

DATE: March 22, 2023
TO: Nicole Morse, T&B Planning, Inc.
FROM: Alex So, Urban Crossroads, Inc.
JOB NO: 14501-04 VMT

4665 LAMPSON AVENUE VEHICLE MILES TRAVELED (VMT) ALTERNATIVE USE ANALYSIS

Urban Crossroads, Inc. is pleased to provide the following Vehicle Miles Traveled (VMT) Alternative Use Analysis for the 4665 Lampson Avenue, which is located in the City of Los Alamitos.

PROJECT OVERVIEW

The Project (**Project**) consists of the development of 246 dwelling units, detailed as follows:

- 55 single family detached residential dwelling units (cluster homes)
- 114 multifamily (low-rise) residential dwelling units
- 77 affordable apartment dwelling units

The proposed alternative (**Project Alternative**) replaces the affordable housing component with age-restricted housing, which would result in the following:

- 55 single family detached residential dwelling units (cluster homes),
- 114 multifamily (low-rise) residential dwelling units,
- 77 attached age-restricted affordable housing dwelling units

ANALYSIS APPROACH

A comparative analysis was prepared to compare the VMT forecasts for the proposed Project Alternative to the Project. The Orange County Transportation Analysis Model (OCTAM) model does not have a mechanism to account for age-restricted affordable housing, therefore, VMT was derived based on OCTAM model trip lengths using the Project's trip generation for the Project and the Project Alternative.

TRAFFIC MODELING METHODOLOGY

The OCTAM sub regional model is the appropriate tool for conducting VMT analysis for land use projects in the County of Orange, as it considers interaction between

different land uses based on socio-economic data (SED), such as population, households, and employment.

PROJECT LAND USE CONVERSION

Adjustments in SED have been made to the appropriate Project transportation analysis zone (TAZ 634) within the OCTAM model to reflect the Project and Project Alternative land use information. The population conversion factors were obtained from the State of California Department of Finance [E-5 Population and Housing Estimates for Cities, Counties, and the State, 2011-2021 with 2010 Census Benchmark](#) (3). Table 1 summarizes the SED inputs used to reflect the Original Project and Project Alternative.

TABLE 1: POPULATION ESTIMATES

	Project
Households	246
Population	647

PASSENGER CAR TRIP LENGTHS

Passenger car trip lengths can be derived from the OCTAM outputs once SED inputs are made to the OCTAM inputs and the OCTAM model is run. Table 2 shows the average trip length for the Project's transportation analysis zone (TAZ) 634 for the base year and cumulative year models. The trip lengths for both model years are interpolated using linear interpolation to the Notice of Preparation year or baseline year 2022.

TABLE 2: OCTAM TRIP LENGTHS FOR TAZ 634

	Base Year (2016)	Cumulative Year (2045)	Baseline Year (2022)
Trip Length	12.12	12.26	12.15

PROJECT TRIP GENERATION

The Project's [Traffic Analysis](#) (Urban Crossroads, 2023) estimates the Project to generate 1,658 vehicle trip ends per day. Table 3 shows the results of the Project's trip generation.

TABLE 3: PROJECT TRIP GENERATION SUMMARY

Land Use ¹	Units ²	ITE LU Code	AM Peak Hour			PM Peak Hour			Daily
			In	Out	Total	In	Out	Total	
Single Family Detached	DU	210	0.18	0.52	0.70	0.59	0.35	0.94	9.43
Multifamily Housing (Low-Rise) (2-3 Floors)	DU	220	0.10	0.30	0.40	0.32	0.19	0.51	6.74
Affordable Housing	DU	223	0.10	0.26	0.36	0.27	0.19	0.46	4.81

¹ Trip Generation & Vehicle Mix Source: Institute of Transportation Engineers (ITE), [Trip Generation Manual](#), Eleventh Edition (2021).

² DU = Dwelling Units

Land Use	Quantity Units ¹	AM Peak Hour			PM Peak Hour			Daily
		In	Out	Total	In	Out	Total	
Single Family Detached	55 DU	10	28	38	33	19	52	520
Multifamily Housing	114 DU	11	35	46	37	22	59	768
Affordable Housing	77 DU	8	20	28	21	15	36	370
Project Total Trips		29	83	112	91	56	147	1,658

¹ DU = Dwelling Units

PROJECT ALTERNATIVE TRIP GENERATION

The Project’s [Alternative Use Traffic Assessment](#) (Urban Crossroads, 2023) estimates the Project Alternative to generate 1,538 vehicle trip ends per day. Table 4 shows the results of the Project Alternative’s trip generation, resulting in a net decrease of 120 daily vehicle trip ends per day.

TABLE 4: PROJECT ALTERNATIVE TRIP GENERATION SUMMARY

Land Use ¹	Units ²	ITE LU Code	AM Peak Hour			PM Peak Hour			Daily
			In	Out	Total	In	Out	Total	
Single Family Detached	DU	210	0.18	0.52	0.70	0.59	0.35	0.94	9.43
Multifamily Housing (Low-Rise) (2-3 Floors)	DU	220	0.10	0.30	0.40	0.32	0.19	0.51	6.74
Senior Housing - Attached	DU	252	0.07	0.13	0.20	0.14	0.11	0.25	3.24

¹ Trip Generation & Vehicle Mix Source: Institute of Transportation Engineers (ITE), [Trip Generation Manual](#), Eleventh Edition (2021).

² DU = Dwelling Units

Land Use	Quantity Units ¹	AM Peak Hour			PM Peak Hour			Daily
		In	Out	Total	In	Out	Total	
Single Family Detached	55 DU	10	28	38	33	19	52	520
Multifamily Housing	114 DU	11	35	46	37	22	59	768
Senior Housing Attached	77 DU	5	10	15	11	8	19	250
Project Alternative Total Trips		26	73	99	81	49	130	1,538
Proposed Project Total Trips		29	83	112	91	56	147	1,658
Variance (Alternative - Proposed)		-3	-10	-13	-10	-7	-17	-120

¹ DU = Dwelling Units

VMT ESTIMATES

The Project and Project Alternative VMT estimates are the result of the product of the daily vehicle trip generation and the OCTAM model vehicle trip lengths. Table 5 shows the results of the VMT estimates for baseline and cumulative conditions.

TABLE 5: VMT COMARISON

	Baseline (2022)	Cumulative (2045)
Project VMT	20,145	20,327
Project Alternative VMT	18,687	18,856
Net Reduction in VMT	-1,458	-1,471

SUMMARY

As the Project Alternative is estimated to decrease daily vehicle trips and subsequent VMT, the Project Alternative will not incur any new or additional VMT impacts as compared to the Project.

If you have any questions, please contact me directly at aso@urbanxroads.com.

REFERENCES

1. **State of California Department of Finance.** *E-5 Population and Housing Estimates for Cities, Counties, and the State, 2011-2021 with 2010 Census Benchmark.* May 2021.

DATE: June 1, 2023
TO: Nicole Morse, T&B Planning, Inc.
FROM: Charlene So, Urban Crossroads
JOB NO: 14501-08 TA Memo



4665 LAMPSON AVENUE ALTERNATIVE USE TRAFFIC ASSESSMENT

Urban Crossroads, Inc. is pleased to submit the following Alternative Use Traffic Assessment for the proposed 4665 Lampson Avenue development (**Project**), which is located in the City of Los Alamitos but adjacent to the City of Seal Beach. This assessment has been prepared to evaluate the proposed Project land use alternative that replaces the affordable housing with age-restricted housing (attached product type). The trip generation for the proposed Project Alternative was presented in the [4665 Lampson Avenue Traffic Analysis](#) (dated February 3, 2023, referred to as 2023 Traffic Study). The purpose of this assessment is to determine whether the proposed Project Alternative would have different deficiencies and recommended improvements from those presented in the 2023 Traffic Study.

SUMMARY OF FINDINGS

For the purposes of this assessment, only Opening Year Cumulative (2026) With Project and General Plan Buildout With Project scenarios have been evaluated assuming the proposed Project Alternative use which would be compared to the results presented in the 2023 Traffic Study.

- The level of service (LOS) operations during the peak hour are anticipated to continue to operate at an acceptable LOS under Opening Year Cumulative (2026) traffic conditions, with the exception of Seal Beach Boulevard & Lampson Avenue (#1), consistent with the 2023 Traffic Study.
- No site adjacent queues are anticipated under General Plan Buildout With Project traffic conditions, consistent with the 2023 Traffic Study.
- Consistent with the 2023 Traffic Study, improvements to deficient intersections have been recommended to maintain acceptable peak hour operations under Opening Year Cumulative (2026) With Project and General Plan Buildout With Project traffic conditions.

PROJECT TRIP GENERATION

The proposed Project Alternative replaces the affordable housing with age-restricted housing (attached product type). As such, the Project Alternative consists of the development of 55 single family detached residential dwelling units (cluster homes), 114 multifamily (low-rise) residential dwelling units, and 77 attached senior housing dwelling units (total of 246 dwelling units). No other changes are proposed to the other uses from those evaluated in the 2023 Traffic Study. In order to develop the traffic characteristics of the Project Alternative, trip-generation statistics published in the Institute of Transportation Engineers (ITE) Trip Generation Manual (11th Edition, 2021) for the Single Family Detached Residential Land Use category (ITE Land Use Code 210), Multifamily (Low-Rise) Housing (ITE Land Use Code 220), and Attached Senior Housing (ITE Land Use Code 252) were used to calculate the Project Alternative trip generation (see Table 1 for rates). The trip generation summary illustrating daily, and peak hour trip generation estimates for the Project Alternative are also shown on Table 1. The Project Alternative is anticipated to generate 1,538 two-way trip-ends per day with 99 AM peak hour trips and 130 PM peak hour trips. As shown, the Project Alternative generates 120 fewer two-way trip-ends per day with 13 fewer AM peak hour trips and 17 fewer PM peak hour trips.

TABLE 1: PROJECT ALTERNATIVE TRIP GENERATION SUMMARY

Land Use ¹	Units ²	ITE LU Code	AM Peak Hour			PM Peak Hour			Daily
			In	Out	Total	In	Out	Total	
Single Family Detached	DU	210	0.18	0.52	0.70	0.59	0.35	0.94	9.43
Multifamily Housing (Low-Rise) (2-3 Floors)	DU	220	0.10	0.30	0.40	0.32	0.19	0.51	6.74
Senior Housing - Attached	DU	252	0.07	0.13	0.20	0.14	0.11	0.25	3.24

¹ Trip Generation & Vehicle Mix Source: Institute of Transportation Engineers (ITE), Trip Generation Manual, Eleventh Edition (2021).

² DU = Dwelling Units

Land Use	Quantity Units ¹	AM Peak Hour			PM Peak Hour			Daily
		In	Out	Total	In	Out	Total	
Single Family Detached	55 DU	10	28	38	33	19	52	520
Multifamily Housing	114 DU	11	35	46	37	22	59	768
Senior Housing Attached	77 DU	5	10	15	11	8	19	250
Project Alternative Total Trips		26	73	99	81	49	130	1,538
Proposed Project Total Trips		29	83	112	91	56	147	1,658
Variance (Alternative - Proposed)		-3	-10	-13	-10	-7	-17	-120

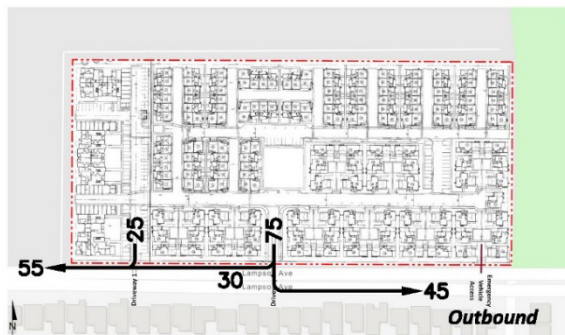
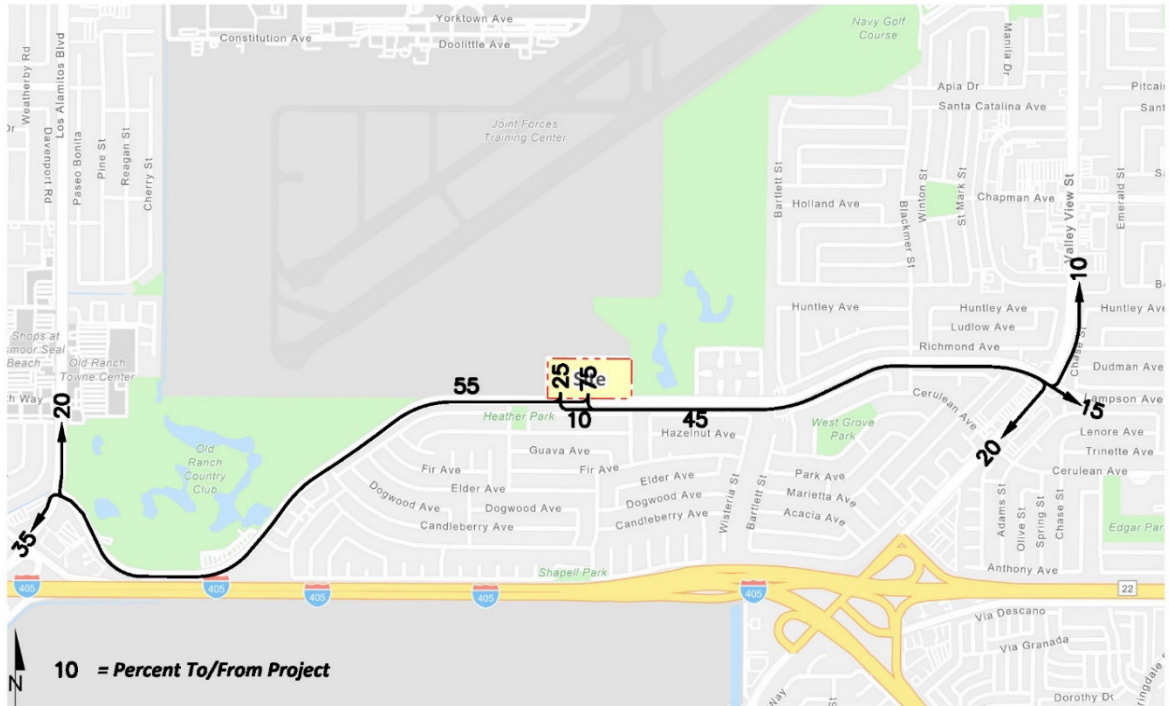
¹ DU = Dwelling Units

PROJECT TRIP DISTRIBUTION

The Project trip distribution represents the directional orientation of traffic to and from the Project site. Trip distribution is the process of identifying the probable destinations, directions or traffic routes that will be utilized by Project traffic. The potential interaction between the planned land uses and surrounding regional access routes are considered, to identify the route where the Project traffic would distribute. The trip distribution used for the proposed Project identified in the 2023 Traffic Study, has also been used for the Project Alternative. Exhibit 1 shows the Project

trip distribution patterns. Access to the Project site will remain the same as described and evaluated in the 2023 Traffic Study.

EXHIBIT 1: PROJECT TRIP DISTRIBUTION



PROJECT TRIP ASSIGNMENT

The assignment of traffic from the Project area to the adjoining roadway system is based upon the Project Alternative trip generation, trip distribution, and the arterial highway and local street system improvements that would be in place by the time of initial occupancy of the Project. Based on the identified Project Alternative traffic generation and trip distribution patterns, Project Alternative only ADT and weekday AM and PM peak hour intersection turning movement volumes are shown in Attachment A.

QUEUING ANALYSIS

A queuing analysis has been performed for the Project driveways on Lampson Avenue under General Plan Buildout With Project Alternative traffic conditions. The traffic modeling and signal timing optimization software package SimTraffic has been utilized to assess the queues consistent with the 2023 Traffic Study. Queuing analysis worksheets for the weekday AM and PM peak hours are provided in Attachment B of this report for General Plan Buildout With Project Alternative traffic conditions.

As shown in Table 2, no site adjacent queues are anticipated under General Plan Buildout With Project Alternative traffic conditions, consistent with the 2023 Traffic Study. An eastbound left turn lane with a minimum of 150-feet of storage at Driveway 2 is anticipated to sufficiently accommodate 95th percentile peak hour queues.

TABLE 2: PEAK HOUR QUEUING SUMMARY FOR GENERAL PLAN BUILDOUT CONDITIONS

# Intersection	Movement	Available Stacking Distance (Feet)	95th Percentile Queue (Feet)		Acceptable? ¹	
			AM Peak Hour	PM Peak Hour	AM	PM
6 Driveway 1 & Lampson Av.	SBR	100	38	34	Yes	Yes
7 Driveway 2 & Lampson Av.	SBL/R	100	58	52	Yes	Yes
	EBL	150	21	39	Yes	Yes
	WBT	520	0	0	Yes	Yes

* SB = Southbound, EB = Eastbound, WB = Westbound, L = Left, T = Through, R = Right

¹ Stacking Distance is acceptable if the required stacking distance is less than or equal to the stacking distance provided. An additional 25 feet of stacking which is assumed to be provided in the transition for turn pockets is reflected in the stacking distance shown on this table, where applicable.

OPENING YEAR CUMULATIVE (2026) INTERSECTION OPERATIONS ANALYSIS

Opening Year Cumulative (2026) forecasts apply an ambient growth of 8.24% plus traffic from pending and approved but not yet constructed known development projects in the area. The weekday ADT and weekday AM and PM peak hour volumes which can be expected for Opening Year Cumulative (2026) With Project Alternative conditions are shown in Attachment C.

As shown on Table 3, the following study area intersection is anticipated to operate at an unacceptable LOS during the peak hours under Opening Year Cumulative (2026) With Project Alternative traffic conditions, consistent with the 2023 Traffic Study:

- Seal Beach Bl. & Lampson Av. (#1) – LOS E AM and PM peak hours

Although the intersection of Valley View Street at Lampson Avenue is anticipated to operate at a deficient LOS based on the Intersection Capacity Utilization (ICU) methodology, a more detailed peak hour intersection operations assessment based on the Highway Capacity Manual (HCM) methodology indicates acceptable peak hour operations. As such, no improvements have been recommended at the intersection of Valley View Street and Lampson Avenue. The intersection operations analysis worksheets for Opening Year Cumulative (2026) With Project Alternative traffic conditions are included in Attachment D.

TABLE 3: INTERSECTION ANALYSIS FOR OPENING YEAR CUMULATIVE (2026) CONDITIONS

# Intersection	Traffic Control ²	2026 Without Project				2026 With Project							
		Delay (secs.) or ICU (v/c) ¹		Level of Service		Primary Use				Alternative Use			
		AM	PM	AM	PM	Delay (secs.) or ICU (v/c) ¹	Level of Service	Delay (secs.) or ICU (v/c) ¹	Level of Service	Delay (secs.) or ICU (v/c) ¹	Level of Service	Delay (secs.) or ICU (v/c) ¹	Level of Service
1 Seal Beach Bl. & Lampson Av.	TS	0.92	0.95	E	E	0.93	0.96	E	E	0.93	0.96	E	E
2 Old Ranch Plaza & Lampson Av.	TS	0.46	0.44	A	A	0.48	0.45	A	A	0.48	0.45	A	A
3 Basswood St. & Lampson Av.	TS	0.51	0.49	A	A	0.53	0.51	A	A	0.53	0.50	A	A
4 Candleberry Av. & Lampson Av.	TS	0.44	0.42	A	A	0.46	0.43	A	A	0.46	0.43	A	A
5 Heather St. & Lampson Av.	TS	0.38	0.42	A	A	0.39	0.44	A	A	0.39	0.43	A	A
6 Driveway 1 & Lampson Av.	CSS	Future Intersection				10.9	10.8	B	B	10.8	10.7	B	B
7 Driveway 2 & Lampson Av.	CSS	Future Intersection				14.8	15.6	B	C	14.6	15.3	B	C
8 Rose St. & Lampson Av.	TS	0.34	0.41	A	A	0.35	0.42	A	A	0.35	0.42	A	A
9 Tulip St. & Lampson Av.	TS	0.34	0.42	A	A	0.35	0.42	A	A	0.35	0.42	A	A
10 Valley View St. & Lampson Av.	TS	0.80	0.90	D	E	0.82	0.92	D	E	0.81	0.92	D	E
HCM Analysis:		31.4	36.5	C	D	33.3	40.6	C	D	35.2	40.3	D	D

* **BOLD** = Level of Service (LOS) does not meet the applicable jurisdictional requirements (i.e., unacceptable LOS).

¹ Per the Highway Capacity Manual (6th Edition), overall average intersection delay and level of service are shown for intersections with a traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown.

ICU is reported in volume-to-capacity (v/c). ICU analysis conducted for signalized intersections only.

² CSS = Cross-street Stop; TS = Traffic Signal; **CSS** = Improvement

OPENING YEAR CUMULATIVE (2026) RECOMMENDED IMPROVEMENTS

This section provides a summary of deficiencies and recommended improvements for Opening Year Cumulative (2026) traffic conditions. Improvements necessary to improve the traffic deficiency back to acceptable levels is discussed below and is consistent with the 2023 Traffic Study.

Seal Beach Boulevard & Lampson Avenue (#1): The intersection of Seal Beach Boulevard and Lampson Avenue is anticipated to meet the applicable deficiency criteria for LOS. As shown in Table 4, restriping the number two westbound left turn lane as a shared left-right turn lane would improve the intersection operations back to acceptable LOS.

The intersection operations analysis worksheets for Opening Year Cumulative (2026) With Project Alternative traffic conditions, with improvements, are included in Attachment E.

TABLE 4: INTERSECTION ANALYSIS FOR OPENING YEAR CUMULATIVE (2026) CONDITIONS WITH IMPROVEMENTS

#	Intersection	Traffic Control ³	Intersection Approach Lanes ¹												Delay ² (secs.)		Level of Service	
			Northbound			Southbound			Eastbound			Westbound			AM	PM	AM	PM
			L	T	R	L	T	R	L	T	R	L	T	R				
1	Seal Beach Bl. & Lampson Av.																	
	- Without Improvements	TS	0	3	1>	2	3	0	0	0	0	2	0	1>	0.93	0.96	E	E
	- With Improvements ⁴	TS	0	3	1>	2	3	0	0	0	0	1	1	1>	0.74	0.82	C	D

¹ When a right turn is designated, the lane can either be striped or unstriped. To function as a right turn lane there must be sufficient width for right turning vehicles to travel outside the through lanes.

L = Left; T = Through; R = Right; > = Right-Turn Overlap Phasing; **1** = Improvement

² Per the Highway Capacity Manual 6th Edition, overall average intersection delay and level of service are shown for intersections with a traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown.

³ TS = Traffic Signal

⁴ Improvement includes restriping the number 2 westbound left as a shared left-right turn lane.

GENERAL PLAN BUILDOUT INTERSECTION OPERATIONS ANALYSIS

For the intersections of Seal Beach Boulevard at Lampson Avenue and Valley View Steet at Lampson Avenue the General Plan Buildout Without Project weekday AM and PM peak hour volumes are based on the OCTAM traffic model. For the remaining study intersections, General Plan Buildout forecasts include Existing traffic volumes plus an ambient growth factor of 14% plus traffic from pending and approved but not yet constructed known development projects in the area. The weekday ADT and weekday AM and PM peak hour volumes which can be expected for General Plan Buildout With Project traffic conditions for the alternative use are shown in Attachment F.

As shown on Table 5, the following study area intersection is anticipated to operate at an unacceptable LOS during the peak hours under General Plan Buildout With Project Alternative traffic conditions, consistent with the 2023 Traffic Study:

- Seal Beach Bl. & Lampson Av. (#1) – LOS F AM and PM peak hours

Although the intersection of Valley View Street at Lampson Avenue is anticipated to operate at a deficient LOS based on the ICU methodology, a more detailed peak hour intersection operations assessment based on the HCM methodology indicates acceptable peak hour operations. As such, no intersection improvements have been recommended at the intersection of Valley View Street and Lampson Avenue. The intersection operations analysis worksheets for General Plan Buildout With Project Alternative traffic conditions are included in Attachment G.

TABLE 5: INTERSECTION ANALYSIS FOR GENERAL PLAN BUILDOUT CONDITIONS

# Intersection	Traffic Control ²	GPBO Without Project				GPBO With Project							
		Delay (secs.) or ICU (v/c) ¹		Level of Service		Primary Use				Alternative Use			
		AM	PM	AM	PM	AM	PM	AM	PM	AM	PM	AM	PM
1 Seal Beach Bl. & Lampson Av.	TS	1.02	1.00	F	F	1.04	1.01	F	F	1.03	1.01	F	F
2 Old Ranch Plaza & Lampson Av.	TS	0.48	0.44	A	A	0.50	0.45	A	A	0.49	0.44	A	A
3 Basswood St. & Lampson Av.	TS	0.54	0.43	A	A	0.55	0.53	A	A	0.54	0.52	A	A
4 Candleberry Av. & Lampson Av.	TS	0.45	0.43	A	A	0.46	0.45	A	A	0.46	0.45	A	A
5 Heather St. & Lampson Av.	TS	0.38	0.43	A	A	0.40	0.45	A	A	0.40	0.45	A	A
6 Driveway 1 & Lampson Av.	CSS	Future Intersection				11.0	10.9	B	B	11.0	10.9	B	B
7 Driveway 2 & Lampson Av.	CSS	Future Intersection				15.3	16.1	C	C	15.0	15.8	C	C
8 Rose St. & Lampson Av.	TS	0.34	0.42	A	A	0.34	0.43	A	A	0.34	0.42	A	A
9 Tulip St. & Lampson Av.	TS	0.35	0.43	A	A	0.36	0.44	A	A	0.36	0.44	A	A
10 Valley View St. & Lampson Av.	TS	0.91	0.95	E	E	0.92	0.96	E	E	0.92	0.96	E	E
HCM Analysis:		52.3	49.8	D	D	54.0	52.4	D	D	53.8	52.1	D	D

* **BOLD** = Level of Service (LOS) does not meet the applicable jurisdictional requirements (i.e., unacceptable LOS).

¹ Per the Highway Capacity Manual (6th Edition), overall average intersection delay and level of service are shown for intersections with a traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown.

ICU is reported in volume-to-capacity (v/c). ICU analysis conducted for signalized intersections only.

² CSS = Cross-street Stop; TS = Traffic Signal; **CSS** = Improvement

GENERAL PLAN BUILDOUT RECOMMENDED IMPROVEMENTS

This section provides a summary of deficiencies and recommended improvements for General Plan Buildout traffic conditions. Improvements necessary to improve this traffic deficiency back to acceptable levels is discussed below and is consistent with the 2023 Traffic Study.

Seal Beach Boulevard & Lampson Avenue (#1): the same improvements as those identified previously for Opening Year Cumulative traffic conditions would improve the intersection operations back to acceptable LOS, as shown in Table 6.

The intersection operations analysis worksheets for General Plan Buildout With Project Alternative traffic conditions, with improvements, are included in Attachment H.

If you have any questions or comments, I can be reached at cso@urbanxroads.com.

TABLE 6: INTERSECTION ANALYSIS FOR GENERAL PLAN BUILDOUT CONDITIONS WITH IMPROVEMENTS

#	Intersection	Traffic Control ³	Intersection Approach Lanes ¹												Delay ² (secs.)		Level of Service	
			Northbound			Southbound			Eastbound			Westbound			AM	PM	AM	PM
			L	T	R	L	T	R	L	T	R	L	T	R				
1	Seal Beach Bl. & Lampson Av.																	
	- Without Improvements	TS	0	3	1>	2	3	0	0	0	0	2	0	1>	1.04	1.01	F	F
	- With Improvements ⁴	TS	0	3	1>	2	3	0	0	0	0	1	1	1>	0.86	0.89	D	D

¹ When a right turn is designated, the lane can either be striped or unstriped. To function as a right turn lane there must be sufficient width for right turning vehicles to travel outside the through lanes.

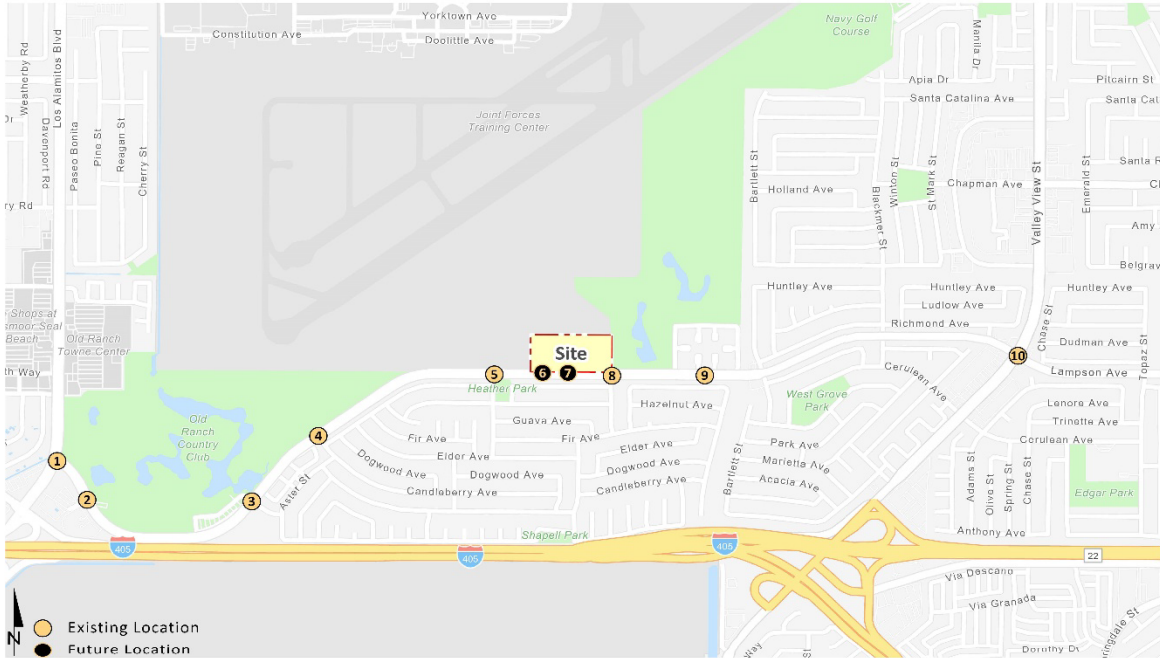
L = Left; T = Through; R = Right; > = Right-Turn Overlap Phasing; **1** = Improvement

² Per the Highway Capacity Manual 6th Edition, overall average intersection delay and level of service are shown for intersections with a traffic signal or all way stop control. For intersections with cross street stop control, the delay and level of service for the worst individual movement (or movements sharing a single lane) are shown.

³ TS = Traffic Signal

⁴ Improvement includes restriping the number 2 westbound left as a shared left-right turn lane.

ATTACHMENT A: PROJECT ALTERNATIVE ONLY TRAFFIC VOLUMES



1	Seal Beach Bl. & Lampson Av.	2	Old Ranch Plaza & Lampson Av.	3	Basswood St. & Lampson Av.	4	Candleberry Av. & Lampson Av.	5	Heather St. & Lampson Av.
300	850	850	850	850	850	850	850	850	850
↓ 5(16) ↑ 15(10) ↓ 26(17) ↑ 9(28)		← 40(27)		← 40(27)		← 40(27)		← 40(27)	
	550	14(45) →		14(45) →		14(45) →		14(45) →	
	850			850		850		850	
6	Driveway 1 & Lampson Av.	7	Driveway 2 & Lampson Av.	8	Rose St. & Lampson Av.	9	Tulip St. & Lampson Av.	10	Valley View St. & Lampson Av.
400	850	1,150	700	700	700	700	150	250	
↓ 18(12) ↑ 7(20) ↓ 22(15)		↓ 22(15) ↑ 33(22) ↓ 14(45)	↑ 5(16) ↓ 7(20)	← 12(36)		← 12(36)	↓ 3(8)	← 4(12)	
	14(45) →			33(22) →		33(22) →	7(5) → 11(7) → 15(10) ↓	↑ 5(16) ↓ 5(16)	
	850	850	700	700	700	700	700	300	

##(##) AM(PM) Peak Hour Intersection Volumes
 ## Average Daily Trips

**ATTACHMENT B: SITE ADJACENT QUEUING ANALYSIS
WORKSHEETS**

Intersection: 6: Lampson Av. & Driveway 1

Movement	SB
Directions Served	R
Maximum Queue (ft)	41
Average Queue (ft)	13
95th Queue (ft)	38
Link Distance (ft)	176
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 7: Lampson Av. & Driveway 2

Movement	EB	SB
Directions Served	L	LR
Maximum Queue (ft)	29	71
Average Queue (ft)	4	29
95th Queue (ft)	21	58
Link Distance (ft)		154
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	150	
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 0

Intersection: 6: Lampson Av. & Driveway 1

Movement	SB
Directions Served	R
Maximum Queue (ft)	31
Average Queue (ft)	10
95th Queue (ft)	34
Link Distance (ft)	176
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

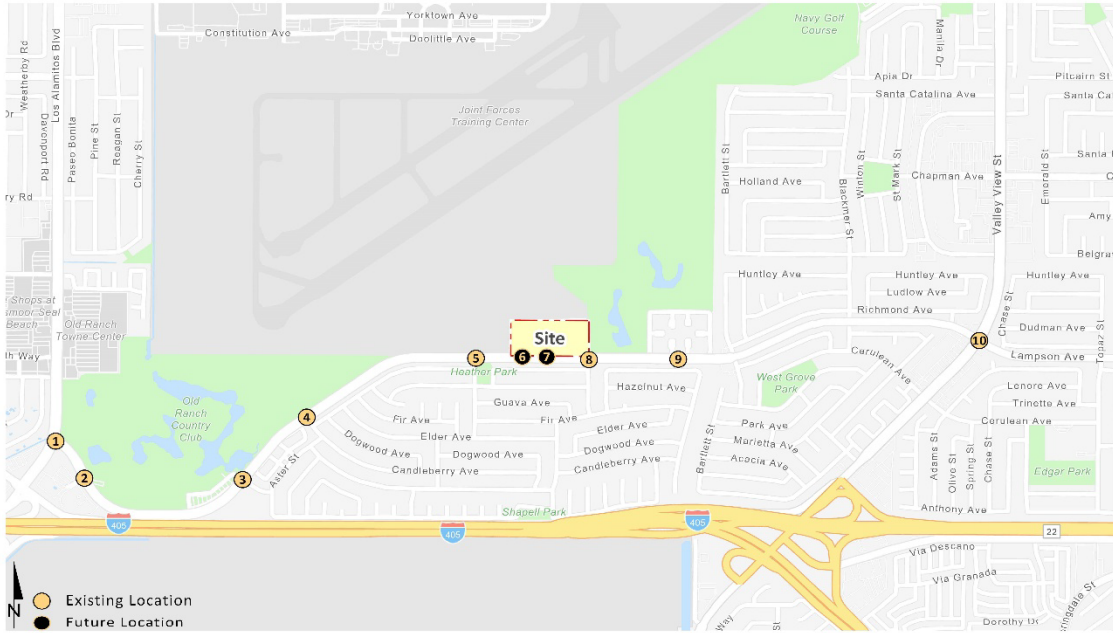
Intersection: 7: Lampson Av. & Driveway 2

Movement	EB	SB
Directions Served	L	LR
Maximum Queue (ft)	44	66
Average Queue (ft)	14	23
95th Queue (ft)	39	52
Link Distance (ft)		154
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)	150	
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 0

**ATTACHMENT C: OPENING YEAR CUMULATIVE (2026) WITH
PROJECT ALTERNATIVE TRAFFIC VOLUMES**



1 Seal Beach Bl. & Lampson Av.	2 Old Ranch Plaza & Lampson Av.	3 Basswood St. & Lampson Av.	4 Candleberry Av. & Lampson Av.	5 Heather St. & Lampson Av.	
44,450 ← 1447(1367) ← 440(535) ↑ 661(530) ← 426(395) 1345(1721) → 325(512) → 42,800	850 34(20) 0(1) 6(3) 5(6) 1022(860) 11(8) 18(37) → 712(974) → 35(37) ↓ 31(45) → 3(0) → 8(11) → 1,100	850 8(41) 0(3) 1(4) 6(3) 860(693) 10(12) 19(28) → 532(778) → 86(126) ↓ 221(134) → 1(0) → 3(11) → 3,050	16,050 691(632) 22(30) 453(673) → 83(120) ↓ 185(76) → 37(24) → 2,650	14,350 624(606) 37(74) 423(632) → 67(65) ↓ 90(56) → 66(55) → 2,650	
21,200	21,200	19,250	16,050	14,550	
6 Driveway 1 & Lampson Av.	7 Driveway 2 & Lampson Av.	8 Rose St. & Lampson Av.	9 Tulip St. & Lampson Av.	10 Valley View St. & Lampson Av.	
400 18(12) 479(704) → 14,450	1,150 22(15) 33(22) 5(16) 633(639) 14(45) → 465(659) → 14,450	14,300 598(624) 32(74) 472(646) → 26(36) ↓ 40(30) → 56(60) → 14,300	15,000 598(624) 32(74) 598(624) 32(74) 40(30) → 56(60) → 2,100	550 15(12) 1(0) 13(11) 10(20) 572(658) 40(86) 5(8) → 509(669) → 12(29) ↓ 42(29) → 1(0) → 85(70) → 2,250	16,150 10(20) 572(658) 40(86) 203(259) → 264(223) → 170(148) ↓ 146(238) → 146(238) → 187(161) → 11,650 45,450
14,450	14,450	14,300	2,100	15,000	

##(##) AM(PM) Peak Hour Intersection Volumes
Average Daily Trips

**ATTACHMENT D: OPENING YEAR CUMULATIVE (2026) WITH
PROJECT ALTERNATIVE CONDITIONS INTERSECTION
OPERATION ANALYSIS WORKSHEETS**

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #1 Seal Beach & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.931
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 118 Level Of Service: E

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Protected Protected Protected
Rights: Ovl Include Include Ovl
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 0 3 0 1 2 0 3 0 0 0 0 0 0 0 2 0 0 0 1

Volume Module:
Base Vol: 0 1199 266 364 1244 0 0 0 0 340 0 572
Growth Adj: 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08
Initial Bse: 0 1298 288 394 1347 0 0 0 0 368 0 619
Added Vol: 0 47 37 46 100 0 0 0 0 57 0 42
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 0 1345 325 440 1447 0 0 0 0 425 0 661
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 0 1345 325 440 1447 0 0 0 0 425 0 661
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 0 1345 325 440 1447 0 0 0 0 425 0 661
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 0 1345 325 440 1447 0 0 0 0 425 0 661
OvlAdjVol: 112

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.00 3.00 1.00 2.00 3.00 0.00 0.00 0.00 0.00 2.00 0.00 1.00
Final Sat.: 0 4800 1600 3200 4800 0 0 0 0 3200 0 1600

Capacity Analysis Module:
Vol/Sat: 0.00 0.28 0.20 0.14 0.30 0.00 0.00 0.00 0.00 0.13 0.00 0.41
OvlAdjV/S: 0.07
Crit Moves: **** * 0.07 ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #2 Old Ranch Plaza & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.477
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 32 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Protected Protected
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 1 0 0 1 0 0 1! 0 0 1 0 2 0 1 1 0 1 1 0

Volume Module:
Base Vol: 29 3 7 1 0 3 7 591 32 10 880 3
Growth Adj: 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08
Initial Bse: 31 3 8 1 0 3 8 640 35 11 953 3
Added Vol: 0 0 0 5 0 31 10 72 0 0 69 2
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 31 3 8 6 0 34 18 712 35 11 1022 5
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 31 3 8 6 0 34 18 712 35 11 1022 5
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 31 3 8 6 0 34 18 712 35 11 1022 5
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 31 3 8 6 0 34 18 712 35 11 1022 5

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.91 0.09 1.00 0.15 0.00 0.85 1.00 2.00 1.00 1.00 1.99 0.01
Final Sat.: 1450 150 1600 241 0 1359 1600 3200 1600 1600 3184 16

Capacity Analysis Module:
Vol/Sat: 0.02 0.02 0.00 0.00 0.00 0.03 0.01 0.22 0.02 0.01 0.32 0.32
Crit Moves: **** **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #3 Basswood & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.526
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 35 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Protected Protected
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 1 0 0 1 0 0 0 1! 0 0 1 0 1 1 0 1 0 1 1 0

Volume Module:
Base Vol: 204 1 3 1 0 7 18 420 79 9 730 6
Growth Adj: 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08
Initial Bse: 221 1 3 1 0 8 19 455 86 10 790 6
Added Vol: 0 0 0 0 0 0 0 0 78 0 0 70 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 221 1 3 1 0 8 19 533 86 10 860 6
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 221 1 3 1 0 8 19 533 86 10 860 6
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 221 1 3 1 0 8 19 533 86 10 860 6
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 221 1 3 1 0 8 19 533 86 10 860 6

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 0.25 0.75 0.12 0.00 0.88 1.00 1.72 0.28 1.00 1.99 0.01
Final Sat.: 1600 400 1200 200 0 1400 1600 2757 443 1600 3176 24

Capacity Analysis Module:
Vol/Sat: 0.14 0.00 0.00 0.00 0.00 0.01 0.01 0.19 0.19 0.01 0.27 0.27
Crit Moves: **** **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #4 Candleberry & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.455
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 31 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 0 1 0 0 0 0 0 0 0 1 1 0 1 0 2 0 0

Volume Module:
Base Vol: 171 0 29 0 0 0 0 347 77 16 574 0
Growth Adj: 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08
Initial Bse: 185 0 31 0 0 0 0 376 83 17 621 0
Added Vol: 0 0 6 0 0 0 0 78 0 5 70 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 185 0 37 0 0 0 0 454 83 22 691 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 185 0 37 0 0 0 0 454 83 22 691 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 185 0 37 0 0 0 0 454 83 22 691 0
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 185 0 37 0 0 0 0 454 83 22 691 0

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.83 0.00 0.17 0.00 0.00 0.00 0.00 1.69 0.31 1.00 2.00 0.00
Final Sat.: 1331 0 269 0 0 0 0 2703 497 1600 3200 0

Capacity Analysis Module:
Vol/Sat: 0.12 0.00 0.14 0.00 0.00 0.00 0.00 0.17 0.17 0.01 0.22 0.00
Crit Moves: **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #5 Heather & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.392
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 28 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 0 1 0 0 0 0 0 0 0 1 1 0 1 0 2 0 0

Volume Module:
Base Vol: 83 0 55 0 0 0 0 314 62 30 507 0
Growth Adj: 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08
Initial Bse: 90 0 60 0 0 0 0 340 67 32 549 0
Added Vol: 0 0 6 0 0 0 0 83 0 5 75 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 90 0 66 0 0 0 0 423 67 37 624 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 90 0 66 0 0 0 0 423 67 37 624 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 90 0 66 0 0 0 0 423 67 37 624 0
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 90 0 66 0 0 0 0 423 67 37 624 0

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.58 0.00 0.42 0.00 0.00 0.00 0.00 1.73 0.27 1.00 2.00 0.00
Final Sat.: 925 0 675 0 0 0 0 2762 438 1600 3200 0

Capacity Analysis Module:
Vol/Sat: 0.06 0.00 0.10 0.00 0.00 0.00 0.00 0.15 0.15 0.02 0.19 0.00
Crit Moves: **** **** ****

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑			↑
Traffic Vol, veh/h	0	479	648	7	0	18
Future Vol, veh/h	0	479	648	7	0	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	521	704	8	0	20

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	-	0	-
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	-
Pot Cap-1 Maneuver	0	-	-
Stage 1	0	-	-
Stage 2	0	-	-
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	10.8
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	640
HCM Lane V/C Ratio	-	-	-	0.031
HCM Control Delay (s)	-	-	-	10.8
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

Intersection						
Int Delay, s/veh	0.8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	14	465	633	5	33	22
Future Vol, veh/h	14	465	633	5	33	22
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	1	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	15	505	688	5	36	24

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	693	0	-	0	974 347
Stage 1	-	-	-	-	691 -
Stage 2	-	-	-	-	283 -
Critical Hdwy	4.14	-	-	-	6.84 6.94
Critical Hdwy Stg 1	-	-	-	-	5.84 -
Critical Hdwy Stg 2	-	-	-	-	5.84 -
Follow-up Hdwy	2.22	-	-	-	3.52 3.32
Pot Cap-1 Maneuver	898	-	-	-	249 649
Stage 1	-	-	-	-	459 -
Stage 2	-	-	-	-	740 -
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	898	-	-	-	245 649
Mov Cap-2 Maneuver	-	-	-	-	356 -
Stage 1	-	-	-	-	451 -
Stage 2	-	-	-	-	740 -

Approach	EB	WB	SB
HCM Control Delay, s	0.3	0	14.6
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	898	-	-	-	434
HCM Lane V/C Ratio	0.017	-	-	-	0.138
HCM Control Delay (s)	9.1	-	-	-	14.6
HCM Lane LOS	A	-	-	-	B
HCM 95th %tile Q(veh)	0.1	-	-	-	0.5

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #8 Rose & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.347
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 27 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 0 1 0 0 0 0 0 0 0 1 1 0 1 0 2 0 0

Volume Module:
Base Vol: 37 0 46 0 0 0 0 0 336 24 25 505 0
Growth Adj: 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08
Initial Bse: 40 0 50 0 0 0 0 0 364 26 27 547 0
Added Vol: 0 0 6 0 0 0 0 0 107 0 5 51 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 40 0 56 0 0 0 0 0 471 26 32 598 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 40 0 56 0 0 0 0 0 471 26 32 598 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 40 0 56 0 0 0 0 0 471 26 32 598 0
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 40 0 56 0 0 0 0 0 471 26 32 598 0

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.41 0.01 0.58 0.00 0.00 0.00 0.00 1.90 0.10 1.00 2.00 0.00
Final Sat.: 669 0 931 0 0 0 0 0 3033 167 1600 3200 0

Capacity Analysis Module:
Vol/Sat: 0.03 0.00 0.06 0.00 0.00 0.00 0.00 0.16 0.16 0.02 0.19 0.00
Crit Moves: **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #9 Tulip & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.349
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 27 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 1 0 0 1 0 0 0 1! 0 0 1 0 1 1 0 1 0 1 1 0

Volume Module:
Base Vol: 39 1 73 10 1 14 5 366 11 32 477 7
Growth Adj: 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08
Initial Bse: 42 1 79 11 1 15 5 396 12 35 516 8
Added Vol: 0 0 6 2 0 0 0 113 0 5 56 2
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 42 1 85 13 1 15 5 509 12 40 572 10
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 42 1 85 13 1 15 5 509 12 40 572 10
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 42 1 85 13 1 15 5 509 12 40 572 10
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 42 1 85 13 1 15 5 509 12 40 572 10

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 0.01 0.99 0.44 0.04 0.52 1.00 1.95 0.05 1.00 1.97 0.03
Final Sat.: 1600 20 1580 706 60 834 1600 3127 73 1600 3147 53

Capacity Analysis Module:
Vol/Sat: 0.03 0.05 0.05 0.01 0.02 0.02 0.00 0.16 0.16 0.02 0.18 0.18
Crit Moves: **** **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #10 Valley View & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.806
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 68 Level Of Service: D

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, Y+R, and Lanes.

Volume Module: Table with 12 columns representing different volume metrics and 12 rows of data.

Saturation Flow Module: Table with 12 columns representing saturation flow metrics and 4 rows of data.

Capacity Analysis Module: Table with 12 columns representing capacity analysis metrics and 2 rows of data.

Timings
10: Valley View St. & Lampson Av.

4665 Lampson (JN 14501)

06/01/2023

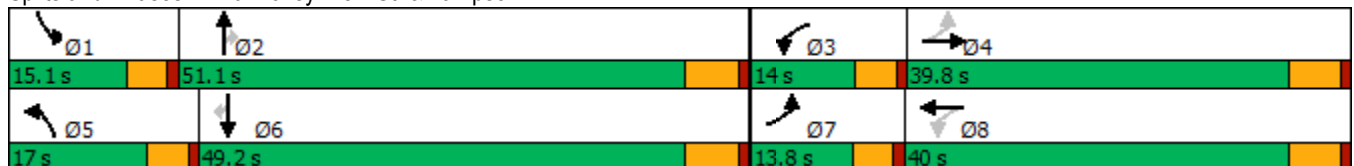


Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↕	↖	↕	↖	↕↕↕	↖	↖	↕↕↕	↖
Traffic Volume (vph)	203	264	234	303	146	1540	187	124	1676	191
Future Volume (vph)	203	264	234	303	146	1540	187	124	1676	191
Turn Type	pm+pt	NA	pm+pt	NA	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4	3	8	5	2		1	6	
Permitted Phases	4		8				2			6
Detector Phase	7	4	3	8	5	2	2	1	6	6
Switch Phase										
Minimum Initial (s)	5.0	10.0	5.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	9.6	39.8	9.6	39.8	9.6	27.8	27.8	9.6	27.8	27.8
Total Split (s)	13.8	39.8	14.0	40.0	17.0	51.1	51.1	15.1	49.2	49.2
Total Split (%)	11.5%	33.2%	11.7%	33.3%	14.2%	42.6%	42.6%	12.6%	41.0%	41.0%
Yellow Time (s)	3.6	4.8	3.6	4.8	3.6	4.8	4.8	3.6	4.8	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.8	4.6	5.8	4.6	5.8	5.8	4.6	5.8	5.8
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	None	None	None
Act Effct Green (s)	30.9	20.5	31.3	20.7	12.0	45.3	45.3	10.3	43.6	43.6
Actuated g/C Ratio	0.29	0.19	0.29	0.19	0.11	0.43	0.43	0.10	0.41	0.41
v/c Ratio	0.95	0.64	0.94	0.74	0.80	0.77	0.28	0.79	0.87	0.30
Control Delay	78.6	32.6	74.2	35.1	76.3	30.2	11.5	79.8	35.7	12.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	78.6	32.6	74.2	35.1	76.3	30.2	11.5	79.8	35.7	12.4
LOS	E	C	E	D	E	C	B	E	D	B
Approach Delay		47.3		47.3		32.0			36.2	
Approach LOS		D		D		C			D	

Intersection Summary

Cycle Length: 120	
Actuated Cycle Length: 106.4	
Natural Cycle: 110	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 0.95	
Intersection Signal Delay: 37.6	Intersection LOS: D
Intersection Capacity Utilization 86.5%	ICU Level of Service E
Analysis Period (min) 15	

Splits and Phases: 10: Valley View St. & Lampson Av.



HCM 6th Signalized Intersection Summary
 10: Valley View St. & Lampson Av.

4665 Lampson (JN 14501)

06/01/2023



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↗	↗↘		↗	↗↘		↗	↑↑↑	↗	↗	↑↑↑	↗
Traffic Volume (veh/h)	203	264	170	234	303	214	146	1540	187	124	1676	191
Future Volume (veh/h)	203	264	170	234	303	214	146	1540	187	124	1676	191
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.98	1.00		0.97	1.00		0.98	1.00		0.99
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	221	287	147	254	329	175	159	1674	140	135	1822	159
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	277	446	222	304	441	229	189	2146	655	164	2074	635
Arrive On Green	0.09	0.20	0.20	0.09	0.20	0.20	0.11	0.42	0.42	0.09	0.41	0.41
Sat Flow, veh/h	1781	2285	1136	1781	2237	1160	1781	5106	1558	1781	5106	1564
Grp Volume(v), veh/h	221	221	213	254	260	244	159	1674	140	135	1822	159
Grp Sat Flow(s),veh/h/ln	1781	1777	1644	1781	1777	1620	1781	1702	1558	1781	1702	1564
Q Serve(g_s), s	9.2	11.8	12.3	9.4	14.2	14.7	9.0	29.2	5.9	7.7	34.0	6.9
Cycle Q Clear(g_c), s	9.2	11.8	12.3	9.4	14.2	14.7	9.0	29.2	5.9	7.7	34.0	6.9
Prop In Lane	1.00		0.69	1.00		0.72	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	277	347	321	304	350	319	189	2146	655	164	2074	635
V/C Ratio(X)	0.80	0.64	0.66	0.84	0.74	0.77	0.84	0.78	0.21	0.82	0.88	0.25
Avail Cap(c_a), veh/h	277	585	542	304	589	537	214	2241	684	181	2147	658
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	33.2	38.2	38.4	34.6	39.0	39.2	45.3	25.8	19.1	46.0	28.3	20.3
Incr Delay (d2), s/veh	13.9	2.0	2.3	17.0	3.1	3.8	20.6	1.8	0.2	21.5	4.4	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.3	5.1	5.0	3.0	6.2	5.9	4.9	11.2	2.0	4.3	13.6	2.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	47.2	40.1	40.8	51.7	42.1	43.0	65.9	27.6	19.2	67.5	32.7	20.5
LnGrp LOS	D	D	D	D	D	D	E	C	B	E	C	C
Approach Vol, veh/h		655			758			1973			2116	
Approach Delay, s/veh		42.7			45.6			30.1			34.0	
Approach LOS		D			D			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	14.1	49.2	14.0	25.9	15.6	47.7	13.8	26.1				
Change Period (Y+Rc), s	4.6	5.8	4.6	5.8	4.6	5.8	4.6	5.8				
Max Green Setting (Gmax), s	10.5	45.3	9.4	34.0	12.4	43.4	9.2	34.2				
Max Q Clear Time (g_c+I1), s	9.7	31.2	11.4	14.3	11.0	36.0	11.2	16.7				
Green Ext Time (p_c), s	0.0	9.4	0.0	2.3	0.0	5.9	0.0	2.6				
Intersection Summary												
HCM 6th Ctrl Delay				35.2								
HCM 6th LOS				D								

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #1 Seal Beach & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.957
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 140 Level Of Service: E

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Protected Protected Protected
Rights: Ovl Include Include Ovl
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 0 3 0 1 2 0 3 0 0 0 0 0 0 0 2 0 0 0 1

Volume Module:
Base Vol: 0 1508 413 447 1202 0 0 0 0 322 0 447
Growth Adj: 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08
Initial Bse: 0 1632 447 484 1301 0 0 0 0 349 0 484
Added Vol: 0 89 65 52 66 0 0 0 0 46 0 46
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 0 1721 512 536 1367 0 0 0 0 395 0 530
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 0 1721 512 536 1367 0 0 0 0 395 0 530
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 0 1721 512 536 1367 0 0 0 0 395 0 530
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 0 1721 512 536 1367 0 0 0 0 395 0 530
OvlAdjVol: 315

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.00 3.00 1.00 2.00 3.00 0.00 0.00 0.00 0.00 2.00 0.00 1.00
Final Sat.: 0 4800 1600 3200 4800 0 0 0 0 3200 0 1600

Capacity Analysis Module:
Vol/Sat: 0.00 0.36 0.32 0.17 0.28 0.00 0.00 0.00 0.00 0.12 0.00 0.33
OvlAdjV/S: 0.20
Crit Moves: **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #2 Old Ranch Plaza & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.452
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 31 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Protected Protected
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 1 0 0 1 0 0 1! 0 0 1 0 2 0 1 1 0 1 1 0

Volume Module:
Base Vol: 42 0 10 0 1 0 4 822 34 7 727 0
Growth Adj: 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08
Initial Bse: 45 0 11 0 1 0 4 890 37 8 787 0
Added Vol: 0 0 0 3 0 20 33 83 0 0 73 6
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 45 0 11 3 1 20 37 973 37 8 860 6
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 45 0 11 3 1 20 37 973 37 8 860 6
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 45 0 11 3 1 20 37 973 37 8 860 6
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 45 0 11 3 1 20 37 973 37 8 860 6

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 0.00 1.00 0.12 0.04 0.84 1.00 2.00 1.00 1.00 1.99 0.01
Final Sat.: 1600 0 1600 199 72 1329 1600 3200 1600 1600 3178 22

Capacity Analysis Module:
Vol/Sat: 0.03 0.00 0.01 0.00 0.02 0.02 0.02 0.30 0.02 0.00 0.27 0.27
Crit Moves: **** **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #3 Basswood & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.504
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 34 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Protected Protected
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 1 0 0 1 0 0 0 1! 0 0 1 0 1 1 0 1 0 1 1 0

Volume Module:
Base Vol: 124 0 10 4 3 38 26 638 116 11 568 3
Growth Adj: 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08
Initial Bse: 134 0 11 4 3 41 28 691 126 12 615 3
Added Vol: 0 0 0 0 0 0 0 87 0 0 78 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 134 0 11 4 3 41 28 778 126 12 693 3
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 134 0 11 4 3 41 28 778 126 12 693 3
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 134 0 11 4 3 41 28 778 126 12 693 3
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 134 0 11 4 3 41 28 778 126 12 693 3

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 0.00 1.00 0.09 0.07 0.84 1.00 1.72 0.28 1.00 1.99 0.01
Final Sat.: 1600 0 1600 142 107 1351 1600 2755 445 1600 3185 15

Capacity Analysis Module:
Vol/Sat: 0.08 0.00 0.01 0.00 0.03 0.03 0.02 0.28 0.28 0.01 0.22 0.22
Crit Moves: **** **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #4 Candleberry & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.429
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 30 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 0 1 0 0 0 0 0 0 0 1 1 0 1 0 2 0 0

Volume Module:
Base Vol: 70 0 18 0 0 0 0 541 111 24 512 0
Growth Adj: 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08
Initial Bse: 76 0 19 0 0 0 0 586 120 26 554 0
Added Vol: 0 0 5 0 0 0 0 87 0 4 78 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 76 0 24 0 0 0 0 673 120 30 632 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 76 0 24 0 0 0 0 673 120 30 632 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 76 0 24 0 0 0 0 673 120 30 632 0
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 76 0 24 0 0 0 0 673 120 30 632 0

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.76 0.00 0.24 0.00 0.00 0.00 0.00 1.70 0.30 1.00 2.00 0.00
Final Sat.: 1209 0 391 0 0 0 0 2715 485 1600 3200 0

Capacity Analysis Module:
Vol/Sat: 0.05 0.00 0.06 0.00 0.00 0.00 0.00 0.25 0.25 0.02 0.20 0.00
Crit Moves: **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #5 Heather & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.434
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 30 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 0 1 0 0 0 0 0 0 0 1 1 0 1 0 2 0 0

Volume Module:
Base Vol: 52 0 46 0 0 0 0 0 499 60 65 484 0
Growth Adj: 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08
Initial Bse: 56 0 50 0 0 0 0 0 540 65 70 524 0
Added Vol: 0 0 5 0 0 0 0 0 92 0 4 82 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 56 0 55 0 0 0 0 0 632 65 74 606 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 56 0 55 0 0 0 0 0 632 65 74 606 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 56 0 55 0 0 0 0 0 632 65 74 606 0
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 56 0 55 0 0 0 0 0 632 65 74 606 0

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.51 0.00 0.49 0.00 0.00 0.00 0.00 1.81 0.19 1.00 2.00 0.00
Final Sat.: 811 0 789 0 0 0 0 2902 298 1600 3200 0

Capacity Analysis Module:
Vol/Sat: 0.04 0.00 0.07 0.00 0.00 0.00 0.00 0.22 0.22 0.05 0.19 0.00
Crit Moves: **** **** ****

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑			↑
Traffic Vol, veh/h	0	704	634	20	0	12
Future Vol, veh/h	0	704	634	20	0	12
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	765	689	22	0	13

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	-	0	-
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	-
Pot Cap-1 Maneuver	0	-	-
Stage 1	0	-	-
Stage 2	0	-	-
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	10.7
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	640
HCM Lane V/C Ratio	-	-	-	0.02
HCM Control Delay (s)	-	-	-	10.7
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

Intersection						
Int Delay, s/veh	0.7					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	45	659	639	16	22	15
Future Vol, veh/h	45	659	639	16	22	15
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	1	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	49	716	695	17	24	16

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	712	0	-	0	1160 356
Stage 1	-	-	-	-	704 -
Stage 2	-	-	-	-	456 -
Critical Hdwy	4.14	-	-	-	6.84 6.94
Critical Hdwy Stg 1	-	-	-	-	5.84 -
Critical Hdwy Stg 2	-	-	-	-	5.84 -
Follow-up Hdwy	2.22	-	-	-	3.52 3.32
Pot Cap-1 Maneuver	884	-	-	-	189 640
Stage 1	-	-	-	-	452 -
Stage 2	-	-	-	-	605 -
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	884	-	-	-	179 640
Mov Cap-2 Maneuver	-	-	-	-	306 -
Stage 1	-	-	-	-	427 -
Stage 2	-	-	-	-	605 -

Approach	EB	WB	SB
HCM Control Delay, s	0.6	0	15.3
HCM LOS			C

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	884	-	-	-	388
HCM Lane V/C Ratio	0.055	-	-	-	0.104
HCM Control Delay (s)	9.3	-	-	-	15.3
HCM Lane LOS	A	-	-	-	C
HCM 95th %tile Q(veh)	0.2	-	-	-	0.3

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #8 Rose & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.416
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 29 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 0 1 0 0 0 0 0 0 0 1 1 0 1 0 2 0 0

Volume Module:
Base Vol: 28 0 51 0 0 0 0 0 528 33 65 489 0
Growth Adj: 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08
Initial Bse: 30 0 55 0 0 0 0 0 572 36 70 529 0
Added Vol: 0 0 5 0 0 0 0 0 74 0 4 96 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 30 0 60 0 0 0 0 0 646 36 74 625 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 30 0 60 0 0 0 0 0 646 36 74 625 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 30 0 60 0 0 0 0 0 646 36 74 625 0
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 30 0 60 0 0 0 0 0 646 36 74 625 0

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.33 0.00 0.67 0.00 0.00 0.00 0.00 1.90 0.10 1.00 2.00 0.00
Final Sat.: 536 0 1064 0 0 0 0 3032 168 1600 3200 0

Capacity Analysis Module:
Vol/Sat: 0.02 0.00 0.06 0.00 0.00 0.00 0.00 0.21 0.21 0.05 0.20 0.00
Crit Moves: **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #9 Tulip & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.422
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 30 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 1 0 0 1 0 0 0 1! 0 0 1 0 1 1 0

Volume Module:
Base Vol: 27 0 60 8 0 11 7 545 27 76 516 17
Growth Adj: 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08
Initial Bse: 29 0 65 9 0 12 8 590 29 82 559 18
Added Vol: 0 0 5 2 0 0 0 79 0 4 100 2
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 29 0 70 11 0 12 8 669 29 86 659 20
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 29 0 70 11 0 12 8 669 29 86 659 20
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 29 0 70 11 0 12 8 669 29 86 659 20
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 29 0 70 11 0 12 8 669 29 86 659 20

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 0.00 1.00 0.47 0.00 0.53 1.00 1.92 0.08 1.00 1.94 0.06
Final Sat.: 1600 0 1600 756 0 844 1600 3066 134 1600 3104 96

Capacity Analysis Module:
Vol/Sat: 0.02 0.00 0.04 0.01 0.00 0.01 0.00 0.22 0.22 0.05 0.21 0.21
Crit Moves: **** **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #10 Valley View & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.921
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 112 Level Of Service: E

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, Y+R, and Lanes.

-----|-----|-----|-----|

Volume Module: Table with 12 columns representing different volume metrics and 12 rows of data.

-----|-----|-----|-----|

Saturation Flow Module: Table with 12 columns representing saturation flow metrics and 4 rows of data.

-----|-----|-----|-----|

Capacity Analysis Module: Table with 12 columns representing capacity analysis metrics and 2 rows of data.

Timings
10: Valley View St. & Lampson Av.

4665 Lampson (JN 14501)

06/01/2023

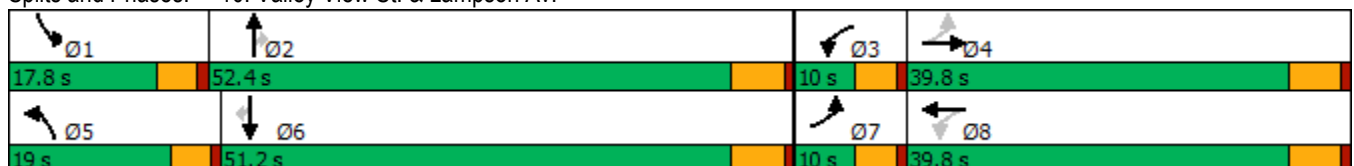


Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations										
Traffic Volume (vph)	259	223	214	237	238	1704	161	172	1844	243
Future Volume (vph)	259	223	214	237	238	1704	161	172	1844	243
Turn Type	pm+pt	NA	pm+pt	NA	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4	3	8	5	2		1	6	
Permitted Phases	4		8				2			6
Detector Phase	7	4	3	8	5	2	2	1	6	6
Switch Phase										
Minimum Initial (s)	5.0	10.0	5.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	9.6	39.8	9.6	39.8	9.6	27.8	27.8	9.6	27.8	27.8
Total Split (s)	10.0	39.8	10.0	39.8	19.0	52.4	52.4	17.8	51.2	51.2
Total Split (%)	8.3%	33.2%	8.3%	33.2%	15.8%	43.7%	43.7%	14.8%	42.7%	42.7%
Yellow Time (s)	3.6	4.8	3.6	4.8	3.6	4.8	4.8	3.6	4.8	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.8	4.6	5.8	4.6	5.8	5.8	4.6	5.8	5.8
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	None	None	None
Act Effct Green (s)	23.4	16.8	23.4	16.8	14.5	47.2	47.2	12.9	45.7	45.7
Actuated g/C Ratio	0.23	0.16	0.23	0.16	0.14	0.46	0.46	0.12	0.44	0.44
v/c Ratio	1.24	0.59	1.05	0.61	0.99	0.76	0.22	0.80	0.85	0.33
Control Delay	171.2	29.2	110.2	35.5	100.8	27.1	8.3	71.4	31.2	12.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	171.2	29.2	110.2	35.5	100.8	27.1	8.3	71.4	31.2	12.4
LOS	F	C	F	D	F	C	A	E	C	B
Approach Delay		87.5		63.4		34.0			32.2	
Approach LOS		F		E		C			C	

Intersection Summary

Cycle Length: 120	
Actuated Cycle Length: 103.3	
Natural Cycle: 120	
Control Type: Actuated-Uncoordinated	
Maximum v/c Ratio: 1.24	
Intersection Signal Delay: 42.4	Intersection LOS: D
Intersection Capacity Utilization 92.2%	ICU Level of Service F
Analysis Period (min) 15	





























Splits and Phases: 10: Valley View St. & Lampson Av.



HCM 6th Signalized Intersection Summary
 10: Valley View St. & Lampson Av.

4665 Lampson (JN 14501)

06/01/2023

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 			 			  			  	
Traffic Volume (veh/h)	259	223	148	214	237	122	238	1704	161	172	1844	243
Future Volume (veh/h)	259	223	148	214	237	122	238	1704	161	172	1844	243
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		0.98	1.00		0.99	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	267	230	119	221	244	104	245	1757	127	177	1901	173
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	221	336	167	220	358	147	262	2394	733	209	2242	695
Arrive On Green	0.06	0.15	0.15	0.06	0.15	0.15	0.15	0.47	0.47	0.12	0.44	0.44
Sat Flow, veh/h	1781	2291	1142	1781	2439	1006	1781	5106	1564	1781	5106	1583
Grp Volume(v), veh/h	267	176	173	221	176	172	245	1757	127	177	1901	173
Grp Sat Flow(s),veh/h/ln	1781	1777	1656	1781	1777	1668	1781	1702	1564	1781	1702	1583
Q Serve(g_s), s	5.4	9.2	9.7	5.4	9.2	9.6	13.3	27.3	4.6	9.5	32.6	6.7
Cycle Q Clear(g_c), s	5.4	9.2	9.7	5.4	9.2	9.6	13.3	27.3	4.6	9.5	32.6	6.7
Prop In Lane	1.00		0.69	1.00		0.60	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	221	261	243	220	261	245	262	2394	733	209	2242	695
V/C Ratio(X)	1.21	0.68	0.71	1.00	0.67	0.71	0.94	0.73	0.17	0.85	0.85	0.25
Avail Cap(c_a), veh/h	221	616	575	220	616	579	262	2428	744	240	2365	733
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	40.9	39.6	39.8	40.9	39.6	39.8	41.3	21.1	15.0	42.4	24.6	17.3
Incr Delay (d2), s/veh	127.5	3.1	3.8	61.3	3.0	3.7	38.2	1.2	0.1	19.3	3.0	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	10.4	4.1	4.0	6.3	4.0	4.0	8.4	10.0	1.5	5.1	12.4	2.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	168.4	42.7	43.7	102.2	42.6	43.5	79.5	22.2	15.2	61.7	27.5	17.5
LnGrp LOS	F	D	D	F	D	D	E	C	B	E	C	B
Approach Vol, veh/h		616			569			2129			2251	
Approach Delay, s/veh		97.4			66.0			28.4			29.4	
Approach LOS		F			E			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	16.1	51.7	10.0	20.2	19.0	48.8	10.0	20.2				
Change Period (Y+Rc), s	4.6	5.8	4.6	5.8	4.6	5.8	4.6	5.8				
Max Green Setting (Gmax), s	13.2	46.6	5.4	34.0	14.4	45.4	5.4	34.0				
Max Q Clear Time (g_c+I1), s	11.5	29.3	7.4	11.7	15.3	34.6	7.4	11.6				
Green Ext Time (p_c), s	0.0	11.3	0.0	1.8	0.0	8.4	0.0	1.8				
Intersection Summary												
HCM 6th Ctrl Delay			40.3									
HCM 6th LOS			D									

**ATTACHMENT E: OPENING YEAR CUMULATIVE (2026) WITH
PROJECT ALTERNATIVE CONDITIONS INTERSECTION
OPERATION ANALYSIS WORKSHEETS WITH
IMPROVEMENTS**

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #1 Seal Beach & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.744
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 56 Level Of Service: C

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Permitted, Protected), Rights (Ovl, Include), and various traffic volume metrics (Min. Green, Y+R, Lanes).

-----|-----|-----|-----|

Volume Module: Table showing traffic volume metrics such as Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume, and OvlAdjVol.

-----|-----|-----|-----|

Saturation Flow Module: Table showing saturation flow metrics like Sat/Lane, Adjustment, Lanes, and Final Sat.

-----|-----|-----|-----|

Capacity Analysis Module: Table showing capacity analysis metrics like Vol/Sat, OvlAdjV/S, and Crit Moves.

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #1 Seal Beach & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.819
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 71 Level Of Service: D

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

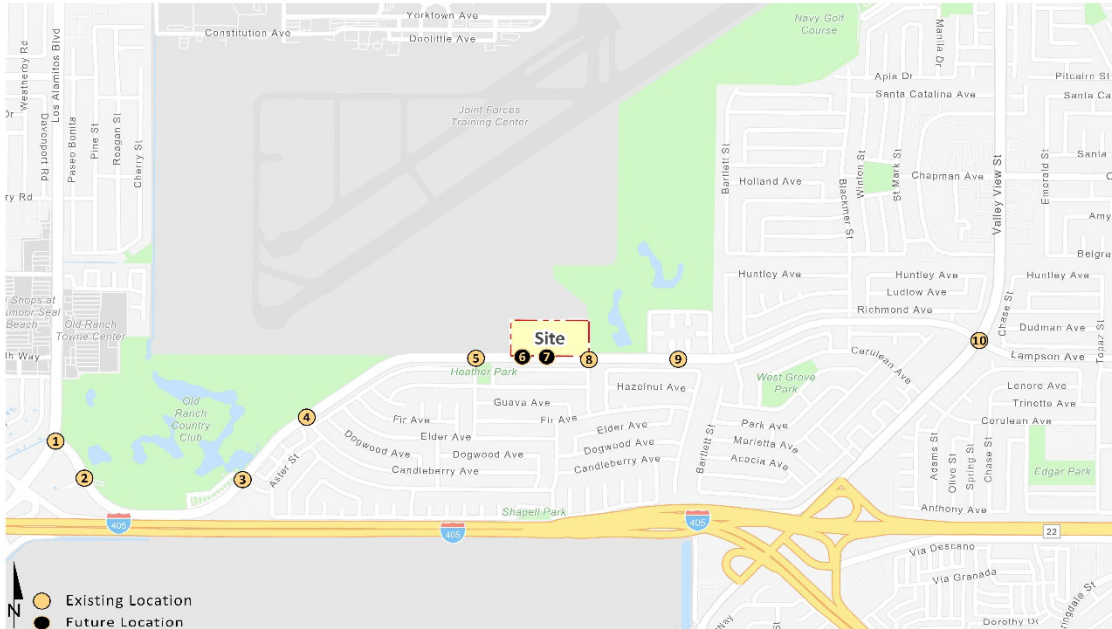
Control: Permitted Protected Protected Protected
Rights: Ovl Include Include Ovl
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 0 3 0 1 2 0 3 0 0 0 0 0 0 0 1 0 1! 0 1

Volume Module:
Base Vol: 0 1508 413 447 1202 0 0 0 0 322 0 447
Growth Adj: 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08
Initial Bse: 0 1632 447 484 1301 0 0 0 0 349 0 484
Added Vol: 0 89 65 52 66 0 0 0 0 46 0 46
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 0 1721 512 536 1367 0 0 0 0 395 0 530
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 0 1721 512 536 1367 0 0 0 0 395 0 530
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 0 1721 512 536 1367 0 0 0 0 395 0 530
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 0 1721 512 536 1367 0 0 0 0 395 0 530
OvlAdjVol: 204

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.00 3.00 1.00 2.00 3.00 0.00 0.00 0.00 0.00 1.28 0.00 1.72
Final Sat.: 0 4800 1600 3200 4800 0 0 0 0 2049 0 2751

Capacity Analysis Module:
Vol/Sat: 0.00 0.36 0.32 0.17 0.28 0.00 0.00 0.00 0.00 0.19 0.00 0.19
OvlAdjV/S: 0.13
Crit Moves: **** *

**ATTACHMENT F: GENERAL PLAN BUILDOUT WITH PROJECT
ALTERNATIVE TRAFFIC VOLUMES**



1 Seal Beach Bl. & Lampson Av.	2 Old Ranch Plaza & Lampson Av.	3 Basswood St. & Lampson Av.	4 Candleberry Av. & Lampson Av.	5 Heather St. & Lampson Av.	
48,850 ↓ 1591(1504) ↑ 694(587) ← 726(605) → 466(505) 1479(1893) 435(560) 47,000	850 34(20) 0(1) 6(3) ↑ 5(6) ← 1072(902) 11(8) 18(38) 746(1021) 36(39) 22,200	900 8(43) 0(3) 1(5) ↑ 7(3) ← 902(726) 10(13) 21(30) 556(814) 90(132) 20,150	16,850 ↑ 724(662) 23(31) 473(704) 88(127) 195(80) 39(26) 16,850	15,300 ← 653(634) 39(78) 441(661) 71(68) 95(59) 69(57) 15,200	
6 Driveway 1 & Lampson Av.	7 Driveway 2 & Lampson Av.	8 Rose St. & Lampson Av.	9 Tulip St. & Lampson Av.	10 Valley View St. & Lampson Av.	
400 18(12) 499(737) 15,100	1,150 22(15) 33(22) ↑ 5(16) ← 664(668) 14(45) 485(592) 15,100	14,950 ↑ 627(652) 34(78) 491(676) 27(38) 42(32) 58(63) 14,950	15,700 ↑ 627(652) 34(78) 491(676) 27(38) 42(32) 58(63) 2,250	16,900 16(13) 1(0) 13(11) ↑ 10(21) ← 600(687) 41(91) 6(8) 530(700) 13(31) 44(31) 1(0) 89(73) 2,400	12,900 50,900 210(266) 1844(2029) 136(198) ↑ 267(135) ← 377(260) 311(235) 223(285) 290(322) 186(162) 185(260) 185(260) 205(177) 49,950

##(##) AM(PM) Peak Hour Intersection Volumes
 ## Average Daily Trips

**ATTACHMENT G: GENERAL PLAN BUILDOUT WITH PROJECT
ALTERNATIVE CONDITIONS INTERSECTION OPERATION
ANALYSIS WORKSHEETS**

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #1 Seal Beach & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 1.034
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with columns for Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, Y+R, and Lanes.

Volume Module table with columns for various volume metrics (Base Vol, Growth Adj, Initial Bse, etc.) and values for each bound.

Saturation Flow Module table with columns for Sat/Lane, Adjustment, Lanes, and Final Sat. values.

Capacity Analysis Module table with columns for Vol/Sat, OvlAdjV/S, and Crit Moves.

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #2 Old Ranch Plaza & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.493
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 33 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Protected Protected
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 1 0 0 1 0 0 1! 0 0 1 0 2 0 1 1 0 1 1 0

Volume Module:
Base Vol: 33 3 8 6 0 34 18 732 36 11 1032 5
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 33 3 8 6 0 34 18 732 36 11 1032 5
Added Vol: 0 0 0 0 0 0 0 14 0 0 40 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 33 3 8 6 0 34 18 746 36 11 1072 5
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 33 3 8 6 0 34 18 746 36 11 1072 5
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 33 3 8 6 0 34 18 746 36 11 1072 5
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 33 3 8 6 0 34 18 746 36 11 1072 5

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.06 1.00 1.00 1.00 1.00 1.06 1.06 1.00 1.03 1.00
Lanes: 0.92 0.08 1.00 0.15 0.00 0.85 1.00 2.00 1.00 1.00 1.99 0.01
Final Sat.: 1467 133 1700 240 0 1360 1600 3400 1700 1600 3285 15

Capacity Analysis Module:
Vol/Sat: 0.02 0.02 0.00 0.00 0.00 0.03 0.01 0.22 0.02 0.01 0.33 0.34
Crit Moves: **** **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #3 Basswood & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.540
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 36 Level Of Service: A

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Permitted, Protected), Rights (Include), and various traffic volume/adjustment metrics.

-----|-----|-----|-----|-----|

Volume Module: Table showing Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume across different movements.

-----|-----|-----|-----|-----|

Saturation Flow Module: Table showing Sat/Lane, Adjustment, Lanes, and Final Sat. for each movement.

-----|-----|-----|-----|-----|

Capacity Analysis Module: Table showing Vol/Sat and Crit Moves for each movement.

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #4 Candleberry & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.459
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 31 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 0 1 0 0 0 0 0 0 0 1 1 0 1 0 2 0 0

Volume Module:
Base Vol: 195 0 39 0 0 0 0 459 88 23 684 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 195 0 39 0 0 0 0 459 88 23 684 0
Added Vol: 0 0 0 0 0 0 0 14 0 0 40 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 195 0 39 0 0 0 0 473 88 23 724 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 195 0 39 0 0 0 0 473 88 23 724 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 195 0 39 0 0 0 0 473 88 23 724 0
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 195 0 39 0 0 0 0 473 88 23 724 0

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.06 1.06 1.00 1.04 1.00 1.00 1.06 1.06
Lanes: 0.83 0.00 0.17 0.00 0.00 0.00 0.00 1.69 0.31 1.00 2.00 0.00
Final Sat.: 1333 0 267 0 0 0 0 2798 502 1600 3400 0

Capacity Analysis Module:
Vol/Sat: 0.12 0.00 0.15 0.00 0.00 0.00 0.00 0.17 0.18 0.01 0.21 0.00
Crit Moves: **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #5 Heather & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.395
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 28 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 0 1! 0 0 0 0 0 0 0 1 1 0 1 0 2 0 0

Volume Module:
Base Vol: 95 0 69 0 0 0 0 0 427 71 39 613 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 95 0 69 0 0 0 0 0 427 71 39 613 0
Added Vol: 0 0 0 0 0 0 0 0 14 0 0 40 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 95 0 69 0 0 0 0 0 441 71 39 653 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 95 0 69 0 0 0 0 0 441 71 39 653 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 95 0 69 0 0 0 0 0 441 71 39 653 0
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 95 0 69 0 0 0 0 0 441 71 39 653 0

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.06 1.06 1.00 1.04 1.00 1.00 1.06 1.06
Lanes: 0.58 0.00 0.42 0.00 0.00 0.00 0.00 1.72 0.28 1.00 2.00 0.00
Final Sat.: 927 0 673 0 0 0 0 2856 444 1600 3400 0

Capacity Analysis Module:
Vol/Sat: 0.06 0.00 0.10 0.00 0.00 0.00 0.00 0.15 0.16 0.02 0.19 0.00
Crit Moves: **** **** ****

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑			↑
Traffic Vol, veh/h	0	499	679	7	0	18
Future Vol, veh/h	0	499	679	7	0	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	542	738	8	0	20

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	-	0	-	0	373
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	-	-	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	3.32
Pot Cap-1 Maneuver	0	-	-	-	624
Stage 1	0	-	-	-	-
Stage 2	0	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	-	-	624
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	11
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	624
HCM Lane V/C Ratio	-	-	-	0.031
HCM Control Delay (s)	-	-	-	11
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

Intersection						
Int Delay, s/veh	0.8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	14	485	664	5	33	22
Future Vol, veh/h	14	485	664	5	33	22
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	1	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	15	527	722	5	36	24

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	727	0	-	0	1019 364
Stage 1	-	-	-	-	725 -
Stage 2	-	-	-	-	294 -
Critical Hdwy	4.14	-	-	-	6.84 6.94
Critical Hdwy Stg 1	-	-	-	-	5.84 -
Critical Hdwy Stg 2	-	-	-	-	5.84 -
Follow-up Hdwy	2.22	-	-	-	3.52 3.32
Pot Cap-1 Maneuver	872	-	-	-	233 633
Stage 1	-	-	-	-	440 -
Stage 2	-	-	-	-	730 -
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	872	-	-	-	229 633
Mov Cap-2 Maneuver	-	-	-	-	341 -
Stage 1	-	-	-	-	433 -
Stage 2	-	-	-	-	730 -

Approach	EB	WB	SB
HCM Control Delay, s	0.3	0	15
HCM LOS			C

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	872	-	-	-	418
HCM Lane V/C Ratio	0.017	-	-	-	0.143
HCM Control Delay (s)	9.2	-	-	-	15
HCM Lane LOS	A	-	-	-	C
HCM 95th %tile Q(veh)	0.1	-	-	-	0.5

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #8 Rose & Lampson

Cycle (sec):	100	Critical Vol./Cap.(X):	0.340
Loss Time (sec):	10	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	26	Level Of Service:	A

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R

Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	1 0	0	0	0 0	0	0	1 1	0	1	0 2 0

Volume Module:

Base Vol:	42	0	58	0	0	0	0	458	27	34	615	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	42	0	58	0	0	0	0	458	27	34	615	0
Added Vol:	0	0	0	0	0	0	0	33	0	0	12	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	42	0	58	0	0	0	0	491	27	34	627	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	42	0	58	0	0	0	0	491	27	34	627	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	42	0	58	0	0	0	0	491	27	34	627	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	42	0	58	0	0	0	0	491	27	34	627	0

Saturation Flow Module:

Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.06	1.06	1.00	1.03	1.00	1.00	1.06	1.06
Lanes:	0.42	0.00	0.58	0.00	0.00	0.00	0.00	1.90	0.10	1.00	2.00	0.00
Final Sat.:	672	0	928	0	0	0	0	3133	167	1600	3400	0

Capacity Analysis Module:

Vol/Sat:	0.03	0.00	0.06	0.00	0.00	0.00	0.00	0.16	0.16	0.02	0.18	0.00
Crit Moves:			****					****		****		

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #9 Tulip & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.360
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 27 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 1 0 0 1 0 0 0 1! 0 0 1 0 1 1 0 1 0 1 1 0

Volume Module:
Base Vol: 44 1 89 13 1 16 6 497 13 41 588 10
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 44 1 89 13 1 16 6 497 13 41 588 10
Added Vol: 0 0 0 0 0 0 0 33 0 0 12 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 44 1 89 13 1 16 6 530 13 41 600 10
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 44 1 89 13 1 16 6 530 13 41 600 10
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 44 1 89 13 1 16 6 530 13 41 600 10
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 44 1 89 13 1 16 6 530 13 41 600 10

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.03 1.00 1.00 1.03 1.00
Lanes: 1.00 0.01 0.99 0.43 0.03 0.54 1.00 1.95 0.05 1.00 1.97 0.03
Final Sat.: 1600 18 1582 693 53 853 1600 3223 77 1600 3248 52

Capacity Analysis Module:
Vol/Sat: 0.03 0.06 0.06 0.01 0.02 0.02 0.00 0.16 0.17 0.03 0.18 0.19
Crit Moves: **** **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #10 Valley View & Lampson

Cycle (sec):	100	Critical Vol./Cap.(X):	0.918
Loss Time (sec):	10	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	110	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound										
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit										
Rights:	Include			Include			Include			Include										
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		
Lanes:	1	0	3	0	1	1	0	3	0	1	1	0	1	1	0	1	0	1	1	0

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Volume Module:

Base Vol:	180	1694	205	136	1844	207	216	279	171	311	373	267
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	180	1694	205	136	1844	207	216	279	171	311	373	267
Added Vol:	5	0	0	0	0	3	7	11	15	0	4	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	185	1694	205	136	1844	210	223	290	186	311	377	267
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	185	1694	205	136	1844	210	223	290	186	311	377	267
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	185	1694	205	136	1844	210	223	290	186	311	377	267
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	185	1694	205	136	1844	210	223	290	186	311	377	267

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Saturation Flow Module:

Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.06	1.00	1.06	1.06	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	1.22	0.78	1.00	1.17	0.83
Final Sat.:	1600	5100	1700	1600	5100	1700	1600	2050	1250	1600	1973	1327

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Capacity Analysis Module:

Vol/Sat:	0.12	0.33	0.12	0.09	0.36	0.12	0.14	0.14	0.15	0.19	0.19	0.20
Crit Moves:	****			****			****			****		

Timings
10: Valley View St. & Lampson Av.

4665 Lampson (JN 14501)

06/01/2023

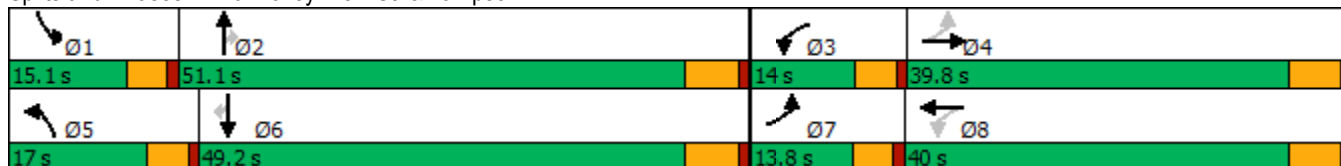


Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↕	↘	↕	↘	↕↕↕	↘	↘	↕↕↕	↘
Traffic Volume (vph)	223	290	311	377	185	1694	205	136	1844	210
Future Volume (vph)	223	290	311	377	185	1694	205	136	1844	210
Turn Type	pm+pt	NA	pm+pt	NA	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4	3	8	5	2		1	6	
Permitted Phases	4		8				2			6
Detector Phase	7	4	3	8	5	2	2	1	6	6
Switch Phase										
Minimum Initial (s)	5.0	10.0	5.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	9.6	39.8	9.6	39.8	9.6	27.8	27.8	9.6	27.8	27.8
Total Split (s)	13.8	39.8	14.0	40.0	17.0	51.1	51.1	15.1	49.2	49.2
Total Split (%)	11.5%	33.2%	11.7%	33.3%	14.2%	42.6%	42.6%	12.6%	41.0%	41.0%
Yellow Time (s)	3.6	4.8	3.6	4.8	3.6	4.8	4.8	3.6	4.8	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.8	4.6	5.8	4.6	5.8	5.8	4.6	5.8	5.8
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	None	None	None
Act Effct Green (s)	35.7	25.2	36.1	25.4	12.4	45.4	45.4	10.5	43.5	43.5
Actuated g/C Ratio	0.32	0.23	0.32	0.23	0.11	0.41	0.41	0.09	0.39	0.39
v/c Ratio	1.12	0.61	1.23	0.81	1.02	0.89	0.32	0.89	1.01	0.34
Control Delay	126.2	32.2	159.4	39.7	119.8	37.9	13.4	96.8	57.1	14.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	126.2	32.2	159.4	39.7	119.8	37.9	13.4	96.8	57.1	14.5
LOS	F	C	F	D	F	D	B	F	E	B
Approach Delay		62.2		78.7		42.7			55.5	
Approach LOS		E		E		D			E	

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 111.5
 Natural Cycle: 120
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 1.23
 Intersection Signal Delay: 55.5
 Intersection LOS: E
 Intersection Capacity Utilization 96.2%
 ICU Level of Service F
 Analysis Period (min) 15

Splits and Phases: 10: Valley View St. & Lampson Av.



HCM 6th Signalized Intersection Summary
 10: Valley View St. & Lampson Av.

4665 Lampson (JN 14501)

06/01/2023

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	223	290	186	311	377	267	185	1694	205	136	1844	210
Future Volume (veh/h)	223	290	186	311	377	267	185	1694	205	136	1844	210
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.98	1.00		0.97	1.00		0.98	1.00		0.99
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	242	315	164	338	410	232	201	1841	160	148	2004	179
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	251	521	264	308	503	281	198	2072	632	168	1985	608
Arrive On Green	0.08	0.23	0.23	0.08	0.23	0.23	0.11	0.41	0.41	0.09	0.39	0.39
Sat Flow, veh/h	1781	2268	1151	1781	2175	1214	1781	5106	1557	1781	5106	1564
Grp Volume(v), veh/h	242	245	234	338	334	308	201	1841	160	148	2004	179
Grp Sat Flow(s),veh/h/ln	1781	1777	1642	1781	1777	1612	1781	1702	1557	1781	1702	1564
Q Serve(g_s), s	9.2	13.8	14.3	9.4	19.9	20.3	12.4	37.4	7.6	9.2	43.4	8.8
Cycle Q Clear(g_c), s	9.2	13.8	14.3	9.4	19.9	20.3	12.4	37.4	7.6	9.2	43.4	8.8
Prop In Lane	1.00		0.70	1.00		0.75	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	251	408	377	308	411	373	198	2072	632	168	1985	608
V/C Ratio(X)	0.97	0.60	0.62	1.10	0.81	0.83	1.02	0.89	0.25	0.88	1.01	0.29
Avail Cap(c_a), veh/h	251	541	500	308	544	494	198	2072	632	168	1985	608
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	37.0	38.4	38.6	39.3	40.6	40.8	49.6	30.8	22.0	50.0	34.1	23.6
Incr Delay (d2), s/veh	46.9	1.4	1.7	80.5	6.9	8.4	68.2	5.2	0.2	37.4	22.6	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.4	5.9	5.7	10.7	9.1	8.6	9.1	15.3	2.7	5.7	20.8	3.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	84.0	39.8	40.3	119.8	47.5	49.2	117.8	36.0	22.2	87.3	56.7	23.8
LnGrp LOS	F	D	D	F	D	D	F	D	C	F	F	C
Approach Vol, veh/h		721			980			2202			2331	
Approach Delay, s/veh		54.8			73.0			42.5			56.1	
Approach LOS		D			E			D			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	15.1	51.1	14.0	31.4	17.0	49.2	13.8	31.6				
Change Period (Y+Rc), s	4.6	5.8	4.6	5.8	4.6	5.8	4.6	5.8				
Max Green Setting (Gmax), s	10.5	45.3	9.4	34.0	12.4	43.4	9.2	34.2				
Max Q Clear Time (g_c+I1), s	11.2	39.4	11.4	16.3	14.4	45.4	11.2	22.3				
Green Ext Time (p_c), s	0.0	4.9	0.0	2.4	0.0	0.0	0.0	2.9				
Intersection Summary												
HCM 6th Ctrl Delay				53.8								
HCM 6th LOS				D								

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #1 Seal Beach & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 1.010
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Protected Protected Protected
Rights: Ovl Include Include Ovl
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 0 3 0 1 2 0 3 0 0 0 0 0 0 0 2 0 0 0 1

Volume Module:
Base Vol: 0 1893 532 571 1504 0 0 0 0 488 0 595
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 1893 532 571 1504 0 0 0 0 488 0 595
Added Vol: 0 0 28 16 0 0 0 0 0 17 0 10
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 0 1893 560 587 1504 0 0 0 0 505 0 605
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 0 1893 560 587 1504 0 0 0 0 505 0 605
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 0 1893 560 587 1504 0 0 0 0 505 0 605
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 0 1893 560 587 1504 0 0 0 0 505 0 605
OvlAdjVol: 292

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.06 1.06 1.00 1.06 1.06 1.00 1.06 1.06 1.00 1.06 1.06
Lanes: 0.00 3.00 1.00 2.00 3.00 0.00 0.00 0.00 0.00 2.00 0.00 1.00
Final Sat.: 0 5100 1700 3200 5100 0 0 0 0 3200 0 1700

Capacity Analysis Module:
Vol/Sat: 0.00 0.37 0.33 0.18 0.29 0.00 0.00 0.00 0.00 0.16 0.00 0.36
OvlAdjV/S: 0.17
Crit Moves: **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #2 Old Ranch Plaza & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.444
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 30 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Protected Protected
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 1 0 0 1 0 0 1! 0 0 1 0 2 0 1 1 0 1 1 0

Volume Module:
Base Vol: 48 0 11 3 1 20 38 976 39 8 875 6
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 48 0 11 3 1 20 38 976 39 8 875 6
Added Vol: 0 0 0 0 0 0 0 45 0 0 27 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 48 0 11 3 1 20 38 1021 39 8 902 6
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 48 0 11 3 1 20 38 1021 39 8 902 6
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 48 0 11 3 1 20 38 1021 39 8 902 6
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 48 0 11 3 1 20 38 1021 39 8 902 6

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.06 1.00 1.00 1.00 1.00 1.06 1.06 1.00 1.03 1.00
Lanes: 1.00 0.00 1.00 0.12 0.04 0.84 1.00 2.00 1.00 1.00 1.99 0.01
Final Sat.: 1600 0 1700 200 67 1333 1600 3400 1700 1600 3279 21

Capacity Analysis Module:
Vol/Sat: 0.03 0.00 0.01 0.00 0.01 0.02 0.02 0.30 0.02 0.01 0.28 0.28
Crit Moves: **** **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #3 Basswood & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.524
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 35 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Protected Protected
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 1 0 0 1 0 0 0 1! 0 0 1 0 1 1 0 1 0 1 1 0

Volume Module:
Base Vol: 141 0 11 5 3 43 30 769 132 13 699 3
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 141 0 11 5 3 43 30 769 132 13 699 3
Added Vol: 0 0 0 0 0 0 0 45 0 0 27 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 141 0 11 5 3 43 30 814 132 13 726 3
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 141 0 11 5 3 43 30 814 132 13 726 3
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 141 0 11 5 3 43 30 814 132 13 726 3
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 141 0 11 5 3 43 30 814 132 13 726 3

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.04 1.00 1.00 1.03 1.00
Lanes: 1.00 0.00 1.00 0.10 0.06 0.84 1.00 1.72 0.28 1.00 1.99 0.01
Final Sat.: 1600 0 1600 157 94 1349 1600 2853 447 1600 3287 13

Capacity Analysis Module:
Vol/Sat: 0.09 0.00 0.01 0.00 0.03 0.03 0.02 0.29 0.30 0.01 0.22 0.23
Crit Moves: **** **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #4 Candleberry & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.445
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 31 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 0 1 0 0 0 0 0 0 0 1 1 0 1 0 2 0 0

Volume Module:
Base Vol: 80 0 26 0 0 0 0 659 127 31 635 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 80 0 26 0 0 0 0 659 127 31 635 0
Added Vol: 0 0 0 0 0 0 0 45 0 0 27 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 80 0 26 0 0 0 0 704 127 31 662 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 80 0 26 0 0 0 0 704 127 31 662 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 80 0 26 0 0 0 0 704 127 31 662 0
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 80 0 26 0 0 0 0 704 127 31 662 0

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.06 1.06 1.00 1.04 1.00 1.00 1.06 1.06
Lanes: 0.75 0.00 0.25 0.00 0.00 0.00 0.00 1.69 0.31 1.00 2.00 0.00
Final Sat.: 1208 0 392 0 0 0 0 2811 489 1600 3400 0

Capacity Analysis Module:
Vol/Sat: 0.05 0.00 0.07 0.00 0.00 0.00 0.00 0.25 0.26 0.02 0.19 0.00
Crit Moves: **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #5 Heather & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.449
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 31 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 0 1! 0 0 0 0 0 0 0 1 1 0 1 0 2 0 0

Volume Module:
Base Vol: 59 0 57 0 0 0 0 0 616 68 78 607 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 59 0 57 0 0 0 0 0 616 68 78 607 0
Added Vol: 0 0 0 0 0 0 0 0 45 0 0 27 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 59 0 57 0 0 0 0 0 661 68 78 634 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 59 0 57 0 0 0 0 0 661 68 78 634 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 59 0 57 0 0 0 0 0 661 68 78 634 0
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 59 0 57 0 0 0 0 0 661 68 78 634 0

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.06 1.06 1.00 1.03 1.00 1.00 1.06 1.06
Lanes: 0.51 0.00 0.49 0.00 0.00 0.00 0.00 1.81 0.19 1.00 2.00 0.00
Final Sat.: 814 0 786 0 0 0 0 3002 298 1600 3400 0

Capacity Analysis Module:
Vol/Sat: 0.04 0.00 0.07 0.00 0.00 0.00 0.00 0.22 0.23 0.05 0.19 0.00
Crit Moves: **** ****

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑↑	↑↑			↑
Traffic Vol, veh/h	0	737	663	20	0	12
Future Vol, veh/h	0	737	663	20	0	12
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	801	721	22	0	13

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	-	0	-
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	-
Pot Cap-1 Maneuver	0	-	-
Stage 1	0	-	-
Stage 2	0	-	-
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	10.9
HCM LOS			B

Minor Lane/Major Mvmt	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	-	-	-	625
HCM Lane V/C Ratio	-	-	-	0.021
HCM Control Delay (s)	-	-	-	10.9
HCM Lane LOS	-	-	-	B
HCM 95th %tile Q(veh)	-	-	-	0.1

Intersection						
Int Delay, s/veh	0.7					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	45	692	668	16	22	15
Future Vol, veh/h	45	692	668	16	22	15
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	1	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	49	752	726	17	24	16

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	743	0	-	0	1209 372
Stage 1	-	-	-	-	735 -
Stage 2	-	-	-	-	474 -
Critical Hdwy	4.14	-	-	-	6.84 6.94
Critical Hdwy Stg 1	-	-	-	-	5.84 -
Critical Hdwy Stg 2	-	-	-	-	5.84 -
Follow-up Hdwy	2.22	-	-	-	3.52 3.32
Pot Cap-1 Maneuver	860	-	-	-	175 625
Stage 1	-	-	-	-	435 -
Stage 2	-	-	-	-	592 -
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	860	-	-	-	165 625
Mov Cap-2 Maneuver	-	-	-	-	293 -
Stage 1	-	-	-	-	410 -
Stage 2	-	-	-	-	592 -

Approach	EB	WB	SB
HCM Control Delay, s	0.6	0	15.8
HCM LOS			C

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	860	-	-	-	373
HCM Lane V/C Ratio	0.057	-	-	-	0.108
HCM Control Delay (s)	9.4	-	-	-	15.8
HCM Lane LOS	A	-	-	-	C
HCM 95th %tile Q(veh)	0.2	-	-	-	0.4

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #8 Rose & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.424
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 30 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 0 0 1 0 0 0 0 0 0 0 1 1 0 1 0 2 0 0

Volume Module:
Base Vol: 32 0 63 0 0 0 0 0 654 38 78 616 0
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 32 0 63 0 0 0 0 0 654 38 78 616 0
Added Vol: 0 0 0 0 0 0 0 0 22 0 0 36 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 32 0 63 0 0 0 0 0 676 38 78 652 0
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 32 0 63 0 0 0 0 0 676 38 78 652 0
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 32 0 63 0 0 0 0 0 676 38 78 652 0
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 32 0 63 0 0 0 0 0 676 38 78 652 0

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.06 1.06 1.00 1.03 1.00 1.00 1.06 1.06
Lanes: 0.34 0.00 0.66 0.00 0.00 0.00 0.00 1.89 0.11 1.00 2.00 0.00
Final Sat.: 539 0 1061 0 0 0 0 3130 170 1600 3400 0

Capacity Analysis Module:
Vol/Sat: 0.02 0.00 0.06 0.00 0.00 0.00 0.00 0.22 0.22 0.05 0.19 0.00
Crit Moves: **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #9 Tulip & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.438
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 30 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 1 0 0 1 0 0 0 1! 0 0 1 0 1 1 0

Volume Module:
Base Vol: 31 0 73 11 0 13 8 678 31 91 651 21
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 31 0 73 11 0 13 8 678 31 91 651 21
Added Vol: 0 0 0 0 0 0 0 22 0 0 36 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 31 0 73 11 0 13 8 700 31 91 687 21
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 31 0 73 11 0 13 8 700 31 91 687 21
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 31 0 73 11 0 13 8 700 31 91 687 21
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 31 0 73 11 0 13 8 700 31 91 687 21

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.03 1.00 1.00 1.03 1.00
Lanes: 1.00 0.00 1.00 0.46 0.00 0.54 1.00 1.92 0.08 1.00 1.94 0.06
Final Sat.: 1600 0 1600 733 0 867 1600 3164 136 1600 3205 95

Capacity Analysis Module:
Vol/Sat: 0.02 0.00 0.05 0.01 0.00 0.01 0.01 0.22 0.23 0.06 0.21 0.22
Crit Moves: **** **** **** ****

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #10 Valley View & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.956
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 139 Level Of Service: E

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Protected Protected Prot+Permit Prot+Permit
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
Lanes: 1 0 3 0 1 1 0 3 0 1 1 0 1 1 0

Volume Module:
Base Vol: 244 1875 177 198 2029 258 280 315 152 235 248 135
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 244 1875 177 198 2029 258 280 315 152 235 248 135
Added Vol: 16 0 0 0 0 8 5 7 10 0 12 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 260 1875 177 198 2029 266 285 322 162 235 260 135
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Volume: 260 1875 177 198 2029 266 285 322 162 235 260 135
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 260 1875 177 198 2029 266 285 322 162 235 260 135
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 260 1875 177 198 2029 266 285 322 162 235 260 135

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.06 1.06 1.00 1.06 1.06 1.00 1.05 1.00 1.00 1.05 1.00
Lanes: 1.00 3.00 1.00 1.00 3.00 1.00 1.00 1.33 0.67 1.00 1.32 0.68
Final Sat.: 1600 5100 1700 1600 5100 1700 1600 2229 1071 1600 2206 1094

Capacity Analysis Module:
Vol/Sat: 0.16 0.37 0.10 0.12 0.40 0.16 0.18 0.14 0.15 0.15 0.12 0.12
Crit Moves: **** **** **** ****

Timings
10: Valley View St. & Lampson Av.

4665 Lampson (JN 14501)

06/01/2023

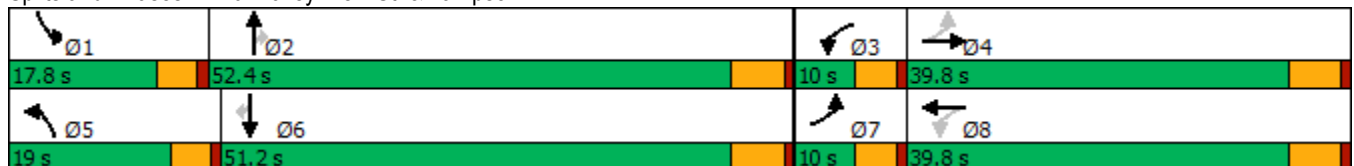


Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations										
Traffic Volume (vph)	285	322	235	260	260	1875	177	198	2029	266
Future Volume (vph)	285	322	235	260	260	1875	177	198	2029	266
Turn Type	pm+pt	NA	pm+pt	NA	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4	3	8	5	2		1	6	
Permitted Phases	4		8				2			6
Detector Phase	7	4	3	8	5	2	2	1	6	6
Switch Phase										
Minimum Initial (s)	5.0	10.0	5.0	10.0	5.0	10.0	10.0	5.0	10.0	10.0
Minimum Split (s)	9.6	39.8	9.6	39.8	9.6	27.8	27.8	9.6	27.8	27.8
Total Split (s)	10.0	39.8	10.0	39.8	19.0	52.4	52.4	17.8	51.2	51.2
Total Split (%)	8.3%	33.2%	8.3%	33.2%	15.8%	43.7%	43.7%	14.8%	42.7%	42.7%
Yellow Time (s)	3.6	4.8	3.6	4.8	3.6	4.8	4.8	3.6	4.8	4.8
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.6	5.8	4.6	5.8	4.6	5.8	5.8	4.6	5.8	5.8
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	None	None	None
Act Effct Green (s)	27.2	20.6	27.2	20.6	14.5	46.8	46.8	13.3	45.6	45.6
Actuated g/C Ratio	0.25	0.19	0.25	0.19	0.14	0.44	0.44	0.12	0.43	0.43
v/c Ratio	1.28	0.71	1.27	0.58	1.12	0.87	0.24	0.93	0.96	0.38
Control Delay	189.5	39.8	189.4	34.3	138.3	33.4	9.8	93.6	43.6	14.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	189.5	39.8	189.4	34.3	138.3	33.4	9.8	93.6	43.6	14.5
LOS	F	D	F	C	F	C	A	F	D	B
Approach Delay		95.3		92.1		43.4			44.5	
Approach LOS		F		F		D			D	

Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 106.9
 Natural Cycle: 120
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 1.28
 Intersection Signal Delay: 55.2
 Intersection LOS: E
 Intersection Capacity Utilization 99.5%
 ICU Level of Service F
 Analysis Period (min) 15

Splits and Phases: 10: Valley View St. & Lampson Av.



HCM 6th Signalized Intersection Summary
 10: Valley View St. & Lampson Av.

4665 Lampson (JN 14501)

06/01/2023



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↗	↗↘		↗	↗↘		↗	↗↘↙	↗	↗	↗↘↙	↗
Traffic Volume (veh/h)	285	322	162	235	260	135	260	1875	177	198	2029	266
Future Volume (veh/h)	285	322	162	235	260	135	260	1875	177	198	2029	266
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		0.98	1.00		0.99	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	294	332	133	242	268	117	268	1933	143	204	2092	196
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	233	443	174	206	430	182	246	2270	695	225	2211	686
Arrive On Green	0.05	0.18	0.18	0.05	0.18	0.18	0.14	0.44	0.44	0.13	0.43	0.43
Sat Flow, veh/h	1781	2488	978	1781	2418	1025	1781	5106	1564	1781	5106	1583
Grp Volume(v), veh/h	294	235	230	242	195	190	268	1933	143	204	2092	196
Grp Sat Flow(s),veh/h/ln	1781	1777	1689	1781	1777	1666	1781	1702	1564	1781	1702	1583
Q Serve(g_s), s	5.4	13.1	13.5	5.4	10.6	11.1	14.4	35.3	5.8	11.8	41.1	8.4
Cycle Q Clear(g_c), s	5.4	13.1	13.5	5.4	10.6	11.1	14.4	35.3	5.8	11.8	41.1	8.4
Prop In Lane	1.00		0.58	1.00		0.62	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	233	316	300	206	316	296	246	2270	695	225	2211	686
V/C Ratio(X)	1.26	0.74	0.76	1.17	0.62	0.64	1.09	0.85	0.21	0.91	0.95	0.29
Avail Cap(c_a), veh/h	233	579	550	206	579	543	246	2280	698	225	2221	688
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	42.6	40.7	40.8	42.1	39.6	39.8	45.0	25.9	17.7	45.0	28.4	19.1
Incr Delay (d2), s/veh	147.8	3.5	4.0	117.7	2.0	2.3	83.7	3.3	0.1	34.8	9.4	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	12.8	5.8	5.7	9.3	4.6	4.5	11.8	13.7	2.0	7.2	17.1	2.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	190.4	44.1	44.9	159.9	41.6	42.1	128.7	29.2	17.9	79.8	37.8	19.4
LnGrp LOS	F	D	D	F	D	D	F	C	B	E	D	B
Approach Vol, veh/h		759			627			2344			2492	
Approach Delay, s/veh		101.0			87.4			39.9			39.8	
Approach LOS		F			F			D			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	17.8	52.2	10.0	24.4	19.0	51.0	10.0	24.4				
Change Period (Y+Rc), s	4.6	5.8	4.6	5.8	4.6	5.8	4.6	5.8				
Max Green Setting (Gmax), s	13.2	46.6	5.4	34.0	14.4	45.4	5.4	34.0				
Max Q Clear Time (g_c+I1), s	13.8	37.3	7.4	15.5	16.4	43.1	7.4	13.1				
Green Ext Time (p_c), s	0.0	7.5	0.0	2.4	0.0	2.1	0.0	2.0				

Intersection Summary

HCM 6th Ctrl Delay	52.1
HCM 6th LOS	D

**ATTACHMENT H: GENERAL PLAN BUILDOUT WITH PROJECT
ALTERNATIVE CONDITIONS INTERSECTION OPERATION
ANALYSIS WORKSHEETS WITH IMPROVEMENTS**

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #1 Seal Beach & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.855
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 82 Level Of Service: D

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Permitted, Protected), Rights (Ovl, Include), and various traffic volume and delay metrics.

Volume Module: Table showing Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume, and OvlAdjVol.

Saturation Flow Module: Table showing Sat/Lane, Adjustment, Lanes, and Final Sat. values.

Capacity Analysis Module: Table showing Vol/Sat, OvlAdjV/S, and Crit Moves.

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #1 Seal Beach & Lampson

Cycle (sec): 100 Critical Vol./Cap.(X): 0.886
Loss Time (sec): 10 Average Delay (sec/veh): xxxxxx
Optimal Cycle: 93 Level Of Service: D

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Permitted, Protected), Rights (Ovl, Include), and various traffic volume and delay metrics.

Volume Module: Table showing Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume, and OvlAdjVol.

Saturation Flow Module: Table showing Sat/Lane, Adjustment, Lanes, and Final Sat. values.

Capacity Analysis Module: Table showing Vol/Sat, OvlAdjV/S, and Crit Moves.
