

TRAVELERS STATION

PROJECT APPENDICES:

- 1. Hydrological Report**
- 2. Water Testing Results**
- 3. Traffic Impact Analysis**
- 4. Preliminary Biological Assessment**
- 5. Geotechnical / Soils Evaluation**
- 6. Septic / Percolation Test Results**
- 7. Greenhouse Gas Analysis**
- 8. Cultural Resource Evaluation**
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- 10. Architectural Plans / Elevations**

TRAVELERS STATION:

Hydrological Report

Kelley Engineering & Surveying

June, 2021

HYDROLOGIC AND HYDRAULIC CALCULATIONS

JOB No. 19019
San Benito Traveler's Station
PLN 200017
San Benito County, CA

June 2021



6/22/2021

KELLEY
ENGINEERING & SURVEYING
400 PARK CENTER DR, STE #4, HOLLISTER, CA 95023
OFFICE: (831) 636-1104 FAX (831) 636-1837

Introduction

Site Location:

The site is located east of Searl Road at its north end, south of Highway 129, and west of the 101 on-ramp.

Site Description:

Existing topography can be described as a pear-shaped basin that falls north to south with an average slope of 0.8 percent. Embankment slopes from Searl Road vary from 6:1 to 3.5:1. The ground slopes away from the 101 on-ramp into the site at a range of 2% to 6%.

There are no existing drainage facilities on-site, but there are culverts that terminate just outside the property to the south.

Proposed Project Description:

The project is a fuel station and fast food restaurant serving motorists and diesel trucks traveling along the 101 and 129 corridors.

Existing Drainage System

Characteristics:

The existing drainage system consists of overland sheet flow and shallow concentrated flow from north to south leading to a number of ditches and culverts within Caltrans' jurisdiction that carry runoff in a counter-clockwise route back to the north where it confluences with tributaries to the Pajaro River.

Proposed Drainage System

On-site drainage is conveyed to a detention pond via sheet flow, curb & gutter, valley gutter, catch basins and storm pipes.

Hydrologic Method:

The SCS unit-hydrograph method is used to calculate peak flows for use in sizing the drainage facilities, and to develop the 10-year pre-development peak flow. In accordance with the County Subdivision Ordinance, post development runoff is limited to this 10-year pre-development rate during a 100-year storm event. Storm events are simulated by the SCS unit hydrograph procedure provided in HydroCAD 10 storm water modeling software. Pond routing is accomplished by the Storage-Indication method also provided in HydroCAD 10. Rainfall depths used in the calculations are from the San Benito County Subdivision Ordinance.

Storm Frequencies Analyzed:

In accordance with Section 1.1, Chapter 3 of the County Subdivision Ordinance all on-site closed conduit systems and minor channel sections are designed with a capacity sufficient to handle the 10-year storm event. Detentions ponds are designed to accommodate the 100-year storm event. Allowable discharge for developed areas is limited to the 10-year pre-development rate.

Proposed Flow Patterns:

As the existing ground slopes north to south so does the finish grade. Storm waters are conveyed to a detention pond located at the south end of the site via sheet flow, curb & gutter, valley gutter, catch basins and storm pipes.

Hydraulic Controls for Drainage Outfall:

The outlet of the pond is a sharp crested weir. The weir releases storm flows into an earthen ditch that leads to an existing culvert located south of the property line.

Conclusions

All on-site runoff is directed to the on-site detention pond located at the south end of the site. The shape of the basin is trapezoidal and measures roughly 40 feet wide at the south end, 60 feet wide at the north end and spans roughly 164 feet. Pond capacity is just over 26,000 cubic feet. The impact of the development is a net increase in rainfall runoff. This net increase is mitigated by attenuating the 100-year post-development storm to the level of a 10-year pre-development storm. This is accomplished by routing the storm through a detention pond. The calculations in exhibit C-1 show that the 10-year pre-developed runoff is 4.21 CFS. The 100-year post developed runoff is calculated to be 10.81 CFS and is shown on exhibit D-1. When this 100-year post-developed storm is routed through the detention pond the result is a peak runoff release at the rate of 4.17 CFS as shown on exhibit E-1.

Attachments

- A Drainage Area Map
- B Rainfall Volume Calculations
- C-1 Pre-developed Runoff Summary – 10-year event
- C-2 Pre-developed Runoff Hydrograph – 10-year event
- D-1 Post-developed Runoff Summary – 100-year event
- D-2 Post-developed Runoff Hydrograph – 100-year event
- E-1 Post-developed Results Summary – 100-year event
- E-2 Post-developed Results Hydrograph – 100-year event.
- E-3 Post-developed Results Tabulated Hydrograph – 100-year event



**KELLEY
ENGINEERING & SURVEYING**

400 PARK CENTER DRIVE, SUITE #4
HOLLISTER, CA 95023
OFFICE (831) 636-1104 FAX (831) 636-1837

**SAN BENITO TRAVELER'S STATION
HWY 129 & 101, SAN BENITO COUNTY
DRAINAGE AREA MAP
EXHIBIT A**

Job No.:	19019
Designed:	TJK
Scale:	1" = 100'
Date:	May 2021
Sheet:	1
	of 1



KELLEY
ENGINEERING & SURVEYING
400 Park Center Drive, Suite #4
Hollister, CA 95023
831-636-1104/Fax 831-636-1837

JOB TRAVELERS STATION
SHEET NO. 1 OF 1
CALCULATED BY WJK DATE 5/21/2021
CHECKED BY _____ DATE _____
SCALE _____

SBC VOLUME CALCULATIONS

TC = ~19 MW.

MAP = 18"

10 YEAR INTENSITY

$$0.6 \times 1.8 = 1.08''/\text{HR}$$

100 YEAR INTENSITY

$$0.9 \times 1.8 = 1.62''/\text{HR}$$

VOLUME

10 YEAR

$$1.09 \times 1.48 \times 1.8 = 2.90 \text{ IN}$$

100 YEAR

$$1.09 \times 2.22 \times 1.8 = 4.36 \text{ IN}$$

SHALLOW CONC FLOW
L = 700 FT S = 0.0078

FIGURE 3-1

FIGURE 3-2

FIGURE 3-2

FIGURE 3-5

FIGURE 3-5

19019 preliminary pond design

Type I 24-hr SBC 10-year Rainfall=2.90"

Prepared by Kelley Engineering & Surveying

Printed 5/21/2021

HydroCAD® 10.00-25 s/n 05395 © 2019 HydroCAD Software Solutions LLC

Summary for Subcatchment 3S: Predeveloped

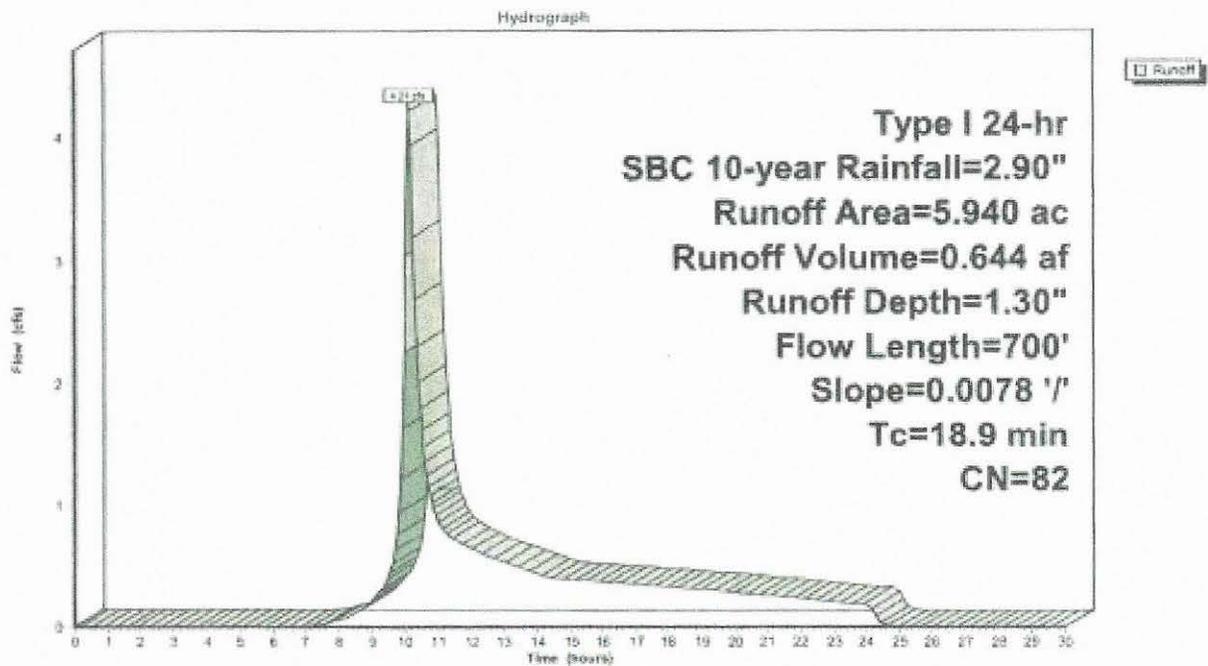
Runoff = 4.21 cfs @ 10.12 hrs, Volume= 0.644 af, Depth= 1.30"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs
Type I 24-hr SBC 10-year Rainfall=2.90"

Area (ac)	CN	Description
1.020	98	Searl Road, Portion 129
4.920	79	50-75% Grass cover, Fair, HSG C
5.940	82	Weighted Average
4.920		82.83% Pervious Area
1.020		17.17% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.9	700	0.0078	0.62		Shallow Concentrated Flow, Shallow Concentrated Flow Short Grass Pasture Kv= 7.0 fps

Subcatchment 3S: Predeveloped



19019 preliminary pond design

Type I 24-hr SBC 100-year Rainfall=4.36"

Prepared by Kelley Engineering & Surveying

Printed 5/21/2021

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Summary for Subcatchment 1S: Proposed Developemnt

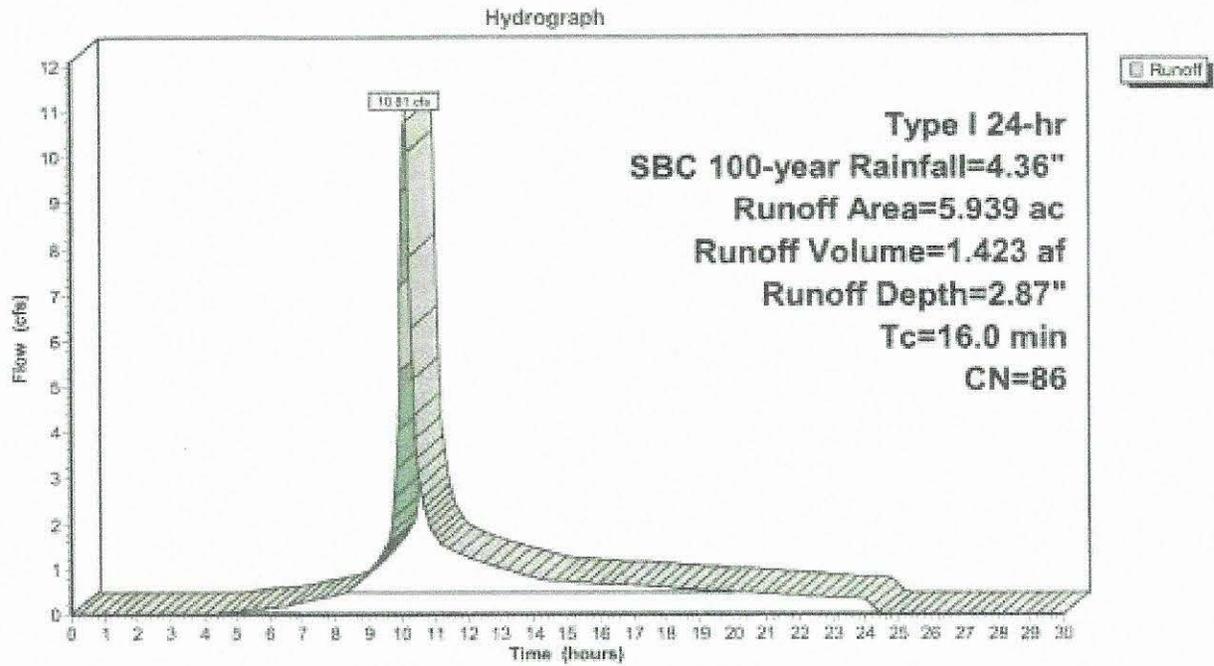
Runoff = 10.81 cfs @ 10.08 hrs, Volume= 1.423 af, Depth= 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs
Type I 24-hr SBC 100-year Rainfall=4.36"

Area (ac)	CN	Description
2.987	74	>75% Grass cover, Good, HSG C
2.952	98	Paved parking, HSG C
5.939	86	Weighted Average
2.987		50.29% Pervious Area
2.952		49.71% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.0					Direct Entry, Direct Entry - Minimum TC

Subcatchment 1S: Proposed Developemnt



19019 preliminary pond design

Type I 24-hr SBC 100-year Rainfall=4.36"

Prepared by Kelley Engineering & Surveying

Printed 5/21/2021

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Summary for Pond 2P: Proposed Pond

Inflow Area = 5.939 ac, 49.71% Impervious, Inflow Depth = 2.87" for SBC 100-year event
 Inflow = 10.81 cfs @ 10.08 hrs, Volume= 1.423 af
 Outflow = 4.20 cfs @ 10.37 hrs, Volume= 1.076 af, Atten= 61%, Lag= 17.6 min
 Primary = 4.20 cfs @ 10.37 hrs, Volume= 1.076 af

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.05 hrs
 Peak Elev= 92.43' @ 10.37 hrs Surf.Area= 8,616 sf Storage= 20,643 cf

Plug-Flow detention time= 242.1 min calculated for 1.074 af (76% of inflow)
 Center-of-Mass det. time= 117.9 min (902.7 - 784.8)

Volume #1	Invert 88.00'	Avail.Storage 26,014 cf	Storage Description Custom Stage Data (Irregular) Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
88.00	51	30.0	0	0	51	
89.00	2,275	272.0	889	889	5,869	
90.00	5,350	439.0	3,705	4,593	15,324	
91.00	6,189	494.0	5,764	10,358	19,434	
92.00	7,492	813.0	6,830	17,188	52,619	
93.00	10,232	921.0	8,826	26,014	67,547	

Device #1	Routing Primary	Invert 91.70'	Outlet Devices 127.0 deg x 1.0' long x 1.25' rise Sharp-Crested Vee/Trap Weir Cv= 2.48 (C= 3.10)
-----------	-----------------	---------------	---

Primary OutFlow Max=4.17 cfs @ 10.37 hrs HW=92.43' (Free Discharge)

↑1=Sharp-Crested Vee/Trap Weir (Weir Controls 4.17 cfs @ 2.33 fps)

19019 preliminary pond design

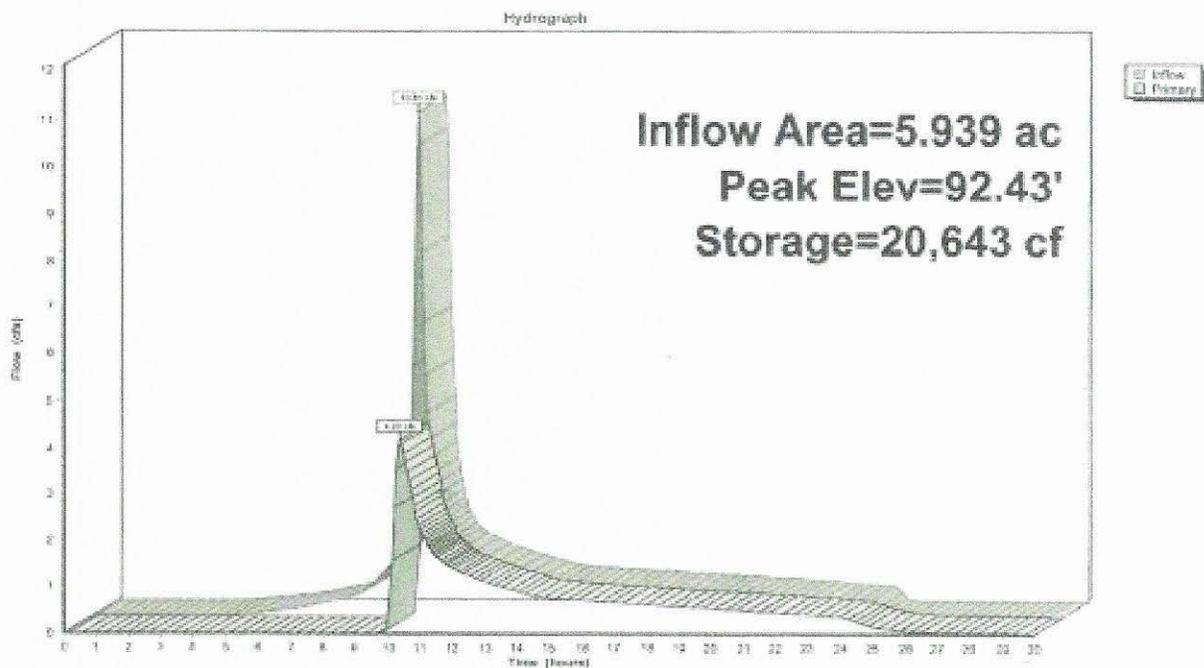
Type I 24-hr SBC 100-year Rainfall=4.36"

Prepared by Kelley Engineering & Surveying

Printed 5/21/2021

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Pond 2P: Proposed Pond



19019 preliminary pond design

Type I 24-hr SBC 100-year Rainfall=4.36"

Prepared by Kelley Engineering & Surveying

Printed 5/21/2021

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Page 3

Hydrograph for Pond 2P: Proposed Pond

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Primary (cfs)
0.00	0.00	0	88.00	0.00
1.00	0.00	0	88.00	0.00
2.00	0.00	0	88.00	0.00
3.00	0.00	0	88.00	0.00
4.00	0.00	0	88.00	0.00
5.00	0.06	91	88.38	0.00
6.00	0.13	438	88.75	0.00
7.00	0.26	1,117	89.10	0.00
8.00	0.41	2,342	89.50	0.00
9.00	0.94	4,570	90.00	0.00
10.00	9.23	12,976	91.41	0.00
11.00	1.74	19,028	92.24	2.27
12.00	1.27	18,085	92.12	1.40
13.00	1.03	17,714	92.07	1.11
14.00	0.81	17,411	92.03	0.90
15.00	0.73	17,191	92.00	0.76
16.00	0.69	17,103	91.99	0.71
17.00	0.64	17,027	91.98	0.66
18.00	0.60	16,951	91.97	0.62
19.00	0.55	16,874	91.96	0.58
20.00	0.51	16,794	91.95	0.53
21.00	0.46	16,703	91.93	0.49
22.00	0.42	16,608	91.92	0.44
23.00	0.37	16,511	91.91	0.40
24.00	0.32	16,411	91.90	0.35
25.00	0.00	15,799	91.81	0.14
26.00	0.00	15,468	91.77	0.06
27.00	0.00	15,311	91.74	0.03
28.00	0.00	15,214	91.73	0.02
29.00	0.00	15,147	91.72	0.02
30.00	0.00	15,101	91.71	0.01

TRAVELERS STATION

WATER TESTING RESULTS

Maggiora Brothers

February, 2021

595 Airport Blvd.
Watsonville, CA 95076
(831) 724-1338

MAGGIORA BROS. DRILLING, INC.

2001 Shelton Drive
Hollister, CA 95023
(831) 637-8228

WELL TEST REPORT

A. **Customer:** Royal Oaks Market Telephone: 831-431-0333
Mail address: 12 Maher Rd, Watsonville, CA 95076
Well Location: 101 & 129 Corner, San Juan Bautista, CA APN: _____
Drilled By: _____ Date: _____

B. **Well Data:** Source (see codes):
Depth of Well: _____ MDT CR OR NM
Diameter of Casing: 8"-STEEL MDT CR OR NM
Depth of Perforation: _____ MDT CR OR NM
Type of Perforation: _____ MDT CR OR NM
Pump Type and HP: 2HP/60-GPM MDT CR OR NM
Depth Pump Set: _____ MDT CR OR NM

(Source codes: MDT=Measured During Testing; CR=Company Records; OR=Owner Records; NM=Not Measured, requires addition testing beyond the scope of report)

C. **Well Test:** Date of Test: 2/25/2021

(1) Water Level at Start: 44 ft. (2) Sustained Pumping Level: 44 ft.
(3) Drawdown (1-2): 0 ft. (4) Test Pumping Duration: 24-HRS

Measured Production Test:

(5) Observed Total Production: 103,680 gal.
(6) Average Yield for Pumping Duration(5/4): 72 gpm

Constant Pumping Level Test:

(7) Final Observed Yield Rate: _____ gpm
(8) Pumping Duration at Final Observed Yield Rate: _____ min.
(9) Calculated Observed Yield Production (4x7): _____ gal.

Pump Broke Suction During test: Yes No Not Sampled in Testing
Title 22 Report Attached: Yes No Not Sampled in Testing
Bacteriological Analysis Attached: Yes No Not Sampled in Testing
Chemical Analysis Attached: Yes No Not Sampled in Testing

D. **Water System Visual Inspection:**

Well Pump Operation: Functional Deficient Not Observed
Electrical Equip.: Functional Deficient Not Observed
Pressure Tanks: Functional Deficient Not Observed
Water Pipes: Functional Deficient Not Observed
Storage Tanks: Functional Deficient Not Observed
Booster Pump Operation: Functional Deficient Not Observed

E. **Comments:** _____

Dated: 2/25/2021

By: SERGIO ROCHA

Rev. 03/00

WELL TEST REPORT DEFINITIONS AND ADDITIONAL TERMS

Sustained yield. Sustained yield is the pumping rate at which long-term pumping can be maintained, and is the rate normally used to compare wells. If the test is of sufficient duration (and assuming the aquifer has a large storage capacity), sustained yield is the best indicator of long term well production during regular operation. As used in this report, sustained yield is the production rate measured at the conclusion of a test in which the pumping level in the well is held constant for the period of time indicated.

Average yield. In many wells, especially wells with small diameter casings, water levels cannot be monitored during pumping, and sustained yield can only be approximated by calculating average yield (which is total volume pumped divided by total pumping time including any period in which the pump breaks suction). Since the pumping level may be declining while testing, and the measured water production may include water in storage in the well and surrounding formation at the start of the test, average yield calculations may be significantly higher than the true sustained yield (particularly where the total pumping time is less than four hours).

Unusual pumping conditions. Wells that break suction while pumping or have high drawdowns in relation to the standing water level are often indicative of marginal long term water producers. These wells should always have protective shutoff devices on the pumps to prevent pump burnout from lack of water. A smaller capacity pump may improve electrical efficiency and sustain less wear by enabling longer pumping cycles. Conversely in stronger wells, the pump itself may be too small to pump the full well capacity, and thus the real sustained (or average) yield may be higher than that observed in this test.

Sole report. This report contains the sole observations and conclusions of the company pertaining to the testing of the Customer's well. Any prior statements of the agents or employees of the company which are not contained herein are superseded by this report. Such prior statements shall be relied upon at the Customer's own sole voluntary risk.

Test limitations. The data and conclusions provided are based upon the tests and measurements of the company using standard and accepted practices of the groundwater industry. However, conditions in water wells are subject to dramatic changes in even short periods of time. Additionally, the techniques employed may be subject to considerable error due to factors within the well and groundwater formation that are beyond the company's immediate control and/or observation. Therefore, the data are valid only as of the date of test and to the extent of the observational limitations of the test or installation indicated.

Use of test. The test conclusions are intended for general comparison of the well in its present condition against known water well standards or guidelines, and should not be relied upon to predict either the future quantity or quality of water that the well will produce. Wells should be periodically re-tested to show both seasonal and long-term production fluctuations or declines.

Disclaimers. In presenting the data and conclusions, the company makes no warranties, either express or implied, as to future water production of the well. Further, the company, unless expressly stated to the contrary, does not represent (1) that the well or pump system is in any particular condition or state of repair, or (2) that the test results will satisfy cognizant governmental ordinances or regulations, or (3) that the test duration or methodology is sufficient to meet local water system or new construction permit standards (these usually require 24 hour or longer test measurement), or (4) that the water is adequate for a particular purpose contemplated by Customer, (5) the accuracy and reliability of the report for any purpose more than one year after the date of the test.

Customer's release. In accepting this report, the Customer releases and holds the company harmless from liability for consequential or incidental damages arising (1) out of the breach of an express or implied warranty of future water production, or (2) in any manner through the further dissemination of this report, or its conclusions, by either Customer or third parties, except as the dissemination is required to complete the project or other activity for which the report was originally prepared.

SOIL CONTROL LAB

42 HANGAR WAY
WATSONVILLE
CALIFORNIA
95076
USA

Maggiora Bros.
595 Airport Blvd.
Watsonville, CA 95076
Attn: Mike Maggiora

Work Order #: 1020703
Reporting Date: March 17, 2021

Date Received: February 25, 2021
Project # / Name: None / P13990
Water System #: NA
Sample Identification: 101& 129 Corner-Information Only do not send to State-Royal Oak Market, sampled 2/25/2021 11:45:00AM
Sampler Name / Co.: Josh Franz / Maggiora Bros
Matrix: Drinking Water
Laboratory #: 1020703-01

	Results	Units	RL	State Drinking Water Limits 1	Analysis Method	Date Analyzed	Flags
General Mineral							
Nitrate as N	ND	mg/L	0.10	10	EPA 300.0	02/28/21	-
pH	7.6	pH Units	0.1	-	SM4500-H+ B	02/25/21	
Specific Conductance (EC)	450	uS/cm	1.0	1600	SM2510B	02/25/21	
Hydroxide as OH	ND	mg/L	2.0	-	SM 2320B	02/25/21	
Carbonate as CO3	ND	mg/L	2.0	-	SM 2320B	02/25/21	
Bicarbonate as HCO3	230	mg/L	2.0	-	SM 2320B	02/25/21	
Total Alkalinity as CaCO3	190	mg/L	14	-	SM 2320B	02/25/21	
Hardness	170	mg/L	5.0	-	SM 2340 B	03/16/21	
Total Dissolved Solids	300	mg/L	10	1000	SM2540C	03/03/21	
Chloride	16	mg/L	1.0	500	EPA 300.0	02/28/21	
Sulfate as SO4	47	mg/L	0.50	500	EPA 300.0	02/28/21	
Fluoride	0.23	mg/L	0.10	2	EPA 300.0	02/28/21	
Calcium	28	mg/L	0.50	-	EPA 200.7	03/16/21	
Magnesium	25	mg/L	0.50	-	EPA 200.7	03/16/21	
Potassium	1.0	mg/L	0.50	-	EPA 200.7	03/16/21	
Sodium	37	mg/L	0.50	-	EPA 200.7	03/16/21	
* Iron	770	ug/L	50	300	EPA 200.7	03/16/21	
* Manganese	230	ug/L	20	50	EPA 200.7	03/16/21	
Copper	ND	ug/L	50	1000	EPA 200.7	03/16/21	
Zinc	ND	ug/L	50	5000	EPA 200.7	03/16/21	
Inorganics							
Nitrate+Nitrite as N	ND	mg/L	0.10	10	EPA 300.0	02/28/21	
Arsenic	ND	ug/L	2.0	10	EPA 200.8	03/04/21	
Barium	ND	ug/L	100	1000	EPA 200.7	03/16/21	

RL - are levels down to which we can quantify with reliability, a result below this level is reported as "ND" for Not Detected.

State Drinking Water Limits - as listed by California Administrative Code, Title 22.

* - a * in the left hand margin of the report means that particular constituent is above the California Drinking Water Limits.

Mike Galloway

SOIL CONTROL LAB

42 HANGAR WAY
WATSONVILLE
CALIFORNIA
95076
USA

Maggiora Bros.
595 Airport Blvd.
Watsonville, CA 95076
Attn: Mike Maggiora

Work Order #: 1020703
Reporting Date: March 17, 2021

Date Received: February 25, 2021
Project # / Name: None / P13990
Water System #: NA
Sample Identification: 101& 129 Corner-Information Only do not send to State-Royal Oak Market, sampled 2/25/2021 11:45:00AM
Sampler Name / Co.: Josh Franz / Maggiora Bros
Matrix: Drinking Water
Laboratory #: 1020703-01

	Results	Units	RL	State Drinking Water Limits *	Analysis Method	Date Analyzed	Flags
Inorganics							
Boron	120	ug/L	100	-	EPA 200.7	03/16/21	-
Cadmium	ND	ug/L	1.0	5	EPA 200.8	03/04/21	
Chromium	ND	ug/L	1.0	50	EPA 200.8	03/04/21	
Cyanide (total)	ND	ug/L	100	200	SM 4500-CN F	03/02/21	
Lead	ND	ug/L	5.0	15	EPA 200.8	03/04/21	
Mercury	ND	ug/L	1.0	2	EPA 245.1	03/04/21	
Selenium	ND	ug/L	5.0	50	EPA 200.8	03/04/21	
Silver	ND	ug/L	10	100	EPA 200.8	03/04/21	
MBAS (Surfactants)	ND	mg/L	0.025	0.5	SM5540C	02/26/21	
Aluminum	ND	ug/L	50	1000	EPA 200.7	03/16/21	
Antimony	ND	ug/L	6.0	6	EPA 200.8	03/04/21	
Beryllium	ND	ug/L	1.0	4	EPA 200.7	03/16/21	
Nickel	ND	ug/L	10	100	EPA 200.7	03/16/21	
Thallium	ND	ug/L	1.0	2	EPA 200.8	03/04/21	
Nitrite as N	ND	mg/L	0.10	1	EPA 300.0	02/28/21	
General Physical							
Color	ND	Color Units	3.0	-	SM 2120B	02/25/21	
Threshold Odor No.	ND	T.O.N.	1.0	-	SM 2150B	02/25/21	
Turbidity	5.0	NTU	0.10	-	SM 2130B	02/25/21	

RL - are levels down to which we can quantify with reliability, a result below this level is reported as "ND" for Not Detected.

State Drinking Water Limits, - as listed by California Administrative Code, Title 22.

* - a * in the left hand margin of the report means that particular constituent is above the California Drinking Water Limits.

Mike Galloway

SOIL CONTROL LAB

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WATSONVILLE
CALIFORNIA
95076
USA

Maggiora Bros.
595 Airport Blvd.
Watsonville, CA 95076
Attn: Mike Maggiora

Work Order #: 1020703
Reporting Date: February 26, 2021

Bacteriological Examination of Water for Coliform Organisms

Date Received: Water sample(s) received February 25, 2021
Project # / Name: P13990 / None
Water System #/Name: NA
Sampling Type: Routine Sampling Period: February 2021
Sampler's Name: Josh Franz / Maggiora Bros
Matrix: Drinking Water

<u>Sample Identification</u>	<u>Sampling Date</u>	<u>Sampling Time</u>	<u>Total Coliforms</u>	<u>E. coli</u>
101& 129 Corner-Information Only do not send to State-Rt	02/25/21	11:45	Absent	Absent

Date/Time Analyzed: 02/25/21 17:07
Method of Analysis: SM 9223 B

CA ELAP Certificate #1494 (This identifies our Laboratory to the Health Department)

Mike Galloway