discharge	11.927	ft ³ /s
Flow Area	1.71	ft ²
Wetted Perimeter	3.91	ft
Hydraulic Radius	0.438	ft
Top Width	0.77	ft
Critical Depth	1.311	ft
Percent Full	93.0	%
Critical Slope	0.01170	ft/ft
Velocity	6.96	ft/s
Velocity Head	0.75	ft
Specific Energy	2.15	ft
Froude Number	0.82	
Maximum Discharge	11.93	ft ³ /s
Discharge Full	10.98	ft ³ /s
Slope Full	0.01320	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.000	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENT 38

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	1.395	ft
Critical Depth	1.311	ft
Channel Slope	0.01120	ft/ft
Critical Slope	0.01170	ft/ft

Worksheet for SEGMENT 41

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.01840	ft/ft
Normal Depth	1.395	ft
Diameter	1.500	ft

Results

Discharge	15.295	ft ³ /s
Flow Area	1.71	ft ²
Wetted Perimeter	3.91	ft
Hydraulic Radius	0.438	ft
Top Width	0.77	ft
Critical Depth	1.412	ft
Percent Full	93.0	%
Critical Slope	0.01841	ft/ft
Velocity	8.93	ft/s
Velocity Head	1.24	ft
Specific Energy	2.63	ft
Froude Number	1.05	
Maximum Discharge	15.30	ft ³ /s
Discharge Full	14.09	ft ³ /s
Slope Full	0.02169	ft/ft
Flow Type	SuperCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	93.000	%
Downstream Velocity	Infinity	ft/s

Worksheet for SEGMENT 41

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	1.395	ft
Critical Depth	1.412	ft
Channel Slope	0.01840	ft/ft
Critical Slope	0.01841	ft/ft

Worksheet for MH 28 DISCHARGE

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.00280	ft/ft
Normal Depth	0.917	ft
Diameter	1.500	ft

Results

Discharge	3.809	ft ³ /s
Flow Area	1.13	ft ²
Wetted Perimeter	2.69	ft
Hydraulic Radius	0.420	ft
Top Width	1.46	ft
Critical Depth	0.746	ft
Percent Full	61.1	%
Critical Slope	0.00550	ft/ft
Velocity	3.36	ft/s
Velocity Head	0.18	ft
Specific Energy	1.09	ft
Froude Number	0.67	
Maximum Discharge	5.94	ft ³ /s
Discharge Full	5.47	ft ³ /s
Slope Full	0.00137	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	61.133	%
Downstream Velocity	Infinity	ft/s

Worksheet for MH 28 DISCHARGE

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	0.917	ft
Critical Depth	0.746	ft
Channel Slope	0.00280	ft/ft
Critical Slope	0.00550	ft/ft

Worksheet for MH 780 DISCHARGE

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.00240	ft/ft
Normal Depth	1.098	ft
Diameter	1.500	ft

Results

Discharge	4.535	ft ³ /s
Flow Area	1.39	ft ²
Wetted Perimeter	3.08	ft
Hydraulic Radius	0.450	ft
Top Width	1.33	ft
Critical Depth	0.818	ft
Percent Full	73.2	%
Critical Slope	0.00571	ft/ft
Velocity	3.27	ft/s
Velocity Head	0.17	ft
Specific Energy	1.26	ft
Froude Number	0.56	
Maximum Discharge	5.49	ft ³ /s
Discharge Full	5.06	ft ³ /s
Slope Full	0.00194	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	73.200	%
Downstream Velocity	Infinity	ft/s

Worksheet for MH 780 DISCHARGE

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	1.098	ft
Critical Depth	0.818	ft
Channel Slope	0.00240	ft/ft
Critical Slope	0.00571	ft/ft

Worksheet for 18" TRUNK LINE MH DISCHARGE

Project Description

Friction Method	Kutter Formula
Solve For	Discharge

Input Data

Roughness Coefficient	0.013	
Channel Slope	0.00480	ft/ft
Normal Depth	0.088	ft
Diameter	1.500	ft

Results

Discharge	0.038	ft ³ /s
Flow Area	0.04	ft ²
Wetted Perimeter	0.73	ft
Hydraulic Radius	0.057	ft
Top Width	0.70	ft
Critical Depth	0.071	ft
Percent Full	5.8	%
Critical Slope	0.01260	ft/ft
Velocity	0.91	ft/s
Velocity Head	0.01	ft
Specific Energy	0.10	ft
Froude Number	0.66	
Maximum Discharge	7.79	ft ³ /s
Discharge Full	7.18	ft ³ /s
Slope Full	0.00000	ft/ft
Flow Type	SubCritical	

GVF Input Data

Downstream Depth	0.000	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.000	ft
Profile Description		
Profile Headloss	0.00	ft
Average End Depth Over Rise	0.00	%
Normal Depth Over Rise	5.833	%
Downstream Velocity	Infinity	ft/s

Worksheet for 18" TRUNK LINE MH DISCHARGE

GVF Output Data

Upstream Velocity	Infinity	ft/s
Normal Depth	0.088	ft
Critical Depth	0.071	ft
Channel Slope	0.00480	ft/ft
Critical Slope	0.01260	ft/ft

Flow Test Results



Alliance

MH at ~25457 Langston St

Santa Clarita, CA 91355

2020.06 Langston MH 28

MH # 28

Access:

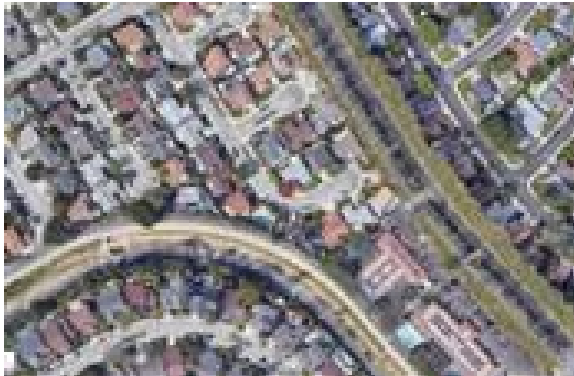
MH in northbound lane, north of address

System Type:

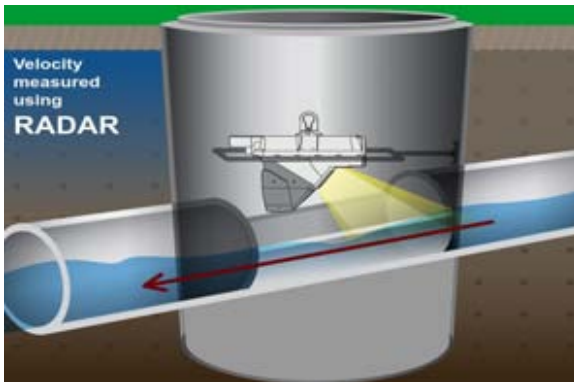
Sanitary Storm

Install Date: 6/04/2020

Map



Technology



Sewer Plan



Flow Meter

Meter Depth: 222"

MH Coordinates: 34.390918, -118.549891

Moderate open channel hydraulics; difficult to calibrate due to depth of MH

Avg Velocity	Avg Measured Level	Multiplier
2.0 fps	8.66"	1.0

Gas

O2	H2S	CO	LEL
20.9	0	0	0

Notes

No laterals; monitored the upstream line as it generally provides the best hydraulics.

Traffic Safety

No formal TCP required; used cones & signs per site-specific CA MUTCD TC requirements.

Land Use

Residential	Commercial	Industrial	Trunk
X			

Manhole Depth	247"
Monitored Pipe Size	18"
Inner Pipe Size (In/Out)	18"/18"
Pipe Shape	Round
Pipe Condition	Good
Manhole Material	Concrete
Silt	None observed
Velocity Profile Data	*
Velocity Profile Taken	0.4 2-D
Sensor Offset	24.32"
Sensor Dist. to Crown	6.32"
Sensor Direction	Upstream
Flow Heading	East



Meter Site Document

2020.06 Langston MH 28

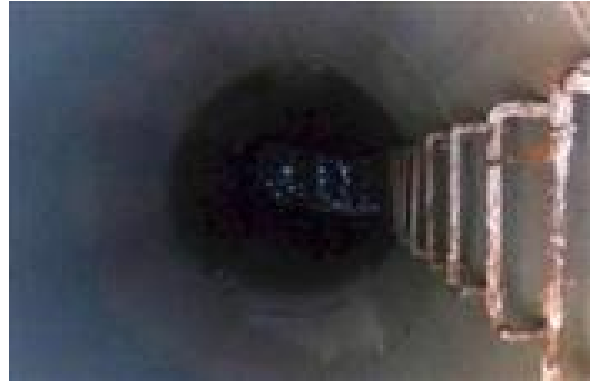
MH at ~25457 Langston St

Santa Clarita, CA 91355

Site



Manhole Before Install



Installation Process



Installed



Upstream



Downstream





601 N. Parkcenter Dr, Suite 209
Santa Ana, CA 92705

9314 Bond Av, Suite A
El Cajon, CA 92021

Utility Systems, Science and Software

Meter Start Date		From	6/4/2020
Meter Stop Date		To	6/12/2020
Velocity (fps)		Level (in)	Flow (mgd)
Average	1.935	8.732	1.123
Maximum	2.500	11.010	1.827
Minimum	0.940	5.070	0.278
Pipe Size		18.000	
Estimated Capacity (mgd)		Not Calculated	
Capacity Used		Not Calculated	
Sensor Type		Hach - Flodar	

Temporary Flow Study

Alliance

2020.06 Langston MH 28



Alliance

MH at ~25456 Orchard Village Rd

Santa Clarita, CA 91355

2020.06 Orchard Village MH 780

MH # 780

Access:

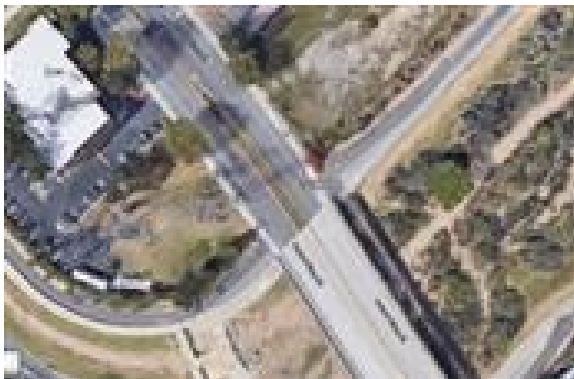
MH on sidewalk next to river trailhead, north of road

System Type:

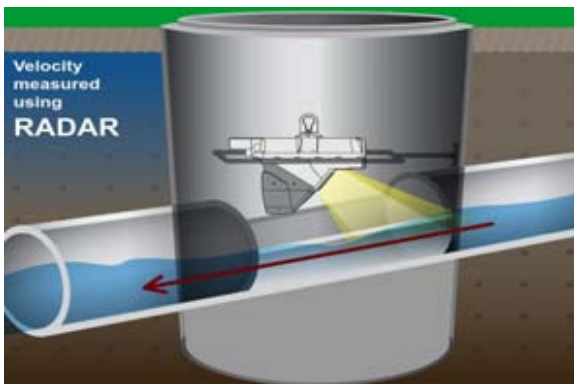
Sanitary Storm

Install Date: 6/04/2020

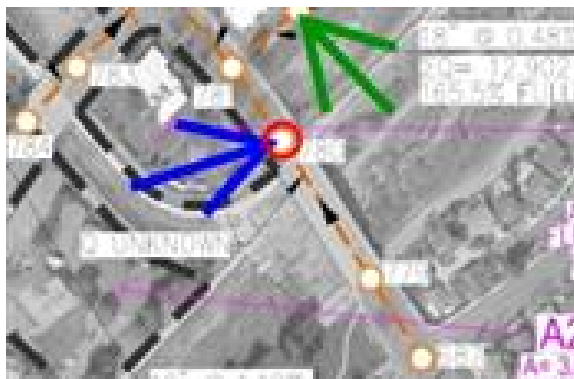
Map



Technology



Sewer Plan



Flow Meter

Meter Depth: 280"		
MH Coordinates: 34.389020, -118.546608		
Moderate open channel hydraulics; difficult to calibrate due to depth of MH		
Avg Velocity	Avg Measured Level	Multiplier
1.5 fps	10.75"	1.0

Gas

O2	H2S	CO	LEL
20.9	0	0	0

Notes

No laterals; monitored the upstream line as it generally provides the best hydraulics.

Traffic Safety

No formal TCP required; used cones & signs to designate work space for pedestrians.

Land Use

Residential	Commercial	Industrial	Trunk
X			

Manhole Depth	305"
Monitored Pipe Size	18"
Inner Pipe Size (In/Out)	18"/18"
Pipe Shape	Round
Pipe Condition	Good
Manhole Material	Concrete
Silt	1.5"
Velocity Profile Data	*
Velocity Profile Taken	0.4 2-D
Sensor Offset	25.11"
Sensor Dist. to Crown	7.11"
Sensor Direction	Upstream
Flow Heading	North



Meter Site Document

2020.06 Orchard Village MH 780

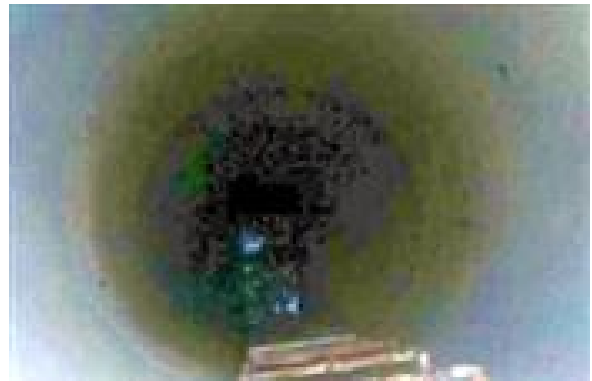
MH at ~25456 Orchard Village Rd

Santa Clarita, CA 91355

Site



Manhole Before Install



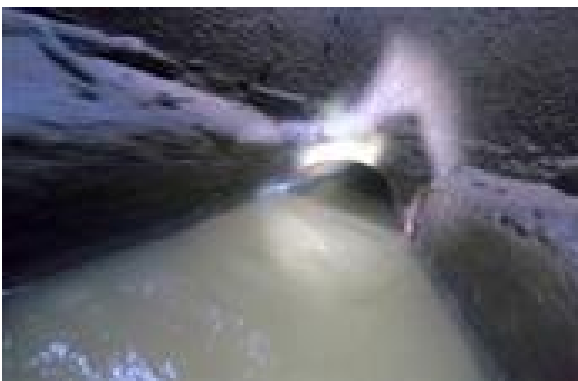
Installation Process



Installed



Upstream



Downstream





Utility Systems, Science and Software
 9314 Bond Av, Suite A
 El Cajon, CA 92021
 601 N. Parkcenter Dr, Suite 209
 Santa Ana, CA 92705

Meter Start Date		From	6/4/2020
Meter Stop Date		To	6/12/2020
Velocity (fps)		Level (in)	Flow (mgd)
Average	1.506	10.838	1.050
Maximum	2.230	13.180	1.794
Minimum	0.510	8.410	0.250
Pipe Size		18.000	
Estimated Capacity (mgd)		Not Calculated	
Capacity Used		Not Calculated	
Sensor Type		Hach - Flodar	

Temporary Flow Study
 Alliance
 2020.06 Orchard Village MH 780



Alliance

MH at ~23520 Wiley Canyon Rd

Santa Clarita, CA 91355

2020.06 Wiley Canyon MH

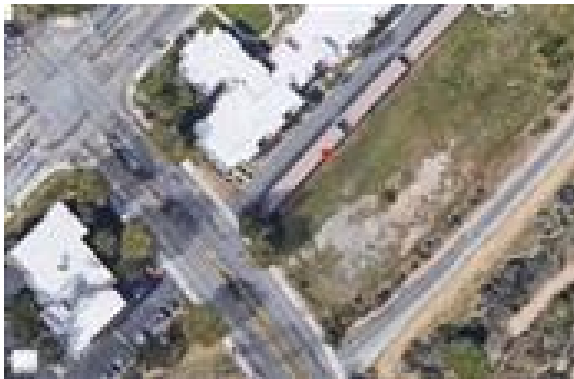
MH # unknown

Access:
MH in open space, east of address

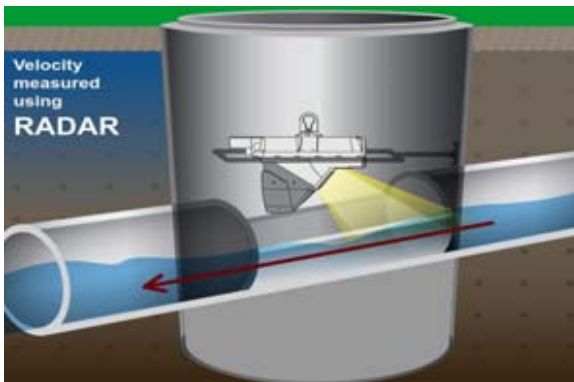
System Type:
Sanitary Storm

Install Date: 6/04/2020

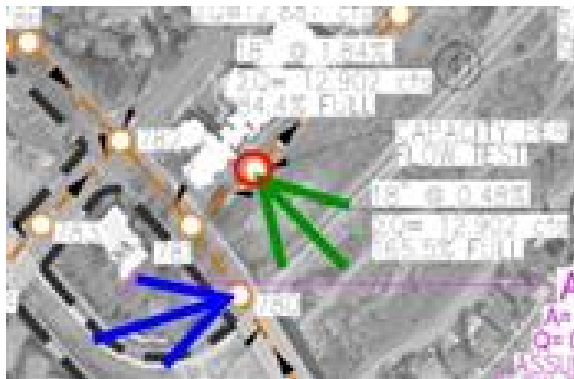
Map



Technology



Sewer Plan



Flow Meter

Meter Depth: 192"

MH Coordinates: 34.389601, -118.546630

Low to no flow; difficult to calibrate due to depth of MH & flow levels

Avg Velocity	Avg Measured Level	Multiplier
0.5 fps	0.25"	1.0

Gas

O2	H2S	CO	LEL
20.9	0	0	0

Notes

No laterals; monitored the upstream line as it generally provides the best hydraulics.

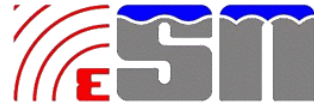
Traffic Safety

MH in open space, no traffic control required.

Land Use

Residential	Commercial	Industrial	Trunk
X			X

Manhole Depth	226"
Monitored Pipe Size	18"
Inner Pipe Size (In/Out)	18"/18"
Pipe Shape	Round
Pipe Condition	Good
Manhole Material	Concrete
Silt	Intermittent
Velocity Profile Data	*
Velocity Profile Taken	0.4 2-D
Sensor Offset	34.28"
Sensor Dist. to Crown	16.28"
Sensor Direction	Upstream
Flow Heading	East



Meter Site Document

2020.06 Wiley Canyon MH

MH at ~23520 Wiley Canyon Rd

Santa Clarita, CA 91355



Site



Manhole Before Install



Installation Process



Installed



Upstream



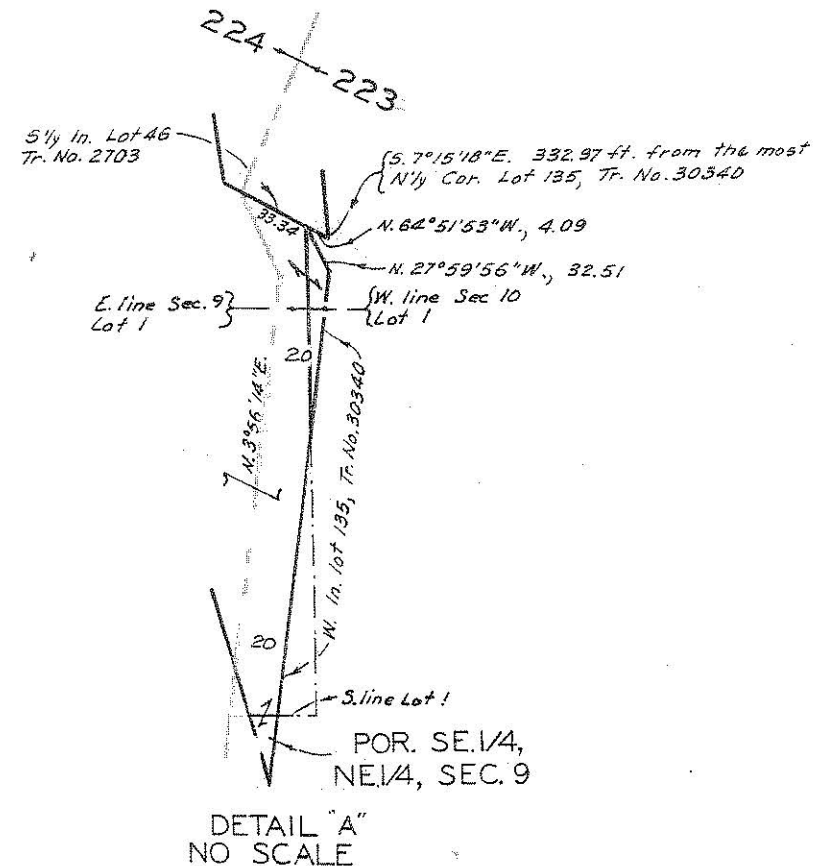
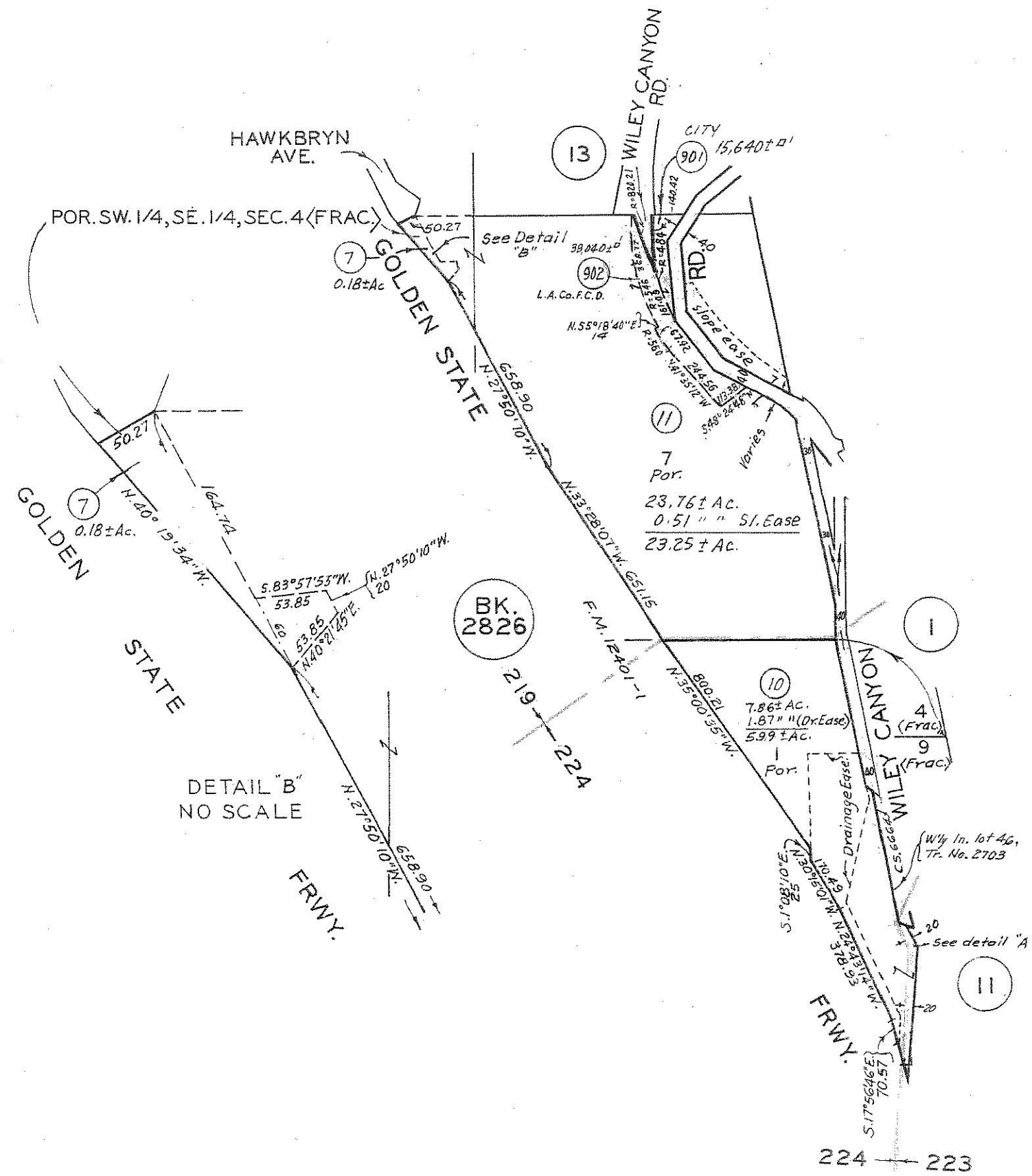
Downstream

Appendix E

APN/Tract Maps

7404
7407
7410
7410
7410
7410
771027
86062
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880321
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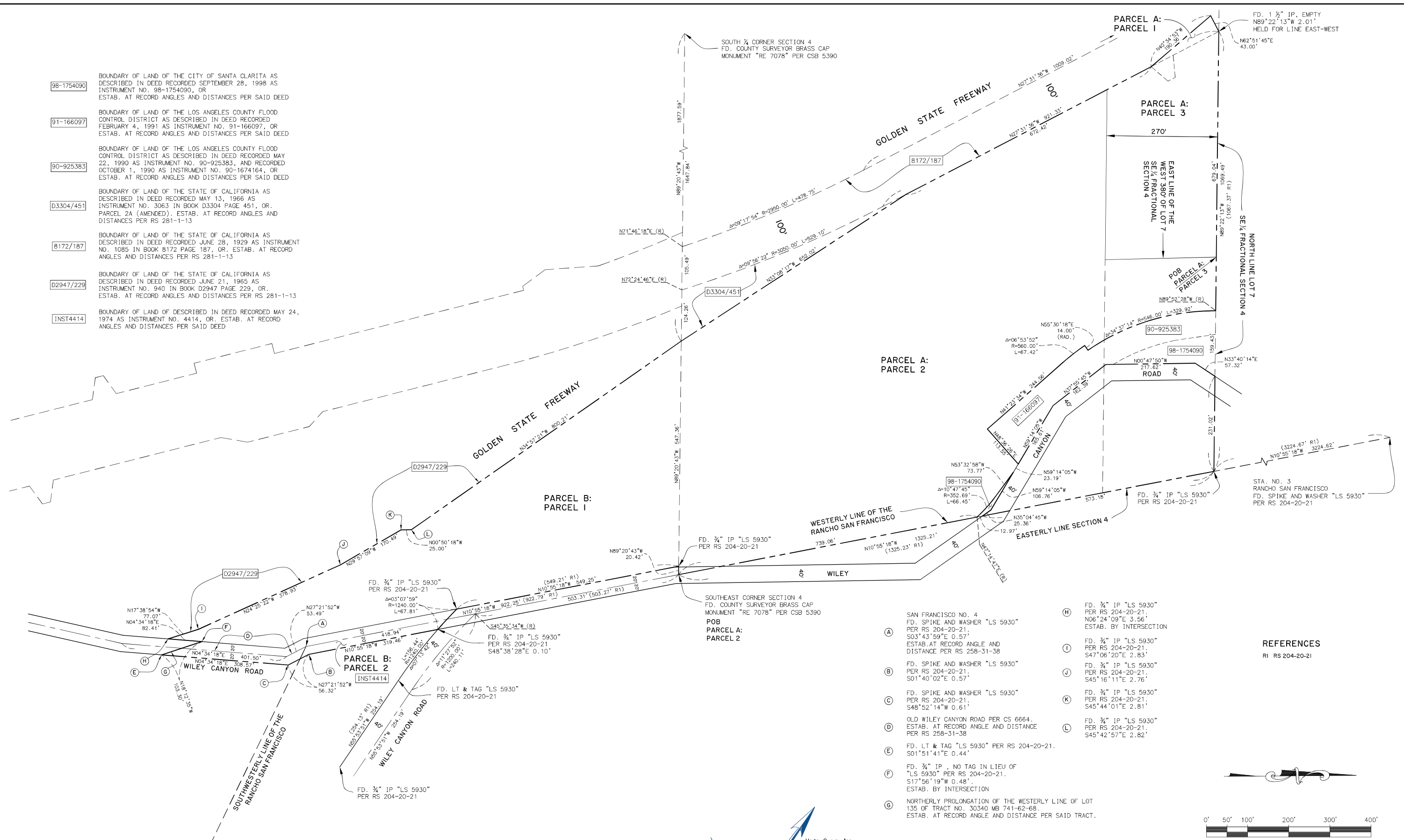


CODE
219
223
224

FDR PREV ASSMT SEC
2825 - 5 & 7

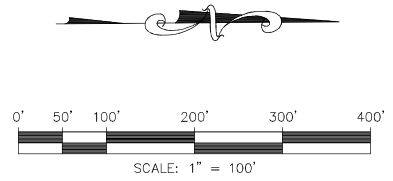
T.3N., R.16W.

- 98-1754090 BOUNDARY OF LAND OF THE CITY OF SANTA CLARITA AS DESCRIBED IN DEED RECORDED SEPTEMBER 28, 1998 AS INSTRUMENT NO. 98-1754090, OR ESTAB. AT RECORD ANGLES AND DISTANCES PER SAID DEED
- 91-166097 BOUNDARY OF LAND OF THE LOS ANGELES COUNTY FLOOD CONTROL DISTRICT AS DESCRIBED IN DEED RECORDED FEBRUARY 4, 1991 AS INSTRUMENT NO. 91-166097, OR ESTAB. AT RECORD ANGLES AND DISTANCES PER SAID DEED
- 90-925383 BOUNDARY OF LAND OF THE LOS ANGELES COUNTY FLOOD CONTROL DISTRICT AS DESCRIBED IN DEED RECORDED MAY 22, 1990 AS INSTRUMENT NO. 90-925383, AND RECORDED OCTOBER 1, 1990 AS INSTRUMENT NO. 90-1674164, OR ESTAB. AT RECORD ANGLES AND DISTANCES PER SAID DEED
- D3304/451 BOUNDARY OF LAND OF THE STATE OF CALIFORNIA AS DESCRIBED IN DEED RECORDED MAY 13, 1966 AS INSTRUMENT NO. 3063 IN BOOK D3304 PAGE 451, OR, PARCEL 2A (AMENDED), ESTAB. AT RECORD ANGLES AND DISTANCES PER RS 281-1-13
- 8172/187 BOUNDARY OF LAND OF THE STATE OF CALIFORNIA AS DESCRIBED IN DEED RECORDED JUNE 28, 1929 AS INSTRUMENT NO. 1085 IN BOOK 8172 PAGE 187, OR, ESTAB. AT RECORD ANGLES AND DISTANCES PER RS 281-1-13
- D2947/229 BOUNDARY OF LAND OF THE STATE OF CALIFORNIA AS DESCRIBED IN DEED RECORDED JUNE 21, 1965 AS INSTRUMENT NO. 940 IN BOOK D2947 PAGE 229, OR, ESTAB. AT RECORD ANGLES AND DISTANCES PER RS 281-1-13
- INST4414 BOUNDARY OF LAND OF DESCRIBED IN DEED RECORDED MAY 24, 1974 AS INSTRUMENT NO. 4414, OR, ESTAB. AT RECORD ANGLES AND DISTANCES PER SAID DEED



- (A) SAN FRANCISCO NO. 4 FD. SPIKE AND WASHER "LS 5930" PER RS 204-20-21, S03°43'59"E 0.57', ESTAB. AT RECORD ANGLE AND DISTANCE PER RS 258-31-38
- (B) FD. SPIKE AND WASHER "LS 5930" PER RS 204-20-21, S01°40'02"E 0.57'
- (C) FD. SPIKE AND WASHER "LS 5930" PER RS 204-20-21, S48°52'14"W 0.61'
- (D) OLD WILEY CANYON ROAD PER CS 6664, ESTAB. AT RECORD ANGLE AND DISTANCE PER RS 258-31-38
- (E) FD. LT & TAG "LS 5930" PER RS 204-20-21, S01°51'41"E 0.44'
- (F) FD. 3/4" IP, NO TAG IN LIEU OF "LS 5930" PER RS 204-20-21, S17°56'19"W 0.48', ESTAB. BY INTERSECTION
- (G) NORTHERLY PROLONGATION OF THE WESTERLY LINE OF LOT 135 OF TRACT NO. 30340 MB 741-62-68, ESTAB. AT RECORD ANGLE AND DISTANCE PER SAID TRACT.
- (H) FD. 3/4" IP "LS 5930" PER RS 204-20-21, N06°24'09"E 3.56' ESTAB. BY INTERSECTION
- (I) FD. 3/4" IP "LS 5930" PER RS 204-20-21, S47°06'20"E 2.83'
- (J) FD. 3/4" IP "LS 5930" PER RS 204-20-21, S45°16'11"E 2.76'
- (K) FD. 3/4" IP "LS 5930" PER RS 204-20-21, S45°44'01"E 2.81'
- (L) FD. 3/4" IP "LS 5930" PER RS 204-20-21, S45°42'57"E 2.82'

REFERENCES
RI RS 204-20-21



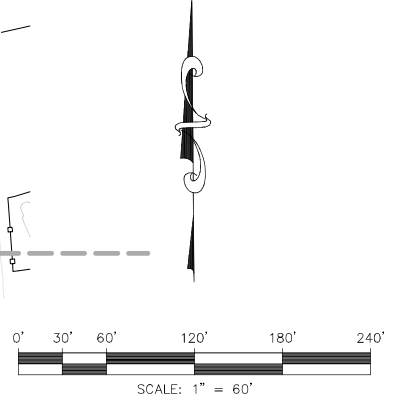
PREPARED FOR:
WILEY CYN LLC
13120 TELFAIR AVENUE
SYLMAR, CA 91342

NO.	REVISION	DATE	CHK'D BY





- LEGEND**
- BELL BELL SYSTEM MANHOLE
 - BM PAC BELL UNDERGROUND MARKER RISER
 - CH CONCRETE CHANNEL
 - GP GUY POLE
 - GW GUY WIRE
 - POL POINT ON CHAIN LINK FENCE
 - PP POWER POLE
 - TP TELEPHONE COMPANY PEDESTAL



Vertex Survey, Inc.
 28348 Constellation Road, Suite 800
 Santa Clarita, CA 91355
 Tel: 661-254-1928
 Fax: 661-254-1929

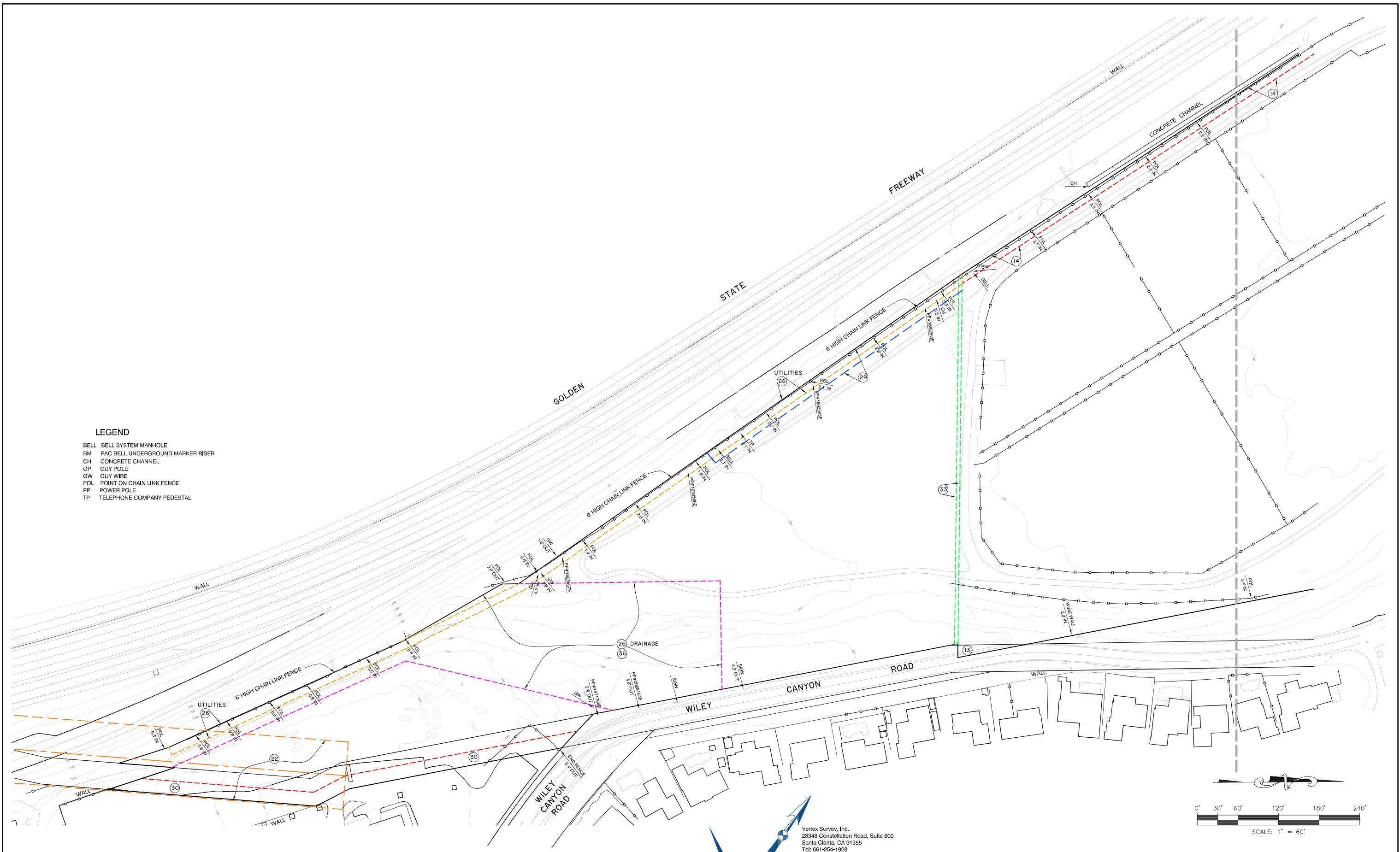
PREPARED FOR:

WILEY CYN LLC
 13120 TELFAIR AVENUE
 SYLMAR, CA 91342

NO.	REVISION	DATE	CHK'D BY

ALTA/NSPS LAND TITLE SURVEY
 FOR
SMISER PROPERTY

W.O.: _____ SCALE: 1" = 60' DATE: 05/28/2018 SHEET **3** OF **4** SHEETS

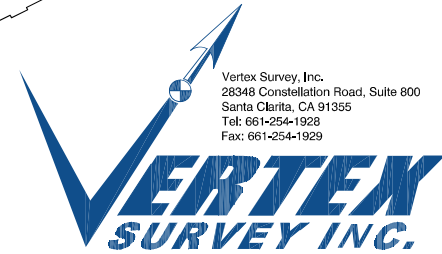


LEGEND

- BELL BELL SYSTEM MANHOLE
- BM PAC BELL UNDERGROUND MARKER RISER
- CH CONCRETE CHANNEL
- GP GUY POLE
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WILEY CYN LLC
 13120 TELFAIR AVENUE
 SYLMAR, CA 91342

NO.	REVISION	DATE	CHK'D BY



ALTA/NSPS LAND TITLE SURVEY
 FOR
 SMISER PROPERTY

W.O.: _____ SCALE: 1" = 60' DATE: 05/28/2018 SHEET 4 OF 4 SHEETS

OVERALL USE/FOOTAGE/FAR MATRIX

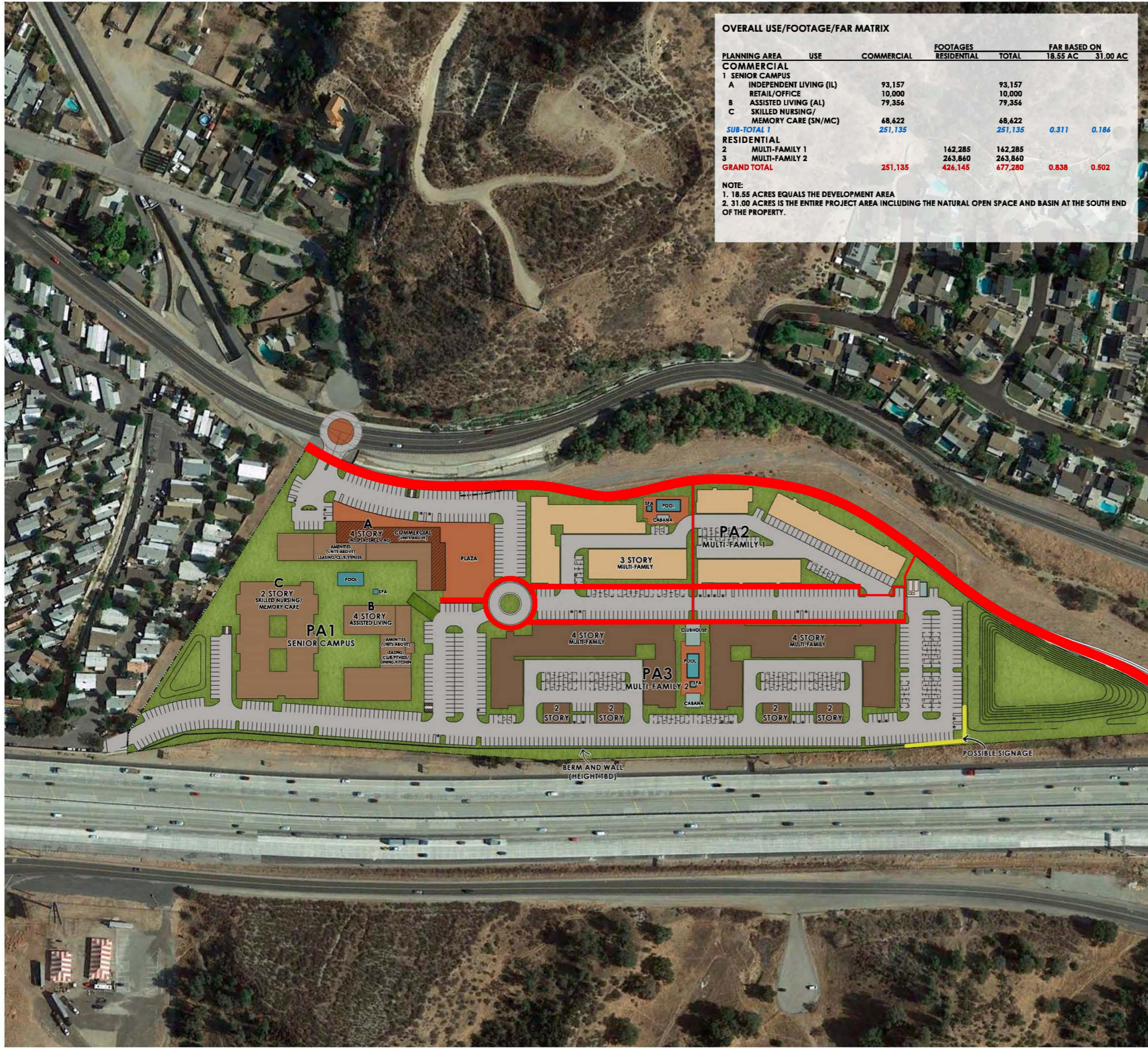
PLANNING AREA	USE	COMMERCIAL	FOOTAGES		FAR BASED ON	
			RESIDENTIAL	TOTAL	18.55 AC	31.00 AC
COMMERCIAL						
1 SENIOR CAMPUS						
A	INDEPENDENT LIVING (IL)	93,157		93,157		
	RETAIL/OFFICE	10,000		10,000		
B	ASSISTED LIVING (AL)	79,356		79,356		
C	SKILLED NURSING/MEMORY CARE (SN/MC)	48,622		48,622		
SUB-TOTAL 1		251,135		251,135	0.311	0.186
RESIDENTIAL						
2	MULTI-FAMILY 1		162,285	162,285		
3	MULTI-FAMILY 2		263,860	263,860		
GRAND TOTAL		251,135	426,145	677,280	0.838	0.502

NOTE:
 1. 18.55 ACRES EQUALS THE DEVELOPMENT AREA
 2. 31.00 ACRES IS THE ENTIRE PROJECT AREA INCLUDING THE NATURAL OPEN SPACE AND BASIN AT THE SOUTH END OF THE PROPERTY.

DETAILED PROGRAM/PARKING MATRIX

PA	USE	STORIES	UNITS/ ROOMS	ACRES	PROGRAM MIX	%	NRSF	PARKING REQ/PROP		PARKING PROVIDED	
								RATIO	NUMBER	GARAGE	CARPORT
COMMERCIAL											
1 SENIOR CAMPUS											
A	IL	4	87 DU		65 1 BEDROOM	75%	700				
					22 2 BEDROOM	25%	875				
					87 FLATS			1.0 SP/DU	87		87
								1 SP/8 DU GUEST	11		11
								1.0 SP/200 SF	50		50
									1		1
COMMERCIAL		GRND LVL			10,000 SF						
B	AL	4	90 DU/100 BEDS		DELIVERY VAN						
					10 STUDIOS	11%	450				
					70 1 BEDROOM	78%	550				
					10 2 BEDROOM	11%	750				
					90 FLATS			0.5 SP/DU	45		45
								1 SP/8 DU GUEST	12		12
								0.5 SP/RM	21		21
									1		1
SUB-TOTAL 1			177 DU/42 RMS	6.67					228		228
RESIDENTIAL											
2 MULTI-FAMILY											
		3 + MEZ			1 BEDROOM						
					23 CARRIAGE		725				
					18 ST FLATS		750				
					41 1 BEDRMS	28%		1.0 SP/DU	41		
					1 BED + MEZ						
					13 CARRIAGE		925				
					9 ST FLATS		950				
					22 1 BD+MEZ	15%		1.0 SP/DU	22		
					2 BEDROOM						
					35 CARRIAGE		950				
					6 ST FLATS		1050				
					41 2 BEDRMS	28%		2.0 SP/DU	82		
					2 BED + MEZ						
					29 CARRIAGE		1150				
					3 ST FLATS		1250				
					32 2 BD+MEZ	22%		2.0 SP/DU	64		
					3 BEDROOM						
					9 ST FLATS		1250				
					9 3 BEDRMS	6%		2.0 SP/DU	18		
					GUEST 145			0.50 SP/DU	73		
SUB-TOTAL 2		145	145	4.16			138,650		300	89	57
								(2.07 SP/DU)			154
											300
											(2.07 SP/DU)
3 MULTI-FAMILY											
		4+ MEZ			EFFICIENCY						
					24 ST FLATS		600				
					24 EFF	11%		1.0 SP/DU	24		
					EFF + MEZ						
					8 ST FLATS		800				
					8 EFF+MEZ	4%		1.0 SP/DU	8		
					1 BEDROOM						
					45 ST FLATS		750				
					45 1 BEDRMS	20%		1.0 SP/DU	45		
					1 BED + MEZ						
					15 ST FLATS		950				
					15 1 BD+MEZ	7%		1.0 SP/DU	15		
					2 BEDROOM						
					36 CARRIAGE		950				
					42 ST FLATS		1050				
					78 2 BEDRMS	34%		2.0 SP/DU	156		
					2 BED + MEZ						
					18 CARRIAGE		1150				
					14 ST FLATS		1250				
					32 2 BD+MEZ	14%		2.0 SP/DU	64		
					2 BEDROOM						
					8 REMOTE CARRIAGE		1100	2.0 SP/DU	16		
					3 BEDROOM						
					20 ST FLATS		1250				
					20 3 BEDRMS	9%		2.0 SP/DU	40		
					GUEST 230			0.50 SP/DU	115		
SUB-TOTAL 3		230	230	7.72			219,100		483	110	120
								(2.10 SP/DU)			253
											483
											(2.10 SP/DU)
GRAND TOTAL			552 + 42 ROOMS				18.55		1011	199	177
			(375 DU ARE MF RESIDENTIAL AND 177 ARE COMMERCIAL)								635
											1011

- NOTES:
1. ACCESSIBLE AND ELECTRIC VEHICLE PARKING PER STATE REQUIREMENTS ARE INCLUDED IN THE ABOVE COUNTS
 2. PA 1 A-C IS PROPOSED TO HAVE ALL OPEN PARKING
 3. PA 2 AND 3 ARE PROPOSED TO HAVE 1 SINGLE CAR GARAGE OR 1 CARPORT PER UNIT
 4. CARPORTS STALLS ARE PROPOSED TO BE 9' WIDE AND THE 9' DIMENSIONS INCLUDE THE STRUCTURAL SUPPORTS FOR THE CARPORTS
 5. IT IS PROPOSED THAT CARPORTS ARE NO ENCLOSED AND ARE OPEN ON THE SIDE AND REAR
 6. IT IS PROPOSED THAT LOCKABLE STORAGE FOR UNITS THAT DO NOT HAVE GARAGES CAN PROVIDE THE 250 CF OFF A PRIVATE DECK
 7. IT IS ASSUMED THE POSTAL PARKING SHALL BE INCLUDED IN THE OVERALL PARKING COUNT FOR EACH NEIGHBORHOOD
 8. THERE ARE 109 UNITS WITH MEZZANINES. IT IS ASSUMED THAT FOR PURPOSES OF PARKING THE MEZZANINES ARE NOT CONSIDERED BEDROOMS
 9. THE MEZZANINE SPACE FOR THE 109 UNITS HAVE AN AREA/OF 10,800 SF
 10. UNDER THE MXN CODE UP TO 187 PARKING STALLS CAN BE SHARED BETWEEN THE RESIDENTIAL AND COMMERCIAL USES (375 RESIDENTIAL UNITS X 0.5 SPACES/DU)
 11. IT IS ASSUMED THAT THE 200 SF OPEN SPACE REQUIREMENT PER UNIT WILL BE MET BY A COMBINATION OF PRIVATE OUTDOOR SPACE AND COMMON OPEN SPACE. THERE IS A 15' MINIMUM DIMENSION FOR THE COMMON OPEN SPACE TO BE COUNTABLE. WATER QUALITY BASINS, NATURAL WATER COURSE AND AREA EAST OF WILEY CANYON ROAD NOT COUNTABLE.
 12. IT IS ASSUMED THAT A 55' HEIGHT FOR PA 3 WILL BE ALLOWED SO THAT THE MEZZANINE SPACE WILL BE FEASIBLE
 13. IT IS ASSUMED THAT THE 10,000 SF COMMERCIAL (RETAIL/OFFICE USES) AND THE 68,622 SF SKILLED NURSING/MEMORY CARE USES WILL REQUIRE 1 DELIVERY VAN SPACE EACH AT 12' X 20'

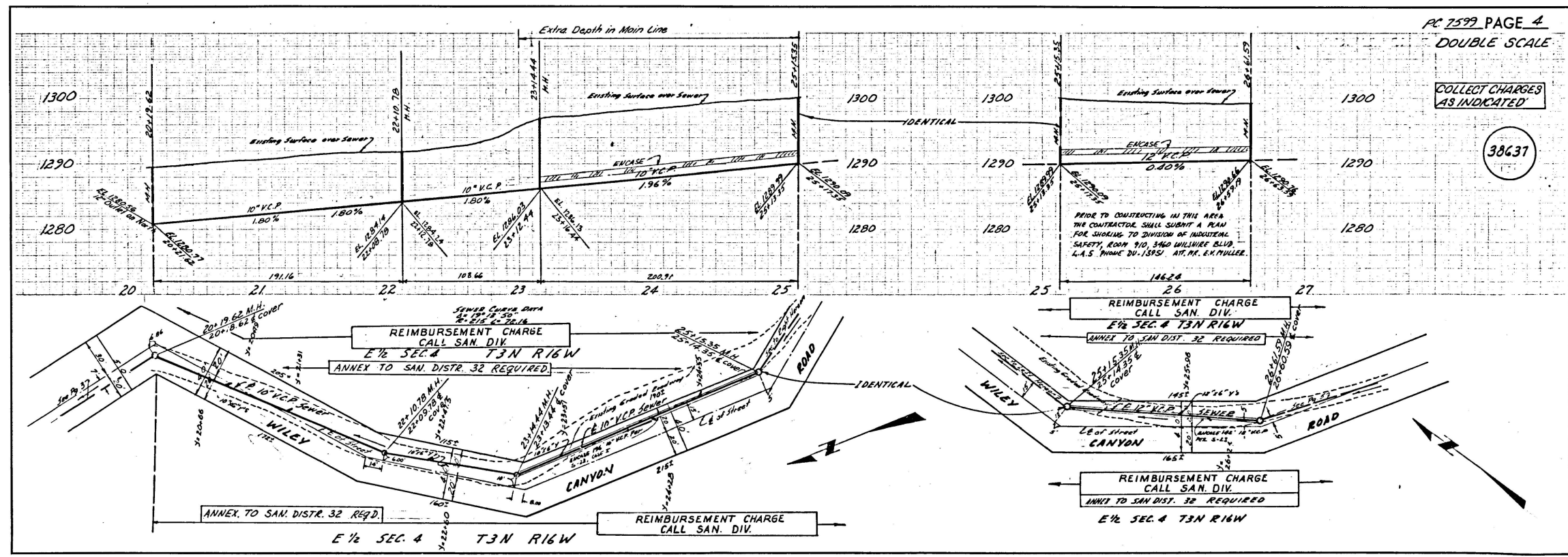
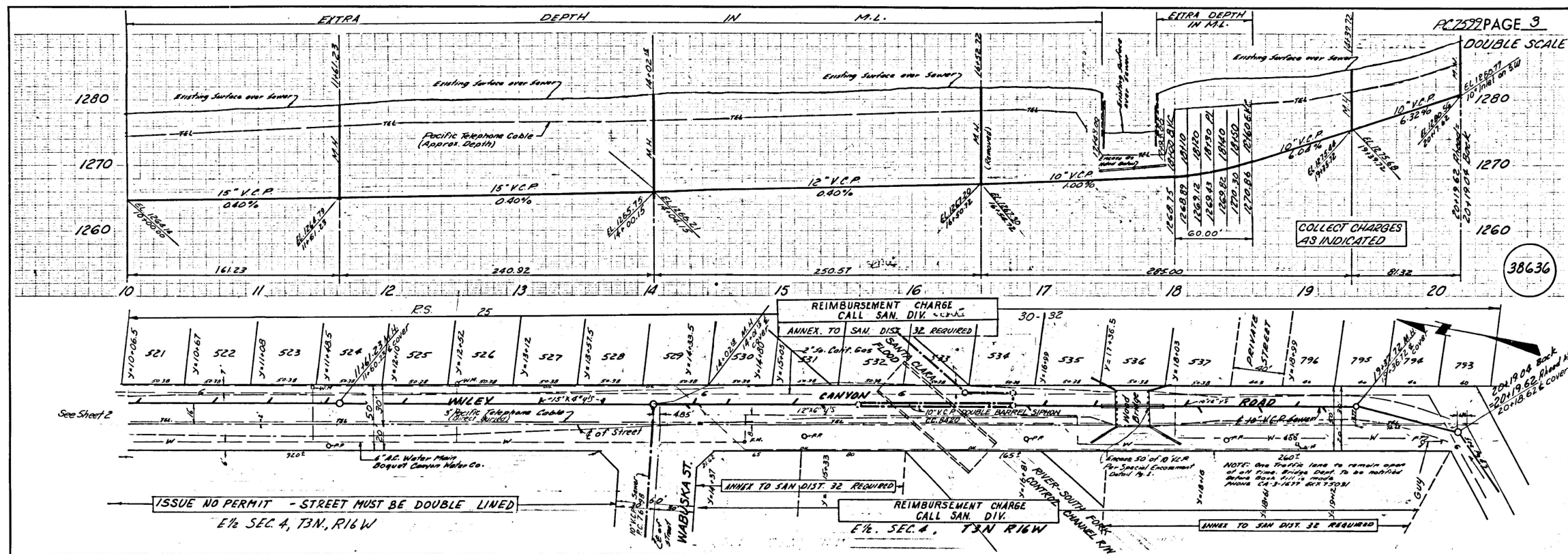


Appendix F

Sewer Plans

SEWER REFERENCE

PC 7599

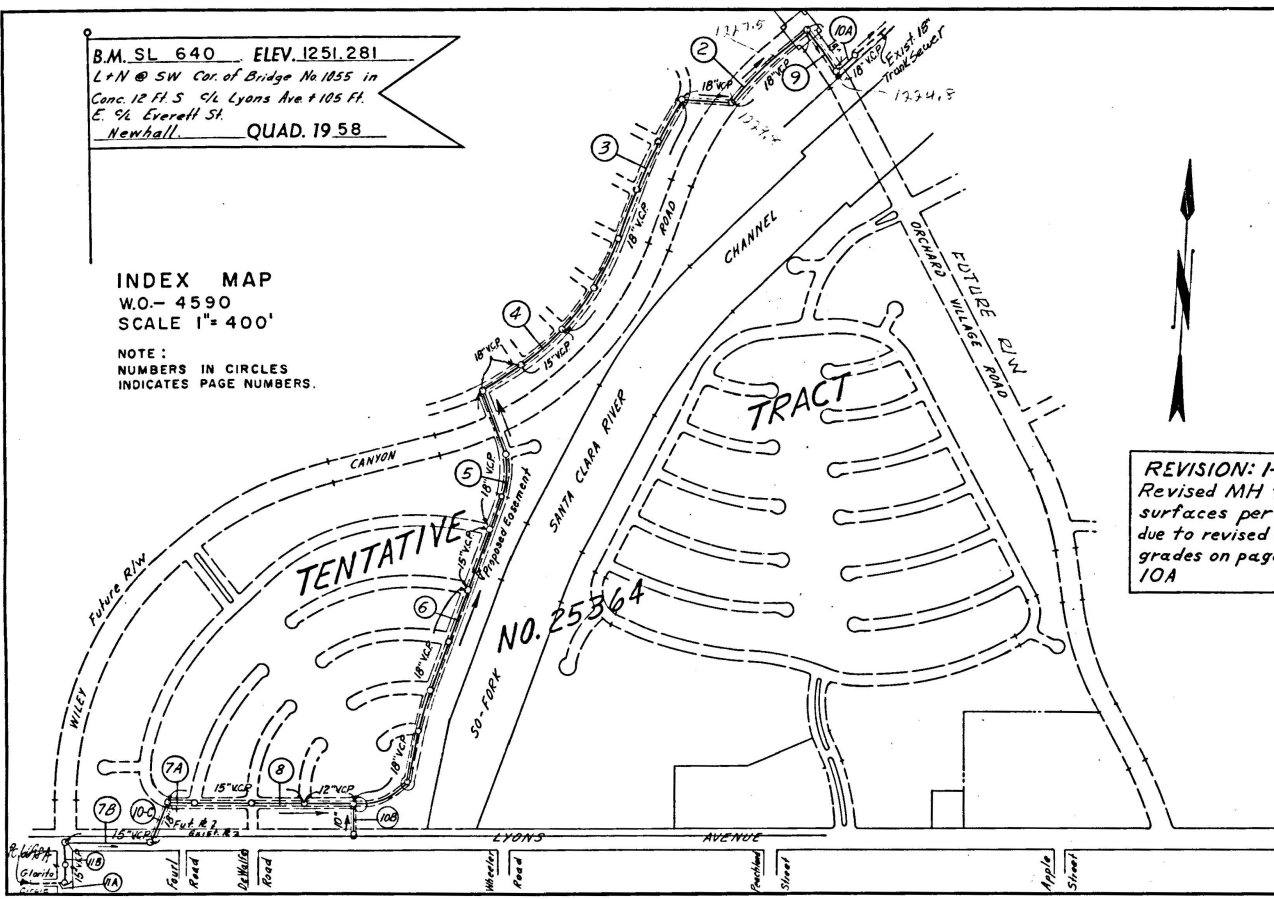


SEWER REFERENCE

PC 7549

B.M. SL 640 ELEV. 1251.281
 L+N @ SW Cor. of Bridge No 1055 in
 Conc. 12 Ft S 1/4 Lyons Ave 105 Ft.
 E. 1/4 Everett St.
 Newhall. QUAD. 19 58

INDEX MAP
 W.O.- 4590
 SCALE 1" = 400'
 NOTE:
 NUMBERS IN CIRCLES
 INDICATES PAGE NUMBERS.



REVISION: 1-31-69
 Revised MH tops &
 surfaces per PC8197
 due to revised street
 grades on page 2, 9, &
 10A

COLLECT CHARGES
 AS INDICATED
Ralph Schaper

DOUBLE SCALE
 NEW HALL BLDG. DIST. NO. 8.2

PROFILE ALIGNMENT AND GRADE OF
 SANITARY SEWERS
 P.C. 7549
 PAGE 1

NORTH OF LYONS AVENUE
 PRIVATE CONTRACT NO. 7549

W.S. 62
 6 SHEETS, 12 PAGES
 SCALE: HORIZ. 1" = 40' VERT. 1" = 4'
 FEBRUARY, 1966
 PREPARED IN THE OFFICES OF
 VOORHEIS-TRINDLE CO.

38652

BY: *J.P. Nelson*
 REG. C. E. NO. 8781

FOR LEGEND SEE PLAN NO. S-1

- PROVIDE STAKES ON THE PROPERTY LINE OR PROPERTY LINES PRODUCED AT RIGHT ANGLES TO THE SEWER LINE AT THE CENTER LINE OF EACH MANHOLE.
- NO REPRESENTATIVE OF THE COUNTY ENGINEER WILL SURVEY OR LAY OUT ANY PORTION OF THE WORK.
- THE PRIVATE ENGINEER SHALL FURNISH THE COUNTY ENGINEER WITH GRADE SHEETS AND STATIONING FOR ALL HOUSE LATERALS AND Y BRANCHES AND SHALL PROVIDE STAKES FOR THEM AT THEIR PROPER LOCATIONS WITH STATIONING PLAINLY MARKED. ALL HOUSE LATERALS SHALL BE CONSTRUCTED AT A STRAIGHT ALIGNMENT TO THE MAIN LINE SEWER EXCEPT AS SHOWN ON THE PLANS. HOUSE LATERALS FROM CHIMNEYS SHALL HAVE AN ANGLE OF LESS THAN 45° WITH THE MAIN LINE SEWER. ANY CHANGE IN ALIGNMENT SHALL BE REQUESTED IN WRITING BY THE PRIVATE ENGINEER.
- THE PRIVATE ENGINEER SHALL FURNISH THE HOUSE LATERAL DEPTH AT THE PROPERTY LINE BELOW THE TOP OF CURB ELEVATION FOR EACH HOUSE LATERAL ON THE GRADE SHEET.
- NO REVISIONS SHALL BE MADE IN THESE PLANS WITHOUT THE APPROVAL OF THE COUNTY ENGINEER.
- THE CONTRACTOR SHALL NOTIFY THE CONSTRUCTION DIVISION BY TELEPHONE, MADISON 9-4747, EXT. 8133, AT LEAST TWENTY-FOUR HOURS BEFORE STARTING ANY WORK UNDER THIS CONTRACT.
- MANHOLES SHALL BE BRICK SEWER STRUCTURES PER S-23 - PRECAST CONCRETE MANHOLES PER S-5 OR S-6 MAY BE USED AS AN ALTERNATE IN LOCATIONS APPROVED BY THE COUNTY ENGINEER.
- USE STANDARD MANHOLE FRAMES AND COVERS, S-15, EXCEPT AS NOTED.
- MANHOLE TOPS IN UNIMPROVED RIGHTS-OF-WAY TO BE SIX INCHES ABOVE FINISHED GRADE.
- USE EXTRA STRENGTH PIPE. ALL PIPE IS STANDARD DEPTH EXCEPT AS NOTED.
- USE EXTRA STRENGTH OR MECHANICAL COMPRESSION JOINTS FOR ALL V.C.P. JOINTS PER SPECS. 36 & 48.
- WHERE OVER EXCESSIVE WATER IS ENCOUNTERED DURING THE CONSTRUCTION OF THE SEWER, THE V.C.P. JOINTS SHALL BE OF COMPOUND JOINT TYPE, SUCH AS S-18, OR MECHANICAL-COMPRESSION JOINTS PER SPECS. 36-23.
- IF A POWER POLE IS WITHIN THREE FEET OF THE SEWER, THE SEWER SHALL BE ENCASED, PER S-23 TWO FEET ON EACH SIDE FROM THE POINT OF INTERFERENCE.
- IF DURING THE COURSE OF CONSTRUCTION IT IS DETERMINED THAT THERE IS LESS THAN FOUR FEET OF COVER OVER THE TOP OF A MAIN LINE OR HOUSE LATERAL V.C.P. SEWER WHICH IS NOT INDICATED ON THE PLANS, THE PIPE SHALL BE ENCASED PER S-23 UNLESS OTHERWISE APPROVED BY THE COUNTY ENGINEER.
- HOUSE LATERALS TO BE CONSTRUCTED WITHIN DIVISIONS OF PROPERTY LINE - PER S-23 - ENCASED - PER S-23 - UNLESS OTHERWISE APPROVED BY THE COUNTY ENGINEER.
- RESURFACE ALL TRENCHES WITHIN PAVED AREAS TO MEET L.A. COUNTY ROAD DEPT. OR CALIFORNIA STATE HIGHWAY REQUIREMENTS IN ACCORDANCE WITH PERMITS.
- FOR ALLOWABLE LEAKAGE TEST USE FORMULA NO. 1, SPECS. SEC. 54.
- ALL STATE AND LOCAL TRENCH SAFETY ORDERS WILL BE RIGIDLY ENFORCED.
- ALL JOINTS BETWEEN CAST IRON PIPE AND VITRIFIED CLAY PIPE SHALL BE MADE WITH A RUBBER SLEEVE JOINT (WITH HOODS IF NECESSARY) PER STANDARD SPECIFICATIONS, SEC. 36, OR APPROVED EQUAL.

NOTE:
 GRADES TO WHICH THIS IMPROVEMENT IS TO BE CONSTRUCTED ARE SHOWN ON PLANS AND PROFILES. GRADE POINTS FOR TOP OF CURBS, CENTER LINE OF STREETS, OR CENTER LINE OF ALLEYS ARE SHOWN BY CIRCLES ON PROFILES. GRADE POINTS FOR TOP OF POINTS THE GRADE SHALL BE ESTABLISHED SO AS TO CONFORM TO A STRAIGHT LINE DRAWN BETWEEN SAID DESIGNATED POINTS. ELEVATIONS ARE IN FEET ABOVE U.S.C. & G.S. SEA LEVEL DATUM OF 1929.
 THIS DRAWING AND THE DATA HEREON ARE HEREBY MADE A PART OF THE SPECIFICATIONS. BEFORE WORK CAN BE STARTED, THE CONTRACTOR MUST OBTAIN A PERMIT TO EXCAVATE IN COUNTY STREETS FROM THE L.A. COUNTY ROAD DEPT., DISTRICT OFFICE NO. 5, AND PAY A FEE TO THE COUNTY ENGINEER, ROOM 300, COUNTY ENGINEERING BUILDING, THE WEST SECOND STREET OFFICE, ANTELOPE VALLEY REGIONAL OFFICE, SUFFICIENT TO COVER THE COST OF CONSTRUCTION INSPECTION AND RECORD PLANS.
 APPROVAL OF THIS PLAN BY THE COUNTY OF LOS ANGELES DOES NOT CONSTITUTE A REPRESENTATION AS TO THE ACCURACY OF THE LOCATION OR OF THE EXISTENCE OR NON-EXISTENCE OF ANY UNDERGROUND UTILITY PIPE OR STRUCTURE WITHIN THE LIMITS OF THIS PROJECT. THIS NOTE APPLIES TO ALL PAGES.
 IF WORK IS TO BE DONE IN A STATE HIGHWAY, A PERMIT MUST BE OBTAINED FROM THE STATE OF CALIFORNIA, DIVISION OF HIGHWAYS, 120 SOUTH SPRING STREET, LOS ANGELES, CALIFORNIA.

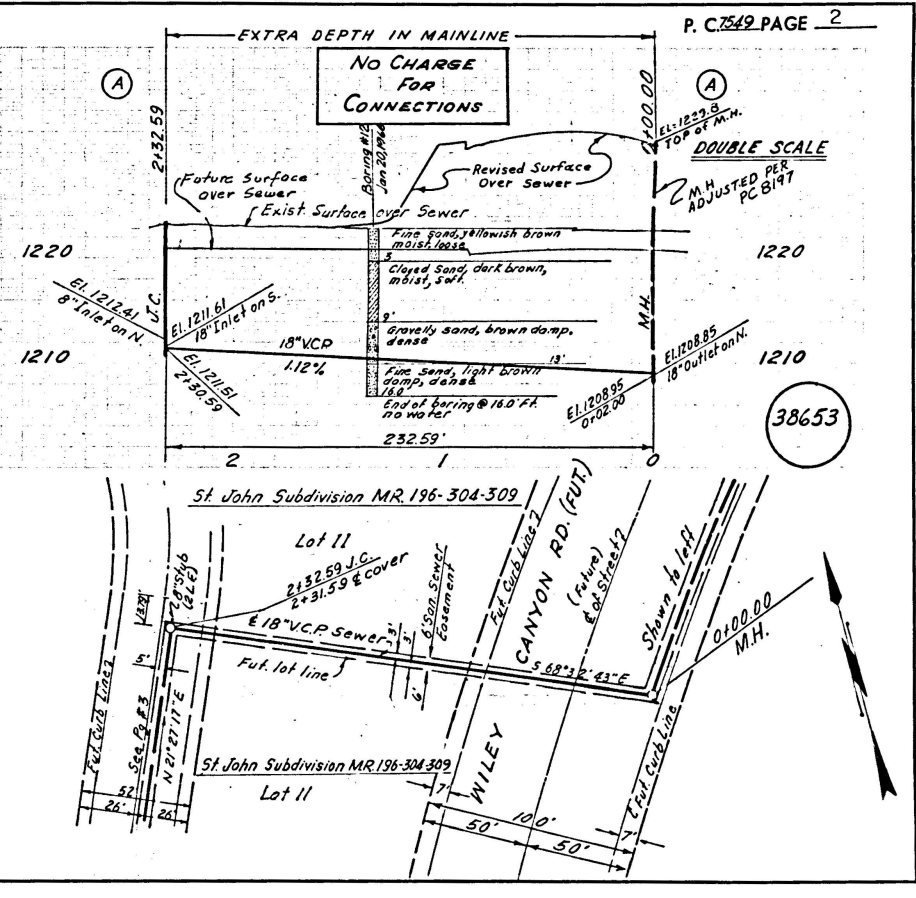
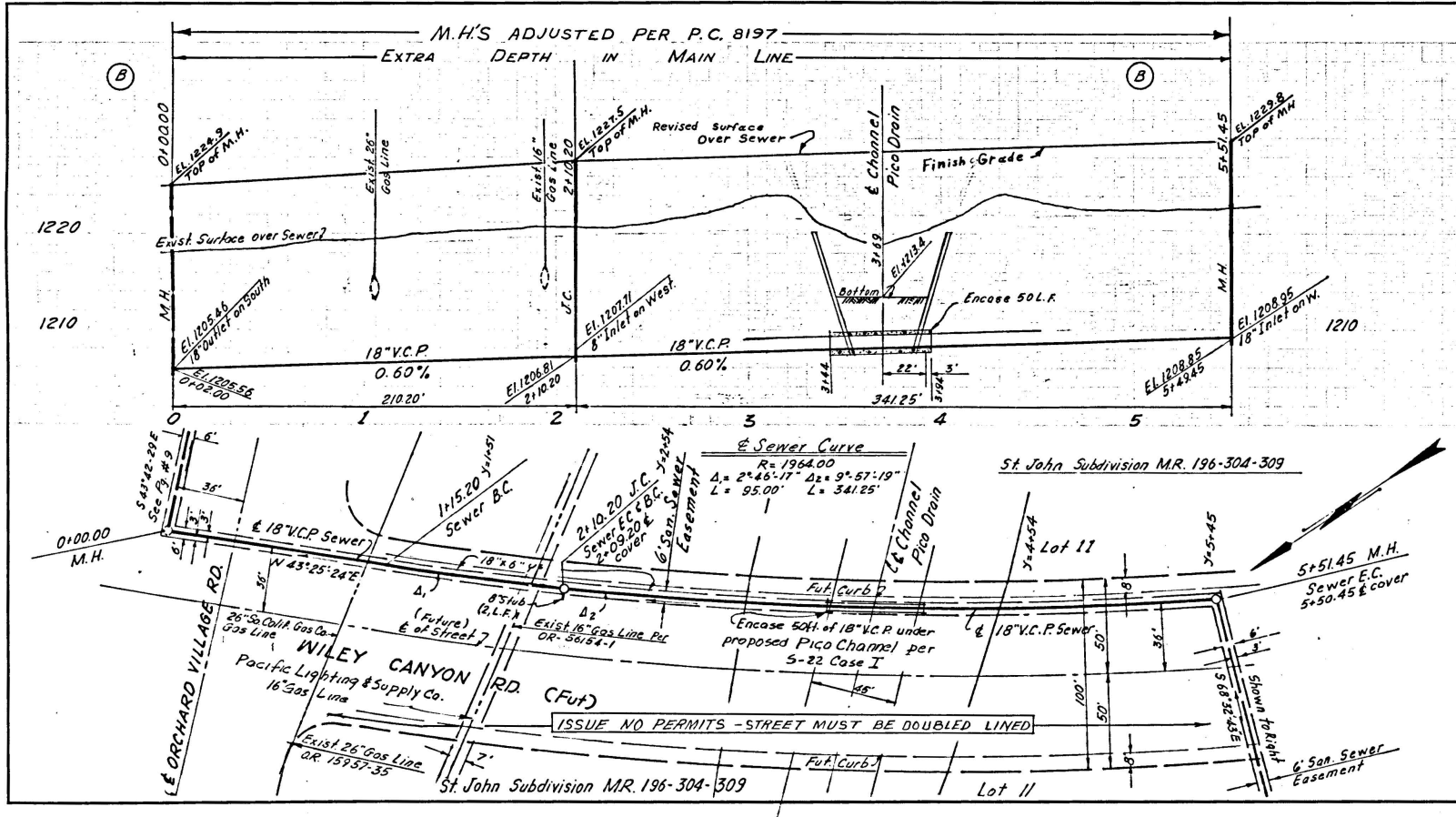
COUNTY OF LOS ANGELES, CALIFORNIA
 JOHN A. LAMBIE, COUNTY ENGINEER J. D. PARKHURST, CHIEF ENGINEER
 CO. SAN. DIST. NO. 32
 APPROVED BY: *Ralph Schaper* ASSISTANT SANITATION ENGINEER APPROVED BY: *J.P. Nelson* OFFICE ENGINEER

CHECKED BY: *Ralph Schaper* 5/18/66
 REG. C. E. NO. 13926

J. N. 0366.91

BEFORE BREAKING INTO ANY EXISTING STRUCTURE AND BEFORE FINAL ACCEPTANCE OF THIS WORK, COUNTY SANITATION DISTRICT (DU A-1281) SHALL BE NOTIFIED IN ORDER THAT REQUIRED INSPECTION CAN BE MADE.

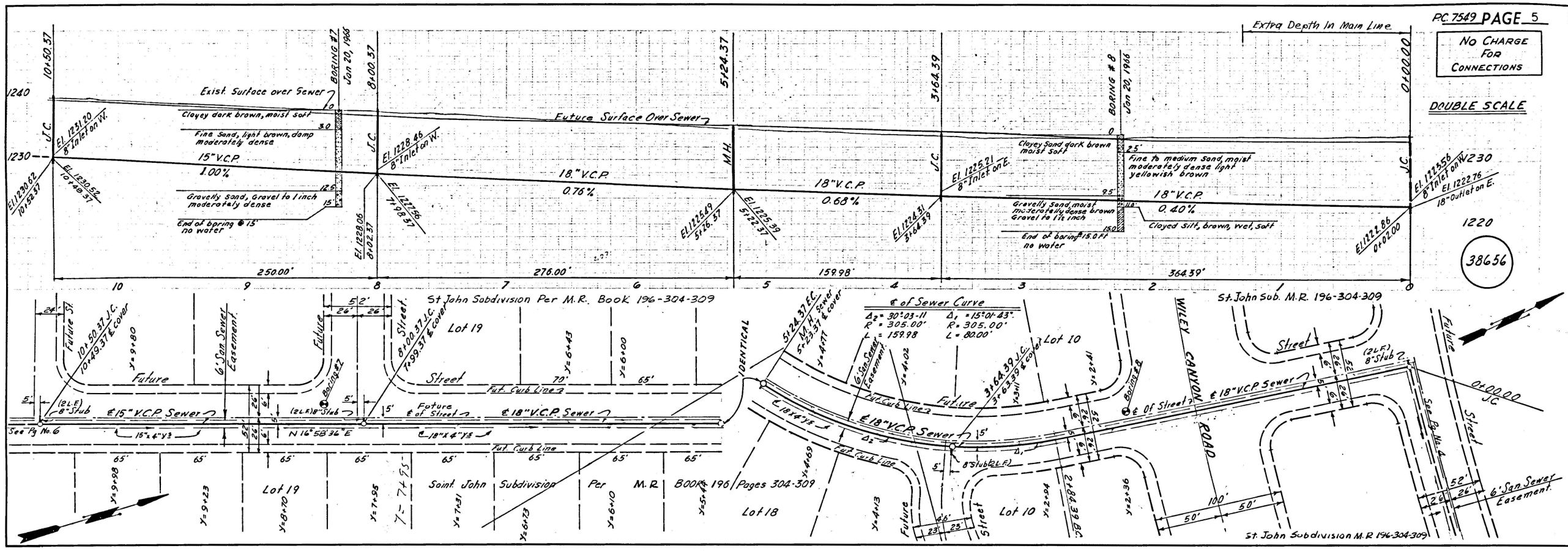
NO CONNECTIONS FOR THE DISPOSAL OF INDUSTRIAL WASTES SHALL BE MADE TO SEWERS SHOWN ON THESE DRAWINGS WITHOUT WRITTEN PERMISSION FROM THE CHIEF ENGINEER AND GENERAL MANAGER OF THE COUNTY SANITATION DISTRICTS.



NO CHARGE FOR CONNECTIONS

DOUBLE SCALE

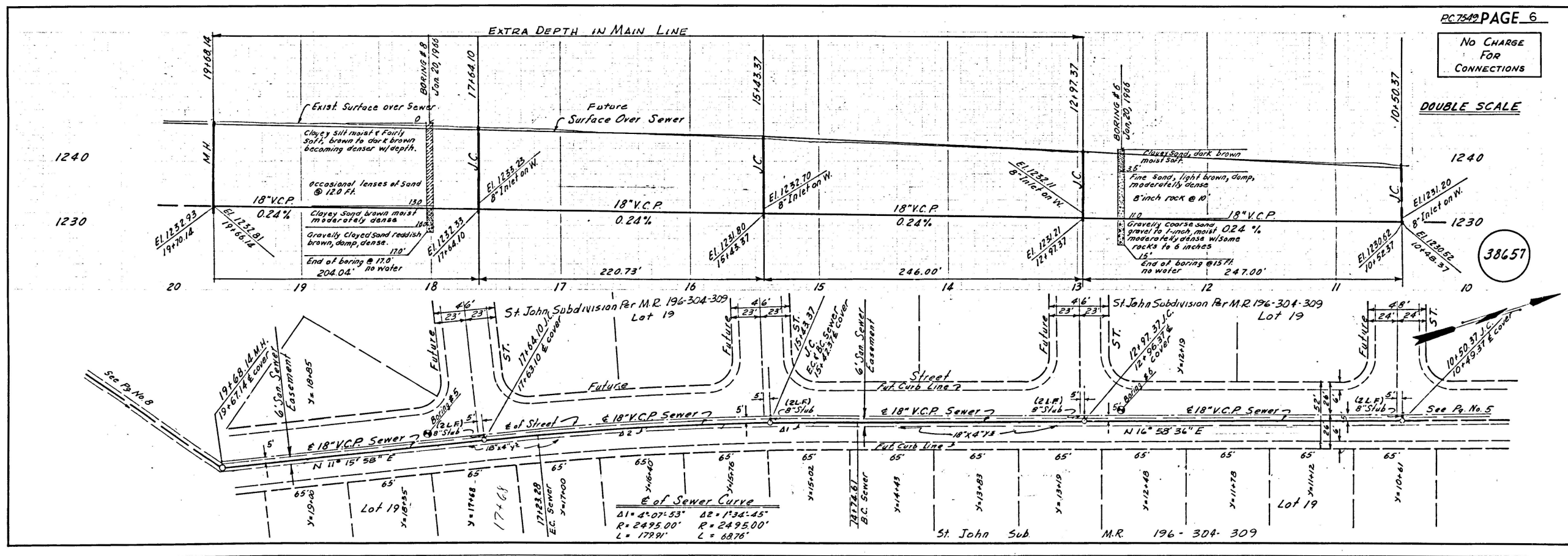
1220
38656



NO CHARGE FOR CONNECTIONS

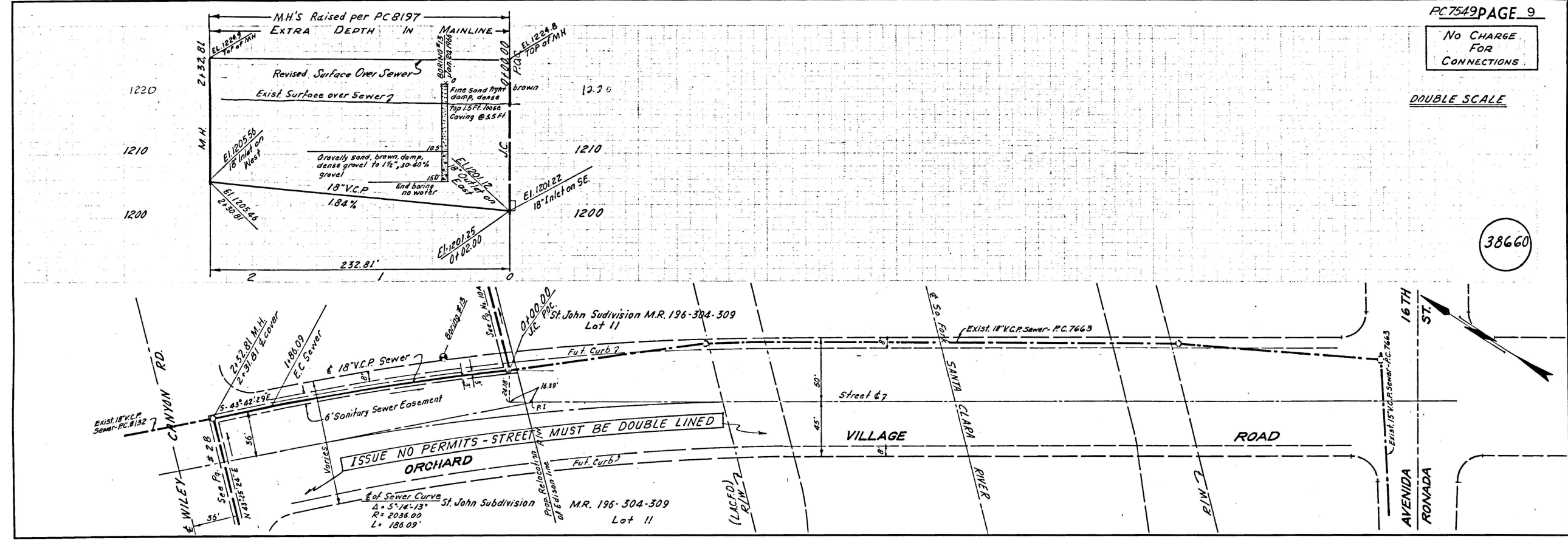
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1240
38657



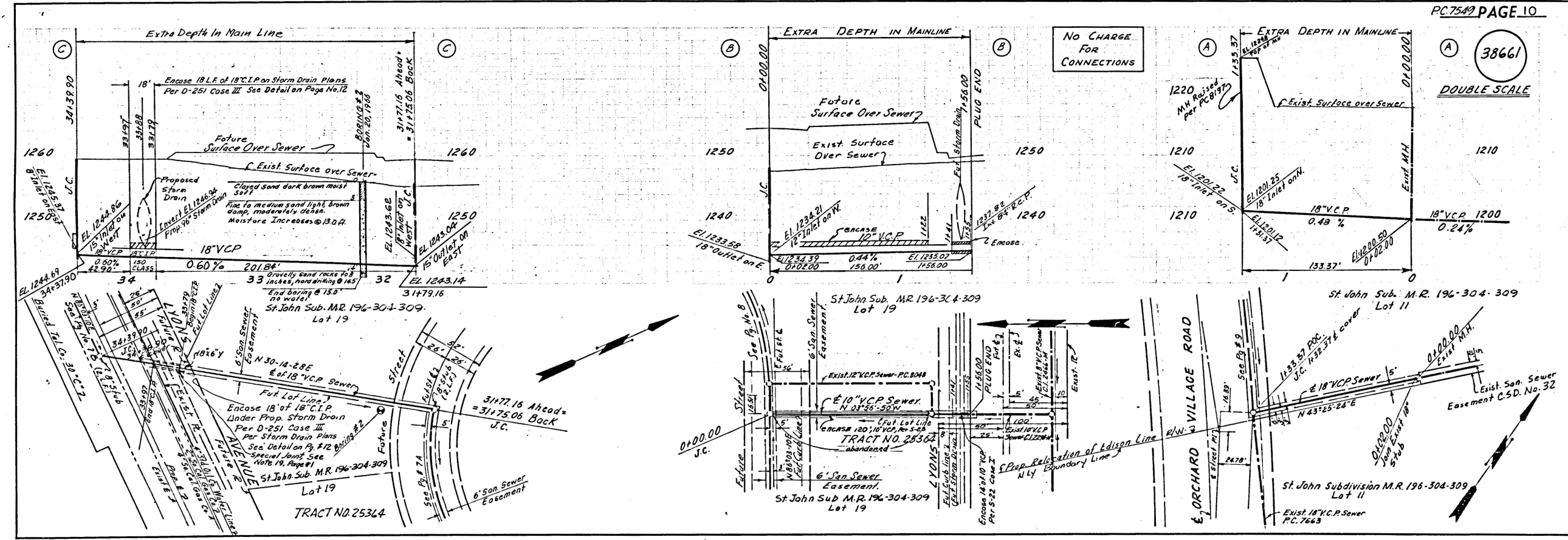
No CHARGE FOR CONNECTIONS

DOUBLE SCALE

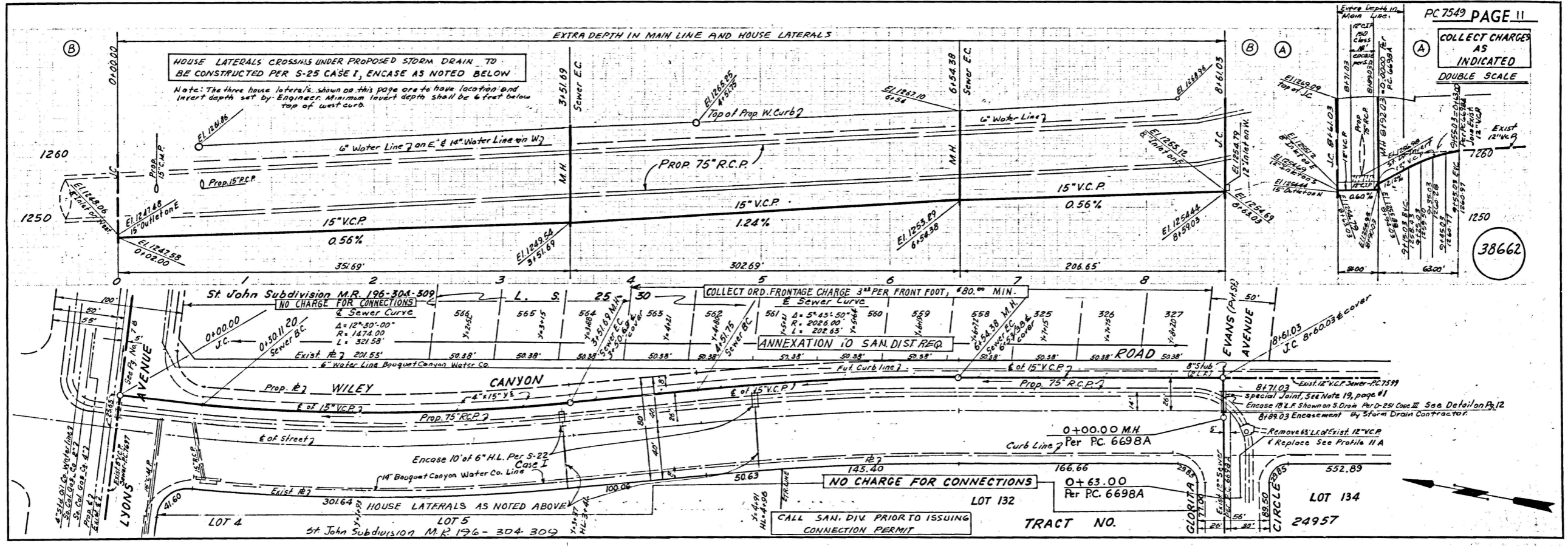


No CHARGE FOR CONNECTIONS

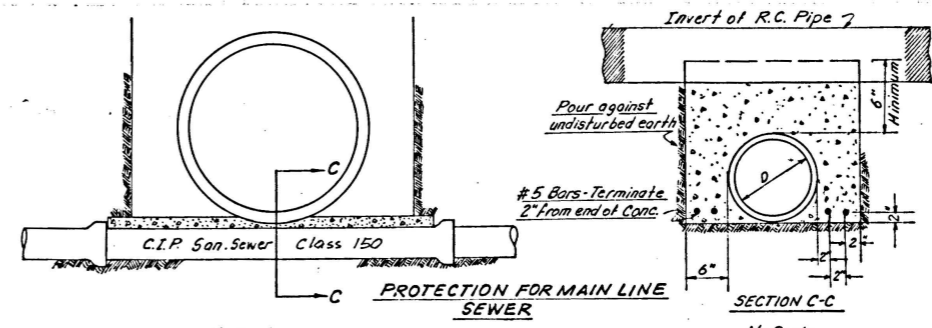
DOUBLE SCALE



COLLECT CHARGES AS INDICATED DOUBLE SCALE



38663



CASE III Per No 2-D 251

Where clearance between bottom on R.C. Pipe Storm Drain and top of Sanitary Sewer is less than 6". San. Sewer shall be C.I.P. unless otherwise specified.

SEWER REFERENCE

PC 8132

SEWER REFERENCE

SAS TM No. 43896

**STEVENSON RANCH AND VICINITY
SEWER AREA STUDY**

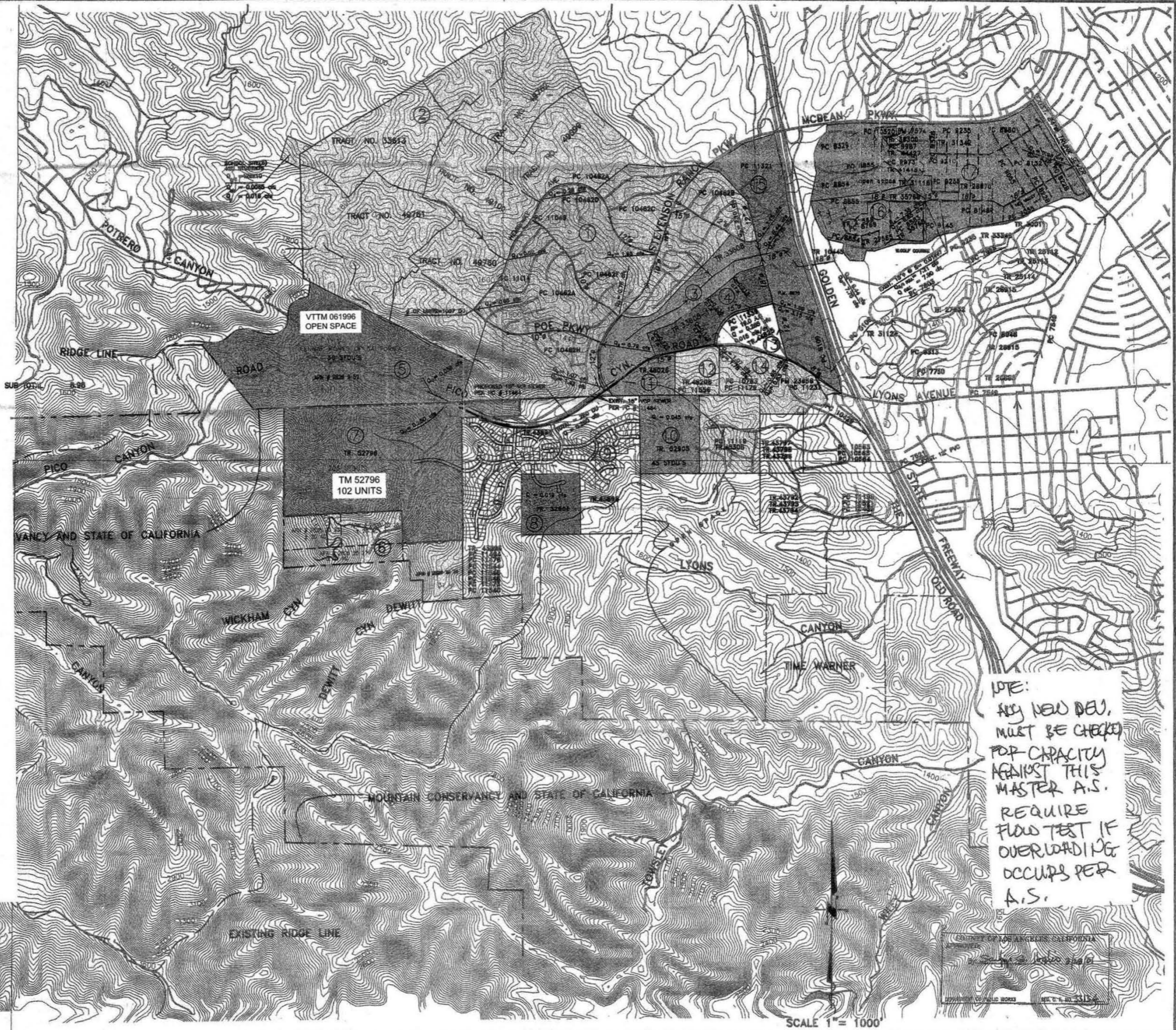
TRACT NO. 43896 - P.C. 11476
SEWER AREA STUDY
TRACT 43896
SUMMARY

AREA	Q peak (cfs)	REMARKS
1	2.303	(EXISTING)
2	1.642	(EXISTING)
3	0.184	APPROVED TENT. TRACT 33818
4	0.706	APPROVED TENT. TRACT 33808-1
5	0.036	LENNAR HOMES FUTURE
6	0.046	FUTURE
7	0.203	PROPOSED TENT. TRACT 52796
8	0.019	APPROVED TENT. TRACT 52908
9	0.280	(EXISTING)
10	0.046	PROPOSED TENT. TRACT 52905
11	0.066	TENT. TR. 48026
12	0.046	APPROVED TENT. TRACT 48208
13	0.285	(EXISTING)
14	0.63	(EXISTING)
15	1.30	(EXISTING)
16	0.909	(EXISTING)
Exist. Peak Q(Qe) = 7.35 cfs (1,2,8,13, 14, 15 & 16) Ultimate Peak Q (Qu) = 8.85 cfs (1 Thru 16)		
17	0.631	(EXISTING)
TOTAL Qe	7.98	Flow to existing 24" trunk sewer
TOTAL Qu	9.28	

SEWER AREA STUDY
WITH CURRENT LAND USE
FOR TM 52796 AND VTTM 061996

AREA	Q peak (cfs)	REMARKS
1	2.303	UNCHANGED
2	1.642	UNCHANGED
3	0.154	UNCHANGED
4	0.706	UNCHANGED
5	0.000	OPEN SPACE
6	0.046	UNCHANGED
7	0.102	102 UNIT Q
8	0.019	UNCHANGED
9	0.28	UNCHANGED
10	0.045	UNCHANGED
11	0.066	UNCHANGED
12	0.046	UNCHANGED
13	0.285	UNCHANGED
14	0.63	UNCHANGED
15	1.3	UNCHANGED
16	0.909	UNCHANGED
Exist. Peak Q(Qe) = 7.35 cfs (1,2,9,13,14,15 & 16) Ultimate Peak Q (Qu) = 8.53 cfs (1 thru 16)		
17	0.631	UNCHANGED
TOTAL Qe	7.98	Flows to existing 24" trunk sewer
TOTAL Qu	9.16	

TABLE PER ALLIANCE LAND PLANNING AND ENGINEERING
APRIL 7, 2014



NOTE:
ANY NEW DEV.
MUST BE CHECKED
FOR CAPACITY
AGAINST THIS
MASTER A.S.
REQUIRE
FLOW TEST IF
OVERLOADING
OCCURS PER
A.S.

DESIGNED:	H.HARRICH
DRAWN:	H.HARRICH
CHECKED:	H.HARRICH
SUPERVISED:	H.HARRICH
PROJ. ENGINEER:	H.HARRICH
DATE:	05/27/2007
SCALE:	1"=1000'
JOB NUMBER:	891-001

SEWER AREA STUDY
TRACT NO. 43896 - P.C. 11476
SR CONSULTANTS, INC.
LAND PLANNING & SURVEYING
10000 WILSON AVENUE, SUITE 100
DANA POINT, CALIFORNIA 92629
Phone: (949) 853-0000, Fax: (949) 853-0005

DATE: _____ BY: _____

REVISION DESCRIPTION: _____

SR CONSULTANTS

SHEET NO. 1

OF 1 SHEET

DDD

SEWER REFERENCE

Diversion Structure (24" CDS Trunk Line)



COUNTY SANITATION DISTRICTS

OF LOS ANGELES, CALIF.

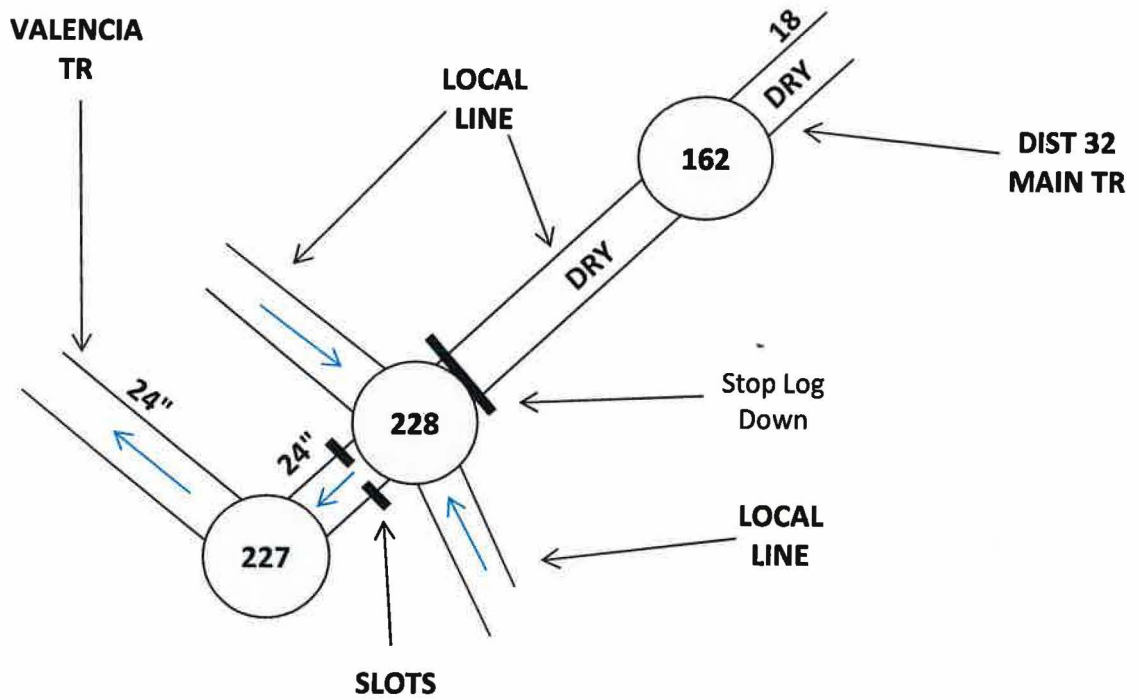
Date: 03/14/18

Page: 13

Prepared by: SA

Checked by: RR/DS

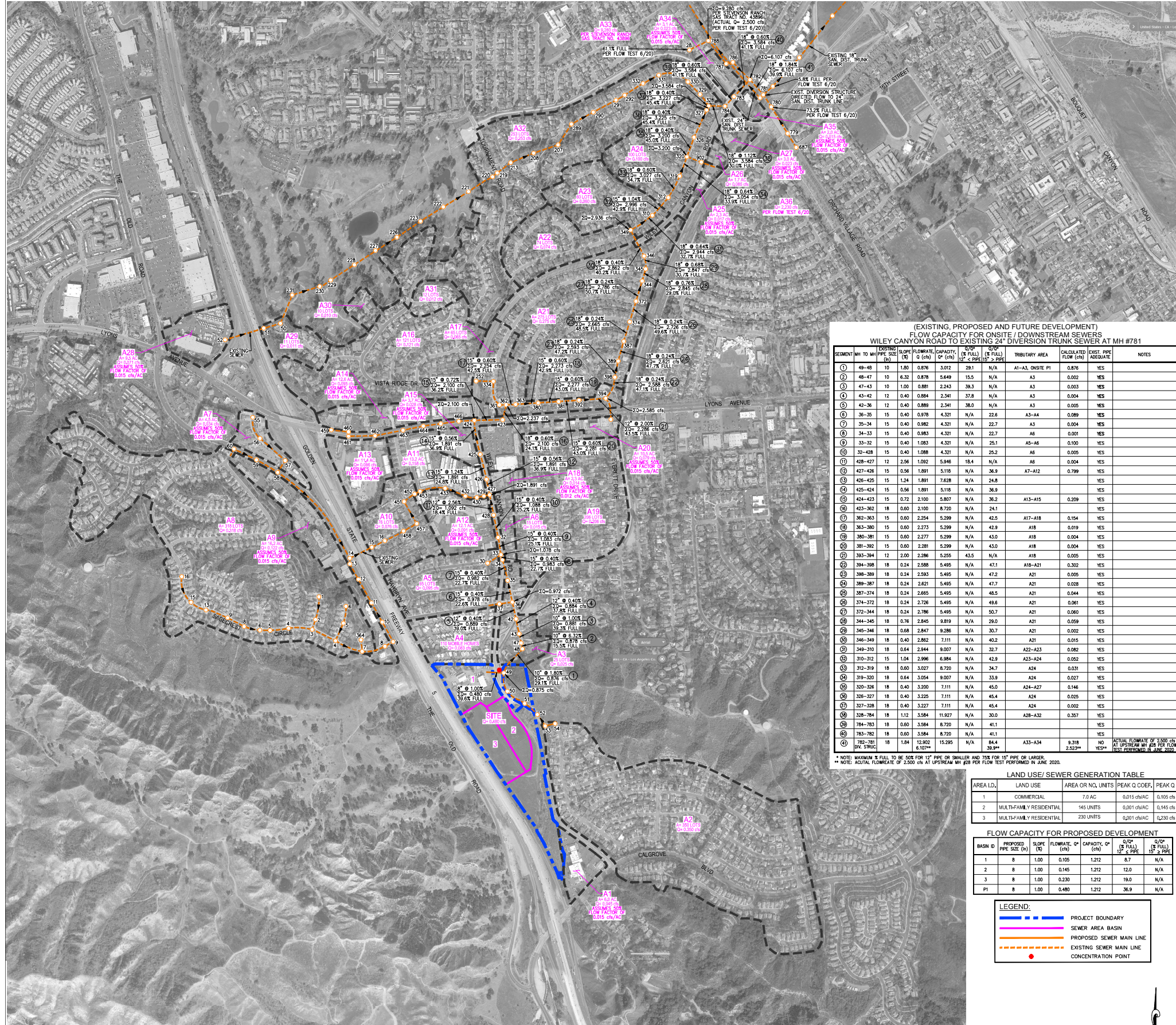
DISTRICT: 32 MAIN TR



Appendix G

Sewer Area Study Map

WILEY CANYON SEWER AREA STUDY EXHIBIT 8/10/20



(EXISTING, PROPOSED AND FUTURE DEVELOPMENT)
FLOW CAPACITY FOR ONSITE / DOWNSTREAM SEWERS
WILEY CANYON ROAD TO EXISTING 24" DIVERSION TRUNK SEWER AT MH #781

SEWER BASIN ID	EXISTING MH TO MH PIPE SIZE (IN)	FLOW RATE, Q (cfs)	FLOW RATE, Q (cfs)	Q/P (CFS/IN)	Q/P (CFS/IN)	TRIBUTARY AREA (AC)	CALCULATED FLOW (cfs)	EXIST. PIPE ADEQUATE	NOTES
①	49-48	10	1.80	0.876	3.012	29.1	N/A	YES	A1-A3, ONSITE P1
②	48-47	10	6.32	0.878	5.649	15.5	N/A	YES	A3
③	47-43	10	1.00	0.881	2.243	39.3	N/A	YES	A3
④	43-42	12	0.40	0.884	2.341	37.8	N/A	YES	A3
⑤	42-36	12	0.40	0.889	2.341	38.0	N/A	YES	A3
⑥	36-35	15	0.40	0.978	4.321	N/A	22.6	YES	A3-A4
⑦	35-34	15	0.40	0.982	4.321	N/A	22.7	YES	A3
⑧	34-33	15	0.40	0.983	4.321	N/A	22.7	YES	A6
⑨	33-32	15	0.40	1.083	4.321	N/A	25.1	YES	A5-A6
⑩	32-428	15	0.40	1.088	4.321	N/A	25.2	YES	A6
⑪	428-427	12	2.56	1.092	5.946	18.4	N/A	YES	A6
⑫	427-426	15	0.56	1.891	5.118	N/A	36.9	YES	A7-A12
⑬	426-425	15	1.24	1.891	7.628	N/A	24.8	YES	
⑭	425-424	15	0.56	1.891	5.118	N/A	36.8	YES	
⑮	424-423	15	0.72	2.100	5.807	N/A	36.2	YES	A13-A15
⑯	423-363	18	0.60	2.100	6.720	N/A	24.1	YES	
⑰	363-362	15	0.60	2.254	5.299	N/A	42.5	YES	A17-A18
⑱	362-360	15	0.60	2.273	5.299	N/A	42.9	YES	A18
⑲	360-361	15	0.60	2.277	5.299	N/A	43.0	YES	A18
⑳	361-362	15	0.60	2.281	5.299	N/A	43.0	YES	A18
㉑	362-394	12	2.00	2.286	5.255	43.5	N/A	YES	A18
㉒	394-398	18	0.24	2.588	5.495	N/A	47.1	YES	A18-A21
㉓	398-399	18	0.24	2.593	5.495	N/A	47.2	YES	A21
㉔	399-387	18	0.24	2.621	5.495	N/A	47.7	YES	A21
㉕	387-374	18	0.24	2.655	5.495	N/A	48.5	YES	A21
㉖	374-372	18	0.24	2.726	5.495	N/A	49.6	YES	A21
㉗	372-344	18	0.24	3.286	5.495	N/A	59.7	YES	A21
㉘	344-345	18	0.76	2.845	9.819	N/A	29.0	YES	A21
㉙	345-346	18	0.68	2.847	8.286	N/A	30.7	YES	A21
㉚	346-349	18	0.40	2.862	7.111	N/A	40.2	YES	A21
㉛	349-310	18	0.84	2.844	9.207	N/A	32.7	YES	A23-A33
㉜	310-312	15	1.04	2.996	6.984	N/A	42.9	YES	A23-A24
㉝	312-319	18	0.60	3.027	8.720	N/A	34.7	YES	A24
㉞	319-320	18	0.64	3.054	9.207	N/A	33.9	YES	A24
㉟	320-326	18	0.40	3.200	7.111	N/A	45.0	YES	A24-A27
㊱	326-327	18	0.40	3.225	7.111	N/A	45.4	YES	A24
㊲	327-328	18	0.40	3.227	7.111	N/A	45.4	YES	A24
㊳	328-784	18	1.12	3.584	11.927	N/A	30.0	YES	A28-A32
㊴	784-783	18	0.60	3.584	8.720	N/A	41.1	YES	
㊵	783-782	18	0.60	3.584	8.720	N/A	41.1	YES	
㊶	782-781	18	1.84	12.902	15.295	N/A	84.4	NO	ACTUAL FLOW RATE OF 2,500 CFS AT UPSTREAM MH #28 PER FLOW TEST PERFORMED IN JUNE 2020.

NOTE: MAXIMUM F FLOW TO BE 50% FOR 12" PIPE OR SMALLER AND 75% FOR 15" PIPE OR LARGER.
NOTE: ACTUAL FLOW RATE OF 2,500 CFS AT UPSTREAM MH #28 PER FLOW TEST PERFORMED IN JUNE 2020.

LAND USE / SEWER GENERATION TABLE

AREA ID	LAND USE	AREA OR NO. UNITS	PEAK Q COEF.	PEAK Q
1	COMMERCIAL	7.0 AC	0.015 cfs/AC	0.105 cfs
2	MULTI-FAMILY RESIDENTIAL	145 UNITS	0.001 cfs/AC	0.145 cfs
3	MULTI-FAMILY RESIDENTIAL	230 UNITS	0.001 cfs/AC	0.230 cfs

FLOW CAPACITY FOR PROPOSED DEVELOPMENT

SEWER BASIN ID	PROPOSED PIPE SIZE (IN)	SLOPE (%)	FLOW RATE, Q (cfs)	CAPACITY, Q (cfs)	Q/P (CFS/IN)	Q/P (CFS/IN)
1	8	1.00	0.105	1.212	8.7	N/A
2	8	1.00	0.145	1.212	12.0	N/A
3	8	1.00	0.230	1.212	19.0	N/A
P1	8	1.00	0.480	1.212	36.9	N/A

LEGEND:

- PROJECT BOUNDARY
- SEWER AREA BASIN
- PROPOSED SEWER MAIN LINE
- EXISTING SEWER MAIN LINE
- CONCENTRATION POINT

