

# Appendix F Preliminary Hydrology Report

## Appendix

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# PRELIMINARY HYDROLOGY REPORT

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Project Name:

## OAK CREEK COMMUNITY PARK

15616 Valley Oak Drive  
Irvine, California CA 92618

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Prepared for:

## City of Irvine

Community Services Department  
One Civic Center Plaza  
Irvine, CA 92623-9575

Prepared by:

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APRIL 2022



The logo for Adams Streeter Civil Engineers, consisting of the letters "AS" in a stylized, orange, sans-serif font.

ADAMS STREETER  
Civil Engineers

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## **I. INTRODUCTION**

This preliminary report is prepared for the City of Irvine Community Services Department as part of the effort to expand the existing facilities at the Oak Creek Community Park located at 15616 Valley Oak Drive in the City of Irvine, California (refer to the Vicinity Map in Appendix A). The project site is bounded by Sand Canyon Avenue to the southeast, Valley Oak Drive to the northwest, Barranca Parkway to the southwest, and corporate offices to the northeast. The existing park facilities that include a baseball field, two soccer fields, a children’s playground area, a flat grassy play area, a restroom building, a picnic shelter, BBQ grills, picnic tables and a paved parking lot are all located at the northerly portion of the site. The southerly portion of the site is owned by Southern California Edison (SCE) and is currently undeveloped. The proposed expansion of park facilities would include improvements within SCE’s property.

The purpose of this report is to provide a hydrologic evaluation of both existing and proposed (post improvement) conditions to estimate peak storm runoff values to support the design of proposed storm drainage and water quality facilities. The project site is located within Zone X per the FEMA Map reference in Appendix C.

## **II. EXISTING DRAINAGE CONDITION**

Existing site topography generally slopes in a westerly direction with elevations ranging from 141.3 to 156 feet above sea level (MSL). No off-site drainage is anticipated to contribute to on-site run-off as shown on the Preliminary Existing Condition Hydrology Map (PECHM) provided in Appendix K.

Drainage from the northerly half of the developed park site is captured by various on-site catch basins and inlets into an existing on-site storm drain line that runs along the park’s existing asphalt drive and parking area. This storm drain line is shown on the AS-BUILT reference plans provided in Appendix M as Lateral “B-5”, Appendix N as Lateral “B-3A”, and Appendix O as Line “A”. For consistency, this report will refer to this storm drain line as Line “A”.

On-site drainage is conveyed by Line “A” to an existing 78” RCP storm drain line in Valley Oak Drive, as shown on the AS-BUILT reference plans provided in Appendix M and N as Line “B”. For consistency, this report will refer to this storm drain line as Line “B”.

Drainage from the southerly half of the undeveloped park site within SCE’s property is captured by an existing 24” CSP riser inlet located at the westerly corner of the site and conveyed by an existing 18” RCP pipe to Line “B” in Valley Oak Drive. This 18” pipe is shown on the AS-BUILT reference plans provided in Appendix N as Lateral “B-1”. For consistency, this report will refer to this storm drain lateral as Line “C”.

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### **III. PROPOSED DRAINAGE CONDITION**

The proposed drainage condition includes provisions to capture and treat run-off from both the northerly and southerly portions of the park site. Minor improvements are proposed for the northerly portion of park site that includes the conversion of the existing baseball field into a synthetic turf soccer field, the conversion of the existing soccer field located at the southeasterly corner of the site into a dog park, and miscellaneous sidewalk and hardscape related improvements. Proposed improvements for the southerly portion of the park site within SCE's property includes an asphalt parking lot with landscape islands, an open turf field, miscellaneous sidewalk, ramps, and steps to facilitate non-ADA and ADA access. Storm drain improvements consisting of a trench drain and 18" RCP pipe conveys drainage from the proposed parking lot to a water quality unit. Treated and overflow runoff from the water quality unit drains towards the westerly corner of the site and is captured by the existing 24" CSP riser that conveys runoff via Line "C" to Line "B" in Valley Oak Drive. Existing drainage patterns will be maintained for the proposed condition as the site is already tabled for draining into the existing 78" RCP pipe (or Line "B") located in Valley Oak Drive. The Proposed Condition Hydrology Map depicting the drainage patterns and peak flow values is shown on the Preliminary Proposed Condition Hydrology Map (PPCHM) provided in Appendix L.

### **IV. HYDROLOGY ANALYSIS**

#### **A. METHODOLOGY**

Hydrologic calculations to determine the storm frequency design discharges for this project were performed using the Orange County Rational Method. The Rational Method equation relates rainfall intensity, runoff coefficient and drainage area size to compute the drainage area peak runoff. Guidelines and parameters that relate to the Rational Method are as follows:

- The rational method equation is only applicable where the rainfall intensity can be assumed to be uniformly distributed over the drainage area at a uniform rate throughout the duration of the storm.
- The equation includes the element of the soil surface characteristics. The Soils Group Map from the Hydrology Manual indicates the existing soils of the project consist primarily of soil type "B" (see Appendix D). Soil ratings are based on a scale of "A" to "D", where "A" is the most pervious. The soil pervious rate is also affected by the type of vegetation and/or ground cover, and the percentage of impervious surface such as asphalt concrete paving, etc.
- The nomograph used to determine the time of concentration of the initial area is

based on the Kirpich formula. The Hydrology Manual specifies that the initial area shall be less than 10 acres and the flow path less than 300 feet.

- Standard intensity-duration curve data was taken from the Orange County Hydrology Manual, dated October 1986.
- For hydrology computations, Antecedent Moisture Content (AMC) I is utilized for 2-year storm events, AMC II is utilized for 10-year and 25-year storm events, and AMC III is utilized for 100-year storm events.
- Land use categories utilized in the computations includes *Commercial* (applicable to paved street for existing and proposed condition), *Barren* for area in the existing SCE site, and *Park Facilities for existing and proposed turf fields*.

Site hydrology was analyzed using the Advance Engineering Software (AES) software package for Orange County, which complies with the County of Orange Hydrology Manual. The software computes peak flows based on the Rational Method and conforms to the County of Orange Hydrology Manual.

## **B. DESIGN ASSUMPTIONS AND OBJECTIVES**

A site hydrology analysis was performed to include the following:

- Delineation of on-site drainage tributary areas based on the proposed grading scheme for the project site and location of drainage inlets and/or drainage convergence points.
- Determination of 2-year, 10-year, and 25-year frequency storm design discharges. The 2-year frequency storm design discharges are computed to support water quality related calculations as provided in the Water Quality Management Plan (WQMP) for the project.
- Peak runoff for design and sizing of proposed storm drain facilities are dependent on parameters such as drainage path lengths, ground and pipe slopes, type and condition of the runoff surface, proposed land use and soil type. Pipe travel times were calculated based actual pipe sizes provided by the AS-BUILT reference plans.

Existing condition drainage patterns for the project site were determined utilizing the following information:

- Topographic Survey (see Appendix B).
- Record Drawings for Line "A" Storm Drain Improvements (see Appendix M, N, O).
- FEMA Flood Map (see Appendix C).

Existing and proposed drainage patterns are plotted on the PECHM and PPCHM

provided in Appendix K and Appendix L respectively. For modeling purposes, the site and all tributary drainage areas are further divided into smaller drainage subareas to more accurately model drainage patterns within the project site. All areas and flow information are utilized for input into the AES software to determine peak flowrates for both existing and proposed conditions.

**C. SUMMARY OF RESULTS**

The AES software calculations that indicate the existing and proposed condition peak flow rates based on the 2-year, 10-year, 25-year are provided in the Appendix E through Appendix J of this report. The 2-year storm event calculations are provided for water quality computations purposes only. The existing condition estimated subarea peak runoff corresponding to the 2-year, 10-year and 25-year storm event is summarized by Table I below.

<b>TABLE I</b>					
<b>EXISTING CONDITION ESTIMATED SUBAREA PEAK RUNOFF</b>					
<b>Drainage Sub-Area</b>	<b>Area (Acres)</b>	<b>Q<sub>2</sub> (CFS)</b>	<b>Q<sub>10</sub> (CFS)</b>	<b>Q<sub>25</sub> (CFS)</b>	<b>TC<sub>25</sub> (Minutes)</b>
Subarea A	11.23	11.51	22.25	26.74	7.41
Subarea B	7.87	5.96	12.73	15.80	12.74
<b>Total</b>	<b>19.10</b>	<b>17.47</b>	<b>34.98</b>	<b>42.54</b>	<b>-</b>

As shown on the PECHM in Appendix K and summarized by Table I above, a peak runoff of about 42.5 CFS based on the 25-year storm event is conveyed from the park site to Line “B”, the 78” RCP storm drain pipe in Valley Oak Drive. About 26.4 CFS of on-site drainage contributions to Line “B” is conveyed by Line “A” with the balance of 15.8 CFS conveyed by Line “C”. As shown on the PECHM and PPCHM, the total drainage subareas that discharges to Line “B” spans approximately 19.1 acres. Peak flow contributions from street parkway areas that surface drain directly to San Canyon Avenue and Valley Oak Drive are not considered as on-site run-off contributions. The proposed condition estimated subarea peak runoff corresponding to the 2-year, 10-year and 25-year storm event is summarized by Table II below.

<b>TABLE II</b>					
<b>PROPOSED CONDITION SUBAREA PEAK RUNOFF</b>					
<b>Drainage Sub-Area</b>	<b>Area (Acres)</b>	<b>Q<sub>2</sub> (CFS)</b>	<b>Q<sub>10</sub> (CFS)</b>	<b>Q<sub>25</sub> (CFS)</b>	<b>TC<sub>25</sub> (Minutes)</b>
Subarea A	11.64	12.10	23.53	28.34	7.41
Subarea B	7.45	5.92	11.81	14.46	11.48
<b>Total</b>	<b>19.09</b>	<b>18.02</b>	<b>35.34</b>	<b>42.80</b>	<b>-</b>

For existing conditions and based on the latest AS-BUILT reference plan for Line “A” provided in Appendix O, a 25-year peak flow of 36.2 CFS is shown captured from the park-site and conveyed by Line “A” to Line “B”. The hydraulic grade line (HGL) shown on the profile sheet also indicates that Line “A” is operating under open-channel or non-pressurized flow conditions during peak flow events. Per Appendix N, a 25-year peak flow of 6.4 CFS is shown captured from the park-site and conveyed by Line “C” to Line “B”. The hydraulic grade line (HGL) shown on the profile indicates that the downstream portion of Line “C” is operating under pressurized flow conditions during peak flow events. Per the PECHM and Table I above, the existing condition 25-year storm peak flow for subarea A and subarea B is estimated at 26.7 CFS and 15.8 CFS respectively. Per the PPCHM and Table II above, the proposed condition 25-year storm peak flow for subarea A and subarea B is estimated at 28.3 CFS and 14.5 CFS respectively. On-site peak flow contributions to Line “B” from Line “A” and Line “C” are summarized by Table III below.

<b>TABLE III</b>				
<b>25-YEAR STORM SUBAREA PEAK RUNOFF CONVEYANCE TO LINE “B”</b>				
<b>Source Document</b>	<b>Peak Flow Rate (CFS)</b>			<b>Comments</b>
	<b>Line “A”</b>	<b>Line “C”</b>	<b>Total</b>	
AS-BUILT Plans (Exist. Condition)	36.2	6.4	42.6	Line “A” - Non-pressurized flow Line “C” – Pressurized flow @ DS portion
PECHM (Exist. Condition)	26.74	15.80	42.54	Total existing on-site runoff to Line “B” is similar to AS-BUILT values
PPCHM (Prop. Condition)	28.34	14.46	42.80	Total proposed on-site runoff to Line “B” is similar to AS-BUILT values

Peak flow values between the AS-BUILT plans and the PECHM should be similar since the PECHM is representative of existing site conditions. As anticipated, the total onsite runoff contributions to Line “B” appears to be the same. However, flow distributions to Line “A” and Line “C” are dissimilar. Since it is unclear how the AS-BUILT plan peak flows were generated, PECHM peak flows for Line “A” and Line “C” will be assumed as the baseline condition. In comparison to the PECHM, PPCHM values indicate a slight increase in peak runoff at Line “A” of 1.60 CFS, a slight reduction in peak runoff at Line “C” of about 1.34 CFS, and a slight increase in the overall on-site contributions to Line “B” of about 0.26 CFS. Due to the minute differences in peak flows, no significant changes to Line “A”, Line “B” or Line “C” is anticipated for the proposed condition.

Table IV below provides for peak flow values contributing to Line “B” based on the 2-year storm event. As shown, the total peak flow rate for the proposed condition of about 18.02 CFS is slightly larger than the existing condition peak flow rate of about 17.47 CFS, translating to an overall increase in onsite runoff of about 0.55 CFS.

<b>TABLE IV</b>				
<b>2-YEAR STORM SUBAREA PEAK RUNOFF CONVEYANCE TO LINE "B"</b>				
<b>Source Document</b>	<b>Peak Flow Rate (CFS)</b>			<b>Comments</b>
	<b>Line "A"</b>	<b>Line "C"</b>	<b>Total</b>	
PECHM (Exist. Condition)	11.51	5.96	17.47	-
PPCHM (Prop. Condition)	12.10	5.92	18.02	-
-	<b>Difference</b>		<b>0.55</b>	-

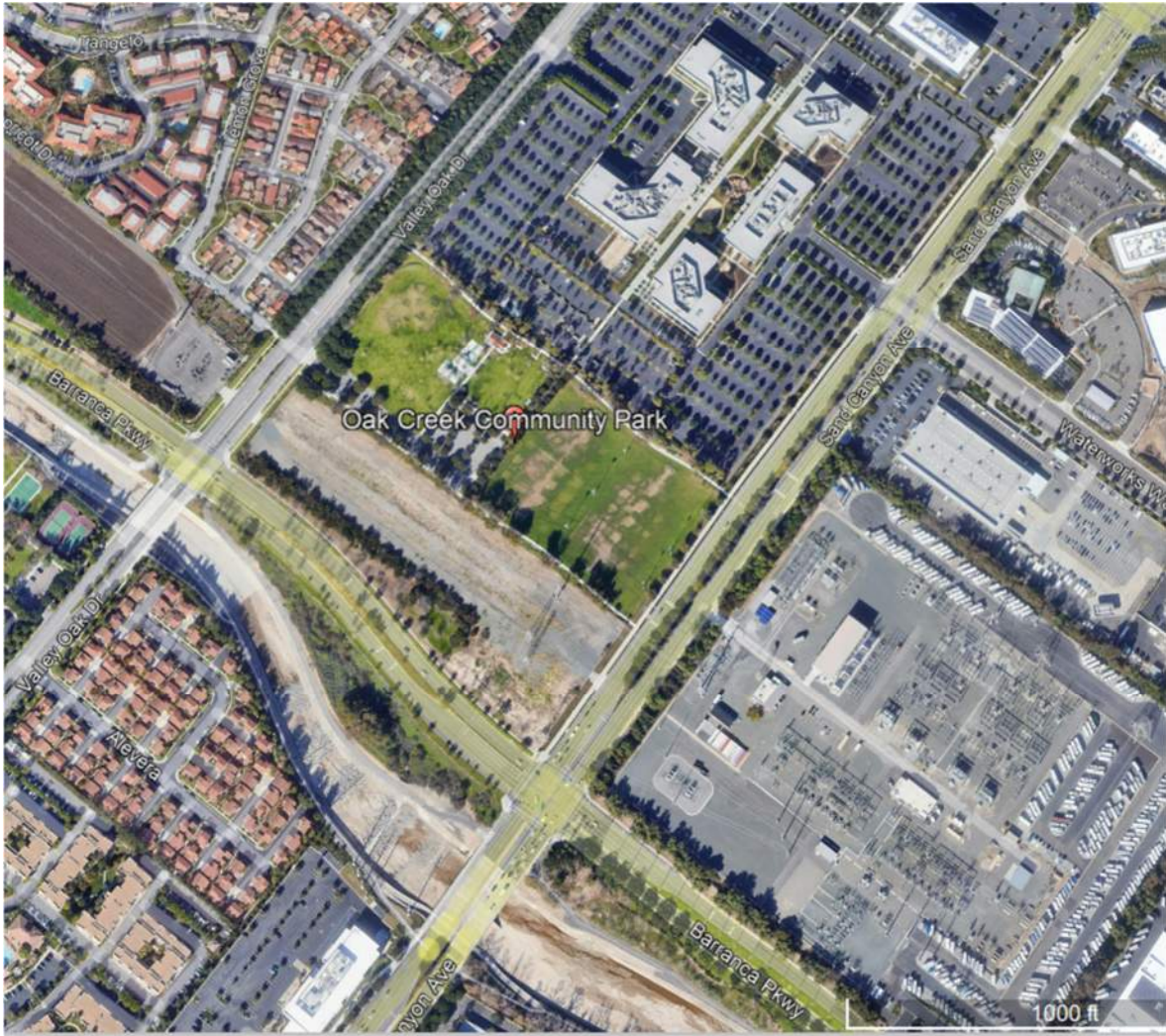
The 0.55 CFS overall increase in onsite peak runoff equates to about a 3.15% increase from the existing condition. Since the increase in onsite peak runoff is less than five percent (5%), hydromodification controls are not anticipated to be required for the project.

END OF SECTION A



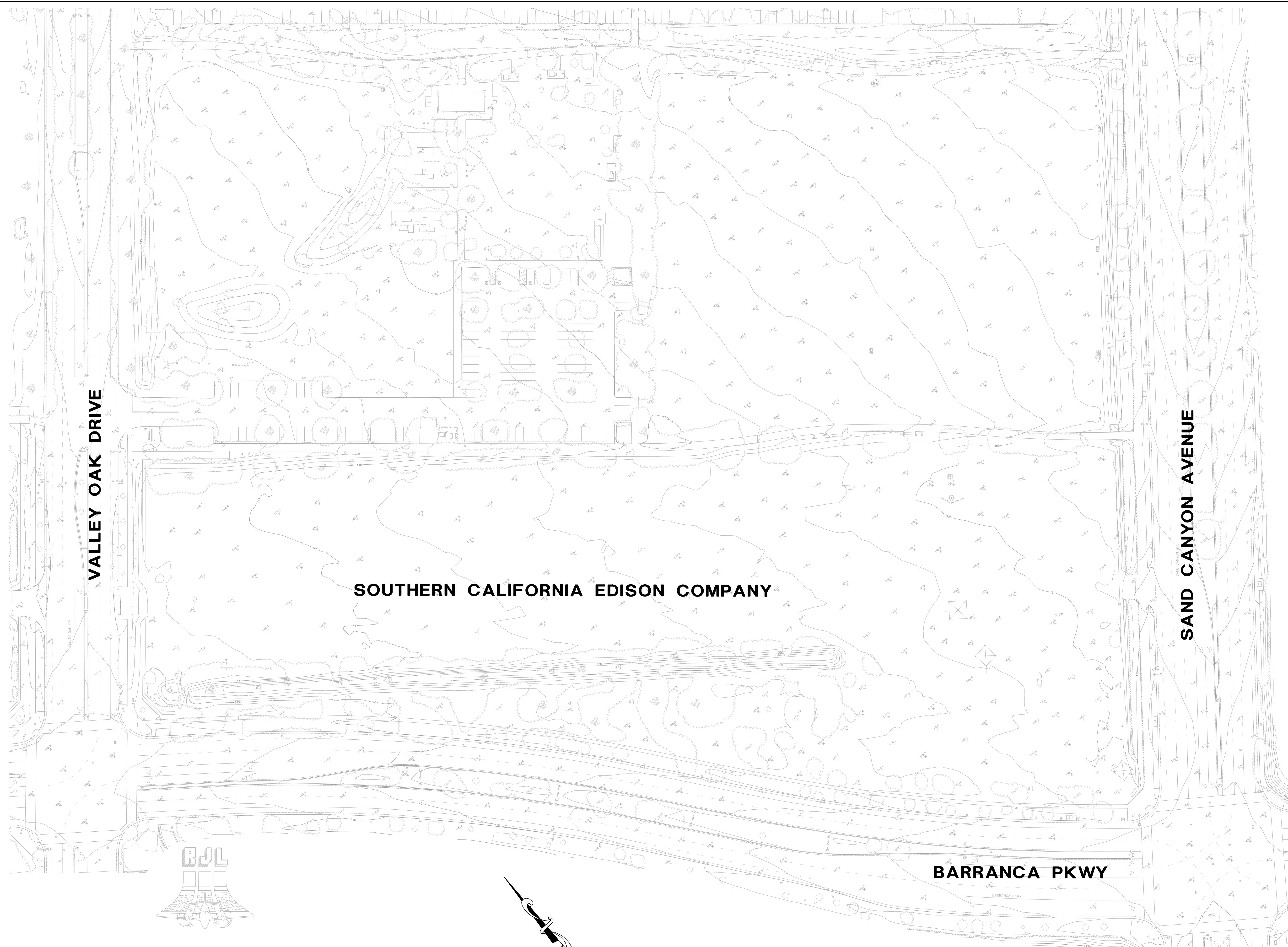
# Appendix A

## Vicinity Map



**Appendix B**  
TOPOGRAPHIC SURVEY MAP



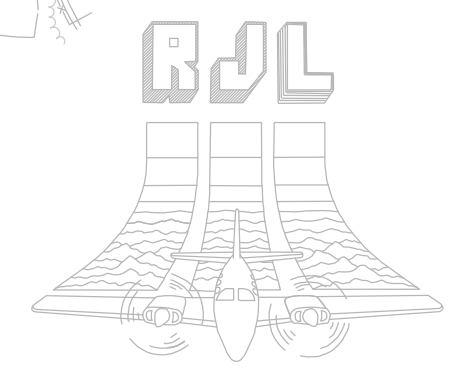


**VALLEY OAK DRIVE**

**SAND CANYON AVENUE**

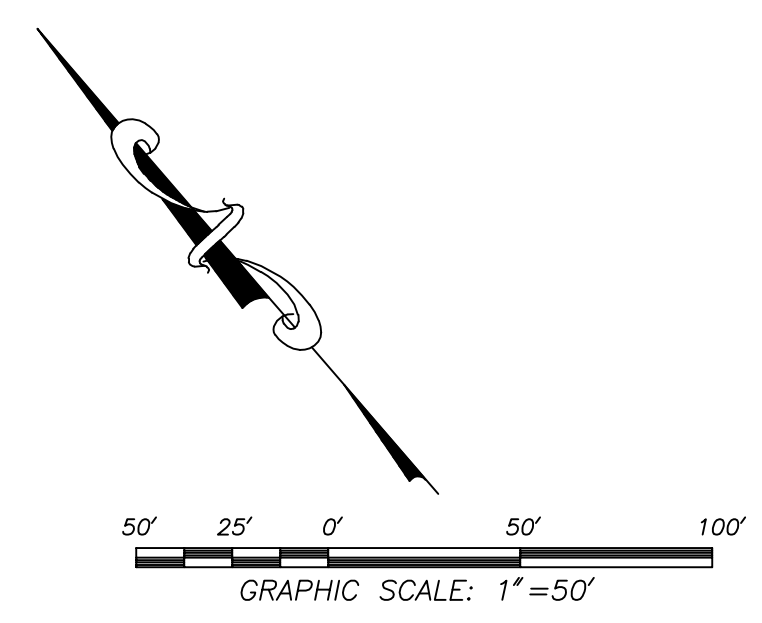
**SOUTHERN CALIFORNIA EDISON COMPANY**

**BARRANCA PKWY**



COMPILED USING PHOTOGRAMMETRIC METHODS BY  
**ROBERT J. LUND & ASSOCIATES**  
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WORK ORDER: 44133  
 DATE OF PHOTOGRAPHY: 7/24/19  
 CONTOUR INTERVAL: 1'



**CITY OF IRVINE  
 OAK CREEK COMMUNITY PARK**

**TOPOGRAPHIC SURVEY MAP**

JOB NO.: 19-2260  
 DATE: 03/17/2022  
 DRAWN BY:  
 DESIGNED BY:  
 APPROVED BY:

SHEET  
 1 OF 1

**Appendix C**  
NATIONAL FLOOD HAZARD LAYER FIRMETTE



# National Flood Hazard Layer FIRMette



117°46'35"W 33°40'14"N



0 250 500 1,000 1,500 2,000 Feet 1:6,000 F-13 117°45'58"W 33°39'44"N  
 Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020

## Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) Zone A, V, A99
		With BFE or Depth Zone AE, AO, AH, VE, AR
		Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
		Future Conditions 1% Annual Chance Flood Hazard Zone X
		Area with Reduced Flood Risk due to Levee. See Notes. Zone X
		Area with Flood Risk due to Levee Zone D

OTHER AREAS		NO SCREEN Area of Minimal Flood Hazard Zone X
		Effective LOMRs
		Area of Undetermined Flood Hazard Zone D

GENERAL STRUCTURES		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall

OTHER FEATURES		20.2 Cross Sections with 1% Annual Chance
		17.5 Water Surface Elevation
		Coastal Transect
		Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary
OTHER FEATURES		Coastal Transect Baseline
		Profile Baseline
		Hydrographic Feature

MAP PANELS		Digital Data Available
		No Digital Data Available
		Unmapped
		The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.



This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on **3/8/2022 at 1:53 PM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

**Appendix D**  
HYDROLOGIC CLASSIFICATION OF SOILS





**HYDROLOGIC CLASSIFICATION OF SOILS**

ORANGE COUNTY, CALIFORNIA

MAY 1986, PLATE B

**Appendix E**  
Existing Q<sub>2</sub> Hydrology Calculations

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RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE  
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)  
(c) Copyright 1983-2008 Advanced Engineering Software (aes)  
Ver. 15.0 Release Date: 04/01/2008 License ID 1204

Analysis prepared by:

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\*\*\*\*\* DESCRIPTION OF STUDY \*\*\*\*\*  
\* 2-YEAR STORM EVENT HYDROLOGY ANALYSIS \*  
\* OAKCREEK COMMUNITY PARK, IRVINE CALIFORNIA \*  
\* EXISTING CONDITION \*  
\*\*\*\*\*

FILE NAME: OAK02EXA.DAT  
TIME/DATE OF STUDY: 15:59 04/04/2022

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--\*TIME-OF-CONCENTRATION MODEL\*--

USER SPECIFIED STORM EVENT (YEAR) = 2.00  
SPECIFIED MINIMUM PIPE SIZE (INCH) = 8.00  
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.90  
\*DATA BANK RAINFALL USED\*  
\*ANTECEDENT MOISTURE CONDITION (AMC) I ASSUMED FOR RATIONAL METHOD\*

\*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL\*

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/ SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH (FT)	LIP (FT)	HIKE (FT)	MANNING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.50	2.00	0.0313	0.125	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:  
1. Relative Flow-Depth = 0.00 FEET  
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)  
2. (Depth)\*(Velocity) Constraint = 6.0 (FT\*FT/S)  
\*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN  
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.\*  
\*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

\*\*\*\*\*

FLOW PROCESS FROM NODE 1.00 TO NODE 2.00 IS CODE = 21

-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 251.00  
ELEVATION DATA: UPSTREAM (FEET) = 157.60 DOWNSTREAM (FEET) = 151.80

Tc = K\*[(LENGTH\*\* 3.00)/(ELEVATION CHANGE)]\*\*0.20

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.355

\* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.580

SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
PUBLIC PARK	B	0.07	0.30	0.850	36	9.36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850

SUBAREA RUNOFF(CFS) = 0.08

TOTAL AREA(ACRES) = 0.07 PEAK FLOW RATE(CFS) = 0.08

\*\*\*\*\*

FLOW PROCESS FROM NODE 2.00 TO NODE 3.00 IS CODE = 41

\*\* WARNING: Computed Flowrate is less than 0.1 cfs,  
Routing Algorithm is UNAVAILABLE.

\*\*\*\*\*

FLOW PROCESS FROM NODE 3.00 TO NODE 3.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 9.36

\* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.580

SUBAREA LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	0.24	0.30	0.850	36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850

SUBAREA AREA(ACRES) = 0.24 SUBAREA RUNOFF(CFS) = 0.29

EFFECTIVE AREA(ACRES) = 0.31 AREA-AVERAGED Fm(INCH/HR) = 0.26

AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85

TOTAL AREA(ACRES) = 0.3 PEAK FLOW RATE(CFS) = 0.37

\*\*\*\*\*

FLOW PROCESS FROM NODE 3.00 TO NODE 4.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0150

FLOW LENGTH(FEET) = 363.00 MANNING'S N = 0.010

DEPTH OF FLOW IN 8.0 INCH PIPE IS 2.4 INCHES

PIPE-FLOW VELOCITY(FEET/SEC.) = 4.12

GIVEN PIPE DIAMETER(INCH) = 8.00 NUMBER OF PIPES = 1

PIPE-FLOW(CFS) = 0.37

PIPE TRAVEL TIME(MIN.) = 1.47 Tc(MIN.) = 10.82

LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4.00 = 614.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 4.00 TO NODE 4.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 10.82



\* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.453  
SUBAREA LOSS RATE DATA (AMC I ):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
PUBLIC PARK B 0.46 0.30 0.850 36  
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
SUBAREA AREA (ACRES) = 0.46 SUBAREA RUNOFF (CFS) = 0.50  
EFFECTIVE AREA (ACRES) = 0.77 AREA-AVERAGED Fm (INCH/HR) = 0.26  
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85  
TOTAL AREA (ACRES) = 0.8 PEAK FLOW RATE (CFS) = 0.83

\*\*\*\*\*

FLOW PROCESS FROM NODE 4.00 TO NODE 5.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0150  
FLOW LENGTH (FEET) = 97.00 MANNING'S N = 0.010  
DEPTH OF FLOW IN 8.0 INCH PIPE IS 3.8 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.11  
GIVEN PIPE DIAMETER (INCH) = 8.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 0.83  
PIPE TRAVEL TIME (MIN.) = 0.32 Tc (MIN.) = 11.14  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 5.00 = 711.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 5.00 TO NODE 5.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 11.14  
\* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.429  
SUBAREA LOSS RATE DATA (AMC I ):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
PUBLIC PARK B 0.11 0.30 0.850 36  
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
SUBAREA AREA (ACRES) = 0.11 SUBAREA RUNOFF (CFS) = 0.12  
EFFECTIVE AREA (ACRES) = 0.88 AREA-AVERAGED Fm (INCH/HR) = 0.26  
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85  
TOTAL AREA (ACRES) = 0.9 PEAK FLOW RATE (CFS) = 0.93

\*\*\*\*\*

FLOW PROCESS FROM NODE 5.00 TO NODE 6.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0150  
FLOW LENGTH (FEET) = 135.00 MANNING'S N = 0.010  
DEPTH OF FLOW IN 8.0 INCH PIPE IS 4.0 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.25  
GIVEN PIPE DIAMETER (INCH) = 8.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 0.93

PIPE TRAVEL TIME (MIN.) = 0.43 Tc (MIN.) = 11.57  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 6.00 = 846.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 6.00 TO NODE 6.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 11.57  
\* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.399  
SUBAREA LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	0.77	0.30	0.850	36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
SUBAREA AREA (ACRES) = 0.77 SUBAREA RUNOFF (CFS) = 0.79  
EFFECTIVE AREA (ACRES) = 1.65 AREA-AVERAGED Fm (INCH/HR) = 0.26  
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85  
TOTAL AREA (ACRES) = 1.6 PEAK FLOW RATE (CFS) = 1.70

\*\*\*\*\*  
FLOW PROCESS FROM NODE 6.00 TO NODE 7.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0150  
FLOW LENGTH (FEET) = 5.00 MANNING'S N = 0.010  
DEPTH OF FLOW IN 12.0 INCH PIPE IS 4.6 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.08  
GIVEN PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 1.70  
PIPE TRAVEL TIME (MIN.) = 0.01 Tc (MIN.) = 11.58  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 7.00 = 851.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 7.00 TO NODE 7.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 11.58  
\* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.398  
SUBAREA LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	3.91	0.30	0.850	36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
SUBAREA AREA (ACRES) = 3.91 SUBAREA RUNOFF (CFS) = 4.02  
EFFECTIVE AREA (ACRES) = 5.56 AREA-AVERAGED Fm (INCH/HR) = 0.26  
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85  
TOTAL AREA (ACRES) = 5.6 PEAK FLOW RATE (CFS) = 5.72

\*\*\*\*\*  
FLOW PROCESS FROM NODE 7.00 TO NODE 8.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0111  
FLOW LENGTH (FEET) = 231.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 8.2 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.00  
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 5.72  
PIPE TRAVEL TIME (MIN.) = 0.64 Tc (MIN.) = 12.22  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 8.00 = 1082.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION (MIN.) = 12.22  
RAINFALL INTENSITY (INCH/HR) = 1.36  
AREA-AVERAGED Fm (INCH/HR) = 0.26  
AREA-AVERAGED Fp (INCH/HR) = 0.30  
AREA-AVERAGED Ap = 0.85  
EFFECTIVE STREAM AREA (ACRES) = 5.56  
TOTAL STREAM AREA (ACRES) = 5.56  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 5.72

\*\*\*\*\*

FLOW PROCESS FROM NODE 8.10 TO NODE 8.20 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00  
ELEVATION DATA: UPSTREAM (FEET) = 150.00 DOWNSTREAM (FEET) = 144.55

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$   
SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 10.542  
\* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.475  
SUBAREA Tc AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
PUBLIC PARK	B	0.76	0.30	0.850	36	10.54

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
SUBAREA RUNOFF (CFS) = 0.83  
TOTAL AREA (ACRES) = 0.76 PEAK FLOW RATE (CFS) = 0.83

\*\*\*\*\*

FLOW PROCESS FROM NODE 8.20 TO NODE 8.30 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0150  
FLOW LENGTH (FEET) = 167.00 MANNING'S N = 0.010

DEPTH OF FLOW IN 8.0 INCH PIPE IS 3.8 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 5.10  
 GIVEN PIPE DIAMETER (INCH) = 8.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 0.83  
 PIPE TRAVEL TIME (MIN.) = 0.55 Tc (MIN.) = 11.09  
 LONGEST FLOWPATH FROM NODE 8.10 TO NODE 8.30 = 467.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 8.30 TO NODE 8.30 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 11.09  
 \* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.433  
 SUBAREA LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.60	0.30	0.100	36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100  
 SUBAREA AREA (ACRES) = 0.60 SUBAREA RUNOFF (CFS) = 0.76  
 EFFECTIVE AREA (ACRES) = 1.36 AREA-AVERAGED Fm (INCH/HR) = 0.16  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.52  
 TOTAL AREA (ACRES) = 1.4 PEAK FLOW RATE (CFS) = 1.56

\*\*\*\*\*

FLOW PROCESS FROM NODE 8.30 TO NODE 8.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0077  
 FLOW LENGTH (FEET) = 37.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 5.2 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 3.74  
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 1.56  
 PIPE TRAVEL TIME (MIN.) = 0.17 Tc (MIN.) = 11.25  
 LONGEST FLOWPATH FROM NODE 8.10 TO NODE 8.00 = 504.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
 TIME OF CONCENTRATION (MIN.) = 11.25  
 RAINFALL INTENSITY (INCH/HR) = 1.42  
 AREA-AVERAGED Fm (INCH/HR) = 0.16  
 AREA-AVERAGED Fp (INCH/HR) = 0.30  
 AREA-AVERAGED Ap = 0.52  
 EFFECTIVE STREAM AREA (ACRES) = 1.36  
 TOTAL STREAM AREA (ACRES) = 1.36  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.56

\*\*\*\*\*

FLOW PROCESS FROM NODE 8.40 TO NODE 8.50 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 252.00  
ELEVATION DATA: UPSTREAM(FEET) = 147.00 DOWNSTREAM(FEET) = 142.68

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM  $T_c$ (MIN.) = 6.261

\* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.990

SUBAREA  $T_c$  AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	$T_c$ (MIN.)
COMMERCIAL	B	0.44	0.30	0.100	36	6.26

SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$ (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.100

SUBAREA RUNOFF(CFS) = 0.78

TOTAL AREA(ACRES) = 0.44 PEAK FLOW RATE(CFS) = 0.78

\*\*\*\*\*

FLOW PROCESS FROM NODE 8.50 TO NODE 8.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0557

FLOW LENGTH(FEET) = 37.70 MANNING'S N = 0.013

DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.2 INCHES

PIPE-FLOW VELOCITY(FEET/SEC.) = 6.13

GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1

PIPE-FLOW(CFS) = 0.78

PIPE TRAVEL TIME(MIN.) = 0.10  $T_c$ (MIN.) = 6.36

LONGEST FLOWPATH FROM NODE 8.40 TO NODE 8.00 = 289.70 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 3

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:

TIME OF CONCENTRATION(MIN.) = 6.36

RAINFALL INTENSITY(INCH/HR) = 1.97

AREA-AVERAGED  $F_m$ (INCH/HR) = 0.03

AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30

AREA-AVERAGED  $A_p$  = 0.10

EFFECTIVE STREAM AREA(ACRES) = 0.44

TOTAL STREAM AREA(ACRES) = 0.44

PEAK FLOW RATE(CFS) AT CONFLUENCE = 0.78

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	$T_c$ (MIN.)	Intensity (INCH/HR)	$F_p$ ( $F_m$ ) (INCH/HR)	$A_p$	$A_e$ (ACRES)	HEADWATER NODE
1	5.72	12.22	1.355	0.30(0.26)	0.85	5.6	1.00
2	1.56	11.25	1.421	0.30(0.16)	0.52	1.4	8.10

3            0.78        6.36        1.971    0.30( 0.03) 0.10            0.4            8.40

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 3 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	6.69	6.36	1.971	0.30( 0.21)	0.71	4.1	8.40
2	7.70	11.25	1.421	0.30( 0.22)	0.74	6.9	8.10
3	7.73	12.22	1.355	0.30( 0.22)	0.74	7.4	1.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 7.73      Tc(MIN.) = 12.22  
 EFFECTIVE AREA(ACRES) = 7.36      AREA-AVERAGED Fm(INCH/HR) = 0.22  
 AREA-AVERAGED Fp(INCH/HR) = 0.30      AREA-AVERAGED Ap = 0.74  
 TOTAL AREA(ACRES) = 7.4  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 8.00 = 1082.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 8.00 TO NODE 9.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0050  
 FLOW LENGTH(FEET) = 261.00      MANNING'S N = 0.013  
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 12.1 INCHES  
 PIPE-FLOW VELOCITY(FEET/SEC.) = 4.85  
 GIVEN PIPE DIAMETER(INCH) = 24.00      NUMBER OF PIPES = 1  
 PIPE-FLOW(CFS) = 7.73  
 PIPE TRAVEL TIME(MIN.) = 0.90      Tc(MIN.) = 13.12  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 9.00 = 1343.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 9.00 TO NODE 9.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 13.12  
 \* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.301  
 SUBAREA LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.52	0.30	0.100	36
PUBLIC PARK	B	3.04	0.30	0.850	36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.740  
 SUBAREA AREA(ACRES) = 3.56      SUBAREA RUNOFF(CFS) = 3.46  
 EFFECTIVE AREA(ACRES) = 10.92      AREA-AVERAGED Fm(INCH/HR) = 0.22  
 AREA-AVERAGED Fp(INCH/HR) = 0.30      AREA-AVERAGED Ap = 0.74  
 TOTAL AREA(ACRES) = 10.9      PEAK FLOW RATE(CFS) = 10.60

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	11.08	7.29	1.823	0.30( 0.22)	0.72	7.7	8.40

2 10.74 12.15 1.360 0.30( 0.22) 0.74 10.5 8.10  
3 10.60 13.12 1.301 0.30( 0.22) 0.74 10.9 1.00

NEW PEAK FLOW DATA ARE:

PEAK FLOW RATE (CFS) = 11.08 Tc (MIN.) = 7.29  
AREA-AVERAGED Fm (INCH/HR) = 0.22 AREA-AVERAGED Fp (INCH/HR) = 0.30  
AREA-AVERAGED Ap = 0.72 EFFECTIVE AREA (ACRES) = 7.66

\*\*\*\*\*  
FLOW PROCESS FROM NODE 9.00 TO NODE 9.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 7.29  
\* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.823  
SUBAREA LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	0.31	0.30	0.850	36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
SUBAREA AREA (ACRES) = 0.31 SUBAREA RUNOFF (CFS) = 0.44  
EFFECTIVE AREA (ACRES) = 7.97 AREA-AVERAGED Fm (INCH/HR) = 0.22  
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.73  
TOTAL AREA (ACRES) = 11.2 PEAK FLOW RATE (CFS) = 11.51

\*\*\*\*\*  
FLOW PROCESS FROM NODE 9.00 TO NODE 10.00 IS CODE = 41  
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0050  
FLOW LENGTH (FEET) = 45.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 15.6 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.31  
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 11.51  
PIPE TRAVEL TIME (MIN.) = 0.14 Tc (MIN.) = 7.43  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 10.00 = 1388.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 10.00 TO NODE 11.00 IS CODE = 41  
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0100  
FLOW LENGTH (FEET) = 43.70 MANNING'S N = 0.013  
DEPTH OF FLOW IN 36.0 INCH PIPE IS 10.4 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.80  
GIVEN PIPE DIAMETER (INCH) = 36.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 11.51  
PIPE TRAVEL TIME (MIN.) = 0.11 Tc (MIN.) = 7.54  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 11.00 = 1431.70 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 11.00 TO NODE 12.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<  
=====

REPRESENTATIVE SLOPE = 0.0100  
FLOW LENGTH(FEET) = 93.90 MANNING'S N = 0.013  
DEPTH OF FLOW IN 36.0 INCH PIPE IS 10.4 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.80  
GIVEN PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 11.51  
PIPE TRAVEL TIME(MIN.) = 0.23 Tc(MIN.) = 7.77  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 12.00 = 1525.60 FEET.  
=====

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END OF RATIONAL METHOD ANALYSIS



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FLOW PROCESS FROM NODE 21.00 TO NODE 22.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 2.35
ELEVATION DATA: UPSTREAM(FEET) = 154.00 DOWNSTREAM(FEET) = 149.00

Tc = K\*[(LENGTH\*\* 3.00)/(ELEVATION CHANGE)]\*\*0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 5.000
\* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 2.264

SUBAREA Tc AND LOSS RATE DATA(AMC I ):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
NATURAL POOR COVER
"BARREN" B 0.63 0.30 1.000 72 5.00
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000
SUBAREA RUNOFF(CFS) = 1.11
TOTAL AREA(ACRES) = 0.63 PEAK FLOW RATE(CFS) = 1.11

\*\*\*\*\*

FLOW PROCESS FROM NODE 22.00 TO NODE 23.00 IS CODE = 52

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 149.00 DOWNSTREAM(FEET) = 139.90
CHANNEL LENGTH THRU SUBAREA(FEET) = 858.00 CHANNEL SLOPE = 0.0106
CHANNEL FLOW THRU SUBAREA(CFS) = 1.11
FLOW VELOCITY(FEET/SEC) = 1.58 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 9.07 Tc(MIN.) = 14.07
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 23.00 = 860.35 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 14.07
\* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.250
SUBAREA LOSS RATE DATA(AMC I ):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
NATURAL POOR COVER
"BARREN" B 4.59 0.30 1.000 72
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000
SUBAREA AREA(ACRES) = 4.59 SUBAREA RUNOFF(CFS) = 3.92
EFFECTIVE AREA(ACRES) = 5.22 AREA-AVERAGED Fm(INCH/HR) = 0.30
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 1.00
TOTAL AREA(ACRES) = 5.2 PEAK FLOW RATE(CFS) = 4.46

\*\*\*\*\*

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 14.07  
RAINFALL INTENSITY(INCH/HR) = 1.25  
AREA-AVERAGED Fm(INCH/HR) = 0.30  
AREA-AVERAGED Fp(INCH/HR) = 0.30  
AREA-AVERAGED Ap = 1.00  
EFFECTIVE STREAM AREA(ACRES) = 5.22  
TOTAL STREAM AREA(ACRES) = 5.22  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.46

\*\*\*\*\*  
FLOW PROCESS FROM NODE 23.10 TO NODE 23.20 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 285.00  
ELEVATION DATA: UPSTREAM(FEET) = 150.80 DOWNSTREAM(FEET) = 147.80

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$   
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.521  
\* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.337

SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
NATURAL POOR COVER						
"BARREN"	B	0.84	0.30	1.000	72	12.52

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000

SUBAREA RUNOFF(CFS) = 0.78

TOTAL AREA(ACRES) = 0.84 PEAK FLOW RATE(CFS) = 0.78

\*\*\*\*\*  
FLOW PROCESS FROM NODE 23.20 TO NODE 23.00 IS CODE = 52

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 147.80 DOWNSTREAM(FEET) = 139.90  
CHANNEL LENGTH THRU SUBAREA(FEET) = 791.00 CHANNEL SLOPE = 0.0100  
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION  
CHANNEL FLOW THRU SUBAREA(CFS) = 0.78  
FLOW VELOCITY(FEET/SEC) = 1.50 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)  
TRAVEL TIME(MIN.) = 8.79 Tc(MIN.) = 21.32  
LONGEST FLOWPATH FROM NODE 23.10 TO NODE 23.00 = 1076.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 21.32  
\* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 0.985  
SUBAREA LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
NATURAL POOR COVER "BARREN"	B	1.81	0.30	1.000	72
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000					
SUBAREA AREA(ACRES) = 1.81 SUBAREA RUNOFF(CFS) = 1.12					
EFFECTIVE AREA(ACRES) = 2.65 AREA-AVERAGED Fm(INCH/HR) = 0.30					
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 1.00					
TOTAL AREA(ACRES) = 2.6 PEAK FLOW RATE(CFS) = 1.63					

\*\*\*\*\*  
FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 21.32  
RAINFALL INTENSITY(INCH/HR) = 0.98  
AREA-AVERAGED Fm(INCH/HR) = 0.30  
AREA-AVERAGED Fp(INCH/HR) = 0.30  
AREA-AVERAGED Ap = 1.00  
EFFECTIVE STREAM AREA(ACRES) = 2.65  
TOTAL STREAM AREA(ACRES) = 2.65  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.63

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.46	14.07	1.250	0.30( 0.30)	1.00	5.2	21.00
2	1.63	21.32	0.985	0.30( 0.30)	1.00	2.6	23.10

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.96	14.07	1.250	0.30( 0.30)	1.00	7.0	21.00
2	4.85	21.32	0.985	0.30( 0.30)	1.00	7.9	23.10

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 5.96 Tc(MIN.) = 14.07  
EFFECTIVE AREA(ACRES) = 6.97 AREA-AVERAGED Fm(INCH/HR) = 0.30  
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 1.00  
TOTAL AREA(ACRES) = 7.9  
LONGEST FLOWPATH FROM NODE 23.10 TO NODE 23.00 = 1076.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 23.00 TO NODE 24.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0100

FLOW LENGTH (FEET) = 45.70 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.0 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 5.89  
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 5.96  
 PIPE TRAVEL TIME (MIN.) = 0.13 Tc (MIN.) = 14.20  
 LONGEST FLOWPATH FROM NODE 23.10 TO NODE 24.00 = 1121.70 FEET.

=====

END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 7.9 TC (MIN.) = 14.20  
 EFFECTIVE AREA (ACRES) = 6.97 AREA-AVERAGED Fm (INCH/HR) = 0.30  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 1.000  
 PEAK FLOW RATE (CFS) = 5.96

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.96	14.20	1.243	0.30 ( 0.30)	1.00	7.0	21.00
2	4.85	21.45	0.981	0.30 ( 0.30)	1.00	7.9	23.10

=====

END OF RATIONAL METHOD ANALYSIS

**Appendix F**  
Existing  $Q_{10}$  Hydrology Calculations

\*\*\*\*\*

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE  
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)  
(c) Copyright 1983-2008 Advanced Engineering Software (aes)  
Ver. 15.0 Release Date: 04/01/2008 License ID 1204

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\*\*\*\*\* DESCRIPTION OF STUDY \*\*\*\*\*  
\* 10-YEAR STORM EVENT HYDROLOGY ANALYSIS \*  
\* OAKCREEK COMMUNITY PARK, IRVINE CALIFORNIA \*  
\* EXISTING CONDITION \*  
\*\*\*\*\*

FILE NAME: OAK10EXA.DAT  
TIME/DATE OF STUDY: 16:11 04/04/2022

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--\*TIME-OF-CONCENTRATION MODEL\*--

USER SPECIFIED STORM EVENT (YEAR) = 10.00  
SPECIFIED MINIMUM PIPE SIZE (INCH) = 8.00  
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.90  
\*DATA BANK RAINFALL USED\*  
\*ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD\*

\*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL\*

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/ SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH (FT)	LIP (FT)	HIKE (FT)	MANNING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.50	2.00	0.0313	0.125	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:  
1. Relative Flow-Depth = 0.00 FEET  
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)  
2. (Depth)\*(Velocity) Constraint = 6.0 (FT\*FT/S)  
\*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN  
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.\*  
\*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

\*\*\*\*\*

FLOW PROCESS FROM NODE 1.00 TO NODE 2.00 IS CODE = 21

-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 251.00  
ELEVATION DATA: UPSTREAM (FEET) = 157.60 DOWNSTREAM (FEET) = 151.80

$T_c = K * [(LENGTH^{**} 3.00) / (ELEVATION CHANGE)]^{**} 0.20$   
 SUBAREA ANALYSIS USED MINIMUM  $T_c$  (MIN.) = 9.355  
 \* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.835  
 SUBAREA  $T_c$  AND LOSS RATE DATA (AMC II):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN	$T_c$ (MIN.)
PUBLIC PARK	B	0.07	0.30	0.850	56	9.36

 SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$  (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.850  
 SUBAREA RUNOFF (CFS) = 0.16  
 TOTAL AREA (ACRES) = 0.07 PEAK FLOW RATE (CFS) = 0.16

\*\*\*\*\*

FLOW PROCESS FROM NODE 2.00 TO NODE 3.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0150  
 FLOW LENGTH (FEET) = 49.40 MANNING'S N = 0.010  
 DEPTH OF FLOW IN 6.0 INCH PIPE IS 1.8 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 3.35  
 GIVEN PIPE DIAMETER (INCH) = 6.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 0.16  
 PIPE TRAVEL TIME (MIN.) = 0.25  $T_c$  (MIN.) = 9.60  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 300.40 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 3.00 TO NODE 3.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE  $T_c$  (MIN.) = 9.60  
 \* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.793  
 SUBAREA LOSS RATE DATA (AMC II):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN
PUBLIC PARK	B	0.24	0.30	0.850	56

 SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$  (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.850  
 SUBAREA AREA (ACRES) = 0.24 SUBAREA RUNOFF (CFS) = 0.55  
 EFFECTIVE AREA (ACRES) = 0.31 AREA-AVERAGED  $F_m$  (INCH/HR) = 0.26  
 AREA-AVERAGED  $F_p$  (INCH/HR) = 0.30 AREA-AVERAGED  $A_p$  = 0.85  
 TOTAL AREA (ACRES) = 0.3 PEAK FLOW RATE (CFS) = 0.71

\*\*\*\*\*

FLOW PROCESS FROM NODE 3.00 TO NODE 4.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0150  
 FLOW LENGTH (FEET) = 363.00 MANNING'S N = 0.010  
 DEPTH OF FLOW IN 8.0 INCH PIPE IS 3.5 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 4.91  
 GIVEN PIPE DIAMETER (INCH) = 8.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 0.71

PIPE TRAVEL TIME (MIN.) = 1.23 Tc (MIN.) = 10.83  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4.00 = 663.40 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 4.00 TO NODE 4.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 10.83  
\* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.607  
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	0.46	0.30	0.850	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
SUBAREA AREA (ACRES) = 0.46 SUBAREA RUNOFF (CFS) = 0.97  
EFFECTIVE AREA (ACRES) = 0.77 AREA-AVERAGED Fm (INCH/HR) = 0.26  
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85  
TOTAL AREA (ACRES) = 0.8 PEAK FLOW RATE (CFS) = 1.63

\*\*\*\*\*  
FLOW PROCESS FROM NODE 4.00 TO NODE 5.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT) <<<<<

=====

REPRESENTATIVE SLOPE = 0.0150  
FLOW LENGTH (FEET) = 97.00 MANNING'S N = 0.010  
DEPTH OF FLOW IN 8.0 INCH PIPE IS 5.9 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.92  
GIVEN PIPE DIAMETER (INCH) = 8.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 1.63  
PIPE TRAVEL TIME (MIN.) = 0.27 Tc (MIN.) = 11.11  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 5.00 = 760.40 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 5.00 TO NODE 5.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 11.11  
\* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.570  
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	0.11	0.30	0.850	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
SUBAREA AREA (ACRES) = 0.11 SUBAREA RUNOFF (CFS) = 0.23  
EFFECTIVE AREA (ACRES) = 0.88 AREA-AVERAGED Fm (INCH/HR) = 0.26  
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85  
TOTAL AREA (ACRES) = 0.9 PEAK FLOW RATE (CFS) = 1.83

\*\*\*\*\*  
FLOW PROCESS FROM NODE 5.00 TO NODE 6.00 IS CODE = 41



>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0150  
FLOW LENGTH(FEET) = 135.00 MANNING'S N = 0.010  
ASSUME FULL-FLOWING PIPELINE  
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.25  
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)  
GIVEN PIPE DIAMETER(INCH) = 8.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 1.83  
PIPE TRAVEL TIME(MIN.) = 0.43 Tc(MIN.) = 11.53  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 6.00 = 895.40 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 6.00 TO NODE 6.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 11.53  
\* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.515  
SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	0.77	0.30	0.850	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
SUBAREA AREA(ACRES) = 0.77 SUBAREA RUNOFF(CFS) = 1.57  
EFFECTIVE AREA(ACRES) = 1.65 AREA-AVERAGED Fm(INCH/HR) = 0.26  
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85  
TOTAL AREA(ACRES) = 1.6 PEAK FLOW RATE(CFS) = 3.36

\*\*\*\*\*  
FLOW PROCESS FROM NODE 6.00 TO NODE 7.00 IS CODE = 41  
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0150  
FLOW LENGTH(FEET) = 5.00 MANNING'S N = 0.010  
DEPTH OF FLOW IN 12.0 INCH PIPE IS 6.9 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.22  
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 3.36  
PIPE TRAVEL TIME(MIN.) = 0.01 Tc(MIN.) = 11.55  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 7.00 = 900.40 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 7.00 TO NODE 7.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 11.55  
\* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.513  
SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	3.91	0.30	0.850	56

SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$  (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.850  
 SUBAREA AREA (ACRES) = 3.91 SUBAREA RUNOFF (CFS) = 7.95  
 EFFECTIVE AREA (ACRES) = 5.56 AREA-AVERAGED  $F_m$  (INCH/HR) = 0.26  
 AREA-AVERAGED  $F_p$  (INCH/HR) = 0.30 AREA-AVERAGED  $A_p$  = 0.85  
 TOTAL AREA (ACRES) = 5.6 PEAK FLOW RATE (CFS) = 11.30

\*\*\*\*\*

FLOW PROCESS FROM NODE 7.00 TO NODE 8.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0111  
 FLOW LENGTH (FEET) = 231.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 12.0 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 7.20  
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 11.30  
 PIPE TRAVEL TIME (MIN.) = 0.54  $T_c$  (MIN.) = 12.08  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 8.00 = 1131.40 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 3  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
 TIME OF CONCENTRATION (MIN.) = 12.08  
 RAINFALL INTENSITY (INCH/HR) = 2.45  
 AREA-AVERAGED  $F_m$  (INCH/HR) = 0.26  
 AREA-AVERAGED  $F_p$  (INCH/HR) = 0.30  
 AREA-AVERAGED  $A_p$  = 0.85  
 EFFECTIVE STREAM AREA (ACRES) = 5.56  
 TOTAL STREAM AREA (ACRES) = 5.56  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 11.30

\*\*\*\*\*

FLOW PROCESS FROM NODE 8.10 TO NODE 8.20 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00  
 ELEVATION DATA: UPSTREAM (FEET) = 150.00 DOWNSTREAM (FEET) = 144.55

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM  $T_c$  (MIN.) = 10.542

\* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.648

SUBAREA  $T_c$  AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN	$T_c$ (MIN.)
PUBLIC PARK	B	0.76	0.30	0.850	56	10.54

SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$  (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.850

SUBAREA RUNOFF (CFS) = 1.64

TOTAL AREA (ACRES) = 0.76 PEAK FLOW RATE (CFS) = 1.64

\*\*\*\*\*  
FLOW PROCESS FROM NODE 8.20 TO NODE 8.30 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0150  
FLOW LENGTH (FEET) = 167.00 MANNING'S N = 0.010  
DEPTH OF FLOW IN 8.0 INCH PIPE IS 5.9 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.92  
GIVEN PIPE DIAMETER (INCH) = 8.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 1.64  
PIPE TRAVEL TIME (MIN.) = 0.47 Tc (MIN.) = 11.01  
LONGEST FLOWPATH FROM NODE 8.10 TO NODE 8.30 = 467.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 8.30 TO NODE 8.30 IS CODE = 81

-----  
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 11.01  
\* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.582  
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.60	0.30	0.100	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100  
SUBAREA AREA (ACRES) = 0.60 SUBAREA RUNOFF (CFS) = 1.38  
EFFECTIVE AREA (ACRES) = 1.36 AREA-AVERAGED Fm (INCH/HR) = 0.16  
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.52  
TOTAL AREA (ACRES) = 1.4 PEAK FLOW RATE (CFS) = 2.97

\*\*\*\*\*  
FLOW PROCESS FROM NODE 8.30 TO NODE 8.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0077  
FLOW LENGTH (FEET) = 37.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 7.2 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.47  
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 2.97  
PIPE TRAVEL TIME (MIN.) = 0.14 Tc (MIN.) = 11.15  
LONGEST FLOWPATH FROM NODE 8.10 TO NODE 8.00 = 504.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION(MIN.) = 11.15  
 RAINFALL INTENSITY(INCH/HR) = 2.56  
 AREA-AVERAGED Fm(INCH/HR) = 0.16  
 AREA-AVERAGED Fp(INCH/HR) = 0.30  
 AREA-AVERAGED Ap = 0.52  
 EFFECTIVE STREAM AREA(ACRES) = 1.36  
 TOTAL STREAM AREA(ACRES) = 1.36  
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.97

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.40 TO NODE 8.50 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 252.00  
 ELEVATION DATA: UPSTREAM(FEET) = 147.00 DOWNSTREAM(FEET) = 142.68

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$   
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 6.261  
 \* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.569

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.44	0.30	0.100	56	6.26

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 1.40

TOTAL AREA(ACRES) = 0.44 PEAK FLOW RATE(CFS) = 1.40

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.50 TO NODE 8.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0557  
 FLOW LENGTH(FEET) = 37.70 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.0 INCHES  
 PIPE-FLOW VELOCITY(FEET/SEC.) = 7.31  
 GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
 PIPE-FLOW(CFS) = 1.40  
 PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 6.35  
 LONGEST FLOWPATH FROM NODE 8.40 TO NODE 8.00 = 289.70 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 3  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:  
 TIME OF CONCENTRATION(MIN.) = 6.35  
 RAINFALL INTENSITY(INCH/HR) = 3.54  
 AREA-AVERAGED Fm(INCH/HR) = 0.03  
 AREA-AVERAGED Fp(INCH/HR) = 0.30

AREA-AVERAGED  $A_p = 0.10$   
 EFFECTIVE STREAM AREA (ACRES) = 0.44  
 TOTAL STREAM AREA (ACRES) = 0.44  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.40

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	11.30	12.08	2.449	0.30 ( 0.26)	0.85	5.6	1.00
2	2.97	11.15	2.564	0.30 ( 0.16)	0.52	1.4	8.10
3	1.40	6.35	3.541	0.30 ( 0.03)	0.10	0.4	8.40

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 3 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	12.67	6.35	3.541	0.30 ( 0.21)	0.71	4.1	8.40
2	14.96	11.15	2.564	0.30 ( 0.22)	0.74	6.9	8.10
3	15.09	12.08	2.449	0.30 ( 0.22)	0.74	7.4	1.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 15.09 Tc (MIN.) = 12.08  
 EFFECTIVE AREA (ACRES) = 7.36 AREA-AVERAGED Fm (INCH/HR) = 0.22  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.74  
 TOTAL AREA (ACRES) = 7.4  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 8.00 = 1131.40 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 8.00 TO NODE 9.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0050  
 FLOW LENGTH (FEET) = 261.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 19.5 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 5.51  
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 15.09  
 PIPE TRAVEL TIME (MIN.) = 0.79 Tc (MIN.) = 12.87  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 9.00 = 1392.40 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 9.00 TO NODE 9.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc (MIN.) = 12.87  
 \* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.362  
 SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.52	0.30	0.100	56
PUBLIC PARK	B	3.04	0.30	0.850	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p = 0.740$   
 SUBAREA AREA (ACRES) = 3.56 SUBAREA RUNOFF (CFS) = 6.85  
 EFFECTIVE AREA (ACRES) = 10.92 AREA-AVERAGED  $F_m$  (INCH/HR) = 0.22  
 AREA-AVERAGED  $F_p$  (INCH/HR) = 0.30 AREA-AVERAGED  $A_p = 0.74$   
 TOTAL AREA (ACRES) = 10.9 PEAK FLOW RATE (CFS) = 21.02

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	21.40	7.15	3.307	0.30 ( 0.22)	0.72	7.7	8.40
2	21.19	11.94	2.465	0.30 ( 0.22)	0.74	10.5	8.10
3	21.02	12.87	2.362	0.30 ( 0.22)	0.74	10.9	1.00

NEW PEAK FLOW DATA ARE:

PEAK FLOW RATE (CFS) = 21.40 Tc (MIN.) = 7.15  
 AREA-AVERAGED  $F_m$  (INCH/HR) = 0.22 AREA-AVERAGED  $F_p$  (INCH/HR) = 0.30  
 AREA-AVERAGED  $A_p = 0.72$  EFFECTIVE AREA (ACRES) = 7.70

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 9.00 TO NODE 9.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc (MIN.) = 7.15  
 \* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 3.307

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN
PUBLIC PARK	B	0.31	0.30	0.850	56

SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$  (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p = 0.850$   
 SUBAREA AREA (ACRES) = 0.31 SUBAREA RUNOFF (CFS) = 0.85  
 EFFECTIVE AREA (ACRES) = 8.01 AREA-AVERAGED  $F_m$  (INCH/HR) = 0.22  
 AREA-AVERAGED  $F_p$  (INCH/HR) = 0.30 AREA-AVERAGED  $A_p = 0.73$   
 TOTAL AREA (ACRES) = 11.2 PEAK FLOW RATE (CFS) = 22.25

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 9.00 TO NODE 10.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0050  
 FLOW LENGTH (FEET) = 45.00 MANNING'S N = 0.013  
 ASSUME FULL-FLOWING PIPELINE  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 7.08  
 PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)  
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 22.25  
 PIPE TRAVEL TIME (MIN.) = 0.11 Tc (MIN.) = 7.26  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 10.00 = 1437.40 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 10.00 TO NODE 11.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0100  
FLOW LENGTH(FEET) = 43.70 MANNING'S N = 0.013  
DEPTH OF FLOW IN 36.0 INCH PIPE IS 14.7 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.17  
GIVEN PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 22.25  
PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 7.35  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 11.00 = 1481.10 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 11.00 TO NODE 12.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0100  
FLOW LENGTH(FEET) = 93.90 MANNING'S N = 0.013  
DEPTH OF FLOW IN 36.0 INCH PIPE IS 14.7 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.17  
GIVEN PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 22.25  
PIPE TRAVEL TIME(MIN.) = 0.19 Tc(MIN.) = 7.54  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 12.00 = 1575.00 FEET.

=====

END OF RATIONAL METHOD ANALYSIS

```

*****
FLOW PROCESS FROM NODE      21.00 TO NODE      22.00 IS CODE =  21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) =      2.35
ELEVATION DATA: UPSTREAM(FEET) =      154.00  DOWNSTREAM(FEET) =      149.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =      5.000
* 10 YEAR RAINFALL INTENSITY(INCH/HR) =      4.060
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp          Ap      SCS  Tc
LAND USE              GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
NATURAL POOR COVER
"BARREN"                B         0.63      0.30      1.000      86   5.00
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  1.000
SUBAREA RUNOFF(CFS) =      2.13
TOTAL AREA(ACRES) =      0.63  PEAK FLOW RATE(CFS) =      2.13

*****
FLOW PROCESS FROM NODE      22.00 TO NODE      23.00 IS CODE =  52
-----
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =      149.00  DOWNSTREAM(FEET) =      139.90
CHANNEL LENGTH THRU SUBAREA(FEET) =      858.00  CHANNEL SLOPE =  0.0106
CHANNEL FLOW THRU SUBAREA(CFS) =      2.13
FLOW VELOCITY(FEET/SEC) =      1.80 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =      7.96  Tc(MIN.) =      12.96
LONGEST FLOWPATH FROM NODE      21.00 TO NODE      23.00 =      860.35 FEET.

*****
FLOW PROCESS FROM NODE      23.00 TO NODE      23.00 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) =      12.96
* 10 YEAR RAINFALL INTENSITY(INCH/HR) =      2.352
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp          Ap      SCS
LAND USE              GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
NATURAL POOR COVER
"BARREN"                B         4.59      0.30      1.000      86
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  1.000
SUBAREA AREA(ACRES) =      4.59  SUBAREA RUNOFF(CFS) =      8.48
EFFECTIVE AREA(ACRES) =      5.22  AREA-AVERAGED Fm(INCH/HR) =  0.30
AREA-AVERAGED Fp(INCH/HR) =  0.30  AREA-AVERAGED Ap =  1.00
TOTAL AREA(ACRES) =      5.2  PEAK FLOW RATE(CFS) =      9.64

*****
FLOW PROCESS FROM NODE      23.00 TO NODE      23.00 IS CODE =  1
-----

```



>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 12.96  
RAINFALL INTENSITY(INCH/HR) = 2.35  
AREA-AVERAGED Fm(INCH/HR) = 0.30  
AREA-AVERAGED Fp(INCH/HR) = 0.30  
AREA-AVERAGED Ap = 1.00  
EFFECTIVE STREAM AREA(ACRES) = 5.22  
TOTAL STREAM AREA(ACRES) = 5.22  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 9.64

\*\*\*\*\*  
FLOW PROCESS FROM NODE 23.10 TO NODE 23.20 IS CODE = 21  
-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 285.00  
ELEVATION DATA: UPSTREAM(FEET) = 150.80 DOWNSTREAM(FEET) = 147.80

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$   
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.521  
\* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.399

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
-------------------------------	-------------------	-----------------	-----------------	-----------------	-----------	--------------

NATURAL POOR COVER						
"BARREN"	B	0.84	0.30	1.000	86	12.52

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000

SUBAREA RUNOFF(CFS) = 1.59

TOTAL AREA(ACRES) = 0.84 PEAK FLOW RATE(CFS) = 1.59

\*\*\*\*\*  
FLOW PROCESS FROM NODE 23.20 TO NODE 23.00 IS CODE = 52  
-----

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 147.80 DOWNSTREAM(FEET) = 139.90  
CHANNEL LENGTH THRU SUBAREA(FEET) = 791.00 CHANNEL SLOPE = 0.0100  
CHANNEL FLOW THRU SUBAREA(CFS) = 1.59  
FLOW VELOCITY(FEET/SEC) = 1.64 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)  
TRAVEL TIME(MIN.) = 8.04 Tc(MIN.) = 20.56  
LONGEST FLOWPATH FROM NODE 23.10 TO NODE 23.00 = 1076.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81  
-----

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 20.56  
\* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 1.806

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/	SCS SOIL	AREA	Fp	Ap	SCS
-------------------	----------	------	----	----	-----

LAND USE                      GROUP      (ACRES)      (INCH/HR)      (DECIMAL)      CN  
 NATURAL POOR COVER  
 "BARREN"                      B              1.81              0.30              1.000              86  
 SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$ (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 1.000  
 SUBAREA AREA (ACRES) = 1.81              SUBAREA RUNOFF (CFS) = 2.45  
 EFFECTIVE AREA (ACRES) = 2.65              AREA-AVERAGED  $F_m$ (INCH/HR) = 0.30  
 AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30              AREA-AVERAGED  $A_p$  = 1.00  
 TOTAL AREA (ACRES) = 2.6              PEAK FLOW RATE (CFS) = 3.59

\*\*\*\*\*

FLOW PROCESS FROM NODE      23.00 TO NODE      23.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
 TIME OF CONCENTRATION (MIN.) = 20.56  
 RAINFALL INTENSITY (INCH/HR) = 1.81  
 AREA-AVERAGED  $F_m$ (INCH/HR) = 0.30  
 AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30  
 AREA-AVERAGED  $A_p$  = 1.00  
 EFFECTIVE STREAM AREA (ACRES) = 2.65  
 TOTAL STREAM AREA (ACRES) = 2.65  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 3.59

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	9.64	12.96	2.352	0.30 ( 0.30)	1.00	5.2	21.00
2	3.59	20.56	1.806	0.30 ( 0.30)	1.00	2.6	23.10

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	12.73	12.96	2.352	0.30 ( 0.30)	1.00	6.9	21.00
2	10.66	20.56	1.806	0.30 ( 0.30)	1.00	7.9	23.10

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 12.73              Tc (MIN.) = 12.96  
 EFFECTIVE AREA (ACRES) = 6.89              AREA-AVERAGED  $F_m$ (INCH/HR) = 0.30  
 AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30              AREA-AVERAGED  $A_p$  = 1.00  
 TOTAL AREA (ACRES) = 7.9  
 LONGEST FLOWPATH FROM NODE      23.10 TO NODE      23.00 = 1076.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE      23.00 TO NODE      24.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0100  
 FLOW LENGTH (FEET) = 45.70              MANNING'S N = 0.013

ASSUME FULL-FLOWING PIPELINE  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 7.20  
 PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)  
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 12.73  
 PIPE TRAVEL TIME (MIN.) = 0.11 Tc (MIN.) = 13.07  
 LONGEST FLOWPATH FROM NODE 23.10 TO NODE 24.00 = 1121.70 FEET.

=====

END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 7.9 TC (MIN.) = 13.07  
 EFFECTIVE AREA (ACRES) = 6.89 AREA-AVERAGED Fm (INCH/HR) = 0.30  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 1.000  
 PEAK FLOW RATE (CFS) = 12.73

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	12.73	13.07	2.341	0.30 ( 0.30)	1.00	6.9	21.00
2	10.66	20.69	1.799	0.30 ( 0.30)	1.00	7.9	23.10

=====

END OF RATIONAL METHOD ANALYSIS

**Appendix G**  
Existing  $Q_{25}$  Hydrology Calculations

\*\*\*\*\*

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE  
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)  
(c) Copyright 1983-2008 Advanced Engineering Software (aes)  
Ver. 15.0 Release Date: 04/01/2008 License ID 1204

Analysis prepared by:

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\*\*\*\*\* DESCRIPTION OF STUDY \*\*\*\*\*  
\* 25-YEAR STORM EVENT HYDROLOGY ANALYSIS \*  
\* OAKCREEK COMMUNITY PARK, IRVINE CALIFORNIA \*  
\* EXISTING CONDITION \*  
\*\*\*\*\*

FILE NAME: OAK25EXA.DAT  
TIME/DATE OF STUDY: 16:19 04/04/2022

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--\*TIME-OF-CONCENTRATION MODEL\*--

USER SPECIFIED STORM EVENT (YEAR) = 25.00  
SPECIFIED MINIMUM PIPE SIZE (INCH) = 8.00  
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.90  
\*DATA BANK RAINFALL USED\*  
\*ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD\*

\*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL\*

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / PARK- SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH (FT)	LIP (FT)	HIKE (FT)	MANNING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.50	2.00	0.0313	0.125	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:  
1. Relative Flow-Depth = 0.00 FEET  
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)  
2. (Depth)\*(Velocity) Constraint = 6.0 (FT\*FT/S)  
\*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN  
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.\*  
\*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

\*\*\*\*\*

FLOW PROCESS FROM NODE 1.00 TO NODE 2.00 IS CODE = 21

-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 251.00  
ELEVATION DATA: UPSTREAM (FEET) = 157.60 DOWNSTREAM (FEET) = 151.80

$T_c = K * [(LENGTH^{**} 3.00) / (ELEVATION CHANGE)]^{**} 0.20$

SUBAREA ANALYSIS USED MINIMUM  $T_c$  (MIN.) = 9.355

\* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.384

SUBAREA  $T_c$  AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN	$T_c$ (MIN.)
PUBLIC PARK	B	0.07	0.30	0.850	56	9.36

SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$  (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.850

SUBAREA RUNOFF (CFS) = 0.20

TOTAL AREA (ACRES) = 0.07 PEAK FLOW RATE (CFS) = 0.20

\*\*\*\*\*

FLOW PROCESS FROM NODE 2.00 TO NODE 3.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0150

FLOW LENGTH (FEET) = 49.40 MANNING'S N = 0.010

DEPTH OF FLOW IN 6.0 INCH PIPE IS 2.0 INCHES

PIPE-FLOW VELOCITY (FEET/SEC.) = 3.51

GIVEN PIPE DIAMETER (INCH) = 6.00 NUMBER OF PIPES = 1

PIPE-FLOW (CFS) = 0.20

PIPE TRAVEL TIME (MIN.) = 0.23  $T_c$  (MIN.) = 9.59

LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 300.40 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 3.00 TO NODE 3.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE  $T_c$  (MIN.) = 9.59

\* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.336

SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN
PUBLIC PARK	B	0.24	0.30	0.850	56

SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$  (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.850

SUBAREA AREA (ACRES) = 0.24 SUBAREA RUNOFF (CFS) = 0.67

EFFECTIVE AREA (ACRES) = 0.31 AREA-AVERAGED  $F_m$  (INCH/HR) = 0.26

AREA-AVERAGED  $F_p$  (INCH/HR) = 0.30 AREA-AVERAGED  $A_p$  = 0.85

TOTAL AREA (ACRES) = 0.3 PEAK FLOW RATE (CFS) = 0.86

\*\*\*\*\*

FLOW PROCESS FROM NODE 3.00 TO NODE 4.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0150

FLOW LENGTH (FEET) = 363.00 MANNING'S N = 0.010

DEPTH OF FLOW IN 8.0 INCH PIPE IS 3.9 INCHES

PIPE-FLOW VELOCITY (FEET/SEC.) = 5.13

GIVEN PIPE DIAMETER (INCH) = 8.00 NUMBER OF PIPES = 1

PIPE-FLOW (CFS) = 0.86

PIPE TRAVEL TIME (MIN.) = 1.18 Tc (MIN.) = 10.77  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4.00 = 663.40 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 4.00 TO NODE 4.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 10.77  
\* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.125  
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	0.46	0.30	0.850	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
SUBAREA AREA (ACRES) = 0.46 SUBAREA RUNOFF (CFS) = 1.19  
EFFECTIVE AREA (ACRES) = 0.77 AREA-AVERAGED Fm (INCH/HR) = 0.26  
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85  
TOTAL AREA (ACRES) = 0.8 PEAK FLOW RATE (CFS) = 1.99

\*\*\*\*\*  
FLOW PROCESS FROM NODE 4.00 TO NODE 5.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0150  
FLOW LENGTH (FEET) = 97.00 MANNING'S N = 0.010  
ASSUME FULL-FLOWING PIPELINE  
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.70  
PIPE FLOW VELOCITY = (TOTAL FLOW) / (PIPE CROSS SECTION AREA)  
GIVEN PIPE DIAMETER (INCH) = 8.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 1.99  
PIPE TRAVEL TIME (MIN.) = 0.28 Tc (MIN.) = 11.05  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 5.00 = 760.40 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 5.00 TO NODE 5.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 11.05  
\* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.079  
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	0.11	0.30	0.850	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
SUBAREA AREA (ACRES) = 0.11 SUBAREA RUNOFF (CFS) = 0.28  
EFFECTIVE AREA (ACRES) = 0.88 AREA-AVERAGED Fm (INCH/HR) = 0.26  
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85  
TOTAL AREA (ACRES) = 0.9 PEAK FLOW RATE (CFS) = 2.24

\*\*\*\*\*  
FLOW PROCESS FROM NODE 5.00 TO NODE 6.00 IS CODE = 41

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-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====
REPRESENTATIVE SLOPE = 0.0150
FLOW LENGTH(FEET) = 135.00 MANNING'S N = 0.010
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.41
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER(INCH) = 8.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.24
PIPE TRAVEL TIME(MIN.) = 0.35 Tc(MIN.) = 11.40
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 6.00 = 895.40 FEET.

*****
FLOW PROCESS FROM NODE 6.00 TO NODE 6.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) = 11.40
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.025
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
PUBLIC PARK B 0.77 0.30 0.850 56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850
SUBAREA AREA(ACRES) = 0.77 SUBAREA RUNOFF(CFS) = 1.92
EFFECTIVE AREA(ACRES) = 1.65 AREA-AVERAGED Fm(INCH/HR) = 0.26
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85
TOTAL AREA(ACRES) = 1.6 PEAK FLOW RATE(CFS) = 4.11

*****
FLOW PROCESS FROM NODE 6.00 TO NODE 7.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====
REPRESENTATIVE SLOPE = 0.0150
FLOW LENGTH(FEET) = 5.00 MANNING'S N = 0.010
DEPTH OF FLOW IN 12.0 INCH PIPE IS 7.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.54
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 4.11
PIPE TRAVEL TIME(MIN.) = 0.01 Tc(MIN.) = 11.41
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 7.00 = 900.40 FEET.

*****
FLOW PROCESS FROM NODE 7.00 TO NODE 7.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) = 11.41
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.023
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN

```



```
PUBLIC PARK                B           3.91       0.30       0.850      56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850
SUBAREA AREA(ACRES) = 3.91       SUBAREA RUNOFF(CFS) = 9.74
EFFECTIVE AREA(ACRES) = 5.56     AREA-AVERAGED Fm(INCH/HR) = 0.26
AREA-AVERAGED Fp(INCH/HR) = 0.30  AREA-AVERAGED Ap = 0.85
TOTAL AREA(ACRES) = 5.6         PEAK FLOW RATE(CFS) = 13.85
```

```
*****
FLOW PROCESS FROM NODE 7.00 TO NODE 8.00 IS CODE = 41
-----
```

```
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
```

```
=====
REPRESENTATIVE SLOPE = 0.0111
FLOW LENGTH(FEET) = 231.00  MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 13.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.56
GIVEN PIPE DIAMETER(INCH) = 24.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 13.85
PIPE TRAVEL TIME(MIN.) = 0.51  Tc(MIN.) = 11.92
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 8.00 = 1131.40 FEET.
```

```
*****
FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1
-----
```

```
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
```

```
=====
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.92
RAINFALL INTENSITY(INCH/HR) = 2.95
AREA-AVERAGED Fm(INCH/HR) = 0.26
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.85
EFFECTIVE STREAM AREA(ACRES) = 5.56
TOTAL STREAM AREA(ACRES) = 5.56
PEAK FLOW RATE(CFS) AT CONFLUENCE = 13.85
```

```
*****
FLOW PROCESS FROM NODE 8.10 TO NODE 8.20 IS CODE = 21
-----
```

```
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
```

```
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 300.00
ELEVATION DATA: UPSTREAM(FEET) = 150.00  DOWNSTREAM(FEET) = 144.55
```

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

```
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 10.542
```

```
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.162
```

```
SUBAREA Tc AND LOSS RATE DATA(AMC II):
```

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
PUBLIC PARK	B	0.76	0.30	0.850	56	10.54
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30						
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850						

```

SUBAREA RUNOFF (CFS) =      1.99
TOTAL AREA (ACRES) =      0.76   PEAK FLOW RATE (CFS) =      1.99

*****
FLOW PROCESS FROM NODE      8.20 TO NODE      8.30 IS CODE =  41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE =  0.0150
FLOW LENGTH (FEET) =  167.00   MANNING'S N =  0.010
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY (FEET/SEC.) =  5.70
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER (INCH) =  8.00   NUMBER OF PIPES =  1
PIPE-FLOW (CFS) =      1.99
PIPE TRAVEL TIME (MIN.) =  0.49   Tc (MIN.) =  11.03
LONGEST FLOWPATH FROM NODE      8.10 TO NODE      8.30 =      467.00 FEET.

*****
FLOW PROCESS FROM NODE      8.30 TO NODE      8.30 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc (MIN.) =  11.03
* 25 YEAR RAINFALL INTENSITY (INCH/HR) =  3.082
SUBAREA LOSS RATE DATA (AMC II):
DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp        Ap        SCS
LAND USE              GROUP   (ACRES)  (INCH/HR) (DECIMAL) CN
COMMERCIAL            B      0.60     0.30     0.100    56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) =  0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.100
SUBAREA AREA (ACRES) =  0.60     SUBAREA RUNOFF (CFS) =  1.65
EFFECTIVE AREA (ACRES) =  1.36   AREA-AVERAGED Fm (INCH/HR) =  0.16
AREA-AVERAGED Fp (INCH/HR) =  0.30   AREA-AVERAGED Ap =  0.52
TOTAL AREA (ACRES) =  1.4     PEAK FLOW RATE (CFS) =  3.58

*****
FLOW PROCESS FROM NODE      8.30 TO NODE      8.00 IS CODE =  41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE =  0.0077
FLOW LENGTH (FEET) =  37.00   MANNING'S N =  0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS  8.0 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) =  4.71
GIVEN PIPE DIAMETER (INCH) = 18.00   NUMBER OF PIPES =  1
PIPE-FLOW (CFS) =      3.58
PIPE TRAVEL TIME (MIN.) =  0.13   Tc (MIN.) =  11.16
LONGEST FLOWPATH FROM NODE      8.10 TO NODE      8.00 =      504.00 FEET.

*****
FLOW PROCESS FROM NODE      8.00 TO NODE      8.00 IS CODE =  1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====

```

TOTAL NUMBER OF STREAMS = 3  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
 TIME OF CONCENTRATION(MIN.) = 11.16  
 RAINFALL INTENSITY(INCH/HR) = 3.06  
 AREA-AVERAGED Fm(INCH/HR) = 0.16  
 AREA-AVERAGED Fp(INCH/HR) = 0.30  
 AREA-AVERAGED Ap = 0.52  
 EFFECTIVE STREAM AREA(ACRES) = 1.36  
 TOTAL STREAM AREA(ACRES) = 1.36  
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 3.58

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.40 TO NODE 8.50 IS CODE = 21  
 -----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 252.00  
 ELEVATION DATA: UPSTREAM(FEET) = 147.00 DOWNSTREAM(FEET) = 142.68

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$   
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 6.261  
 \* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 4.247

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.44	0.30	0.100	56	6.26

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100  
 SUBAREA RUNOFF(CFS) = 1.67  
 TOTAL AREA(ACRES) = 0.44 PEAK FLOW RATE(CFS) = 1.67

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.50 TO NODE 8.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0557  
 FLOW LENGTH(FEET) = 37.70 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.3 INCHES  
 PIPE-FLOW VELOCITY(FEET/SEC.) = 7.68  
 GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
 PIPE-FLOW(CFS) = 1.67  
 PIPE TRAVEL TIME(MIN.) = 0.08 Tc(MIN.) = 6.34  
 LONGEST FLOWPATH FROM NODE 8.40 TO NODE 8.00 = 289.70 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:  
 TIME OF CONCENTRATION(MIN.) = 6.34  
 RAINFALL INTENSITY(INCH/HR) = 4.22

AREA-AVERAGED Fm (INCH/HR) = 0.03  
 AREA-AVERAGED Fp (INCH/HR) = 0.30  
 AREA-AVERAGED Ap = 0.10  
 EFFECTIVE STREAM AREA (ACRES) = 0.44  
 TOTAL STREAM AREA (ACRES) = 0.44  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.67

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	13.85	11.92	2.949	0.30 ( 0.26)	0.85	5.6	1.00
2	3.58	11.16	3.062	0.30 ( 0.16)	0.52	1.4	8.10
3	1.67	6.34	4.216	0.30 ( 0.03)	0.10	0.4	8.40

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 3 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	15.35	6.34	4.216	0.30 ( 0.21)	0.71	4.2	8.40
2	18.30	11.16	3.062	0.30 ( 0.22)	0.74	7.0	8.10
3	18.46	11.92	2.949	0.30 ( 0.22)	0.74	7.4	1.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 18.46 Tc (MIN.) = 11.92  
 EFFECTIVE AREA (ACRES) = 7.36 AREA-AVERAGED Fm (INCH/HR) = 0.22  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.74  
 TOTAL AREA (ACRES) = 7.4  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 8.00 = 1131.40 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.00 TO NODE 9.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0050  
 FLOW LENGTH (FEET) = 261.00 MANNING'S N = 0.013  
 ASSUME FULL-FLOWING PIPELINE  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 5.88  
 PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)  
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 18.46  
 PIPE TRAVEL TIME (MIN.) = 0.74 Tc (MIN.) = 12.66  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 9.00 = 1392.40 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 9.00 TO NODE 9.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc (MIN.) = 12.66  
 \* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 2.851  
 SUBAREA LOSS RATE DATA (AMC II):  
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN

COMMERCIAL B 0.52 0.30 0.100 56  
 PUBLIC PARK B 3.04 0.30 0.850 56  
 SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$ (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.740  
 SUBAREA AREA(ACRES) = 3.56 SUBAREA RUNOFF(CFS) = 8.42  
 EFFECTIVE AREA(ACRES) = 10.92 AREA-AVERAGED  $F_m$ (INCH/HR) = 0.22  
 AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30 AREA-AVERAGED  $A_p$  = 0.74  
 TOTAL AREA(ACRES) = 10.9 PEAK FLOW RATE(CFS) = 25.83

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	$F_p$ ( $F_m$ ) (INCH/HR)	$A_p$	$A_e$ (ACRES)	HEADWATER NODE
1	25.72	7.23	3.914	0.30( 0.22)	0.72	7.7	8.40
2	25.96	11.91	2.952	0.30( 0.22)	0.74	10.6	8.10
3	25.83	12.66	2.851	0.30( 0.22)	0.74	10.9	1.00

NEW PEAK FLOW DATA ARE:

PEAK FLOW RATE(CFS) = 25.96 Tc(MIN.) = 11.91  
 AREA-AVERAGED  $F_m$ (INCH/HR) = 0.22 AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30  
 AREA-AVERAGED  $A_p$  = 0.74 EFFECTIVE AREA(ACRES) = 10.56

\*\*\*\*\*

FLOW PROCESS FROM NODE 9.00 TO NODE 9.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 11.91

\* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.952

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN
PUBLIC PARK	B	0.31	0.30	0.850	56

SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$ (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.850  
 SUBAREA AREA(ACRES) = 0.31 SUBAREA RUNOFF(CFS) = 0.75  
 EFFECTIVE AREA(ACRES) = 10.87 AREA-AVERAGED  $F_m$ (INCH/HR) = 0.22  
 AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30 AREA-AVERAGED  $A_p$  = 0.74  
 TOTAL AREA(ACRES) = 11.2 PEAK FLOW RATE(CFS) = 26.71

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	$F_p$ ( $F_m$ ) (INCH/HR)	$A_p$	$A_e$ (ACRES)	HEADWATER NODE
1	26.74	7.23	3.914	0.30( 0.22)	0.73	8.0	8.40
2	26.71	11.91	2.952	0.30( 0.22)	0.74	10.9	8.10
3	26.55	12.66	2.851	0.30( 0.22)	0.75	11.2	1.00

NEW PEAK FLOW DATA ARE:

PEAK FLOW RATE(CFS) = 26.74 Tc(MIN.) = 7.23  
 AREA-AVERAGED  $F_m$ (INCH/HR) = 0.22 AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30  
 AREA-AVERAGED  $A_p$  = 0.73 EFFECTIVE AREA(ACRES) = 8.04

\*\*\*\*\*

FLOW PROCESS FROM NODE 9.00 TO NODE 10.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0050

FLOW LENGTH(FEET) = 45.00 MANNING'S N = 0.013

```

ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY (FEET/SEC.) = 8.51
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 26.74
PIPE TRAVEL TIME (MIN.) = 0.09 Tc (MIN.) = 7.32
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 10.00 = 1437.40 FEET.

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*****
FLOW PROCESS FROM NODE 10.00 TO NODE 11.00 IS CODE = 41
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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====

```

```

REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH (FEET) = 43.70 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 16.3 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 8.58
GIVEN PIPE DIAMETER (INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 26.74
PIPE TRAVEL TIME (MIN.) = 0.08 Tc (MIN.) = 7.41
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 11.00 = 1481.10 FEET.

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*****
FLOW PROCESS FROM NODE 11.00 TO NODE 12.00 IS CODE = 41
-----

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```

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====

```

```

REPRESENTATIVE SLOPE = 0.0100
FLOW LENGTH (FEET) = 93.90 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 16.3 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 8.58
GIVEN PIPE DIAMETER (INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 26.74
PIPE TRAVEL TIME (MIN.) = 0.18 Tc (MIN.) = 7.59
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 12.00 = 1575.00 FEET.
=====

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END OF RATIONAL METHOD ANALYSIS

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*****
FLOW PROCESS FROM NODE      21.00 TO NODE      22.00 IS CODE =  21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) =      2.35
ELEVATION DATA: UPSTREAM(FEET) =      154.00  DOWNSTREAM(FEET) =      149.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =      5.000
* 25 YEAR RAINFALL INTENSITY(INCH/HR) =      4.824
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp          Ap      SCS   Tc
LAND USE              GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN   (MIN.)
NATURAL POOR COVER
"BARREN"              B        0.63     0.30     1.000     86   5.00
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  1.000
SUBAREA RUNOFF(CFS) =      2.56
TOTAL AREA(ACRES) =      0.63  PEAK FLOW RATE(CFS) =      2.56

*****
FLOW PROCESS FROM NODE      22.00 TO NODE      23.00 IS CODE =  52
-----
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =      149.00  DOWNSTREAM(FEET) =      139.90
CHANNEL LENGTH THRU SUBAREA(FEET) =      858.00  CHANNEL SLOPE =  0.0106
CHANNEL FLOW THRU SUBAREA(CFS) =      2.56
FLOW VELOCITY(FEET/SEC) =      1.87 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =      7.65  Tc(MIN.) =      12.65
LONGEST FLOWPATH FROM NODE      21.00 TO NODE      23.00 =      860.35 FEET.

*****
FLOW PROCESS FROM NODE      23.00 TO NODE      23.00 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) =      12.65
* 25 YEAR RAINFALL INTENSITY(INCH/HR) =      2.852
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp          Ap      SCS
LAND USE              GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
NATURAL POOR COVER
"BARREN"              B        4.59     0.30     1.000     86
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  1.000
SUBAREA AREA(ACRES) =      4.59  SUBAREA RUNOFF(CFS) =      10.54
EFFECTIVE AREA(ACRES) =      5.22  AREA-AVERAGED Fm(INCH/HR) =  0.30
AREA-AVERAGED Fp(INCH/HR) =  0.30  AREA-AVERAGED Ap =  1.00
TOTAL AREA(ACRES) =      5.2  PEAK FLOW RATE(CFS) =      11.99

*****
FLOW PROCESS FROM NODE      23.00 TO NODE      23.00 IS CODE =  1
-----

```

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 12.65
RAINFALL INTENSITY(INCH/HR) = 2.85
AREA-AVERAGED Fm(INCH/HR) = 0.30
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 1.00
EFFECTIVE STREAM AREA(ACRES) = 5.22
TOTAL STREAM AREA(ACRES) = 5.22
PEAK FLOW RATE(CFS) AT CONFLUENCE = 11.99

\*\*\*\*\*
FLOW PROCESS FROM NODE 23.10 TO NODE 23.20 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 285.00
ELEVATION DATA: UPSTREAM(FEET) = 150.80 DOWNSTREAM(FEET) = 147.80

Tc = K\*[(LENGTH\*\* 3.00)/(ELEVATION CHANGE)]\*\*0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.521
\* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.869

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
NATURAL POOR COVER

"BARREN" B 0.84 0.30 1.000 86 12.52

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000

SUBAREA RUNOFF(CFS) = 1.94

TOTAL AREA(ACRES) = 0.84 PEAK FLOW RATE(CFS) = 1.94

\*\*\*\*\*
FLOW PROCESS FROM NODE 23.20 TO NODE 23.00 IS CODE = 52

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 147.80 DOWNSTREAM(FEET) = 139.90
CHANNEL LENGTH THRU SUBAREA(FEET) = 791.00 CHANNEL SLOPE = 0.0100
CHANNEL FLOW THRU SUBAREA(CFS) = 1.94
FLOW VELOCITY(FEET/SEC) = 1.71 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 7.71 Tc(MIN.) = 20.23
LONGEST FLOWPATH FROM NODE 23.10 TO NODE 23.00 = 1076.00 FEET.

\*\*\*\*\*
FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 20.23
\* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.187

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS



LAND USE                      GROUP      (ACRES)      (INCH/HR)      (DECIMAL)      CN  
 NATURAL POOR COVER  
 "BARREN"                      B              1.81              0.30              1.000              86  
 SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$ (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 1.000  
 SUBAREA AREA (ACRES) = 1.81              SUBAREA RUNOFF (CFS) = 3.07  
 EFFECTIVE AREA (ACRES) = 2.65              AREA-AVERAGED  $F_m$ (INCH/HR) = 0.30  
 AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30              AREA-AVERAGED  $A_p$  = 1.00  
 TOTAL AREA (ACRES) = 2.6              PEAK FLOW RATE (CFS) = 4.50

\*\*\*\*\*

FLOW PROCESS FROM NODE      23.00 TO NODE      23.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
 TIME OF CONCENTRATION (MIN.) = 20.23  
 RAINFALL INTENSITY (INCH/HR) = 2.19  
 AREA-AVERAGED  $F_m$ (INCH/HR) = 0.30  
 AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30  
 AREA-AVERAGED  $A_p$  = 1.00  
 EFFECTIVE STREAM AREA (ACRES) = 2.65  
 TOTAL STREAM AREA (ACRES) = 2.65  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 4.50

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	11.99	12.65	2.852	0.30 ( 0.30)	1.00	5.2	21.00
2	4.50	20.23	2.187	0.30 ( 0.30)	1.00	2.6	23.10

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	15.80	12.65	2.852	0.30 ( 0.30)	1.00	6.9	21.00
2	13.36	20.23	2.187	0.30 ( 0.30)	1.00	7.9	23.10

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 15.80              Tc (MIN.) = 12.65  
 EFFECTIVE AREA (ACRES) = 6.88              AREA-AVERAGED  $F_m$ (INCH/HR) = 0.30  
 AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30              AREA-AVERAGED  $A_p$  = 1.00  
 TOTAL AREA (ACRES) = 7.9  
 LONGEST FLOWPATH FROM NODE      23.10 TO NODE      23.00 = 1076.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE      23.00 TO NODE      24.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0100  
 FLOW LENGTH (FEET) = 45.70              MANNING'S N = 0.013

ASSUME FULL-FLOWING PIPELINE  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 8.94  
 PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)  
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 15.80  
 PIPE TRAVEL TIME (MIN.) = 0.09 Tc (MIN.) = 12.74  
 LONGEST FLOWPATH FROM NODE 23.10 TO NODE 24.00 = 1121.70 FEET.

=====

END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 7.9 TC (MIN.) = 12.74  
 EFFECTIVE AREA (ACRES) = 6.88 AREA-AVERAGED Fm (INCH/HR) = 0.30  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 1.000  
 PEAK FLOW RATE (CFS) = 15.80

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	15.80	12.74	2.841	0.30 ( 0.30)	1.00	6.9	21.00
2	13.36	20.33	2.180	0.30 ( 0.30)	1.00	7.9	23.10

=====

END OF RATIONAL METHOD ANALYSIS

**Appendix H**  
Proposed Q<sub>2</sub> Hydrology Calculations

\*\*\*\*\*

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE  
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)  
(c) Copyright 1983-2008 Advanced Engineering Software (aes)  
Ver. 15.0 Release Date: 04/01/2008 License ID 1204

Analysis prepared by:

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\*\*\*\*\* DESCRIPTION OF STUDY \*\*\*\*\*  
\* 2-YEAR STORM EVENT HYDROLOGY ANYLYSIS \*  
\* OAKCREEK COMMUNITY PARK, IRVINE CALIFORNIA \*  
\* PROPOSED CONDITION \*  
\*\*\*\*\*

FILE NAME: OAK02PRA.DAT  
TIME/DATE OF STUDY: 16:01 04/04/2022

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--\*TIME-OF-CONCENTRATION MODEL\*--

USER SPECIFIED STORM EVENT (YEAR) = 2.00  
SPECIFIED MINIMUM PIPE SIZE (INCH) = 8.00  
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.90  
\*DATA BANK RAINFALL USED\*  
\*ANTECEDENT MOISTURE CONDITION (AMC) I ASSUMED FOR RATIONAL METHOD\*

\*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL\*

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / PARK- SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH (FT)	LIP (FT)	HIKE (FT)	MANNING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.50	2.00	0.0313	0.125	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:  
1. Relative Flow-Depth = 0.00 FEET  
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)  
2. (Depth)\*(Velocity) Constraint = 6.0 (FT\*FT/S)  
\*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN  
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.\*  
\*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

\*\*\*\*\*

FLOW PROCESS FROM NODE 1.00 TO NODE 2.00 IS CODE = 21

-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 251.00  
ELEVATION DATA: UPSTREAM (FEET) = 157.00 DOWNSTREAM (FEET) = 151.80

$T_c = K * [(LENGTH^{**} 3.00) / (ELEVATION CHANGE)]^{**} 0.20$   
 SUBAREA ANALYSIS USED MINIMUM  $T_c$  (MIN.) = 9.562  
 \* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.560  
 SUBAREA  $T_c$  AND LOSS RATE DATA (AMC I):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN	$T_c$ (MIN.)
PUBLIC PARK	B	0.07	0.30	0.850	36	9.56

 SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$  (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.850  
 SUBAREA RUNOFF (CFS) = 0.08  
 TOTAL AREA (ACRES) = 0.07 PEAK FLOW RATE (CFS) = 0.08

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 2.00 TO NODE 3.00 IS CODE = 41  
 -----

\*\* WARNING: Computed Flowrate is less than 0.1 cfs,  
 Routing Algorithm is UNAVAILABLE.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 3.00 TO NODE 3.00 IS CODE = 81  
 -----

>>>> ADDITION OF SUBAREA TO MAINLINE PEAK FLOW <<<<<

=====

MAINLINE  $T_c$  (MIN.) = 9.56  
 \* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.560  
 SUBAREA LOSS RATE DATA (AMC I):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN
PUBLIC PARK	B	0.24	0.30	0.850	36

 SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$  (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.850  
 SUBAREA AREA (ACRES) = 0.24 SUBAREA RUNOFF (CFS) = 0.28  
 EFFECTIVE AREA (ACRES) = 0.31 AREA-AVERAGED  $F_m$  (INCH/HR) = 0.26  
 AREA-AVERAGED  $F_p$  (INCH/HR) = 0.30 AREA-AVERAGED  $A_p$  = 0.85  
 TOTAL AREA (ACRES) = 0.3 PEAK FLOW RATE (CFS) = 0.36

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 3.00 TO NODE 4.00 IS CODE = 41  
 -----

>>>> COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA <<<<<  
 >>>> USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT) <<<<<

=====

REPRESENTATIVE SLOPE = 0.0150  
 FLOW LENGTH (FEET) = 363.00 MANNING'S N = 0.010  
 DEPTH OF FLOW IN 8.0 INCH PIPE IS 2.4 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 4.06  
 GIVEN PIPE DIAMETER (INCH) = 8.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 0.36  
 PIPE TRAVEL TIME (MIN.) = 1.49  $T_c$  (MIN.) = 11.05  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4.00 = 614.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 4.00 TO NODE 4.00 IS CODE = 81  
 -----

>>>> ADDITION OF SUBAREA TO MAINLINE PEAK FLOW <<<<<

=====

MAINLINE  $T_c$  (MIN.) = 11.05

\* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.436  
 SUBAREA LOSS RATE DATA (AMC I ):  
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
 PUBLIC PARK B 2.53 0.30 0.850 36  
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
 SUBAREA AREA (ACRES) = 2.53 SUBAREA RUNOFF (CFS) = 2.69  
 EFFECTIVE AREA (ACRES) = 2.84 AREA-AVERAGED Fm (INCH/HR) = 0.26  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85  
 TOTAL AREA (ACRES) = 2.8 PEAK FLOW RATE (CFS) = 3.02

\*\*\*\*\*

FLOW PROCESS FROM NODE 4.00 TO NODE 5.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0150  
 FLOW LENGTH (FEET) = 97.00 MANNING'S N = 0.010  
 ASSUME FULL-FLOWING PIPELINE  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 8.65  
 PIPE FLOW VELOCITY = (TOTAL FLOW) / (PIPE CROSS SECTION AREA)  
 GIVEN PIPE DIAMETER (INCH) = 8.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 3.02  
 PIPE TRAVEL TIME (MIN.) = 0.19 Tc (MIN.) = 11.24  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 5.00 = 711.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 5.00 TO NODE 5.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 11.24  
 \* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.422  
 SUBAREA LOSS RATE DATA (AMC I ):  
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
 PUBLIC PARK B 0.25 0.30 0.850 36  
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
 SUBAREA AREA (ACRES) = 0.25 SUBAREA RUNOFF (CFS) = 0.26  
 EFFECTIVE AREA (ACRES) = 3.09 AREA-AVERAGED Fm (INCH/HR) = 0.26  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85  
 TOTAL AREA (ACRES) = 3.1 PEAK FLOW RATE (CFS) = 3.25

\*\*\*\*\*

FLOW PROCESS FROM NODE 5.00 TO NODE 6.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0150  
 FLOW LENGTH (FEET) = 135.00 MANNING'S N = 0.010  
 ASSUME FULL-FLOWING PIPELINE  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 9.30  
 PIPE FLOW VELOCITY = (TOTAL FLOW) / (PIPE CROSS SECTION AREA)

GIVEN PIPE DIAMETER(INCH) = 8.00 NUMBER OF PIPES = 1  
 PIPE-FLOW(CFS) = 3.25  
 PIPE TRAVEL TIME(MIN.) = 0.24 Tc(MIN.) = 11.48  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 6.00 = 846.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 6.00 TO NODE 6.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 11.48  
 \* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.405  
 SUBAREA LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	0.71	0.30	0.850	36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
 SUBAREA AREA(ACRES) = 0.71 SUBAREA RUNOFF(CFS) = 0.73  
 EFFECTIVE AREA(ACRES) = 3.80 AREA-AVERAGED Fm(INCH/HR) = 0.26  
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85  
 TOTAL AREA(ACRES) = 3.8 PEAK FLOW RATE(CFS) = 3.93

\*\*\*\*\*

FLOW PROCESS FROM NODE 6.00 TO NODE 7.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0150  
 FLOW LENGTH(FEET) = 5.00 MANNING'S N = 0.010  
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 7.6 INCHES  
 PIPE-FLOW VELOCITY(FEET/SEC.) = 7.48  
 GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1  
 PIPE-FLOW(CFS) = 3.93  
 PIPE TRAVEL TIME(MIN.) = 0.01 Tc(MIN.) = 11.49  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 7.00 = 851.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 7.00 TO NODE 7.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 11.49  
 \* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.404  
 SUBAREA LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	2.01	0.30	0.850	36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
 SUBAREA AREA(ACRES) = 2.01 SUBAREA RUNOFF(CFS) = 2.08  
 EFFECTIVE AREA(ACRES) = 5.81 AREA-AVERAGED Fm(INCH/HR) = 0.26  
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85  
 TOTAL AREA(ACRES) = 5.8 PEAK FLOW RATE(CFS) = 6.01

\*\*\*\*\*

FLOW PROCESS FROM NODE 7.00 TO NODE 8.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0111  
FLOW LENGTH (FEET) = 231.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 8.4 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.08  
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 6.01  
PIPE TRAVEL TIME (MIN.) = 0.63 Tc (MIN.) = 12.12  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 8.00 = 1082.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION (MIN.) = 12.12  
RAINFALL INTENSITY (INCH/HR) = 1.36  
AREA-AVERAGED Fm (INCH/HR) = 0.26  
AREA-AVERAGED Fp (INCH/HR) = 0.30  
AREA-AVERAGED Ap = 0.85  
EFFECTIVE STREAM AREA (ACRES) = 5.81  
TOTAL STREAM AREA (ACRES) = 5.81  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 6.01

\*\*\*\*\*

FLOW PROCESS FROM NODE 8.10 TO NODE 8.20 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00  
ELEVATION DATA: UPSTREAM (FEET) = 150.00 DOWNSTREAM (FEET) = 144.55

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 10.542

\* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.475

SUBAREA Tc AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
PUBLIC PARK	B	0.76	0.30	0.850	36	10.54

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850

SUBAREA RUNOFF (CFS) = 0.83

TOTAL AREA (ACRES) = 0.76 PEAK FLOW RATE (CFS) = 0.83

\*\*\*\*\*

FLOW PROCESS FROM NODE 8.20 TO NODE 8.30 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<



REPRESENTATIVE SLOPE = 0.0150  
 FLOW LENGTH (FEET) = 167.00 MANNING'S N = 0.010  
 DEPTH OF FLOW IN 8.0 INCH PIPE IS 3.8 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 5.10  
 GIVEN PIPE DIAMETER (INCH) = 8.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 0.83  
 PIPE TRAVEL TIME (MIN.) = 0.55 Tc (MIN.) = 11.09  
 LONGEST FLOWPATH FROM NODE 8.10 TO NODE 8.30 = 467.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.30 TO NODE 8.30 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc (MIN.) = 11.09  
 \* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.433  
 SUBAREA LOSS RATE DATA (AMC I):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.51	0.30	0.100	36

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100  
 SUBAREA AREA (ACRES) = 0.51 SUBAREA RUNOFF (CFS) = 0.64  
 EFFECTIVE AREA (ACRES) = 1.27 AREA-AVERAGED Fm (INCH/HR) = 0.16  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.55  
 TOTAL AREA (ACRES) = 1.3 PEAK FLOW RATE (CFS) = 1.45

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.30 TO NODE 8.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0077  
 FLOW LENGTH (FEET) = 37.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 5.0 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 3.66  
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 1.45  
 PIPE TRAVEL TIME (MIN.) = 0.17 Tc (MIN.) = 11.26  
 LONGEST FLOWPATH FROM NODE 8.10 TO NODE 8.00 = 504.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 3  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
 TIME OF CONCENTRATION (MIN.) = 11.26  
 RAINFALL INTENSITY (INCH/HR) = 1.42  
 AREA-AVERAGED Fm (INCH/HR) = 0.16  
 AREA-AVERAGED Fp (INCH/HR) = 0.30  
 AREA-AVERAGED Ap = 0.55  
 EFFECTIVE STREAM AREA (ACRES) = 1.27  
 TOTAL STREAM AREA (ACRES) = 1.27  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.45

\*\*\*\*\*  
FLOW PROCESS FROM NODE 8.40 TO NODE 8.50 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 255.00  
ELEVATION DATA: UPSTREAM (FEET) = 147.00 DOWNSTREAM (FEET) = 142.68

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM  $T_c$  (MIN.) = 6.305

\* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.982

SUBAREA  $T_c$  AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN	$T_c$ (MIN.)
COMMERCIAL	B	0.45	0.30	0.100	36	6.31

SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$  (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.100

SUBAREA RUNOFF (CFS) = 0.79

TOTAL AREA (ACRES) = 0.45 PEAK FLOW RATE (CFS) = 0.79

\*\*\*\*\*  
FLOW PROCESS FROM NODE 8.50 TO NODE 8.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0557  
FLOW LENGTH (FEET) = 37.70 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 2.3 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.14  
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 0.79  
PIPE TRAVEL TIME (MIN.) = 0.10  $T_c$  (MIN.) = 6.41  
LONGEST FLOWPATH FROM NODE 8.40 TO NODE 8.00 = 292.70 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:  
TIME OF CONCENTRATION (MIN.) = 6.41  
RAINFALL INTENSITY (INCH/HR) = 1.96  
AREA-AVERAGED  $F_m$  (INCH/HR) = 0.03  
AREA-AVERAGED  $F_p$  (INCH/HR) = 0.30  
AREA-AVERAGED  $A_p$  = 0.10  
EFFECTIVE STREAM AREA (ACRES) = 0.45  
TOTAL STREAM AREA (ACRES) = 0.45  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 0.79

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	$Q$ (CFS)	$T_c$ (MIN.)	Intensity (INCH/HR)	$F_p$ ( $F_m$ ) (INCH/HR)	$A_p$	$A_e$ (ACRES)	HEADWATER NODE
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1	6.01	12.12	1.361	0.30 ( 0.26)	0.85	5.8	1.00
2	1.45	11.26	1.421	0.30 ( 0.16)	0.55	1.3	8.10
3	0.79	6.41	1.963	0.30 ( 0.03)	0.10	0.4	8.40

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 3 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	6.87	6.41	1.963	0.30 ( 0.22)	0.72	4.2	8.40
2	7.90	11.26	1.421	0.30 ( 0.22)	0.75	7.1	8.10
3	7.93	12.12	1.361	0.30 ( 0.23)	0.75	7.5	1.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 7.93 Tc (MIN.) = 12.12  
EFFECTIVE AREA (ACRES) = 7.53 AREA-AVERAGED Fm (INCH/HR) = 0.23  
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.75  
TOTAL AREA (ACRES) = 7.5  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 8.00 = 1082.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 8.00 TO NODE 9.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0050  
FLOW LENGTH (FEET) = 261.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 12.3 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.88  
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 7.93  
PIPE TRAVEL TIME (MIN.) = 0.89 Tc (MIN.) = 13.01  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 9.00 = 1343.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 9.00 TO NODE 9.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc (MIN.) = 13.01  
\* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.307  
SUBAREA LOSS RATE DATA (AMC I):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
COMMERCIAL B 0.50 0.30 0.100 36  
PUBLIC PARK B 3.13 0.30 0.850 36  
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.747  
SUBAREA AREA (ACRES) = 3.63 SUBAREA RUNOFF (CFS) = 3.54  
EFFECTIVE AREA (ACRES) = 11.16 AREA-AVERAGED Fm (INCH/HR) = 0.23  
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.75  
TOTAL AREA (ACRES) = 11.2 PEAK FLOW RATE (CFS) = 10.86

\*\* PEAK FLOW RATE TABLE \*\*

STREAM	Q	Tc	Intensity	Fp (Fm)	Ap	Ae	HEADWATER
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NUMBER	(CFS)	(MIN.)	(INCH/HR)	(INCH/HR)	(ACRES)	NODE
1	11.32	7.33	1.817	0.30 ( 0.22)	0.73	8.40
2	10.98	12.15	1.360	0.30 ( 0.22)	0.75	8.10
3	10.86	13.01	1.307	0.30 ( 0.23)	0.75	1.00

NEW PEAK FLOW DATA ARE:

PEAK FLOW RATE (CFS) = 11.32 Tc (MIN.) = 7.33  
 AREA-AVERAGED Fm (INCH/HR) = 0.22 AREA-AVERAGED Fp (INCH/HR) = 0.30  
 AREA-AVERAGED Ap = 0.73 EFFECTIVE AREA (ACRES) = 7.87

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 9.00 TO NODE 9.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 7.33  
 \* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.817  
 SUBAREA LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	0.33	0.30	0.850	36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
 SUBAREA AREA (ACRES) = 0.33 SUBAREA RUNOFF (CFS) = 0.46  
 EFFECTIVE AREA (ACRES) = 8.20 AREA-AVERAGED Fm (INCH/HR) = 0.22  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.74  
 TOTAL AREA (ACRES) = 11.5 PEAK FLOW RATE (CFS) = 11.79

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 9.00 TO NODE 9.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 7.33  
 \* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.817  
 SUBAREA LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	0.22	0.30	0.850	36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
 SUBAREA AREA (ACRES) = 0.22 SUBAREA RUNOFF (CFS) = 0.31  
 EFFECTIVE AREA (ACRES) = 8.42 AREA-AVERAGED Fm (INCH/HR) = 0.22  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.74  
 TOTAL AREA (ACRES) = 11.7 PEAK FLOW RATE (CFS) = 12.10

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 9.00 TO NODE 10.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0050  
 FLOW LENGTH (FEET) = 45.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 16.2 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 5.36  
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 12.10

PIPE TRAVEL TIME (MIN.) = 0.14      Tc (MIN.) = 7.47  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 10.00 = 1388.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 10.00 TO NODE 11.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0100  
FLOW LENGTH (FEET) = 43.70      MANNING'S N = 0.013  
DEPTH OF FLOW IN 36.0 INCH PIPE IS 10.7 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.90  
GIVEN PIPE DIAMETER (INCH) = 36.00      NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 12.10  
PIPE TRAVEL TIME (MIN.) = 0.11      Tc (MIN.) = 7.58  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 11.00 = 1431.70 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 11.00 TO NODE 12.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0100  
FLOW LENGTH (FEET) = 93.90      MANNING'S N = 0.013  
DEPTH OF FLOW IN 36.0 INCH PIPE IS 10.7 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 6.90  
GIVEN PIPE DIAMETER (INCH) = 36.00      NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 12.10  
PIPE TRAVEL TIME (MIN.) = 0.23      Tc (MIN.) = 7.80  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 12.00 = 1525.60 FEET.

-----  
END OF RATIONAL METHOD ANALYSIS

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FLOW PROCESS FROM NODE 21.00 TO NODE 22.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 314.00
ELEVATION DATA: UPSTREAM(FEET) = 154.00 DOWNSTREAM(FEET) = 148.60

Tc = K\*[(LENGTH\*\* 3.00)/(ELEVATION CHANGE)]\*\*0.20

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.799

\* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.383

SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)

NATURAL POOR COVER

"BARREN" B 0.88 0.30 1.000 72 11.80

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000

SUBAREA RUNOFF(CFS) = 0.86

TOTAL AREA(ACRES) = 0.88 PEAK FLOW RATE(CFS) = 0.86

\*\*\*\*\*

FLOW PROCESS FROM NODE 22.00 TO NODE 22.50 IS CODE = 52

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 148.60 DOWNSTREAM(FEET) = 143.90

CHANNEL LENGTH THRU SUBAREA(FEET) = 472.00 CHANNEL SLOPE = 0.0100

NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION

CHANNEL FLOW THRU SUBAREA(CFS) = 0.86

FLOW VELOCITY(FEET/SEC) = 1.50 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)

TRAVEL TIME(MIN.) = 5.26 Tc(MIN.) = 17.05

LONGEST FLOWPATH FROM NODE 21.00 TO NODE 22.50 = 786.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 22.50 TO NODE 22.50 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 17.05

\* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.119

SUBAREA LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN

PUBLIC PARK B 0.48 0.30 0.850 36

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850

SUBAREA AREA(ACRES) = 0.48 SUBAREA RUNOFF(CFS) = 0.37

EFFECTIVE AREA(ACRES) = 1.36 AREA-AVERAGED Fm(INCH/HR) = 0.28

AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.95

TOTAL AREA(ACRES) = 1.4 PEAK FLOW RATE(CFS) = 1.02

\*\*\*\*\*

FLOW PROCESS FROM NODE 22.50 TO NODE 22.50 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 17.05  
RAINFALL INTENSITY(INCH/HR) = 1.12  
AREA-AVERAGED Fm(INCH/HR) = 0.28  
AREA-AVERAGED Fp(INCH/HR) = 0.30  
AREA-AVERAGED Ap = 0.95  
EFFECTIVE STREAM AREA(ACRES) = 1.36  
TOTAL STREAM AREA(ACRES) = 1.36  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.02

\*\*\*\*\*

FLOW PROCESS FROM NODE 22.51 TO NODE 22.52 IS CODE = 21

-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 283.30  
ELEVATION DATA: UPSTREAM(FEET) = 150.40 DOWNSTREAM(FEET) = 147.40

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$   
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.224  
\* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.833  
SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.71	0.30	0.100	36	7.22

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100  
SUBAREA RUNOFF(CFS) = 1.15  
TOTAL AREA(ACRES) = 0.71 PEAK FLOW RATE(CFS) = 1.15

\*\*\*\*\*

FLOW PROCESS FROM NODE 22.52 TO NODE 22.53 IS CODE = 61

-----

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STANDARD CURB SECTION USED)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0050  
STREET LENGTH(FEET) = 220.70 CURB HEIGHT(INCHES) = 6.0  
STREET HALFWIDTH(FEET) = 20.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1  
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.73  
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
STREET FLOW DEPTH(FEET) = 0.32  
HALFSTREET FLOOD WIDTH(FEET) = 9.90

AVERAGE FLOW VELOCITY (FEET/SEC.) = 1.58  
 PRODUCT OF DEPTH&VELOCITY (FT\*FT/SEC.) = 0.51  
 STREET FLOW TRAVEL TIME (MIN.) = 2.33 Tc (MIN.) = 9.56  
 \* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.561  
 SUBAREA LOSS RATE DATA (AMC I):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.84	0.30	0.100	36

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100  
 SUBAREA AREA (ACRES) = 0.84 SUBAREA RUNOFF (CFS) = 1.16  
 EFFECTIVE AREA (ACRES) = 1.55 AREA-AVERAGED Fm (INCH/HR) = 0.03  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10  
 TOTAL AREA (ACRES) = 1.5 PEAK FLOW RATE (CFS) = 2.14

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH (FEET) = 0.34 HALFSTREET FLOOD WIDTH (FEET) = 10.82  
 FLOW VELOCITY (FEET/SEC.) = 1.66 DEPTH\*VELOCITY (FT\*FT/SEC.) = 0.57  
 LONGEST FLOWPATH FROM NODE 22.51 TO NODE 22.53 = 504.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 22.53 TO NODE 22.53 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc (MIN.) = 9.56  
 \* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.561  
 SUBAREA LOSS RATE DATA (AMC I):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.15	0.30	0.100	36

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100  
 SUBAREA AREA (ACRES) = 0.15 SUBAREA RUNOFF (CFS) = 0.21  
 EFFECTIVE AREA (ACRES) = 1.70 AREA-AVERAGED Fm (INCH/HR) = 0.03  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10  
 TOTAL AREA (ACRES) = 1.7 PEAK FLOW RATE (CFS) = 2.34

\*\*\*\*\*

FLOW PROCESS FROM NODE 22.53 TO NODE 22.50 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW) <<<<<

REPRESENTATIVE SLOPE = 0.0100  
 FLOW LENGTH (FEET) = 50.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 12.0 INCH PIPE IS 7.4 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 4.64  
 ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 2.34  
 PIPE TRAVEL TIME (MIN.) = 0.18 Tc (MIN.) = 9.74  
 LONGEST FLOWPATH FROM NODE 22.51 TO NODE 22.50 = 554.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 22.50 TO NODE 22.50 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<



>>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
TIME OF CONCENTRATION(MIN.) = 9.74  
RAINFALL INTENSITY(INCH/HR) = 1.54  
AREA-AVERAGED Fm(INCH/HR) = 0.03  
AREA-AVERAGED Fp(INCH/HR) = 0.30  
AREA-AVERAGED Ap = 0.10  
EFFECTIVE STREAM AREA(ACRES) = 1.70  
TOTAL STREAM AREA(ACRES) = 1.70  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.34

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	1.02	17.05	1.119	0.30( 0.28)	0.95	1.4	21.00
2	2.34	9.74	1.544	0.30( 0.03)	0.10	1.7	22.51

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.22	9.74	1.544	0.30( 0.11)	0.37	2.5	22.51
2	2.71	17.05	1.119	0.30( 0.14)	0.48	3.1	21.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 3.22 Tc(MIN.) = 9.74  
EFFECTIVE AREA(ACRES) = 2.48 AREA-AVERAGED Fm(INCH/HR) = 0.11  
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.37  
TOTAL AREA(ACRES) = 3.1  
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 22.50 = 786.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 22.50 TO NODE 23.00 IS CODE = 52

>>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<  
>>>>>TRAVELTIME THRU SUBAREA<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 143.90 DOWNSTREAM(FEET) = 139.80  
CHANNEL LENGTH THRU SUBAREA(FEET) = 325.40 CHANNEL SLOPE = 0.0126  
CHANNEL FLOW THRU SUBAREA(CFS) = 3.22  
FLOW VELOCITY(FEET/SEC) = 2.14 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)  
TRAVEL TIME(MIN.) = 2.53 Tc(MIN.) = 12.27  
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 23.00 = 1111.40 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81

>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 12.27  
\* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.352  
SUBAREA LOSS RATE DATA(AMC I):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS

LAND USE                      GROUP      (ACRES)      (INCH/HR)      (DECIMAL)      CN  
 PUBLIC PARK                      B              0.25              0.30              0.850              36  
 SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$ (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.850  
 SUBAREA AREA(ACRES) = 0.25              SUBAREA RUNOFF(CFS) = 0.25  
 EFFECTIVE AREA(ACRES) = 2.73              AREA-AVERAGED  $F_m$ (INCH/HR) = 0.12  
 AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30              AREA-AVERAGED  $A_p$  = 0.41  
 TOTAL AREA(ACRES) = 3.3              PEAK FLOW RATE(CFS) = 3.22  
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

\*\*\*\*\*

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
 TIME OF CONCENTRATION(MIN.) = 12.27  
 RAINFALL INTENSITY(INCH/HR) = 1.35  
 AREA-AVERAGED  $F_m$ (INCH/HR) = 0.12  
 AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30  
 AREA-AVERAGED  $A_p$  = 0.41  
 EFFECTIVE STREAM AREA(ACRES) = 2.73  
 TOTAL STREAM AREA(ACRES) = 3.31  
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 3.22

\*\*\*\*\*

FLOW PROCESS FROM NODE 23.10 TO NODE 23.20 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 285.00  
 ELEVATION DATA: UPSTREAM(FEET) = 150.80      DOWNSTREAM(FEET) = 147.80

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM  $T_c$ (MIN.) = 12.521

\* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 1.337

SUBAREA  $T_c$  AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN	$T_c$ (MIN.)
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NATURAL POOR COVER

"BARREN"	B	0.84	0.30	1.000	72	12.52
----------	---	------	------	-------	----	-------

SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$ (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 1.000

SUBAREA RUNOFF(CFS) = 0.78

TOTAL AREA(ACRES) = 0.84      PEAK FLOW RATE(CFS) = 0.78

\*\*\*\*\*

FLOW PROCESS FROM NODE 23.20 TO NODE 23.00 IS CODE = 52

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 147.80      DOWNSTREAM(FEET) = 139.60

CHANNEL LENGTH THRU SUBAREA(FEET) = 815.80      CHANNEL SLOPE = 0.0101

NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION

CHANNEL FLOW THRU SUBAREA(CFS) = 0.78  
 FLOW VELOCITY(FEET/SEC) = 1.50 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)  
 TRAVEL TIME(MIN.) = 9.04 Tc(MIN.) = 21.56  
 LONGEST FLOWPATH FROM NODE 23.10 TO NODE 23.00 = 1100.80 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 21.56  
 \* 2 YEAR RAINFALL INTENSITY(INCH/HR) = 0.978  
 SUBAREA LOSS RATE DATA(AMC I):  
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
 NATURAL POOR COVER  
 "BARREN" B 1.81 0.30 1.000 72  
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000  
 SUBAREA AREA(ACRES) = 1.81 SUBAREA RUNOFF(CFS) = 1.11  
 EFFECTIVE AREA(ACRES) = 2.65 AREA-AVERAGED Fm(INCH/HR) = 0.30  
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 1.00  
 TOTAL AREA(ACRES) = 2.6 PEAK FLOW RATE(CFS) = 1.62

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
 TIME OF CONCENTRATION(MIN.) = 21.56  
 RAINFALL INTENSITY(INCH/HR) = 0.98  
 AREA-AVERAGED Fm(INCH/HR) = 0.30  
 AREA-AVERAGED Fp(INCH/HR) = 0.30  
 AREA-AVERAGED Ap = 1.00  
 EFFECTIVE STREAM AREA(ACRES) = 2.65  
 TOTAL STREAM AREA(ACRES) = 2.65  
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 1.62

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	3.22	12.27	1.352	0.30( 0.12)	0.41	2.7	22.51
1	2.71	19.69	1.031	0.30( 0.15)	0.50	3.3	21.00
2	1.62	21.56	0.978	0.30( 0.30)	1.00	2.6	23.10

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	4.65	12.27	1.352	0.30( 0.19)	0.62	4.2	22.51
2	4.30	19.69	1.031	0.30( 0.21)	0.71	5.7	21.00
3	4.16	21.56	0.978	0.30( 0.22)	0.72	6.0	23.10

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 4.65 Tc (MIN.) = 12.27  
EFFECTIVE AREA (ACRES) = 4.23 AREA-AVERAGED Fm (INCH/HR) = 0.19  
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.62  
TOTAL AREA (ACRES) = 6.0  
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 23.00 = 1111.40 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc (MIN.) = 12.27  
\* 2 YEAR RAINFALL INTENSITY (INCH/HR) = 1.352  
SUBAREA LOSS RATE DATA (AMC I):  
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
PUBLIC PARK B 1.49 0.30 0.850 36  
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
SUBAREA AREA (ACRES) = 1.49 SUBAREA RUNOFF (CFS) = 1.47  
EFFECTIVE AREA (ACRES) = 5.72 AREA-AVERAGED Fm (INCH/HR) = 0.20  
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.68  
TOTAL AREA (ACRES) = 7.4 PEAK FLOW RATE (CFS) = 5.92

\*\*\*\*\*

FLOW PROCESS FROM NODE 23.00 TO NODE 24.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0100  
FLOW LENGTH (FEET) = 45.70 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 10.0 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.88  
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 5.92  
PIPE TRAVEL TIME (MIN.) = 0.13 Tc (MIN.) = 12.40  
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 24.00 = 1157.10 FEET.

END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 7.4 TC (MIN.) = 12.40  
EFFECTIVE AREA (ACRES) = 5.72 AREA-AVERAGED Fm (INCH/HR) = 0.20  
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.680  
PEAK FLOW RATE (CFS) = 5.92

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.92	12.40	1.344	0.30 (0.20)	0.68	5.7	22.51
2	5.25	19.82	1.027	0.30 (0.22)	0.74	7.2	21.00
3	5.05	21.70	0.975	0.30 (0.22)	0.75	7.4	23.10

END OF RATIONAL METHOD ANALYSIS

**Appendix I**  
Proposed Q<sub>10</sub> Hydrology Calculations

\*\*\*\*\*

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE  
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)  
(c) Copyright 1983-2008 Advanced Engineering Software (aes)  
Ver. 15.0 Release Date: 04/01/2008 License ID 1204

Analysis prepared by:

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\*\*\*\*\* DESCRIPTION OF STUDY \*\*\*\*\*  
\* 10-YEAR STORM EVENT HYDROLOGY ANALYSIS \*  
\* OAKCREEK COMMUNITY PARK, IRVINE CALIFORNIA \*  
\* PROPOSED CONDITION \*  
\*\*\*\*\*

FILE NAME: OAK10PRA.DAT  
TIME/DATE OF STUDY: 16:13 04/04/2022

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--\*TIME-OF-CONCENTRATION MODEL\*--

USER SPECIFIED STORM EVENT (YEAR) = 10.00  
SPECIFIED MINIMUM PIPE SIZE (INCH) = 8.00  
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.90  
\*DATA BANK RAINFALL USED\*  
\*ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD\*

\*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL\*

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/ SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH (FT)	LIP (FT)	HIKE (FT)	MANNING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.50	2.00	0.0313	0.125	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:  
1. Relative Flow-Depth = 0.00 FEET  
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)  
2. (Depth)\*(Velocity) Constraint = 6.0 (FT\*FT/S)  
\*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN  
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.\*  
\*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

\*\*\*\*\*

FLOW PROCESS FROM NODE 1.00 TO NODE 2.00 IS CODE = 21

-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 251.00  
ELEVATION DATA: UPSTREAM (FEET) = 157.00 DOWNSTREAM (FEET) = 151.80

$T_c = K * [(LENGTH^{**} 3.00) / (ELEVATION CHANGE)]^{**} 0.20$   
 SUBAREA ANALYSIS USED MINIMUM  $T_c$  (MIN.) = 9.562  
 \* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.800  
 SUBAREA  $T_c$  AND LOSS RATE DATA (AMC II):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN	$T_c$ (MIN.)
PUBLIC PARK	B	0.07	0.30	0.850	56	9.56

 SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$  (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.850  
 SUBAREA RUNOFF (CFS) = 0.16  
 TOTAL AREA (ACRES) = 0.07 PEAK FLOW RATE (CFS) = 0.16

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 2.00 TO NODE 3.00 IS CODE = 41  
 -----

>>>> COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA <<<<<  
 >>>> USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT) <<<<<  
 =====  
 REPRESENTATIVE SLOPE = 0.0150  
 FLOW LENGTH (FEET) = 49.40 MANNING'S N = 0.010  
 DEPTH OF FLOW IN 6.0 INCH PIPE IS 1.8 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 3.30  
 GIVEN PIPE DIAMETER (INCH) = 6.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 0.16  
 PIPE TRAVEL TIME (MIN.) = 0.25  $T_c$  (MIN.) = 9.81  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 300.40 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 3.00 TO NODE 3.00 IS CODE = 81  
 -----

>>>> ADDITION OF SUBAREA TO MAINLINE PEAK FLOW <<<<<  
 =====  
 MAINLINE  $T_c$  (MIN.) = 9.81  
 \* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.759  
 SUBAREA LOSS RATE DATA (AMC II):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN
PUBLIC PARK	B	0.24	0.30	0.850	56

 SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$  (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.850  
 SUBAREA AREA (ACRES) = 0.24 SUBAREA RUNOFF (CFS) = 0.54  
 EFFECTIVE AREA (ACRES) = 0.31 AREA-AVERAGED  $F_m$  (INCH/HR) = 0.26  
 AREA-AVERAGED  $F_p$  (INCH/HR) = 0.30 AREA-AVERAGED  $A_p$  = 0.85  
 TOTAL AREA (ACRES) = 0.3 PEAK FLOW RATE (CFS) = 0.70

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 3.00 TO NODE 4.00 IS CODE = 41  
 -----

>>>> COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA <<<<<  
 >>>> USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT) <<<<<  
 =====  
 REPRESENTATIVE SLOPE = 0.0150  
 FLOW LENGTH (FEET) = 363.00 MANNING'S N = 0.010  
 DEPTH OF FLOW IN 8.0 INCH PIPE IS 3.4 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 4.88  
 GIVEN PIPE DIAMETER (INCH) = 8.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 0.70

PIPE TRAVEL TIME (MIN.) = 1.24 Tc (MIN.) = 11.05  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4.00 = 663.40 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 4.00 TO NODE 4.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 11.05  
\* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.577  
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	2.53	0.30	0.850	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
SUBAREA AREA (ACRES) = 2.53 SUBAREA RUNOFF (CFS) = 5.29  
EFFECTIVE AREA (ACRES) = 2.84 AREA-AVERAGED Fm (INCH/HR) = 0.26  
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85  
TOTAL AREA (ACRES) = 2.8 PEAK FLOW RATE (CFS) = 5.94

\*\*\*\*\*  
FLOW PROCESS FROM NODE 4.00 TO NODE 5.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0150  
FLOW LENGTH (FEET) = 97.00 MANNING'S N = 0.010  
ASSUME FULL-FLOWING PIPELINE  
PIPE-FLOW VELOCITY (FEET/SEC.) = 17.00  
PIPE FLOW VELOCITY = (TOTAL FLOW) / (PIPE CROSS SECTION AREA)  
GIVEN PIPE DIAMETER (INCH) = 8.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 5.94  
PIPE TRAVEL TIME (MIN.) = 0.10 Tc (MIN.) = 11.15  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 5.00 = 760.40 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 5.00 TO NODE 5.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 11.15  
\* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.564  
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	0.25	0.30	0.850	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
SUBAREA AREA (ACRES) = 0.25 SUBAREA RUNOFF (CFS) = 0.52  
EFFECTIVE AREA (ACRES) = 3.09 AREA-AVERAGED Fm (INCH/HR) = 0.26  
AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85  
TOTAL AREA (ACRES) = 3.1 PEAK FLOW RATE (CFS) = 6.42

\*\*\*\*\*  
FLOW PROCESS FROM NODE 5.00 TO NODE 6.00 IS CODE = 41



```

-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====
REPRESENTATIVE SLOPE = 0.0150
FLOW LENGTH(FEET) = 135.00 MANNING'S N = 0.010
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY(FEET/SEC.) = 18.40
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER(INCH) = 8.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 6.42
PIPE TRAVEL TIME(MIN.) = 0.12 Tc(MIN.) = 11.27
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 6.00 = 895.40 FEET.

*****
FLOW PROCESS FROM NODE 6.00 TO NODE 6.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) = 11.27
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.548
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
PUBLIC PARK B 0.71 0.30 0.850 56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850
SUBAREA AREA(ACRES) = 0.71 SUBAREA RUNOFF(CFS) = 1.47
EFFECTIVE AREA(ACRES) = 3.80 AREA-AVERAGED Fm(INCH/HR) = 0.26
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85
TOTAL AREA(ACRES) = 3.8 PEAK FLOW RATE(CFS) = 7.84

*****
FLOW PROCESS FROM NODE 6.00 TO NODE 7.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====
REPRESENTATIVE SLOPE = 0.0150
FLOW LENGTH(FEET) = 5.00 MANNING'S N = 0.010
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.99
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 7.84
PIPE TRAVEL TIME(MIN.) = 0.01 Tc(MIN.) = 11.28
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 7.00 = 900.40 FEET.

*****
FLOW PROCESS FROM NODE 7.00 TO NODE 7.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) = 11.28
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.547
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS

```

LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN
PUBLIC PARK	B	2.01	0.30	0.850	56
SUBAREA AVERAGE PERVIOUS LOSS RATE, $F_p$ (INCH/HR) = 0.30					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p$ = 0.850					
SUBAREA AREA(ACRES) =		2.01	SUBAREA RUNOFF(CFS) =		4.15
EFFECTIVE AREA(ACRES) =		5.81	AREA-AVERAGED $F_m$ (INCH/HR) =		0.26
AREA-AVERAGED $F_p$ (INCH/HR) =		0.30	AREA-AVERAGED $A_p$ =		0.85
TOTAL AREA(ACRES) =		5.8	PEAK FLOW RATE(CFS) =		11.99

\*\*\*\*\*  
FLOW PROCESS FROM NODE 7.00 TO NODE 8.00 IS CODE = 41  
-----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<

=====

REPRESENTATIVE SLOPE = 0.0111  
FLOW LENGTH(FEET) = 231.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 12.4 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.30  
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 11.99  
PIPE TRAVEL TIME(MIN.) = 0.53  $T_c$ (MIN.) = 11.80  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 8.00 = 1131.40 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1  
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 11.80  
RAINFALL INTENSITY(INCH/HR) = 2.48  
AREA-AVERAGED  $F_m$ (INCH/HR) = 0.26  
AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30  
AREA-AVERAGED  $A_p$  = 0.85  
EFFECTIVE STREAM AREA(ACRES) = 5.81  
TOTAL STREAM AREA(ACRES) = 5.81  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 11.99

\*\*\*\*\*  
FLOW PROCESS FROM NODE 8.10 TO NODE 8.20 IS CODE = 21  
-----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 300.00  
ELEVATION DATA: UPSTREAM(FEET) = 150.00 DOWNSTREAM(FEET) = 144.55

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM  $T_c$ (MIN.) = 10.542

\* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.648

SUBAREA  $T_c$  AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN	$T_c$ (MIN.)
PUBLIC PARK	B	0.76	0.30	0.850	56	10.54
SUBAREA AVERAGE PERVIOUS LOSS RATE, $F_p$ (INCH/HR) = 0.30						

SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p = 0.850$   
 SUBAREA RUNOFF (CFS) = 1.64  
 TOTAL AREA (ACRES) = 0.76 PEAK FLOW RATE (CFS) = 1.64

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.20 TO NODE 8.30 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0150  
 FLOW LENGTH (FEET) = 167.00 MANNING'S N = 0.010  
 DEPTH OF FLOW IN 8.0 INCH PIPE IS 5.9 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 5.92  
 GIVEN PIPE DIAMETER (INCH) = 8.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 1.64  
 PIPE TRAVEL TIME (MIN.) = 0.47  $T_c$  (MIN.) = 11.01  
 LONGEST FLOWPATH FROM NODE 8.10 TO NODE 8.30 = 467.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.30 TO NODE 8.30 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE  $T_c$  (MIN.) = 11.01  
 \* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.582  
 SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN
COMMERCIAL	B	0.51	0.30	0.100	56

SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$  (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p = 0.100$   
 SUBAREA AREA (ACRES) = 0.51 SUBAREA RUNOFF (CFS) = 1.17  
 EFFECTIVE AREA (ACRES) = 1.27 AREA-AVERAGED  $F_m$  (INCH/HR) = 0.16  
 AREA-AVERAGED  $F_p$  (INCH/HR) = 0.30 AREA-AVERAGED  $A_p = 0.55$   
 TOTAL AREA (ACRES) = 1.3 PEAK FLOW RATE (CFS) = 2.76

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.30 TO NODE 8.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0077  
 FLOW LENGTH (FEET) = 37.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 7.0 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 4.39  
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 2.76  
 PIPE TRAVEL TIME (MIN.) = 0.14  $T_c$  (MIN.) = 11.15  
 LONGEST FLOWPATH FROM NODE 8.10 TO NODE 8.00 = 504.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 3  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
 TIME OF CONCENTRATION(MIN.) = 11.15  
 RAINFALL INTENSITY(INCH/HR) = 2.56  
 AREA-AVERAGED Fm(INCH/HR) = 0.16  
 AREA-AVERAGED Fp(INCH/HR) = 0.30  
 AREA-AVERAGED Ap = 0.55  
 EFFECTIVE STREAM AREA(ACRES) = 1.27  
 TOTAL STREAM AREA(ACRES) = 1.27  
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.76

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.40 TO NODE 8.50 IS CODE = 21  
 -----

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 255.00  
 ELEVATION DATA: UPSTREAM(FEET) = 147.00 DOWNSTREAM(FEET) = 142.68

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$   
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 6.305  
 \* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.554

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	B	0.45	0.30	0.100	56	6.31

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 1.43

TOTAL AREA(ACRES) = 0.45 PEAK FLOW RATE(CFS) = 1.43

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.50 TO NODE 8.00 IS CODE = 41  
 -----

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0557  
 FLOW LENGTH(FEET) = 37.70 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.0 INCHES  
 PIPE-FLOW VELOCITY(FEET/SEC.) = 7.34  
 GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1  
 PIPE-FLOW(CFS) = 1.43  
 PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 6.39  
 LONGEST FLOWPATH FROM NODE 8.40 TO NODE 8.00 = 292.70 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1  
 -----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:  
 TIME OF CONCENTRATION(MIN.) = 6.39  
 RAINFALL INTENSITY(INCH/HR) = 3.53

AREA-AVERAGED Fm (INCH/HR) = 0.03  
 AREA-AVERAGED Fp (INCH/HR) = 0.30  
 AREA-AVERAGED Ap = 0.10  
 EFFECTIVE STREAM AREA (ACRES) = 0.45  
 TOTAL STREAM AREA (ACRES) = 0.45  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.43

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	11.99	11.80	2.482	0.30 ( 0.26)	0.85	5.8	1.00
2	2.76	11.15	2.563	0.30 ( 0.16)	0.55	1.3	8.10
3	1.43	6.39	3.527	0.30 ( 0.03)	0.10	0.4	8.40

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 3 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	13.18	6.39	3.527	0.30 ( 0.22)	0.72	4.3	8.40
2	15.54	11.15	2.563	0.30 ( 0.23)	0.75	7.2	8.10
3	15.66	11.80	2.482	0.30 ( 0.23)	0.75	7.5	1.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 15.66 Tc (MIN.) = 11.80  
 EFFECTIVE AREA (ACRES) = 7.53 AREA-AVERAGED Fm (INCH/HR) = 0.23  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.75  
 TOTAL AREA (ACRES) = 7.5  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 8.00 = 1131.40 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 8.00 TO NODE 9.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0050  
 FLOW LENGTH (FEET) = 261.00 MANNING'S N = 0.013  
 ASSUME FULL-FLOWING PIPELINE  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 4.98  
 PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)  
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 15.66  
 PIPE TRAVEL TIME (MIN.) = 0.87 Tc (MIN.) = 12.68  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 9.00 = 1392.40 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 9.00 TO NODE 9.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc (MIN.) = 12.68  
 \* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.382  
 SUBAREA LOSS RATE DATA (AMC II):  
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN

COMMERCIAL B 0.50 0.30 0.100 56  
 PUBLIC PARK B 3.13 0.30 0.850 56  
 SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$ (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.747  
 SUBAREA AREA(ACRES) = 3.63 SUBAREA RUNOFF(CFS) = 7.05  
 EFFECTIVE AREA(ACRES) = 11.16 AREA-AVERAGED  $F_m$ (INCH/HR) = 0.23  
 AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30 AREA-AVERAGED  $A_p$  = 0.75  
 TOTAL AREA(ACRES) = 11.2 PEAK FLOW RATE(CFS) = 21.66

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	$F_p$ ( $F_m$ ) (INCH/HR)	$A_p$	$A_e$ (ACRES)	HEADWATER NODE
1	22.02	7.19	3.297	0.30( 0.22)	0.73	8.0	8.40
2	21.75	12.03	2.454	0.30( 0.22)	0.75	10.8	8.10
3	21.66	12.68	2.382	0.30( 0.23)	0.75	11.2	1.00

NEW PEAK FLOW DATA ARE:

PEAK FLOW RATE(CFS) = 22.02 Tc(MIN.) = 7.19  
 AREA-AVERAGED  $F_m$ (INCH/HR) = 0.22 AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30  
 AREA-AVERAGED  $A_p$  = 0.73 EFFECTIVE AREA(ACRES) = 7.95

\*\*\*\*\*

FLOW PROCESS FROM NODE 9.00 TO NODE 9.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 7.19  
 \* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.297  
 SUBAREA LOSS RATE DATA(AMC II):  
 DEVELOPMENT TYPE/ SCS SOIL AREA  $F_p$   $A_p$  SCS  
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
 PUBLIC PARK B 0.33 0.30 0.850 56  
 SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$ (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.850  
 SUBAREA AREA(ACRES) = 0.33 SUBAREA RUNOFF(CFS) = 0.90  
 EFFECTIVE AREA(ACRES) = 8.28 AREA-AVERAGED  $F_m$ (INCH/HR) = 0.22  
 AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30 AREA-AVERAGED  $A_p$  = 0.74  
 TOTAL AREA(ACRES) = 11.5 PEAK FLOW RATE(CFS) = 22.93

\*\*\*\*\*

FLOW PROCESS FROM NODE 9.00 TO NODE 9.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 7.19  
 \* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.297  
 SUBAREA LOSS RATE DATA(AMC II):  
 DEVELOPMENT TYPE/ SCS SOIL AREA  $F_p$   $A_p$  SCS  
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
 PUBLIC PARK B 0.22 0.30 0.850 56  
 SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$ (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.850  
 SUBAREA AREA(ACRES) = 0.22 SUBAREA RUNOFF(CFS) = 0.60  
 EFFECTIVE AREA(ACRES) = 8.50 AREA-AVERAGED  $F_m$ (INCH/HR) = 0.22  
 AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30 AREA-AVERAGED  $A_p$  = 0.74  
 TOTAL AREA(ACRES) = 11.7 PEAK FLOW RATE(CFS) = 23.53

\*\*\*\*\*

FLOW PROCESS FROM NODE 9.00 TO NODE 10.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0050  
FLOW LENGTH(FEET) = 45.00 MANNING'S N = 0.013  
ASSUME FULL-FLOWING PIPELINE  
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.49  
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)  
GIVEN PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 23.53  
PIPE TRAVEL TIME(MIN.) = 0.10 Tc(MIN.) = 7.29  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 10.00 = 1437.40 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 10.00 TO NODE 11.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0100  
FLOW LENGTH(FEET) = 43.70 MANNING'S N = 0.013  
DEPTH OF FLOW IN 36.0 INCH PIPE IS 15.2 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.29  
GIVEN PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 23.53  
PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 7.38  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 11.00 = 1481.10 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 11.00 TO NODE 12.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0100  
FLOW LENGTH(FEET) = 93.90 MANNING'S N = 0.013  
DEPTH OF FLOW IN 36.0 INCH PIPE IS 15.2 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.29  
GIVEN PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 23.53  
PIPE TRAVEL TIME(MIN.) = 0.19 Tc(MIN.) = 7.57  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 12.00 = 1575.00 FEET.

=====

END OF RATIONAL METHOD ANALYSIS

\*\*\*\*\*

FLOW PROCESS FROM NODE 21.00 TO NODE 22.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 314.00
ELEVATION DATA: UPSTREAM(FEET) = 154.00 DOWNSTREAM(FEET) = 148.60

Tc = K\*[(LENGTH\*\* 3.00)/(ELEVATION CHANGE)]\*\*0.20

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.799

\* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.482

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)

NATURAL POOR COVER

"BARREN" B 0.88 0.30 1.000 86 11.80

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000

SUBAREA RUNOFF(CFS) = 1.73

TOTAL AREA(ACRES) = 0.88 PEAK FLOW RATE(CFS) = 1.73

\*\*\*\*\*

FLOW PROCESS FROM NODE 22.00 TO NODE 22.50 IS CODE = 52

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 148.60 DOWNSTREAM(FEET) = 143.90

CHANNEL LENGTH THRU SUBAREA(FEET) = 472.00 CHANNEL SLOPE = 0.0100

CHANNEL FLOW THRU SUBAREA(CFS) = 1.73

FLOW VELOCITY(FEET/SEC) = 1.67 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)

TRAVEL TIME(MIN.) = 4.72 Tc(MIN.) = 16.52

LONGEST FLOWPATH FROM NODE 21.00 TO NODE 22.50 = 786.00 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 22.50 TO NODE 22.50 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 16.52

\* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.047

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN

PUBLIC PARK B 0.48 0.30 0.850 56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850

SUBAREA AREA(ACRES) = 0.48 SUBAREA RUNOFF(CFS) = 0.77

EFFECTIVE AREA(ACRES) = 1.36 AREA-AVERAGED Fm(INCH/HR) = 0.28

AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.95

TOTAL AREA(ACRES) = 1.4 PEAK FLOW RATE(CFS) = 2.16

\*\*\*\*\*

FLOW PROCESS FROM NODE 22.50 TO NODE 22.50 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<



```

=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 16.52
RAINFALL INTENSITY(INCH/HR) = 2.05
AREA-AVERAGED Fm(INCH/HR) = 0.28
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.95
EFFECTIVE STREAM AREA(ACRES) = 1.36
TOTAL STREAM AREA(ACRES) = 1.36
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.16

*****
FLOW PROCESS FROM NODE 22.51 TO NODE 22.52 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 283.30
ELEVATION DATA: UPSTREAM(FEET) = 150.40 DOWNSTREAM(FEET) = 147.40

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.224
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.288
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL B 0.71 0.30 0.100 56 7.22
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 2.08
TOTAL AREA(ACRES) = 0.71 PEAK FLOW RATE(CFS) = 2.08

*****
FLOW PROCESS FROM NODE 22.52 TO NODE 22.53 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
REPRESENTATIVE SLOPE = 0.0050
STREET LENGTH(FEET) = 220.70 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 20.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.15
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.38
HALFSTREET FLOOD WIDTH(FEET) = 12.70
AVERAGE FLOW VELOCITY(FEET/SEC.) = 1.82

```

PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 0.69  
 STREET FLOW TRAVEL TIME(MIN.) = 2.02 Tc(MIN.) = 9.24  
 \* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.855  
 SUBAREA LOSS RATE DATA(AMC II):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.84	0.30	0.100	56

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100  
 SUBAREA AREA(ACRES) = 0.84 SUBAREA RUNOFF(CFS) = 2.14  
 EFFECTIVE AREA(ACRES) = 1.55 AREA-AVERAGED Fm(INCH/HR) = 0.03  
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10  
 TOTAL AREA(ACRES) = 1.5 PEAK FLOW RATE(CFS) = 3.94

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.41 HALFSTREET FLOOD WIDTH(FEET) = 13.95  
 FLOW VELOCITY(FEET/SEC.) = 1.91 DEPTH\*VELOCITY(FT\*FT/SEC.) = 0.77  
 LONGEST FLOWPATH FROM NODE 22.51 TO NODE 22.53 = 504.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 22.53 TO NODE 22.53 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 9.24  
 \* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.855  
 SUBAREA LOSS RATE DATA(AMC II):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.15	0.30	0.100	56

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100  
 SUBAREA AREA(ACRES) = 0.15 SUBAREA RUNOFF(CFS) = 0.38  
 EFFECTIVE AREA(ACRES) = 1.70 AREA-AVERAGED Fm(INCH/HR) = 0.03  
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10  
 TOTAL AREA(ACRES) = 1.7 PEAK FLOW RATE(CFS) = 4.32

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 22.53 TO NODE 22.50 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

REPRESENTATIVE SLOPE = 0.0100  
 FLOW LENGTH(FEET) = 50.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 15.0 INCH PIPE IS 9.3 INCHES  
 PIPE-FLOW VELOCITY(FEET/SEC.) = 5.41  
 ESTIMATED PIPE DIAMETER(INCH) = 15.00 NUMBER OF PIPES = 1  
 PIPE-FLOW(CFS) = 4.32  
 PIPE TRAVEL TIME(MIN.) = 0.15 Tc(MIN.) = 9.40  
 LONGEST FLOWPATH FROM NODE 22.51 TO NODE 22.50 = 554.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 22.50 TO NODE 22.50 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

```

=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 9.40
RAINFALL INTENSITY(INCH/HR) = 2.83
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 1.70
TOTAL STREAM AREA(ACRES) = 1.70
PEAK FLOW RATE(CFS) AT CONFLUENCE = 4.32

```

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.16	16.52	2.047	0.30( 0.28)	0.95	1.4	21.00
2	4.32	9.40	2.828	0.30( 0.03)	0.10	1.7	22.51

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	6.09	9.40	2.828	0.30( 0.11)	0.36	2.5	22.51
2	5.27	16.52	2.047	0.30( 0.14)	0.48	3.1	21.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

```

PEAK FLOW RATE(CFS) = 6.09 Tc(MIN.) = 9.40
EFFECTIVE AREA(ACRES) = 2.47 AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.36
TOTAL AREA(ACRES) = 3.1
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 22.50 = 786.00 FEET.

```

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*****
FLOW PROCESS FROM NODE 22.50 TO NODE 23.00 IS CODE = 52

```

```

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<<

```

```

=====
ELEVATION DATA: UPSTREAM(FEET) = 143.90 DOWNSTREAM(FEET) = 139.80
CHANNEL LENGTH THRU SUBAREA(FEET) = 325.40 CHANNEL SLOPE = 0.0126
CHANNEL FLOW THRU SUBAREA(CFS) = 6.09
FLOW VELOCITY(FEET/SEC) = 2.48 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 2.18 Tc(MIN.) = 11.58
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 23.00 = 1111.40 FEET.

```

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*****
FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81

```

```

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

```

```

=====
MAINLINE Tc(MIN.) = 11.58
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.508
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN

```

PUBLIC PARK B 0.25 0.30 0.850 56  
SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$ (INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.850  
SUBAREA AREA(ACRES) = 0.25 SUBAREA RUNOFF(CFS) = 0.51  
EFFECTIVE AREA(ACRES) = 2.72 AREA-AVERAGED  $F_m$ (INCH/HR) = 0.12  
AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30 AREA-AVERAGED  $A_p$  = 0.41  
TOTAL AREA(ACRES) = 3.3 PEAK FLOW RATE(CFS) = 6.09  
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

\*\*\*\*\*  
FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION(MIN.) = 11.58  
RAINFALL INTENSITY(INCH/HR) = 2.51  
AREA-AVERAGED  $F_m$ (INCH/HR) = 0.12  
AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30  
AREA-AVERAGED  $A_p$  = 0.41  
EFFECTIVE STREAM AREA(ACRES) = 2.72  
TOTAL STREAM AREA(ACRES) = 3.31  
PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.09

\*\*\*\*\*  
FLOW PROCESS FROM NODE 23.10 TO NODE 23.20 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 285.00  
ELEVATION DATA: UPSTREAM(FEET) = 150.80 DOWNSTREAM(FEET) = 147.80

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM  $T_c$ (MIN.) = 12.521

\* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.399

SUBAREA  $T_c$  AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN	$T_c$ (MIN.)
NATURAL POOR COVER						
"BARREN"	B	0.84	0.30	1.000	86	12.52

SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$ (INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 1.000

SUBAREA RUNOFF(CFS) = 1.59  
TOTAL AREA(ACRES) = 0.84

PEAK FLOW RATE(CFS) = 1.59

\*\*\*\*\*  
FLOW PROCESS FROM NODE 23.20 TO NODE 23.00 IS CODE = 52

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 147.80 DOWNSTREAM(FEET) = 139.60  
CHANNEL LENGTH THRU SUBAREA(FEET) = 815.80 CHANNEL SLOPE = 0.0101  
CHANNEL FLOW THRU SUBAREA(CFS) = 1.59  
FLOW VELOCITY(FEET/SEC) = 1.65 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)

TRAVEL TIME (MIN.) = 8.26 Tc (MIN.) = 20.78  
 LONGEST FLOWPATH FROM NODE 23.10 TO NODE 23.00 = 1100.80 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 20.78  
 \* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 1.794  
 SUBAREA LOSS RATE DATA (AMC II):  
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
 NATURAL POOR COVER  
 "BARREN" B 1.81 0.30 1.000 86  
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000  
 SUBAREA AREA (ACRES) = 1.81 SUBAREA RUNOFF (CFS) = 2.43  
 EFFECTIVE AREA (ACRES) = 2.65 AREA-AVERAGED Fm (INCH/HR) = 0.30  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 1.00  
 TOTAL AREA (ACRES) = 2.6 PEAK FLOW RATE (CFS) = 3.56

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
 TIME OF CONCENTRATION (MIN.) = 20.78  
 RAINFALL INTENSITY (INCH/HR) = 1.79  
 AREA-AVERAGED Fm (INCH/HR) = 0.30  
 AREA-AVERAGED Fp (INCH/HR) = 0.30  
 AREA-AVERAGED Ap = 1.00  
 EFFECTIVE STREAM AREA (ACRES) = 2.65  
 TOTAL STREAM AREA (ACRES) = 2.65  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 3.56

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	6.09	11.58	2.508	0.30 ( 0.12)	0.41	2.7	22.51
1	5.27	18.78	1.902	0.30 ( 0.15)	0.50	3.3	21.00
2	3.56	20.78	1.794	0.30 ( 0.30)	1.00	2.6	23.10

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	9.03	11.58	2.508	0.30 ( 0.19)	0.62	4.2	22.51
2	8.72	18.78	1.902	0.30 ( 0.21)	0.71	5.7	21.00
3	8.51	20.78	1.794	0.30 ( 0.22)	0.72	6.0	23.10

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 9.03 Tc (MIN.) = 11.58  
 EFFECTIVE AREA (ACRES) = 4.20 AREA-AVERAGED Fm (INCH/HR) = 0.19  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.62  
 TOTAL AREA (ACRES) = 6.0  
 LONGEST FLOWPATH FROM NODE 21.00 TO NODE 23.00 = 1111.40 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc (MIN.) = 11.58  
 \* 10 YEAR RAINFALL INTENSITY (INCH/HR) = 2.508  
 SUBAREA LOSS RATE DATA (AMC II):  
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
 PUBLIC PARK B 1.49 0.30 0.850 56  
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
 SUBAREA AREA (ACRES) = 1.49 SUBAREA RUNOFF (CFS) = 3.02  
 EFFECTIVE AREA (ACRES) = 5.69 AREA-AVERAGED Fm (INCH/HR) = 0.20  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.68  
 TOTAL AREA (ACRES) = 7.4 PEAK FLOW RATE (CFS) = 11.81

\*\*\*\*\*

FLOW PROCESS FROM NODE 23.00 TO NODE 24.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0100  
 FLOW LENGTH (FEET) = 45.70 MANNING'S N = 0.013  
 ASSUME FULL-FLOWING PIPELINE  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 6.68  
 PIPE FLOW VELOCITY = (TOTAL FLOW) / (PIPE CROSS SECTION AREA)  
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 11.81  
 PIPE TRAVEL TIME (MIN.) = 0.11 Tc (MIN.) = 11.70  
 LONGEST FLOWPATH FROM NODE 21.00 TO NODE 24.00 = 1157.10 FEET.

END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 7.4 TC (MIN.) = 11.70  
 EFFECTIVE AREA (ACRES) = 5.69 AREA-AVERAGED Fm (INCH/HR) = 0.20  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.678  
 PEAK FLOW RATE (CFS) = 11.81

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	11.81	11.70	2.494	0.30 ( 0.20)	0.68	5.7	22.51
2	10.87	18.91	1.895	0.30 ( 0.22)	0.74	7.2	21.00
3	10.52	20.91	1.788	0.30 ( 0.22)	0.75	7.4	23.10

END OF RATIONAL METHOD ANALYSIS

**Appendix J**  
Proposed Q<sub>25</sub> Hydrology Calculations

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RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE  
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)  
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Ver. 15.0 Release Date: 04/01/2008 License ID 1204

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\*\*\*\*\* DESCRIPTION OF STUDY \*\*\*\*\*  
\* 25-YEAR STORM EVENT HYDROLOGY ANALYSIS \*  
\* OAKCREEK COMMUNITY PARK, IRVINE CALIFORNIA \*  
\* PROPOSED CONDITION \*  
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FILE NAME: OAK25PRA.DAT  
TIME/DATE OF STUDY: 16:23 04/04/2022

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USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

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--\*TIME-OF-CONCENTRATION MODEL\*--

USER SPECIFIED STORM EVENT (YEAR) = 25.00  
SPECIFIED MINIMUM PIPE SIZE (INCH) = 8.00  
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.90  
\*DATA BANK RAINFALL USED\*  
\*ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD\*

\*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL\*

NO.	HALF-WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN-SIDE / OUT-SIDE / PARK-WAY	CURB HEIGHT (FT)	GUTTER WIDTH (FT)	GEOMETRIES: LIP (FT)	HIKE (FT)	MANNING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.50	2.00	0.0313	0.125	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:  
1. Relative Flow-Depth = 0.00 FEET  
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)  
2. (Depth)\*(Velocity) Constraint = 6.0 (FT\*FT/S)  
\*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN  
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.\*  
\*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

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FLOW PROCESS FROM NODE 1.00 TO NODE 2.00 IS CODE = 21

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>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

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INITIAL SUBAREA FLOW-LENGTH (FEET) = 251.00  
ELEVATION DATA: UPSTREAM (FEET) = 157.00 DOWNSTREAM (FEET) = 151.80



$T_c = K * [(LENGTH^{**} 3.00) / (ELEVATION CHANGE)]^{**} 0.20$   
 SUBAREA ANALYSIS USED MINIMUM  $T_c$  (MIN.) = 9.562  
 \* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.342  
 SUBAREA  $T_c$  AND LOSS RATE DATA (AMC II):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN	$T_c$ (MIN.)
PUBLIC PARK	B	0.07	0.30	0.850	56	9.56

 SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$  (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.850  
 SUBAREA RUNOFF (CFS) = 0.19  
 TOTAL AREA (ACRES) = 0.07 PEAK FLOW RATE (CFS) = 0.19

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FLOW PROCESS FROM NODE 2.00 TO NODE 3.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0150  
 FLOW LENGTH (FEET) = 49.40 MANNING'S N = 0.010  
 DEPTH OF FLOW IN 6.0 INCH PIPE IS 2.0 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 3.46  
 GIVEN PIPE DIAMETER (INCH) = 6.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 0.19  
 PIPE TRAVEL TIME (MIN.) = 0.24  $T_c$  (MIN.) = 9.80  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 300.40 FEET.

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FLOW PROCESS FROM NODE 3.00 TO NODE 3.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE  $T_c$  (MIN.) = 9.80  
 \* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.296  
 SUBAREA LOSS RATE DATA (AMC II):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN
PUBLIC PARK	B	0.24	0.30	0.850	56

 SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$  (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.850  
 SUBAREA AREA (ACRES) = 0.24 SUBAREA RUNOFF (CFS) = 0.66  
 EFFECTIVE AREA (ACRES) = 0.31 AREA-AVERAGED  $F_m$  (INCH/HR) = 0.26  
 AREA-AVERAGED  $F_p$  (INCH/HR) = 0.30 AREA-AVERAGED  $A_p$  = 0.85  
 TOTAL AREA (ACRES) = 0.3 PEAK FLOW RATE (CFS) = 0.85

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FLOW PROCESS FROM NODE 3.00 TO NODE 4.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0150  
 FLOW LENGTH (FEET) = 363.00 MANNING'S N = 0.010  
 DEPTH OF FLOW IN 8.0 INCH PIPE IS 3.8 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 5.13  
 GIVEN PIPE DIAMETER (INCH) = 8.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 0.85

PIPE TRAVEL TIME (MIN.) = 1.18      Tc (MIN.) = 10.98  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4.00 = 663.40 FEET.

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 FLOW PROCESS FROM NODE 4.00 TO NODE 4.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 10.98  
 \* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.090  
 SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	2.53	0.30	0.850	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
 SUBAREA AREA (ACRES) = 2.53      SUBAREA RUNOFF (CFS) = 6.46  
 EFFECTIVE AREA (ACRES) = 2.84      AREA-AVERAGED Fm (INCH/HR) = 0.26  
 AREA-AVERAGED Fp (INCH/HR) = 0.30      AREA-AVERAGED Ap = 0.85  
 TOTAL AREA (ACRES) = 2.8      PEAK FLOW RATE (CFS) = 7.25

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 FLOW PROCESS FROM NODE 4.00 TO NODE 5.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0150  
 FLOW LENGTH (FEET) = 97.00      MANNING'S N = 0.010  
 ASSUME FULL-FLOWING PIPELINE  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 20.76  
 PIPE FLOW VELOCITY = (TOTAL FLOW) / (PIPE CROSS SECTION AREA)  
 GIVEN PIPE DIAMETER (INCH) = 8.00      NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 7.25  
 PIPE TRAVEL TIME (MIN.) = 0.08      Tc (MIN.) = 11.06  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 5.00 = 760.40 FEET.

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 FLOW PROCESS FROM NODE 5.00 TO NODE 5.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 11.06  
 \* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.078  
 SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	0.25	0.30	0.850	56

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
 SUBAREA AREA (ACRES) = 0.25      SUBAREA RUNOFF (CFS) = 0.64  
 EFFECTIVE AREA (ACRES) = 3.09      AREA-AVERAGED Fm (INCH/HR) = 0.26  
 AREA-AVERAGED Fp (INCH/HR) = 0.30      AREA-AVERAGED Ap = 0.85  
 TOTAL AREA (ACRES) = 3.1      PEAK FLOW RATE (CFS) = 7.85

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 FLOW PROCESS FROM NODE 5.00 TO NODE 6.00 IS CODE = 41

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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====
REPRESENTATIVE SLOPE = 0.0150
FLOW LENGTH(FEET) = 135.00 MANNING'S N = 0.010
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY(FEET/SEC.) = 22.49
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER(INCH) = 8.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 7.85
PIPE TRAVEL TIME(MIN.) = 0.10 Tc(MIN.) = 11.16
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 6.00 = 895.40 FEET.

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FLOW PROCESS FROM NODE 6.00 TO NODE 6.00 IS CODE = 81
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) = 11.16
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.062
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
PUBLIC PARK B 0.71 0.30 0.850 56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850
SUBAREA AREA(ACRES) = 0.71 SUBAREA RUNOFF(CFS) = 1.79
EFFECTIVE AREA(ACRES) = 3.80 AREA-AVERAGED Fm(INCH/HR) = 0.26
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.85
TOTAL AREA(ACRES) = 3.8 PEAK FLOW RATE(CFS) = 9.60

*****
FLOW PROCESS FROM NODE 6.00 TO NODE 7.00 IS CODE = 41
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<
=====
REPRESENTATIVE SLOPE = 0.0150
FLOW LENGTH(FEET) = 5.00 MANNING'S N = 0.010
ASSUME FULL-FLOWING PIPELINE
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.22
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)
GIVEN PIPE DIAMETER(INCH) = 12.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 9.60
PIPE TRAVEL TIME(MIN.) = 0.01 Tc(MIN.) = 11.16
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 7.00 = 900.40 FEET.

*****
FLOW PROCESS FROM NODE 7.00 TO NODE 7.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) = 11.16
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.061
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS

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LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN
PUBLIC PARK	B	2.01	0.30	0.850	56
SUBAREA AVERAGE PERVIOUS LOSS RATE, $F_p$ (INCH/HR) = 0.30					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p$ = 0.850					
SUBAREA AREA (ACRES) =		2.01	SUBAREA RUNOFF (CFS) =		5.08
EFFECTIVE AREA (ACRES) =		5.81	AREA-AVERAGED $F_m$ (INCH/HR) =		0.26
AREA-AVERAGED $F_p$ (INCH/HR) =		0.30	AREA-AVERAGED $A_p$ =		0.85
TOTAL AREA (ACRES) =		5.8	PEAK FLOW RATE (CFS) =		14.67

\*\*\*\*\*  
FLOW PROCESS FROM NODE 7.00 TO NODE 8.00 IS CODE = 41  
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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0111  
FLOW LENGTH (FEET) = 231.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 24.0 INCH PIPE IS 14.1 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 7.66  
GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 14.67  
PIPE TRAVEL TIME (MIN.) = 0.50  $T_c$  (MIN.) = 11.67  
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 8.00 = 1131.40 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1  
-----

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3  
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
TIME OF CONCENTRATION (MIN.) = 11.67  
RAINFALL INTENSITY (INCH/HR) = 2.99  
AREA-AVERAGED  $F_m$  (INCH/HR) = 0.26  
AREA-AVERAGED  $F_p$  (INCH/HR) = 0.30  
AREA-AVERAGED  $A_p$  = 0.85  
EFFECTIVE STREAM AREA (ACRES) = 5.81  
TOTAL STREAM AREA (ACRES) = 5.81  
PEAK FLOW RATE (CFS) AT CONFLUENCE = 14.67

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FLOW PROCESS FROM NODE 8.10 TO NODE 8.20 IS CODE = 21  
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>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00  
ELEVATION DATA: UPSTREAM (FEET) = 150.00 DOWNSTREAM (FEET) = 144.55

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM  $T_c$  (MIN.) = 10.542

\* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.162

SUBAREA  $T_c$  AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN	$T_c$ (MIN.)
PUBLIC PARK	B	0.76	0.30	0.850	56	10.54
SUBAREA AVERAGE PERVIOUS LOSS RATE, $F_p$ (INCH/HR) = 0.30						

SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p = 0.850$   
SUBAREA RUNOFF (CFS) = 1.99  
TOTAL AREA (ACRES) = 0.76 PEAK FLOW RATE (CFS) = 1.99

\*\*\*\*\*  
FLOW PROCESS FROM NODE 8.20 TO NODE 8.30 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0150  
FLOW LENGTH (FEET) = 167.00 MANNING'S N = 0.010  
ASSUME FULL-FLOWING PIPELINE  
PIPE-FLOW VELOCITY (FEET/SEC.) = 5.70  
PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)  
GIVEN PIPE DIAMETER (INCH) = 8.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 1.99  
PIPE TRAVEL TIME (MIN.) = 0.49  $T_c$  (MIN.) = 11.03  
LONGEST FLOWPATH FROM NODE 8.10 TO NODE 8.30 = 467.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 8.30 TO NODE 8.30 IS CODE = 81

-----  
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

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MAINLINE  $T_c$  (MIN.) = 11.03  
\* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.082  
SUBAREA LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN
COMMERCIAL	B	0.51	0.30	0.100	56

SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$  (INCH/HR) = 0.30  
SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p = 0.100$   
SUBAREA AREA (ACRES) = 0.51 SUBAREA RUNOFF (CFS) = 1.40  
EFFECTIVE AREA (ACRES) = 1.27 AREA-AVERAGED  $F_m$  (INCH/HR) = 0.16  
AREA-AVERAGED  $F_p$  (INCH/HR) = 0.30 AREA-AVERAGED  $A_p = 0.55$   
TOTAL AREA (ACRES) = 1.3 PEAK FLOW RATE (CFS) = 3.33

\*\*\*\*\*  
FLOW PROCESS FROM NODE 8.30 TO NODE 8.00 IS CODE = 41

-----  
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0077  
FLOW LENGTH (FEET) = 37.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 18.0 INCH PIPE IS 7.7 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 4.62  
GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 3.33  
PIPE TRAVEL TIME (MIN.) = 0.13  $T_c$  (MIN.) = 11.16  
LONGEST FLOWPATH FROM NODE 8.10 TO NODE 8.00 = 504.00 FEET.

\*\*\*\*\*  
FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1

-----  
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

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=====
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 11.16
RAINFALL INTENSITY(INCH/HR) = 3.06
AREA-AVERAGED Fm(INCH/HR) = 0.16
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.55
EFFECTIVE STREAM AREA(ACRES) = 1.27
TOTAL STREAM AREA(ACRES) = 1.27
PEAK FLOW RATE(CFS) AT CONFLUENCE = 3.33

*****
FLOW PROCESS FROM NODE 8.40 TO NODE 8.50 IS CODE = 21
=====
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 255.00
ELEVATION DATA: UPSTREAM(FEET) = 147.00 DOWNSTREAM(FEET) = 142.68

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 6.305
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 4.230
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL B 0.45 0.30 0.100 56 6.31
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 1.70
TOTAL AREA(ACRES) = 0.45 PEAK FLOW RATE(CFS) = 1.70

*****
FLOW PROCESS FROM NODE 8.50 TO NODE 8.00 IS CODE = 41
=====
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<
=====
REPRESENTATIVE SLOPE = 0.0557
FLOW LENGTH(FEET) = 37.70 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 3.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.75
GIVEN PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1.70
PIPE TRAVEL TIME(MIN.) = 0.08 Tc(MIN.) = 6.39
LONGEST FLOWPATH FROM NODE 8.40 TO NODE 8.00 = 292.70 FEET.

*****
FLOW PROCESS FROM NODE 8.00 TO NODE 8.00 IS CODE = 1
=====
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 3
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
TIME OF CONCENTRATION(MIN.) = 6.39

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RAINFALL INTENSITY (INCH/HR) = 4.20  
 AREA-AVERAGED Fm (INCH/HR) = 0.03  
 AREA-AVERAGED Fp (INCH/HR) = 0.30  
 AREA-AVERAGED Ap = 0.10  
 EFFECTIVE STREAM AREA (ACRES) = 0.45  
 TOTAL STREAM AREA (ACRES) = 0.45  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 1.70

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	14.67	11.67	2.986	0.30 ( 0.26)	0.85	5.8	1.00
2	3.33	11.16	3.061	0.30 ( 0.16)	0.55	1.3	8.10
3	1.70	6.39	4.200	0.30 ( 0.03)	0.10	0.4	8.40

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 3 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	15.96	6.39	4.200	0.30 ( 0.22)	0.72	4.4	8.40
2	19.00	11.16	3.061	0.30 ( 0.23)	0.75	7.3	8.10
3	19.13	11.67	2.986	0.30 ( 0.23)	0.75	7.5	1.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 19.13 Tc (MIN.) = 11.67  
 EFFECTIVE AREA (ACRES) = 7.53 AREA-AVERAGED Fm (INCH/HR) = 0.23  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.75  
 TOTAL AREA (ACRES) = 7.5  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 8.00 = 1131.40 FEET.

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FLOW PROCESS FROM NODE 8.00 TO NODE 9.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0050  
 FLOW LENGTH (FEET) = 261.00 MANNING'S N = 0.013  
 ASSUME FULL-FLOWING PIPELINE  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 6.09  
 PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)  
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 19.13  
 PIPE TRAVEL TIME (MIN.) = 0.71 Tc (MIN.) = 12.38  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 9.00 = 1392.40 FEET.

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FLOW PROCESS FROM NODE 9.00 TO NODE 9.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc (MIN.) = 12.38  
 \* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 2.887  
 SUBAREA LOSS RATE DATA (AMC II):  
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS

LAND USE	GROUP	(ACRES)	(INCH/HR)	(DECIMAL)	CN
COMMERCIAL	B	0.50	0.30	0.100	56
PUBLIC PARK	B	3.13	0.30	0.850	56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.747					
SUBAREA AREA(ACRES) =		3.63	SUBAREA RUNOFF(CFS) =		8.70
EFFECTIVE AREA(ACRES) =		11.16	AREA-AVERAGED Fm(INCH/HR) =		0.23
AREA-AVERAGED Fp(INCH/HR) =		0.30	AREA-AVERAGED Ap =		0.75
TOTAL AREA(ACRES) =		11.2	PEAK FLOW RATE(CFS) =		26.73

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	26.53	7.24	3.911	0.30( 0.22)	0.73	8.0	8.40
2	26.81	11.88	2.955	0.30( 0.22)	0.75	10.9	8.10
3	26.73	12.38	2.887	0.30( 0.23)	0.75	11.2	1.00

NEW PEAK FLOW DATA ARE:

PEAK FLOW RATE(CFS) = 26.81 Tc(MIN.) = 11.88  
 AREA-AVERAGED Fm(INCH/HR) = 0.22 AREA-AVERAGED Fp(INCH/HR) = 0.30  
 AREA-AVERAGED Ap = 0.75 EFFECTIVE AREA(ACRES) = 10.91

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 9.00 TO NODE 9.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 11.88  
 \* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.955  
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	0.33	0.30	0.850	56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850					
SUBAREA AREA(ACRES) =		0.33	SUBAREA RUNOFF(CFS) =		0.80
EFFECTIVE AREA(ACRES) =		11.24	AREA-AVERAGED Fm(INCH/HR) =		0.23
AREA-AVERAGED Fp(INCH/HR) =		0.30	AREA-AVERAGED Ap =		0.75
TOTAL AREA(ACRES) =		11.5	PEAK FLOW RATE(CFS) =		27.61

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 9.00 TO NODE 9.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 11.88  
 \* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.955  
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	0.22	0.30	0.850	56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850					
SUBAREA AREA(ACRES) =		0.22	SUBAREA RUNOFF(CFS) =		0.53
EFFECTIVE AREA(ACRES) =		11.46	AREA-AVERAGED Fm(INCH/HR) =		0.23
AREA-AVERAGED Fp(INCH/HR) =		0.30	AREA-AVERAGED Ap =		0.75
TOTAL AREA(ACRES) =		11.7	PEAK FLOW RATE(CFS) =		28.14



\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	28.34	7.24	3.911	0.30 ( 0.22)	0.74	8.5	8.40
2	28.14	11.88	2.955	0.30 ( 0.23)	0.75	11.5	8.10
3	28.04	12.38	2.887	0.30 ( 0.23)	0.76	11.7	1.00

NEW PEAK FLOW DATA ARE:

PEAK FLOW RATE (CFS) = 28.34 Tc (MIN.) = 7.24  
 AREA-AVERAGED Fm (INCH/HR) = 0.22 AREA-AVERAGED Fp (INCH/HR) = 0.30  
 AREA-AVERAGED Ap = 0.74 EFFECTIVE AREA (ACRES) = 8.54

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 9.00 TO NODE 10.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0050  
 FLOW LENGTH (FEET) = 45.00 MANNING'S N = 0.013  
 ASSUME FULL-FLOWING PIPELINE  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 9.02  
 PIPE FLOW VELOCITY = (TOTAL FLOW)/(PIPE CROSS SECTION AREA)  
 GIVEN PIPE DIAMETER (INCH) = 24.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 28.34  
 PIPE TRAVEL TIME (MIN.) = 0.08 Tc (MIN.) = 7.33  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 10.00 = 1437.40 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 10.00 TO NODE 11.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0100  
 FLOW LENGTH (FEET) = 43.70 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 36.0 INCH PIPE IS 16.9 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 8.71  
 GIVEN PIPE DIAMETER (INCH) = 36.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 28.34  
 PIPE TRAVEL TIME (MIN.) = 0.08 Tc (MIN.) = 7.41  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 11.00 = 1481.10 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 11.00 TO NODE 12.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

=====

REPRESENTATIVE SLOPE = 0.0100  
 FLOW LENGTH (FEET) = 93.90 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 36.0 INCH PIPE IS 16.9 INCHES  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 8.71  
 GIVEN PIPE DIAMETER (INCH) = 36.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 28.34  
 PIPE TRAVEL TIME (MIN.) = 0.18 Tc (MIN.) = 7.59  
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 12.00 = 1575.00 FEET.

=====

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END OF RATIONAL METHOD ANALYSIS

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*****
FLOW PROCESS FROM NODE      21.00 TO NODE      22.00 IS CODE =  21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) =  314.00
ELEVATION DATA: UPSTREAM(FEET) =  154.00  DOWNSTREAM(FEET) =  148.60

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =  11.799
* 25 YEAR RAINFALL INTENSITY(INCH/HR) =  2.967
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp          Ap      SCS   Tc
LAND USE              GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN   (MIN.)
NATURAL POOR COVER
"BARREN"                B        0.88      0.30      1.000      86   11.80
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  1.000
SUBAREA RUNOFF(CFS) =  2.11
TOTAL AREA(ACRES) =  0.88  PEAK FLOW RATE(CFS) =  2.11

*****
FLOW PROCESS FROM NODE      22.00 TO NODE      22.50 IS CODE =  52
-----
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) =  148.60  DOWNSTREAM(FEET) =  143.90
CHANNEL LENGTH THRU SUBAREA(FEET) =  472.00  CHANNEL SLOPE =  0.0100
CHANNEL FLOW THRU SUBAREA(CFS) =  2.11
FLOW VELOCITY(FEET/SEC) =  1.74 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) =  4.53  Tc(MIN.) =  16.33
LONGEST FLOWPATH FROM NODE      21.00 TO NODE      22.50 =  786.00 FEET.

*****
FLOW PROCESS FROM NODE      22.50 TO NODE      22.50 IS CODE =  81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) =  16.33
* 25 YEAR RAINFALL INTENSITY(INCH/HR) =  2.469
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/      SCS SOIL   AREA      Fp          Ap      SCS
LAND USE              GROUP   (ACRES)  (INCH/HR)  (DECIMAL)  CN
PUBLIC PARK            B        0.48      0.30      0.850      56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.850
SUBAREA AREA(ACRES) =  0.48  SUBAREA RUNOFF(CFS) =  0.96
EFFECTIVE AREA(ACRES) =  1.36  AREA-AVERAGED Fm(INCH/HR) =  0.28
AREA-AVERAGED Fp(INCH/HR) =  0.30  AREA-AVERAGED Ap =  0.95
TOTAL AREA(ACRES) =  1.4  PEAK FLOW RATE(CFS) =  2.67

*****
FLOW PROCESS FROM NODE      22.50 TO NODE      22.50 IS CODE =  1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

```

```

=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 16.33
RAINFALL INTENSITY(INCH/HR) = 2.47
AREA-AVERAGED Fm(INCH/HR) = 0.28
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.95
EFFECTIVE STREAM AREA(ACRES) = 1.36
TOTAL STREAM AREA(ACRES) = 1.36
PEAK FLOW RATE(CFS) AT CONFLUENCE = 2.67

*****
FLOW PROCESS FROM NODE 22.51 TO NODE 22.52 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 283.30
ELEVATION DATA: UPSTREAM(FEET) = 150.40 DOWNSTREAM(FEET) = 147.40

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 7.224
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.917
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL B 0.71 0.30 0.100 56 7.22
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 2.48
TOTAL AREA(ACRES) = 0.71 PEAK FLOW RATE(CFS) = 2.48

*****
FLOW PROCESS FROM NODE 22.52 TO NODE 22.53 IS CODE = 61
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STANDARD CURB SECTION USED)<<<<
=====
REPRESENTATIVE SLOPE = 0.0050
STREET LENGTH(FEET) = 220.70 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 20.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 10.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.77
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.40
HALFSTREET FLOOD WIDTH(FEET) = 13.63
AVERAGE FLOW VELOCITY(FEET/SEC.) = 1.91

```

PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 0.76  
 STREET FLOW TRAVEL TIME(MIN.) = 1.93 Tc(MIN.) = 9.15  
 \* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.426  
 SUBAREA LOSS RATE DATA(AMC II):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.84	0.30	0.100	56

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100  
 SUBAREA AREA(ACRES) = 0.84 SUBAREA RUNOFF(CFS) = 2.57  
 EFFECTIVE AREA(ACRES) = 1.55 AREA-AVERAGED Fm(INCH/HR) = 0.03  
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10  
 TOTAL AREA(ACRES) = 1.5 PEAK FLOW RATE(CFS) = 4.74

END OF SUBAREA STREET FLOW HYDRAULICS:  
 DEPTH(FEET) = 0.43 HALFSTREET FLOOD WIDTH(FEET) = 14.96  
 FLOW VELOCITY(FEET/SEC.) = 2.01 DEPTH\*VELOCITY(FT\*FT/SEC.) = 0.86  
 LONGEST FLOWPATH FROM NODE 22.51 TO NODE 22.53 = 504.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 22.53 TO NODE 22.53 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 9.15  
 \* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.426  
 SUBAREA LOSS RATE DATA(AMC II):  

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	0.15	0.30	0.100	56

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100  
 SUBAREA AREA(ACRES) = 0.15 SUBAREA RUNOFF(CFS) = 0.46  
 EFFECTIVE AREA(ACRES) = 1.70 AREA-AVERAGED Fm(INCH/HR) = 0.03  
 AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.10  
 TOTAL AREA(ACRES) = 1.7 PEAK FLOW RATE(CFS) = 5.20

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 22.53 TO NODE 22.50 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

REPRESENTATIVE SLOPE = 0.0100  
 FLOW LENGTH(FEET) = 50.00 MANNING'S N = 0.013  
 DEPTH OF FLOW IN 15.0 INCH PIPE IS 10.6 INCHES  
 PIPE-FLOW VELOCITY(FEET/SEC.) = 5.60  
 ESTIMATED PIPE DIAMETER(INCH) = 15.00 NUMBER OF PIPES = 1  
 PIPE-FLOW(CFS) = 5.20  
 PIPE TRAVEL TIME(MIN.) = 0.15 Tc(MIN.) = 9.30  
 LONGEST FLOWPATH FROM NODE 22.51 TO NODE 22.50 = 554.00 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 22.50 TO NODE 22.50 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

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=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 9.30
RAINFALL INTENSITY(INCH/HR) = 3.39
AREA-AVERAGED Fm(INCH/HR) = 0.03
AREA-AVERAGED Fp(INCH/HR) = 0.30
AREA-AVERAGED Ap = 0.10
EFFECTIVE STREAM AREA(ACRES) = 1.70
TOTAL STREAM AREA(ACRES) = 1.70
PEAK FLOW RATE(CFS) AT CONFLUENCE = 5.20

```

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	2.67	16.33	2.469	0.30( 0.28)	0.95	1.4	21.00
2	5.20	9.30	3.394	0.30( 0.03)	0.10	1.7	22.51

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	7.36	9.30	3.394	0.30( 0.11)	0.37	2.5	22.51
2	6.44	16.33	2.469	0.30( 0.14)	0.48	3.1	21.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

```

PEAK FLOW RATE(CFS) = 7.36 Tc(MIN.) = 9.30
EFFECTIVE AREA(ACRES) = 2.47 AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.37
TOTAL AREA(ACRES) = 3.1
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 22.50 = 786.00 FEET.

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*****
FLOW PROCESS FROM NODE 22.50 TO NODE 23.00 IS CODE = 52

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>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA<<<<<

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=====
ELEVATION DATA: UPSTREAM(FEET) = 143.90 DOWNSTREAM(FEET) = 139.80
CHANNEL LENGTH THRU SUBAREA(FEET) = 325.40 CHANNEL SLOPE = 0.0126
CHANNEL FLOW THRU SUBAREA(CFS) = 7.36
FLOW VELOCITY(FEET/SEC) = 2.60 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)
TRAVEL TIME(MIN.) = 2.09 Tc(MIN.) = 11.39
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 23.00 = 1111.40 FEET.

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*****
FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81

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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

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=====
MAINLINE Tc(MIN.) = 11.39
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.027
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN

```

PUBLIC PARK B 0.25 0.30 0.850 56  
 SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$ (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 0.850  
 SUBAREA AREA (ACRES) = 0.25 SUBAREA RUNOFF (CFS) = 0.62  
 EFFECTIVE AREA (ACRES) = 2.72 AREA-AVERAGED  $F_m$ (INCH/HR) = 0.12  
 AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30 AREA-AVERAGED  $A_p$  = 0.41  
 TOTAL AREA (ACRES) = 3.3 PEAK FLOW RATE (CFS) = 7.36  
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:  
 TIME OF CONCENTRATION (MIN.) = 11.39  
 RAINFALL INTENSITY (INCH/HR) = 3.03  
 AREA-AVERAGED  $F_m$ (INCH/HR) = 0.12  
 AREA-AVERAGED  $F_p$ (INCH/HR) = 0.30  
 AREA-AVERAGED  $A_p$  = 0.41  
 EFFECTIVE STREAM AREA (ACRES) = 2.72  
 TOTAL STREAM AREA (ACRES) = 3.31  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 7.36

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 23.10 TO NODE 23.20 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 285.00  
 ELEVATION DATA: UPSTREAM (FEET) = 150.80 DOWNSTREAM (FEET) = 147.80

$$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$$

SUBAREA ANALYSIS USED MINIMUM  $T_c$  (MIN.) = 12.521

\* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 2.869

SUBAREA  $T_c$  AND LOSS RATE DATA (AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	$F_p$ (INCH/HR)	$A_p$ (DECIMAL)	SCS CN	$T_c$ (MIN.)
NATURAL POOR COVER						

"BARREN"	B	0.84	0.30	1.000	86	12.52
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SUBAREA AVERAGE PERVIOUS LOSS RATE,  $F_p$ (INCH/HR) = 0.30

SUBAREA AVERAGE PERVIOUS AREA FRACTION,  $A_p$  = 1.000

SUBAREA RUNOFF (CFS) = 1.94

TOTAL AREA (ACRES) = 0.84 PEAK FLOW RATE (CFS) = 1.94

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 23.20 TO NODE 23.00 IS CODE = 52

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 147.80 DOWNSTREAM (FEET) = 139.60  
 CHANNEL LENGTH THRU SUBAREA (FEET) = 815.80 CHANNEL SLOPE = 0.0101  
 CHANNEL FLOW THRU SUBAREA (CFS) = 1.94  
 FLOW VELOCITY (FEET/SEC) = 1.71 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)

TRAVEL TIME (MIN.) = 7.93 Tc (MIN.) = 20.45  
 LONGEST FLOWPATH FROM NODE 23.10 TO NODE 23.00 = 1100.80 FEET.

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc (MIN.) = 20.45  
 \* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 2.173  
 SUBAREA LOSS RATE DATA (AMC II):  
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
 NATURAL POOR COVER  
 "BARREN" B 1.81 0.30 1.000 86  
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 1.000  
 SUBAREA AREA (ACRES) = 1.81 SUBAREA RUNOFF (CFS) = 3.05  
 EFFECTIVE AREA (ACRES) = 2.65 AREA-AVERAGED Fm (INCH/HR) = 0.30  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 1.00  
 TOTAL AREA (ACRES) = 2.6 PEAK FLOW RATE (CFS) = 4.47

\*\*\*\*\*  
 FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<  
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2  
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:  
 TIME OF CONCENTRATION (MIN.) = 20.45  
 RAINFALL INTENSITY (INCH/HR) = 2.17  
 AREA-AVERAGED Fm (INCH/HR) = 0.30  
 AREA-AVERAGED Fp (INCH/HR) = 0.30  
 AREA-AVERAGED Ap = 1.00  
 EFFECTIVE STREAM AREA (ACRES) = 2.65  
 TOTAL STREAM AREA (ACRES) = 2.65  
 PEAK FLOW RATE (CFS) AT CONFLUENCE = 4.47

\*\* CONFLUENCE DATA \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	7.36	11.39	3.027	0.30 ( 0.12)	0.41	2.7	22.51
1	6.44	18.48	2.301	0.30 ( 0.15)	0.50	3.3	21.00
2	4.47	20.45	2.173	0.30 ( 0.30)	1.00	2.6	23.10

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO  
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	10.99	11.39	3.027	0.30 ( 0.19)	0.62	4.2	22.51
2	10.75	18.48	2.301	0.30 ( 0.21)	0.71	5.7	21.00
3	10.52	20.45	2.173	0.30 ( 0.22)	0.72	6.0	23.10

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:



PEAK FLOW RATE (CFS) = 10.99 Tc (MIN.) = 11.39  
 EFFECTIVE AREA (ACRES) = 4.20 AREA-AVERAGED Fm (INCH/HR) = 0.19  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.62  
 TOTAL AREA (ACRES) = 6.0  
 LONGEST FLOWPATH FROM NODE 21.00 TO NODE 23.00 = 1111.40 FEET.

\*\*\*\*\*

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc (MIN.) = 11.39  
 \* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.027  
 SUBAREA LOSS RATE DATA (AMC II):  
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS  
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN  
 PUBLIC PARK B 1.49 0.30 0.850 56  
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.30  
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850  
 SUBAREA AREA (ACRES) = 1.49 SUBAREA RUNOFF (CFS) = 3.72  
 EFFECTIVE AREA (ACRES) = 5.69 AREA-AVERAGED Fm (INCH/HR) = 0.20  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.68  
 TOTAL AREA (ACRES) = 7.4 PEAK FLOW RATE (CFS) = 14.46

\*\*\*\*\*

FLOW PROCESS FROM NODE 23.00 TO NODE 24.00 IS CODE = 41

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
 >>>>USING USER-SPECIFIED PIPESIZE (EXISTING ELEMENT)<<<<<

REPRESENTATIVE SLOPE = 0.0100  
 FLOW LENGTH (FEET) = 45.70 MANNING'S N = 0.013  
 ASSUME FULL-FLOWING PIPELINE  
 PIPE-FLOW VELOCITY (FEET/SEC.) = 8.18  
 PIPE FLOW VELOCITY = (TOTAL FLOW) / (PIPE CROSS SECTION AREA)  
 GIVEN PIPE DIAMETER (INCH) = 18.00 NUMBER OF PIPES = 1  
 PIPE-FLOW (CFS) = 14.46  
 PIPE TRAVEL TIME (MIN.) = 0.09 Tc (MIN.) = 11.48  
 LONGEST FLOWPATH FROM NODE 21.00 TO NODE 24.00 = 1157.10 FEET.

END OF STUDY SUMMARY:

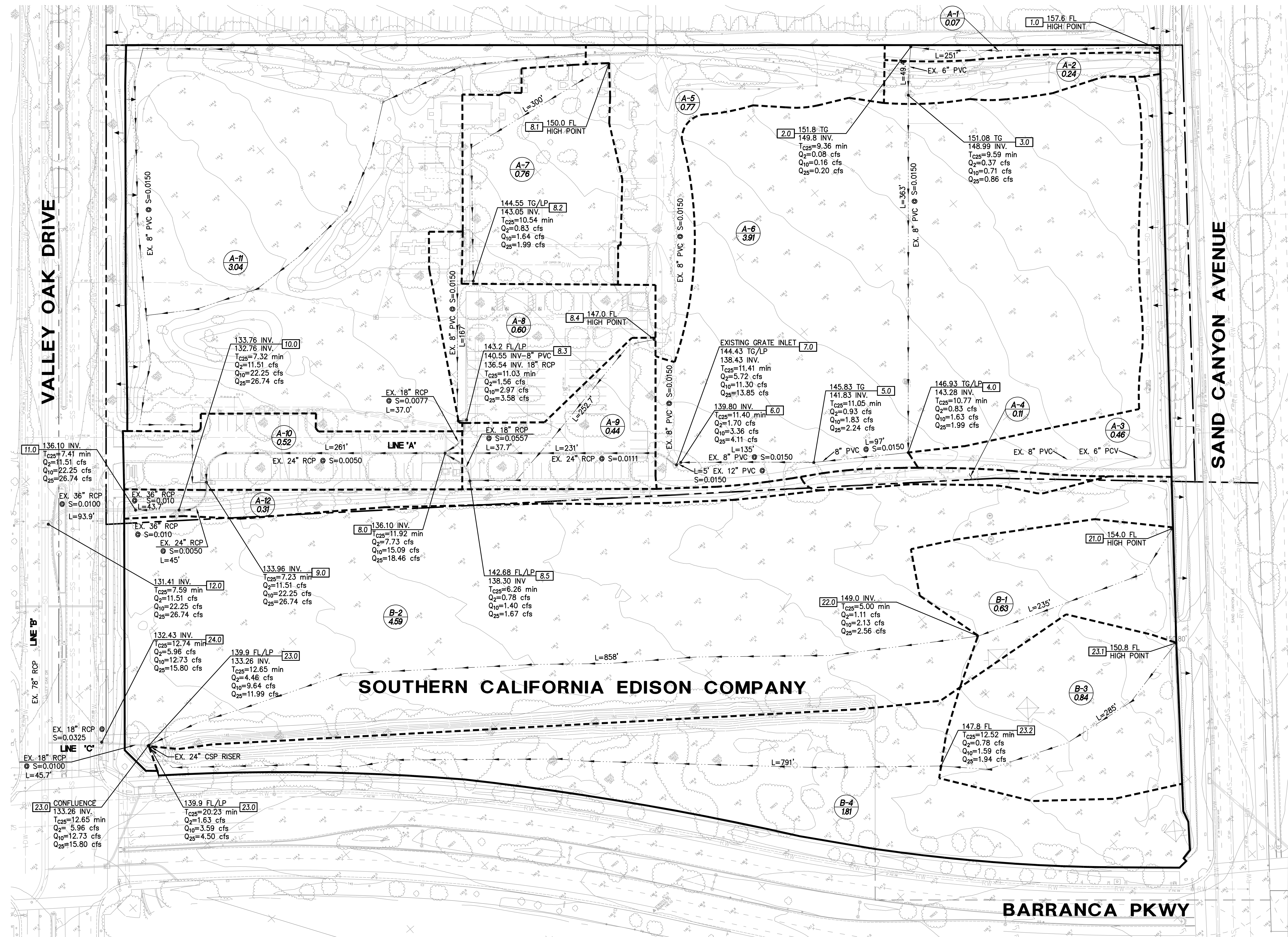
TOTAL AREA (ACRES) = 7.4 TC (MIN.) = 11.48  
 EFFECTIVE AREA (ACRES) = 5.69 AREA-AVERAGED Fm (INCH/HR) = 0.20  
 AREA-AVERAGED Fp (INCH/HR) = 0.30 AREA-AVERAGED Ap = 0.678  
 PEAK FLOW RATE (CFS) = 14.46

\*\* PEAK FLOW RATE TABLE \*\*

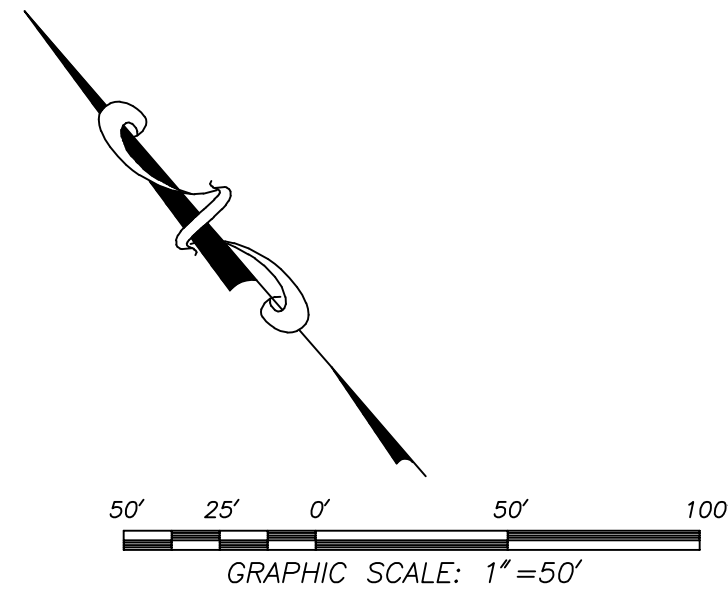
STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp (Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	14.46	11.48	3.013	0.30 ( 0.20)	0.68	5.7	22.51
2	13.46	18.58	2.294	0.30 ( 0.22)	0.74	7.2	21.00
3	13.06	20.55	2.167	0.30 ( 0.22)	0.75	7.4	23.10

END OF RATIONAL METHOD ANALYSIS

**Appendix K**  
Hydrology Map - Existing Condition



- LEGEND:**
- FLOW PATH
  - BOUNDARY LINE
  - 1520 CONTOURS
  - DRAINAGE AREA BOUNDARY
  - DRAINAGE SUB-AREA BOUNDARY
  - FLOW PROCESS NODE
  - ELEVATION
  - 10.0 100.0 ELEV.
  - T<sub>c</sub> = 3.00 min — TIME OF CONCENTRATION
  - Q<sub>2</sub> = 0.1 cfs — PEAK FLOW RATE FOR 10 YEAR STORM
  - Q<sub>10</sub> = 0.1 cfs — PEAK FLOW RATE FOR 25 YEAR STORM
  - Q<sub>25</sub> = 0.2 cfs — PEAK FLOW RATE FOR 100 YEAR STORM
  - A-1 1.0 — DRAINAGE SUB-AREA DESIGNATION
  - DRAINAGE SUB-AREA ACREAGE



CITY OF IRVINE  
OAK CREEK COMMUNITY PARK

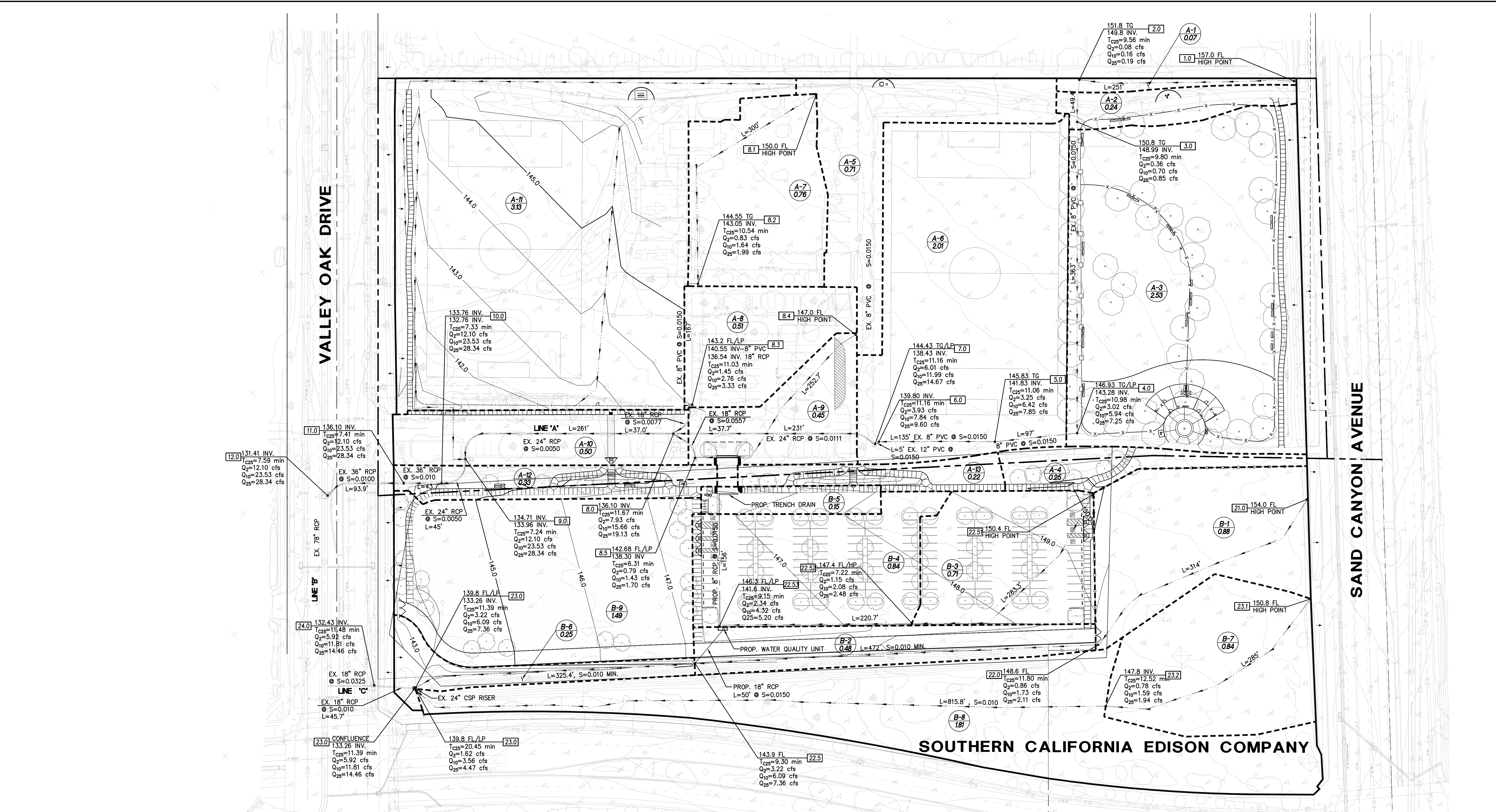
**PRELIMINARY HYDROLOGY MAP  
EXISTING CONDITION**

JOB NO.: 19-2260  
DATE: 04/05/2022  
DRAWN BY: GC  
DESIGNED BY: GC  
APPROVED BY: MO

SHEET  
1 OF 1

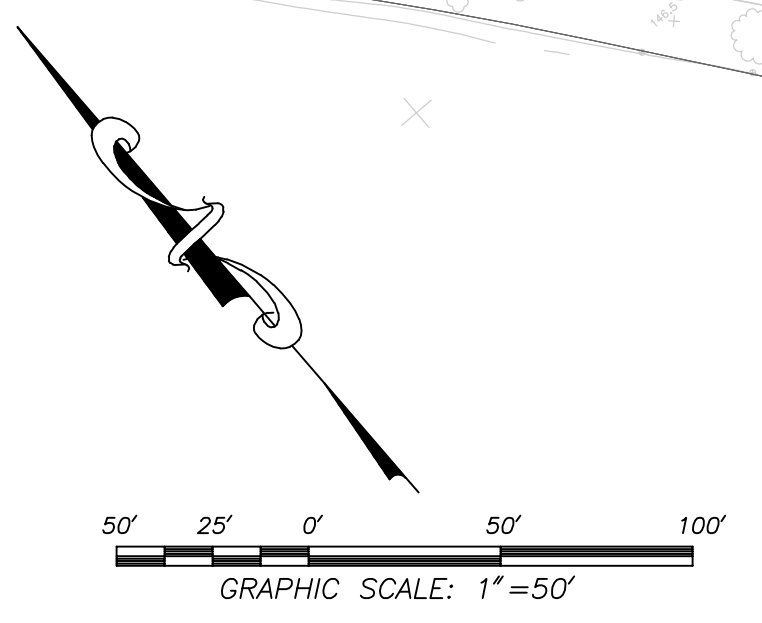
**Appendix L**  
Hydrology Map – Proposed Condition





**LEGEND:**

- FLOW PATH
- - - BOUNDARY LINE
- 1520 CONTOURS
- DRAINAGE AREA BOUNDARY
- DRAINAGE SUB-AREA BOUNDARY
- FLOW PROCESS NODE
- 100.0 ELEV. ELEVATION
- $T_c = 3.00$  min TIME OF CONCENTRATION
- $Q_{10} = 0.1$  cfs PEAK FLOW RATE FOR 10 YEAR STORM
- $Q_{25} = 0.1$  cfs PEAK FLOW RATE FOR 25 YEAR STORM
- $Q_{100} = 0.2$  cfs PEAK FLOW RATE FOR 100 YEAR STORM
- A-1 DRAINAGE SUB-AREA DESIGNATION
- 10 DRAINAGE SUB-AREA ACREAGE



**BARRANCA PKWY**

**SOUTHERN CALIFORNIA EDISON COMPANY**

**CITY OF IRVINE  
OAK CREEK COMMUNITY PARK**

**PRELIMINARY HYDROLOGY MAP  
PROPOSED CONDITION**

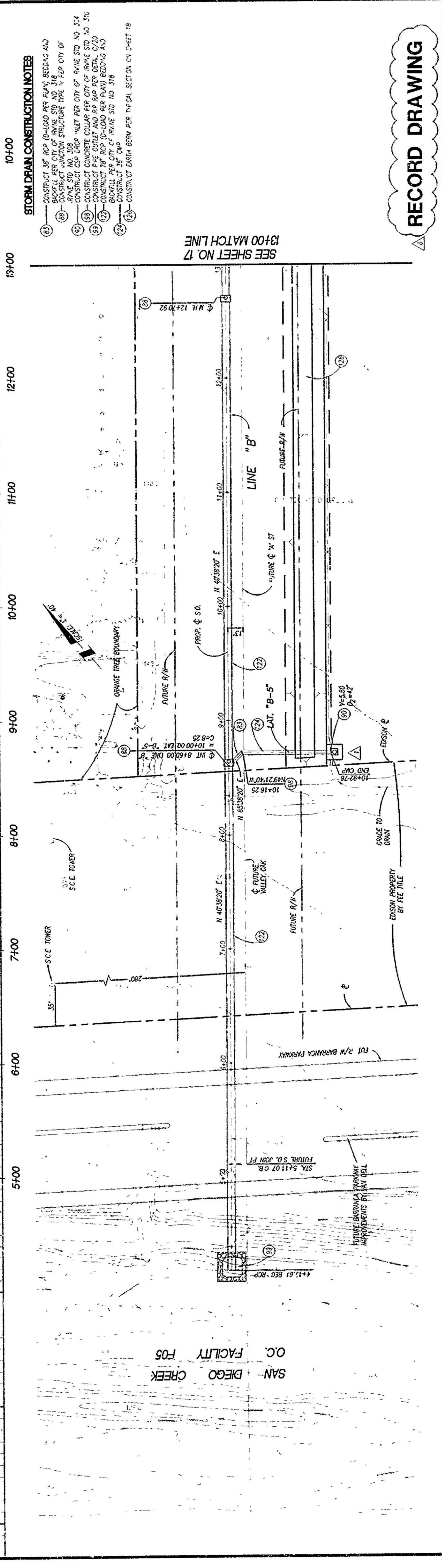
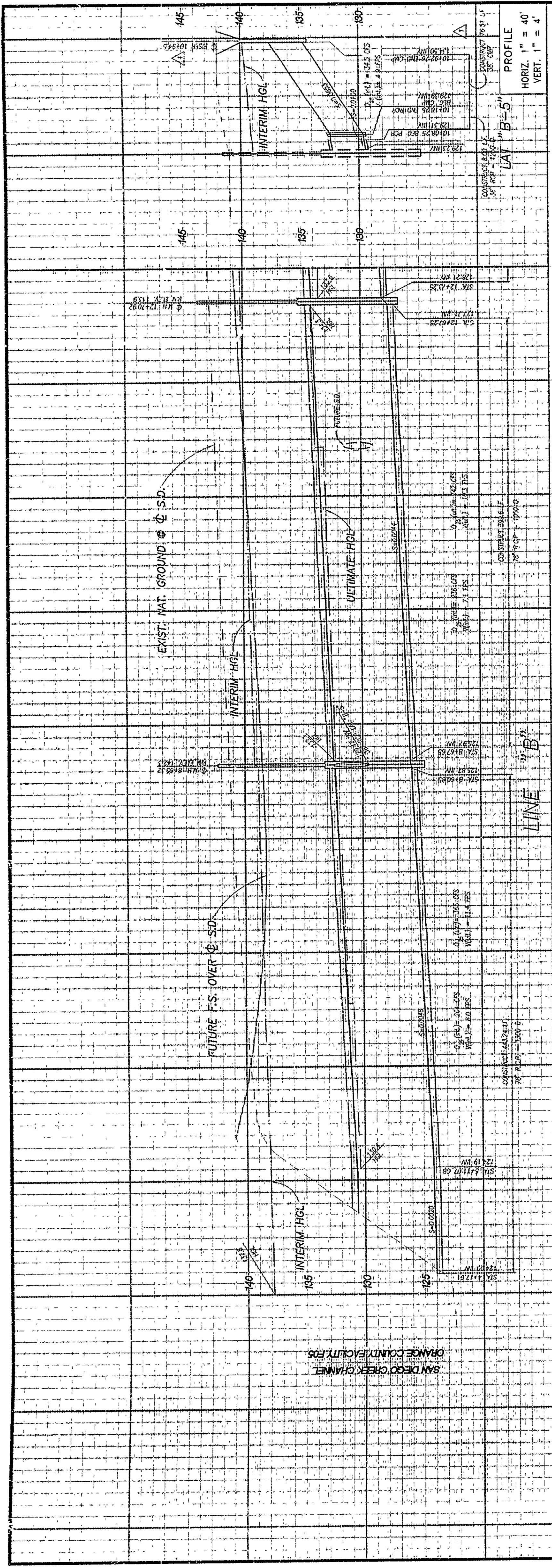
JOB NO.:	19-2260
DATE:	04/05/2022
DRAWN BY:	GC
DESIGNED BY:	GC
APPROVED BY:	MO

SHEET  
**1 OF 1**

**Appendix M**

Record Drawing Permit No. 30010-TC Storm Drain  
Improvement Line "B" sheet 16 of 40.





NO.	DATE	REVISIONS
1		
2		
3		
4		
5		
6		

CITY OF IRVINE	DATE
APP	DATE

<b>THE KEITH COMPANIES</b> Civil Engineering • Land Surveying Mapping • Environmental Services Water Resources • Land Planning	2855 Reg. H. Avenue Costa Mesa, CA 92626 (714) 440-1800
---	---

<b>THE IRVINE COMPANY</b> 5500 W. BEACH, CLACKAMASH 97058 (503) 720-2200	DEVELOPER: THE IRVINE COMPANY DATE APPROVED: 12/26/20 LAST REVISED: 1/21/98
--	---

<b>GOFFMAN MCCORMICK AND URBAN, INC.</b> 23241 WILSON AVENUE, SUITE 200 RANCHO SANTA ANITA, CA 92688 (714) 888-6513	SOILS ENGINEER: GOFFMAN MCCORMICK AND URBAN, INC. DATE APPROVED: 12/26/20 LAST REVISED: 1/21/98
--	---

STREET IMPROVEMENT PLAN - PHASE 1 STORM DRAIN - LINE "B" FROM STA. 4+17.61 TO STA. 13+00.00 CITY OF IRVINE COMMUNITY DEVELOPMENT DEPARTMENT	PERMIT NO. 30010-7C SHEET 16 OF 40 JOB NO. 12962.000
---	--

STORM DRAIN CONSTRUCTION NOTES

- CONSTRUCT 36" RCP (0-LOAD PER PLAN) BEDDING AND BACKFILL PER SECTION STRUCTURE TYPE IN PER CITY OF IRVINE STD. NO. 308
- CONSTRUCT CONCRETE COLLAR PER CITY OF IRVINE STD. NO. 314
- CONSTRUCT P.P.E. OUTLET AND R.P. PER DETAIL C/20
- CONSTRUCT 78" RCP (0-LOAD PER PLAN) BEDDING AND BACKFILL PER CITY OF IRVINE STD. NO. 318
- CONSTRUCT 36" RCP
- CONSTRUCT EARTH BERM PER TYPICAL SECTION ON SHEET 18

SEE SHEET NO. 17 @ 13+00 MATCH LINE

RECORD DRAWING

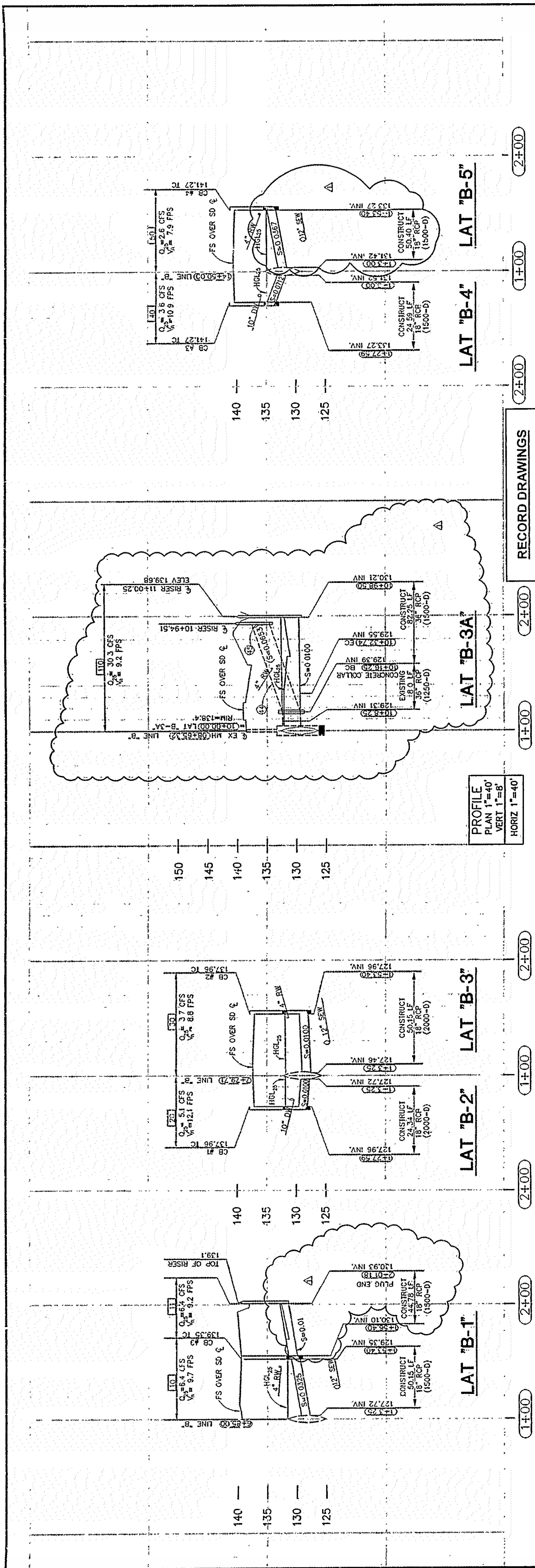
PLAN 887



**Appendix N**

Record Drawing Project No. 34071-MC Valley Oak Plan  
And Profile Storm Drain Improvement Plan for  
Line "B" Laterals sheet 7 of 15.



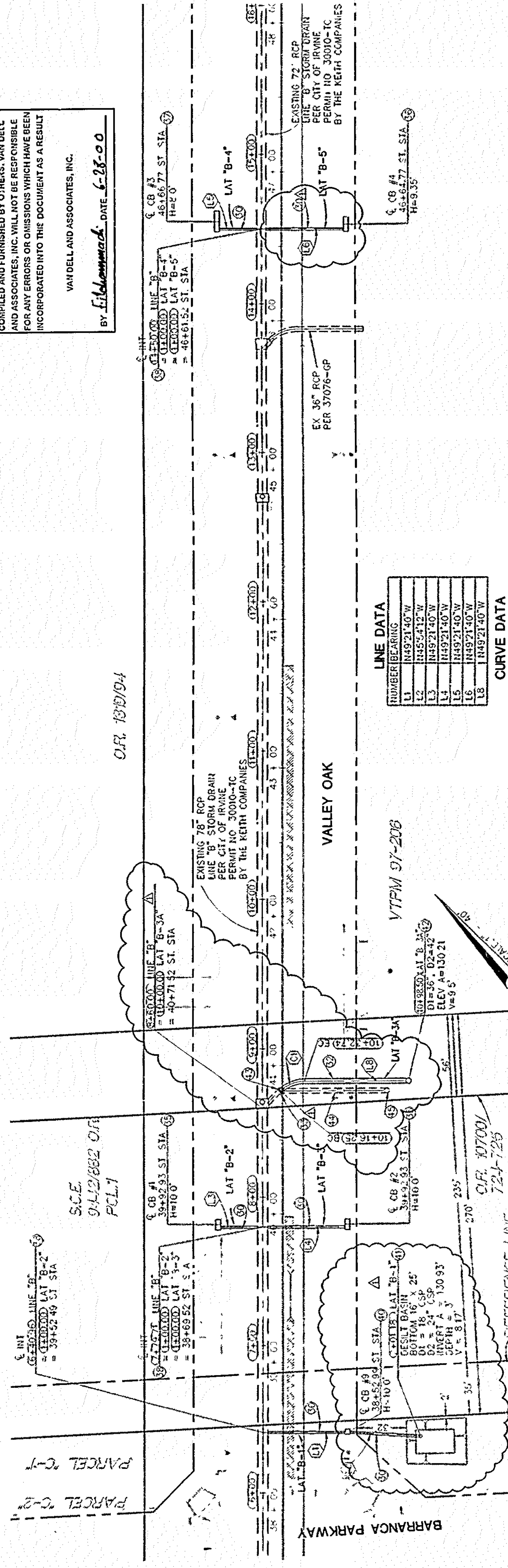


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VAN DELL AND ASSOCIATES, INC.  
 BY: *[Signature]* DATE: 6-28-00

- CONSTRUCTION NOTES:**
- 50 CONSTRUCT 18" RCP (0-LOAD PER PROFILE) WITH BEDDING AND BACKFILL PER CITY OF IRVINE STANDARD PLAN NO. 318.
  - 51 CONSTRUCT 30" RCP (0-LOAD PER PROFILE) WITH BEDDING AND BACKFILL PER CITY OF IRVINE STANDARD PLAN NO. 318.
  - 52 CONSTRUCT 36" RCP (0-LOAD PER PROFILE) WITH BEDDING AND BACKFILL PER CITY OF IRVINE STANDARD PLAN NO. 318.
  - 53 REMOVE EXISTING CONCRETE COLLAR.
  - 54 CONSTRUCT 7" CATCH BASIN TYPE I PER CITY OF IRVINE STANDARD PLAN NO. 301.
  - 55 CONSTRUCT 10" CATCH BASIN TYPE II PER CITY OF IRVINE STANDARD PLAN NO. 301.
  - 56 CONSTRUCT 14" CATCH BASIN TYPE II PER CITY OF IRVINE STANDARD PLAN NO. 301.
  - 57 CONSTRUCT JUNCTION STRUCTURE TYPE V PER CITY OF IRVINE STANDARD PLAN NO. 308B.
  - 58 CONSTRUCT 3.5' CATCH BASIN TYPE I PER CITY OF IRVINE STANDARD PLAN NO. 300.
  - 59 CONSTRUCT 2.4' RISER PER CITY OF IRVINE STANDARD PLAN NO. 304.
  - 60 CONSTRUCT 2' RISER PER CITY OF IRVINE STANDARD PLAN NO. 304.
  - 61 CONSTRUCT CONCRETE COLLAR PER CITY OF IRVINE STANDARD PLAN NO. 310.
  - 62 REMOVE EXISTING 36" CHD PIPE.
  - 63 REMOVE EXISTING RISER.



**LINE DATA**

NUMBER	BEARING	RADIUS	LENGTH	TANGENT
L1	N49°21'40" W		21.00	8.70
L2	S45°41'2" W		16.49	8.70
L3	N49°21'40" W		16.49	8.70
L4	N49°21'40" W		16.49	8.70
L5	S45°41'2" W		16.49	8.70
L6	N49°21'40" W		16.49	8.70
L7	N49°21'40" W		16.49	8.70

**CURVE DATA**

NUMBER	DELTA	RADIUS	LENGTH	TANGENT
CT	45°00'00"	21.00	16.49	8.70

IRVINE COMMUNITY DEVELOPMENT COMPANY  
 PREPARED FOR: *[Signature]*

REGISTERED PROFESSIONAL ENGINEER  
 STATE OF CALIFORNIA  
 No. 51754  
 Exp. 12/31/04

PLAN AND PROFILE STORM DRAIN IMPROVEMENT PLANS FOR:  
**VALLEY OAK**  
 LINE "B" LATERALS  
 VTPM 97-206

CITY OF IRVINE  
 PUBLIC WORKS DEPARTMENT

PROJECT NO. 34071-MC  
 SHEET 7 OF 15

DATE: 8-5-99  
 REVISION: 8-5-99  
 CHECKED BY: J.Y.  
 UNDER THE SUPERVISION OF: *[Signature]*  
 DATE: 8-5-99  
 DATE: 8-5-99

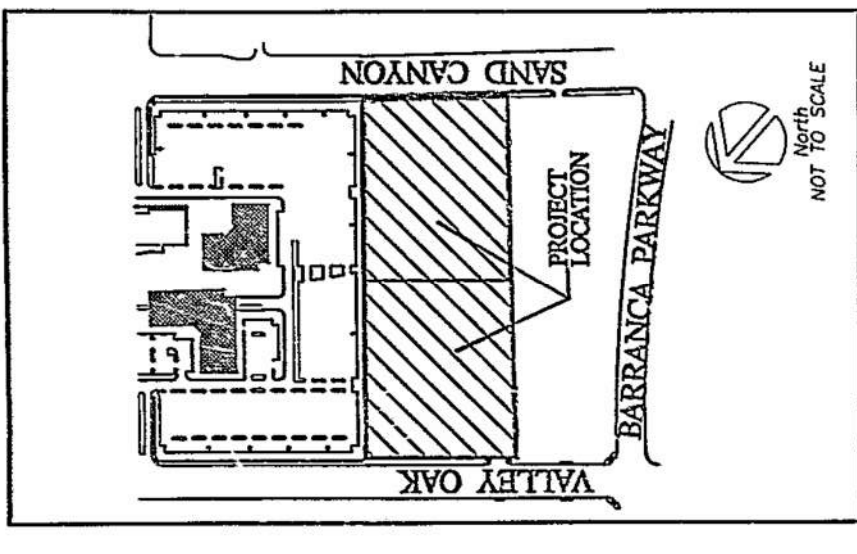
DATE: 8-5-99

**Appendix O**

Record Drawing Grading Permit No. 4319-CCG  
Sheet No. CE-09 and CE-10.



Landscapes Architecture  
Planning  
Urban Design  
Environmental Analysis  
Site Engineering  
Graphic Design  
17875 Von Karman Ave., #400  
Irvine, California 92614  
TEL: (949) 680-8044  
FAX: (949) 680-1046



IRVINE COMMUNITY DEVELOPMENT COMPANY  
530 NEWPORT CENTER DRIVE  
NEWPORT BEACH, CALIFORNIA 92660  
TEL: (949) 707-2065 FAX: (949) 707-2065  
TOM HEGGI, VICE PRESIDENT

Table with 2 columns: No., Date, Revision. Multiple empty rows for revisions.



OAKCREEK PARK  
STORM DRAIN PLAN  
LINE "A"

PHASE ONE  
LOT 259  
ACCESSOR PARCEL NO. 97206

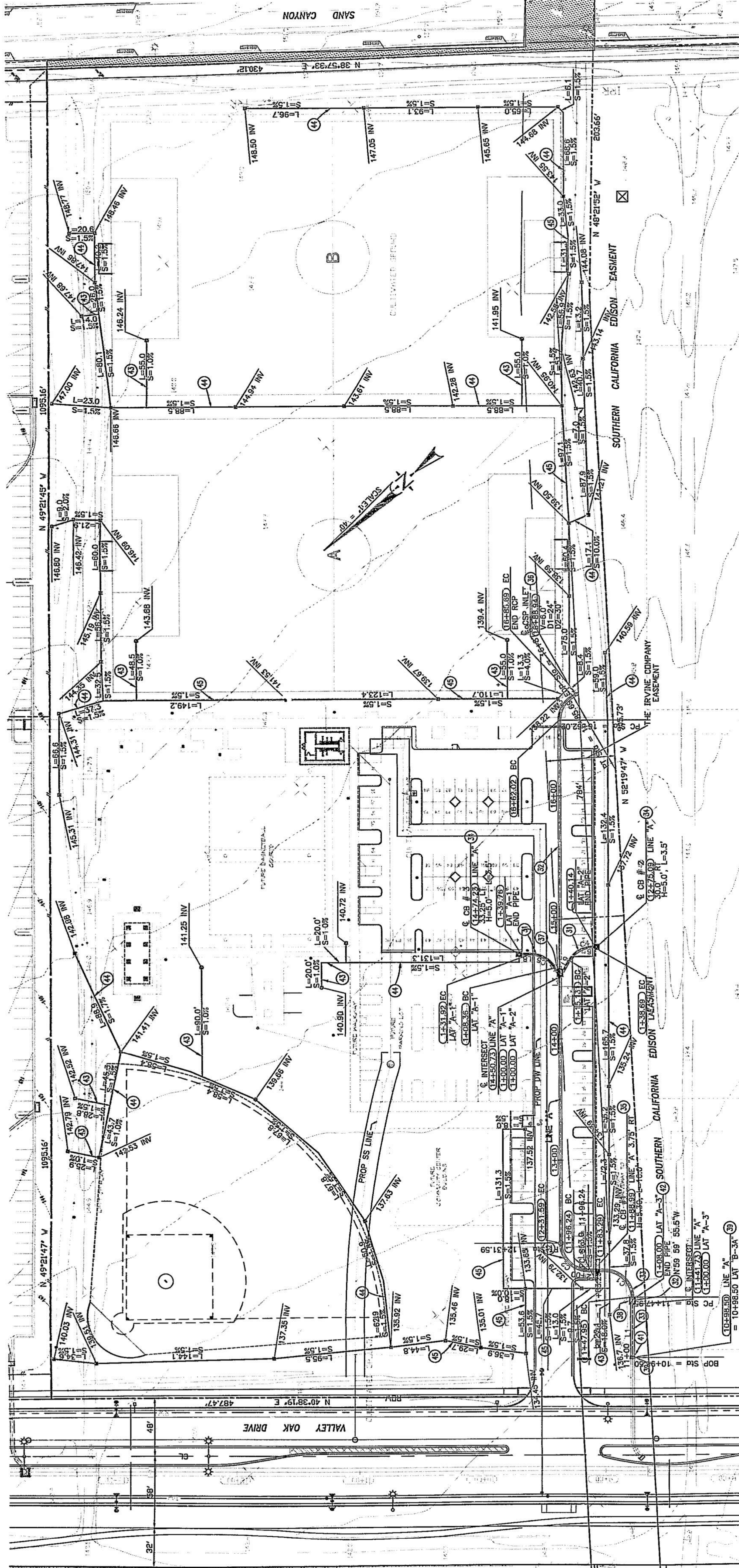
OAKCREEK COMMUNITY PARK

DEVELOPMENT CASE NO. 47070-PA  
PROJECT NO.  
Scale: 1" = 40'  
Date: 01/07/09  
Drawn By: FM  
Checked By: DRG  
Sheet No: 9 OF 13

CE-09

GRADING PERMIT NO. 4319-003  
CADD File N .

PLANS PREPARED BY:  
VAN DELL AND ASSOCIATES, INC.  
LAND PLANNED  
DESIGNED  
CONSTRUCTED  
BY THE CITY OF IRVINE  
UNDER THE SUPERVISION OF:  
DONALD R. GORDON, R.C.E. 0242725, DATE

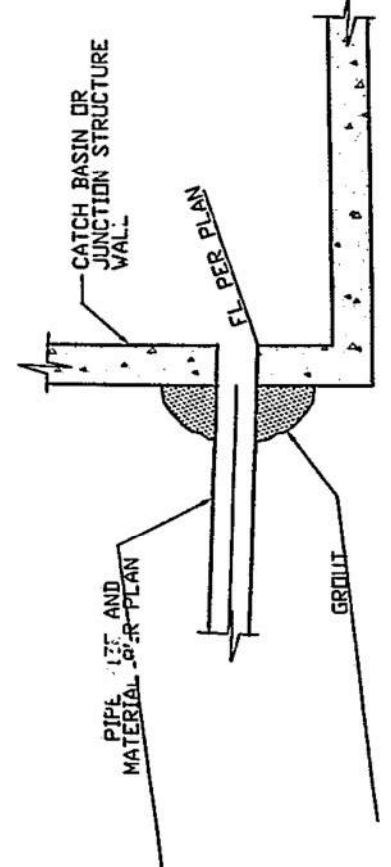


CONSTRUCTION NOTES:

- 1. CONSTRUCT 18" RCP 0-LOAD PER PROFILED WITH BEDDING AND BACKFILL PER CITY OF IRVINE STD. PLAN NO. 318.
- 2. CONSTRUCT 24" RCP 0-LOAD PER PROFILED WITH BEDDING AND BACKFILL PER CITY OF IRVINE STD. PLAN NO. 318.
- 3. CONSTRUCT 36" RCP 0-LOAD PER PROFILED WITH BEDDING AND BACKFILL PER CITY OF IRVINE STD. PLAN NO. 318.
- 4. CONSTRUCT 36" CATCH BASIN TYPE I PER CITY OF IRVINE STD. PLAN NO. 300.
- 5. CONSTRUCT 18" CATCH BASIN TYPE II PER CITY OF IRVINE STD. PLAN NO. 300.
- 6. CONSTRUCT BROOKS BOX 3636 CB OR EQUIVALENT PER CITY OF IRVINE STD. PLAN NO. 306.
- 7. CONSTRUCT JUNCTION STRUCTURE TYPE I PER CITY OF IRVINE STD. PLAN NO. 306.
- 8. CONSTRUCT JUNCTION STRUCTURE TYPE II PER CITY OF IRVINE STD. PLAN NO. 306.
- 9. CONSTRUCT CONCRETE COLLAR PER CITY OF IRVINE STD. PLAN NO. 310.
- 10. CONSTRUCT BRUNPIPE TO CATCH BASIN/JUNCTION STRUCTURE CONNECTION PER DETAIL A.
- 11. REMOVE CSP DROP INLET.
- 12. CONSTRUCT 4" PVC PER COI STD PLAN NO. 318.
- 13. CONSTRUCT 6" PVC PER COI STD PLAN NO. 318.
- 14. CONSTRUCT 8" PVC PER COI STD PLAN NO. 318.
- 15. CONSTRUCT 12" PVC PER COI STD PLAN NO. 318.

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VAN DELL AND ASSOCIATES, INC.  
DATE 02/16/09

Table with 2 columns: NUMBER, DIRECTION, DISTANCE. Lists various line segments and their lengths.



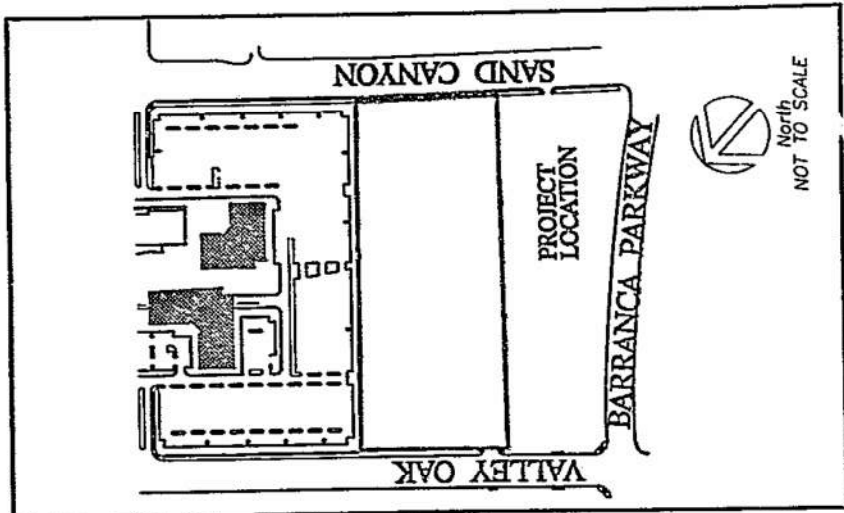
SEE SHEET CE-10 FOR STORM DRAIN PROFILES



# EDAW

Landscape Architecture  
 Planning  
 Urban Design  
 Environmental Analysis  
 Site Engineering  
 Graphic Design

17875 Von Karman Ave., #400  
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IRVINE COMMUNITY  
 DEVELOPMENT COMPANY  
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 NEWPORT BEACH, CALIFORNIA 92660  
 TEL: (949) 79-2656 FAX: (949) 79-2828  
 TOM HEGGL, VICE PRESIDENT

No.	Date	Revision



OAKCREEK  
 PARK

STORM DRAIN  
 PROFILE

DEVELOPMENT CASE NO. 47070-PA

PHASE ONE

LOT 259

ACCESSOR PARCEL NO. 97206

OAKCREEK  
 COMMUNITY PARK

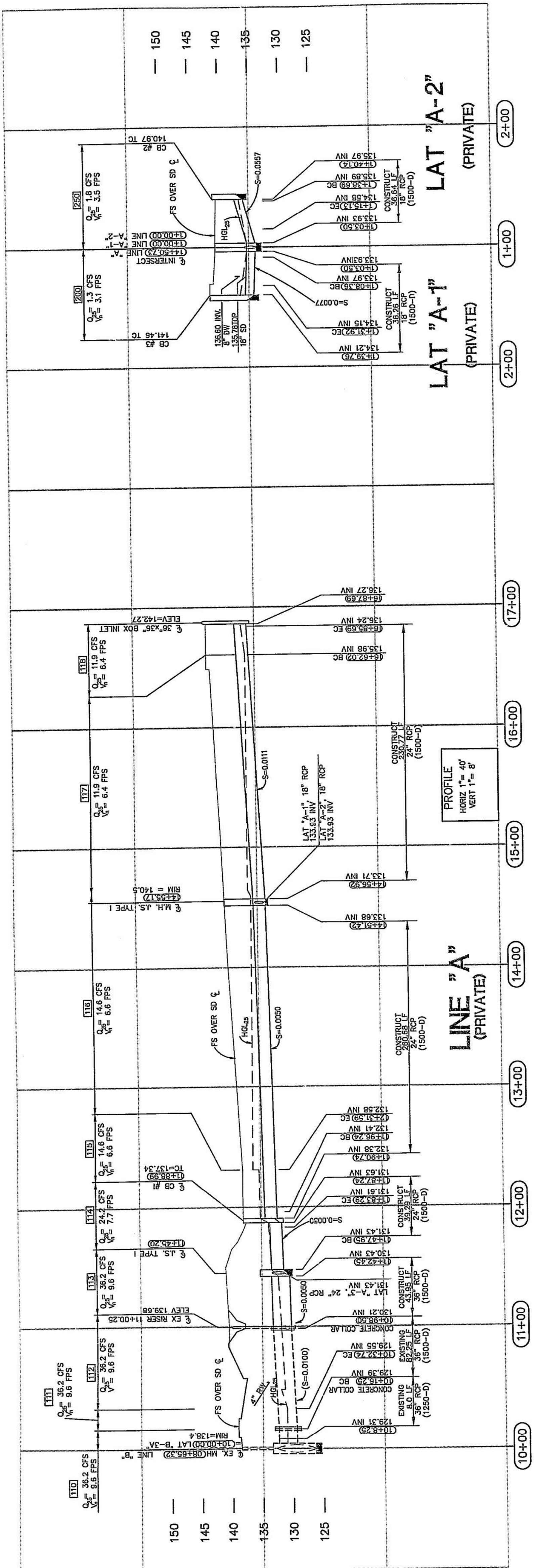
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 Project No. \_\_\_\_\_  
 Section \_\_\_\_\_  
 Date: \_\_\_\_\_  
 Drawn By: DWG  
 Checked By: DWG  
 Sheet No. 10 OF 13

CE-10

GRADING PERMIT NO. \_\_\_\_\_

CADD File No. \_\_\_\_\_

PLANS PREPARED BY:  
 VAN BELL AND ASSOCIATES, INC.  
 17875 VON KARMAN AVENUE, #400  
 IRVINE, CALIFORNIA 92614  
 TEL: (949) 660-8044  
 FAX: (949) 660-1046  
 UNDER THE SUPERVISION OF:  
 DONALD R. GORBOON R.C.E. 024723 DATE \_\_\_\_\_



AS-GRADED

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VAN BELL AND ASSOCIATES, INC.  
 BY: \_\_\_\_\_ DATE: \_\_\_\_\_