

TRAFFIC STUDY

MAPES AND TRUMBLE INDUSTRIAL FACILITY PROJECT (PLN22-05023)

CITY OF PERRIS

RIVERSIDE COUNTY, CALIFORNIA

This Traffic Study has been prepared under the supervision of
Ambarish Mukherjee, P.E.



LSA

December 2022

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CITY OF PERRIS

RIVERSIDE COUNTY, CALIFORNIA

Prepared for:

City of Perris
101 N D Street
Perris, California 92570

Prepared by:

LSA Associates, Inc.
1500 Iowa Avenue, Suite 200
Riverside, California 92507
(951) 781-9310

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1.0 INTRODUCTION

This Traffic Study (TS) has been prepared to assess the potential circulation impacts associated with the proposed Mapes Road and Trumble Road Industrial Facility Project to be located at the southwest corner of Mapes Road and Trumble Road, in the City of Perris (City). The project site is bounded by Mapes Road to the north, Exceed Road a commercial development with undeveloped property to the south, Interstate 215 (I-215) to the west, and Trumble Road to the east.

The site is currently undeveloped and the proposed project will add one high cube fulfillment center warehouse building with a total area of 395,500 square feet (SF). The project opening year is anticipated to be 2024. Figure 1-1 illustrates the regional and project location. (Figures and tables are located at the end of each chapter.)

The City has adopted the *Draft City of Perris Transportation Impact Analysis Guidelines for CEQA*, dated May 2020. However, this guideline does not provide guidance for Levels of Service (LOS) Analysis for General Plan consistency. Therefore, as recommended by City staff, the LOS analysis for the project was prepared in accordance with the *City of Perris LOS Standards and Threshold of Significance*. The scope of work for this TS, including trip generation, trip distribution, study area, and analysis methodologies, has been approved by City staff via the Scoping Agreement process. A copy of the approved Scoping Agreement is included as Appendix A. (Appendices are attached at the end of the TS.)

This study examines traffic operations in the vicinity of the proposed project under the following four scenarios:

- Existing Conditions;
- Existing with Project Conditions;
- Cumulative (2024) without Project Conditions; and
- Cumulative (2024) with Project Conditions.

Traffic conditions in the study area were examined for weekday a.m. and p.m. peak hour conditions. The a.m. peak hour is defined as the one hour of highest traffic volumes occurring between 7:00 and 9:00 a.m. The p.m. peak hour is the one hour of highest traffic volumes occurring between 4:00 and 6:00 p.m.

1.1 PROJECT DESCRIPTION

As previously mentioned, the proposed project will add one high cube fulfillment center warehouse building with a total area of 395,500 SF. Figure 1-2 illustrates a conceptual site plan for the proposed project. As illustrated in Figure 1-2, access to the project will be provided via three driveways. Following are detailed access description of each driveway:

- Driveway 1 – This driveway located on Mapes Road will be used by only passenger vehicles.
- Driveway 2 – This driveway located on Trumble Road will be used by only passenger vehicles.

- Driveway 3 – This driveway located on Exceed Road will be used by only trucks.

All three project driveways will operate as full access driveways. The project will also be improving the project frontage by adding curb, gutter and sidewalk along the project frontage.

It should be noted that though the total area of the building is 395,500 SF, the traffic analysis was conducted using 396,000 SF as a conservative estimate and rounding up to the nearest thousand square feet (TSF).

Additionally, the project proposes to include 50 percent High-Cube Cold Storage Warehouse as an alternative. However, peak hour trip generation rate for High-Cube Cold Storage Warehouse is lower than corresponding trip generation rates for High Cube Fulfillment Center Warehouse – Non-Sort land use. Since this study evaluates the project’s effect on adjacent circulation network under a.m. and p.m. peak hours, the project was considered as 100 percent High Cube Fulfillment Center Warehouse – Non-Sort as a conservative approach.

1.2 STUDY AREA

The study area was approved by City staff via the City’s scoping agreement process (Appendix A). Based on discussion with City staff, the study area includes the following intersections:

1.2.1 Study Intersections

The following intersections are analyzed in this study:

1. Interstate 215 (I-215) Southbound Ramps/Bonnie Drive (Perris/Caltrans);
2. I-215 Northbound Ramps/California State Route 74 (CA-74) (Perris/Menifee/Caltrans);
3. Trumble Road/Mapes Road (Perris/Menifee);
4. Trumble Road/Exceed Road-Sturgeon Electric Driveway (Perris/Menifee);
5. Trumble Road/CA-74 (Menifee/Caltrans);
6. Project Driveway 1/Mapes Road (Perris); and
7. Project Driveway 2/Trumble Road (Perris/Menifee).

Figure 1-3 illustrates the study area intersections.

1.3 LIST OF CHAPTER 1.0 FIGURES

- Figure 1-1: Regional and Project Location
- Figure 1-2: Conceptual Site Plan
- Figure 1-3: Study Area Intersections

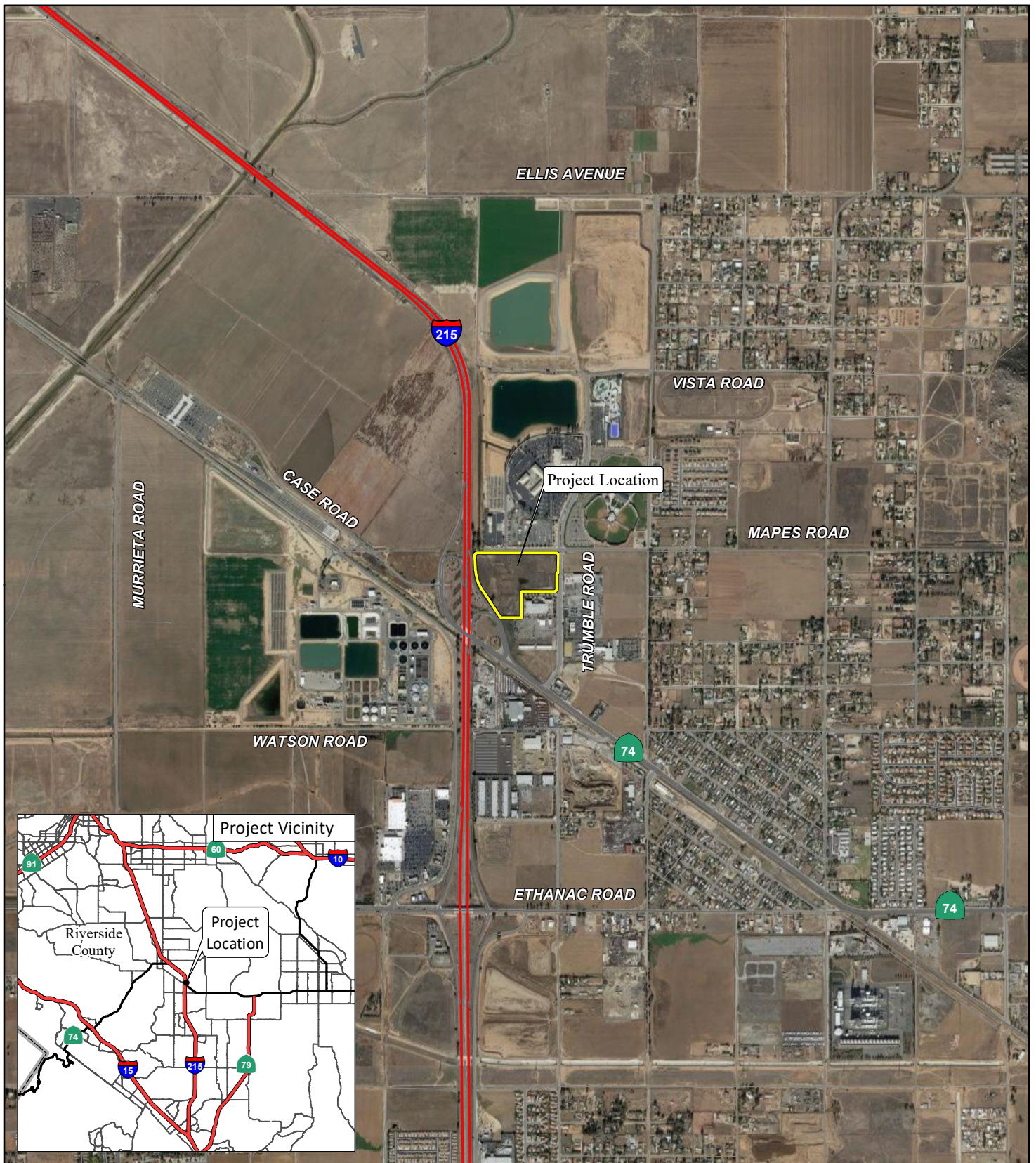
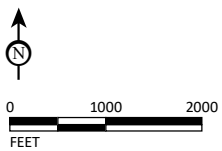


FIGURE 1-1

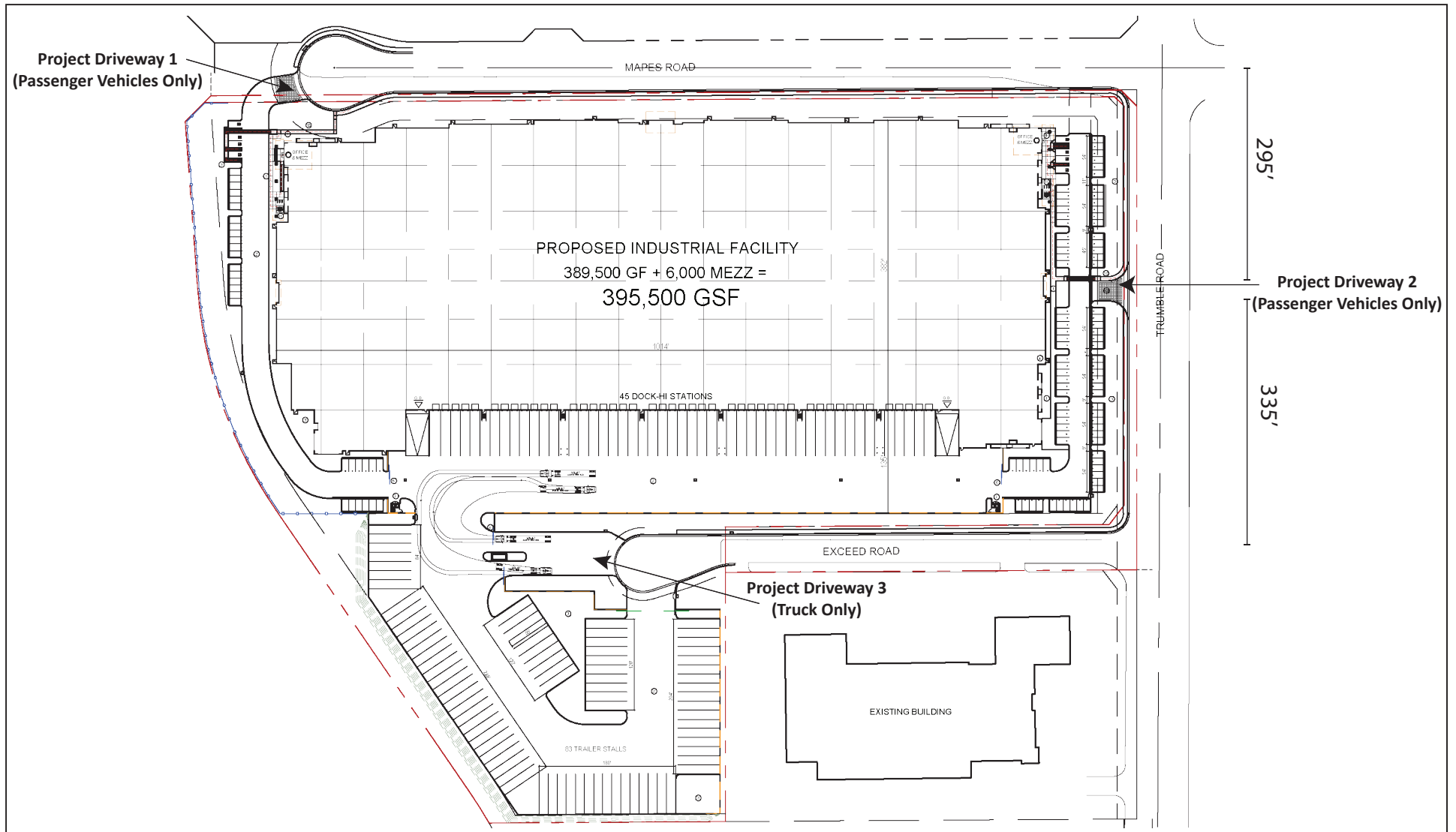
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SOURCE: ESRI Streetmap, 2021; Google Earth, 2019.

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Mapes and Trumble Industrial Facility Project
 Traffic Study
 Regional and Project Location



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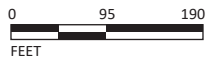


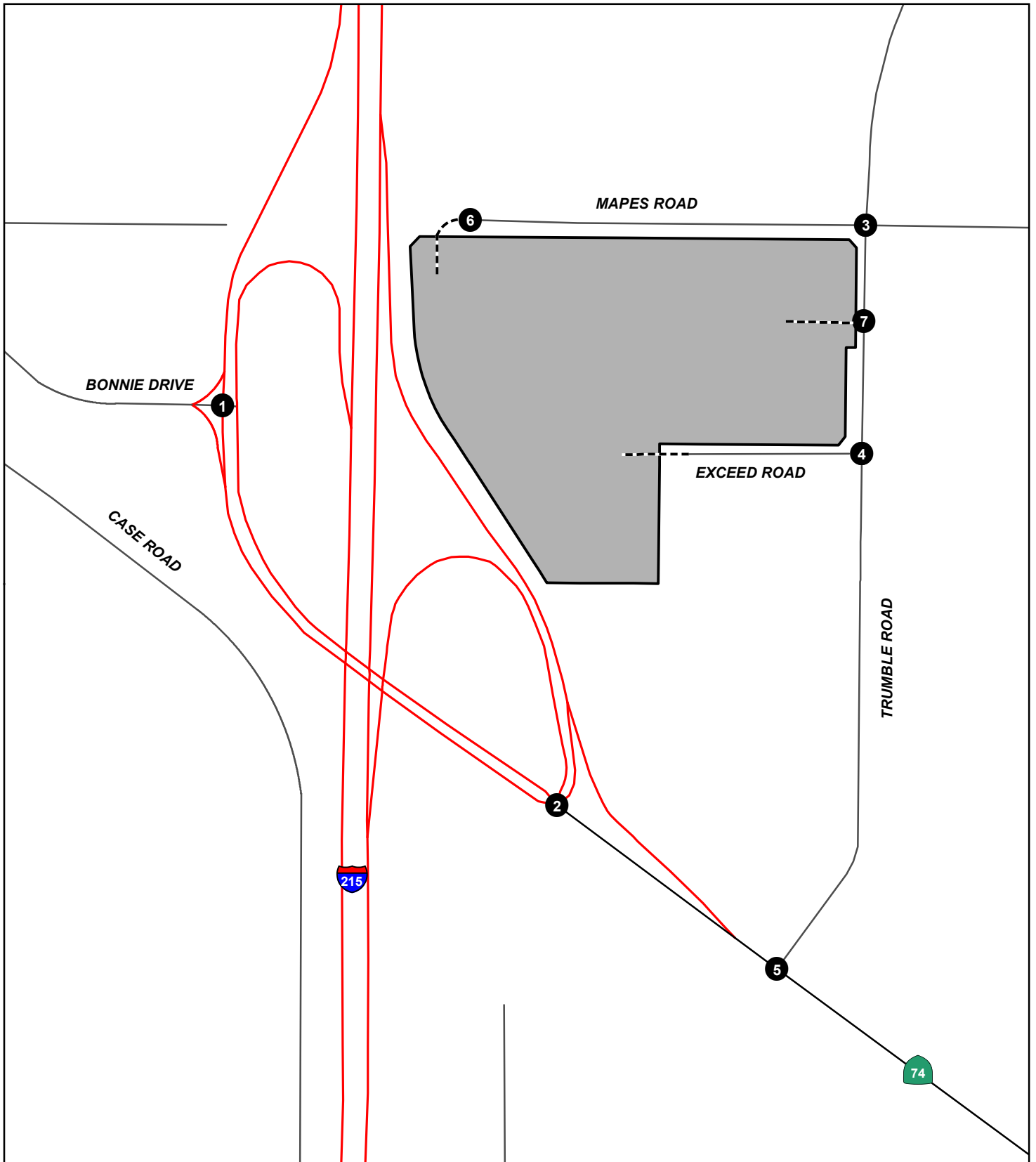
FIGURE 2

Mapes and Trumble Industrial Facility Project
Traffic Study

Conceptual Site Plan

SOURCE: AO Architects, June 2022

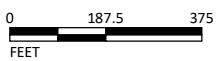
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LEGEND

- Project Location
- Study Area Intersections
- Project Driveway



SOURCE: Riverside County, 2016

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FIGURE 1-3

Mapes and Trumble Industrial Facility Project
Traffic Study

Study Area Intersections

2.0 ANALYSIS METHODOLOGY

2.1 LEVEL OF SERVICE DEFINITIONS

Level of service (LOS) can be characterized for the whole intersection, each intersection approach, and by each lane group. Control delay alone is used to characterize LOS for the entire intersection. Control delay quantifies the increase in travel time due to the traffic signal control, and is a surrogate measure of driver discomfort and fuel consumption.

A complete description of the meaning of LOS can be found in the Transportation Research Board Special Report 209, *Highway Capacity Manual* (HCM). The HCM establishes LOS A through F for intersections. The HCM establishes levels of service A through F for intersections as shown in Table 2-A.

For all study area intersections, the *Highway Capacity Manual 6th Edition* (HCM 6) analysis methodologies were used to determine intersection LOS. Intersection LOS was calculated using Synchro 11 software, which uses HCM 6 methodologies. Table 2-B shows the level of service criteria for unsignalized and signalized intersections.

2.2 LEVEL OF SERVICE PROCEDURES AND THRESHOLDS

Study intersections analyzed in this report are under the jurisdictions of the City of Perris, at the border of City of Perris and Menifee or California Department of Transportation (Caltrans). Both Cities use LOS D as their minimum level of service criteria for intersections. Based on the City of Perris LOS standards, LOS E is acceptable at intersections of any arterials and Expressways with SR-74, the Ramona-Cajalco Expressway or at I-215 Freeway ramps, and may be allowed within the boundaries of the downtown specific plan area to the extent that would support transit-oriented developments and walkable communities.

2.3 LEVEL OF SERVICE REQUIREMENTS

As aforementioned, the City has identified LOS D as the standard for acceptable operating conditions for intersections and roadway segments, except intersections of any arterials and Expressways with SR-74, the Ramona-Cajalco Expressway or at I-215 Freeway ramps. Per the City, a direct project related operational deficiency occurs at a study intersection when it operates at an acceptable Level of Service (LOS) for existing conditions (without the project) and the addition of 50 or more a.m. or p.m. peak hour project trips causes the intersection to operate at an unacceptable LOS for existing plus project conditions. Additionally, a direct project related operational deficiency occurs at a study intersection when it operates at an unacceptable LOS for existing conditions (without the project) and the addition of 50 or more a.m. or p.m. peak hour project trips causes the intersection delay to increase by 2 seconds or more. A cumulative operational deficiency occurs when a study intersection is forecast to operate at an unacceptable LOS with the addition of cumulative/background traffic and 50 or more a.m. or p.m. peak hour project trips. Additionally, improvements need to be recommended to conform to the City's LOS standards. As such, feasible improvements need to be recommended to eliminate or the existing or forecasted deficiency within the study area.

2.4 LIST OF CHAPTER 2.0 TABLES

- Table 2-A: Intersection Level of Service Definitions
- Table 2-B: Level of Service Criteria for Unsignalized and Signalized Intersections

Table 2-A: Intersection Level of Service Definitions

LOS	Description
A	Traffic operations with a control delay of 10 seconds per vehicle or less and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is exceptionally favorable or the cycle length is very short. If LOS A is the result of favorable progression, most vehicles arrive during the green indication and travel through the intersection without stopping.
B	Traffic operations with control delay between 10 seconds per vehicle and 20 seconds per vehicle and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is highly favorable or the cycle length is short. More vehicles stop than with LOS A.
C	Traffic operations with control delay between 20 and 35 seconds per vehicle and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when progression is favorable or the cycle length is moderate. Individual cycle failures (i.e., one or more queued vehicles are not able to depart as a result of the insufficient capacity during the cycle) may begin to appear at this level. The number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.
D	Traffic operations with control delay between 35 and 55 seconds per vehicle and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high and either progression is ineffective or the cycle length is long. Many vehicles stop and individual cycle failures are noticeable.
E	Traffic operations with control delay between 55 and 80 seconds per vehicle and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when volume-to-capacity ratio is high, progression is unfavorable, and the cycle length is long. Individual cycle failures are frequent.
F	Traffic operations with control delay exceeding 80 seconds per vehicle or a volume-to-capacity ratio greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is very high, progression is very poor, and the cycle length is long. Most cycles fail to clear the queue.

Source: *Highway Capacity Manual* (6th Edition)

Table 2-B: Level of Service Criteria for Unsignalized and Signalized Intersections

Level of Service	Unsignalized Intersection Average Delay per Vehicle (sec.)	Signalized Intersection Average Delay per Vehicle (sec.)
A	≤ 10	≤ 10
B	> 10 and ≤ 15	> 10 and ≤ 20
C	> 15 and ≤ 25	> 20 and ≤ 35
D	> 25 and ≤ 35	> 35 and ≤ 55
E	> 35 and ≤ 50	> 55 and ≤ 80
F	> 50	> 80

Source: *Highway Capacity Manual* (6th Edition)

3.0 CIRCULATION NETWORK SETTING

3.1 EXISTING CIRCULATION NETWORK

Figure 3-1 illustrates existing with project study intersection geometrics and traffic control.

It should be noted that though the southbound right turn lane at intersection 1, I-215 Southbound Ramps/Bonnie Drive is channelized and free as constructed, it was coded as a yield in the synchro network, since it does not have its own dedicated merge lane and needs to yield to westbound through traffic. The turn pocket storage length was estimated from the yield point to the point where the north leg through lane expands enough to create a de facto space for cars making southbound right turn.

For the eastbound right turn lane, the channelized right turn was coded as a free right turn in the synchro network based on existing geometry. The turn pocket storage length was estimated from the merge point to the point where the west leg left turn lane expands enough to create a de facto space for cars making eastbound right turn.

As previously mentioned, the project could be accessed via three driveways located on Trumble Road, Mapes road and Exceed Road.

Within the City, all major roadways are classified based on the roadway classification provided in the City of Perris General Plan Circulation Element, updated January 2022. Figure 3-2 summarizes the classifications on major roadways within the City. Following is a brief description of major roadways within the study area:

- **Trumble Road:** Trumble Road is shared between the Cities of Perris and Menifee. Within the City of Perris, it designated as a Secondary Arterial between Ellis Avenue and City boundary in the City's General Plan. In the City of Menifee General Plan Circulation Element, it is designated as a 4-lane divided Major Roadway. Within the study area, Trumble Road is a four-lane roadway with a two-way left-turn lane (TWLTL) median. Currently there's no provision for on-street parking along Trumble Road between Mapes Road and SR-74.
- **Mapes Road:** Mapes Road is designated as a local road in the City's General Plan. Within the study area, Mapes Road is a two-lane, undivided local road. There is no provision for on-street parking along Mapes Road within the study area.
- **SR-74:** SR-74 is not designated in the City's General Plan Circulation Element. However, in the City of Menifee General Plan Circulation Element, it is designated as a 4-lane divided Major Roadway between I-215 and Antelope Road, and as Expressway east of Antelope Road. It is the major connection between I-215 and unincorporated Riverside County, City of Hemet and San Jacinto and City of Menifee east of the City of Perris. Within the study area, SR-74 is a 4-lane roadway that terminates at I-215. There is no provision for bicycle lanes or on-street parking along SR-74 within the study area.
- **Bonnie Drive:** Bonnie Drive is designated as an Arterial in the City's General Plan. Bonnie Drive Road is a two-lane, undivided road west of I-215 that merges with Case Road. There are no provision for bicycle lanes or on-street parking along Bonnie Drive within the study area.

3.2 CITY OF PERRIS MODES OF TRANSPORTATION

3.2.1 Bicycle Circulation

The City of Perris Active Transportation Plan (ATP), identifies goals and policies concerning pedestrian and bicycle accommodations. The ATP aims to improve health and equity for Perris residents, and to provide improved access to community destinations, supporting safe walking and biking. The ATP classifies six categories of bikeway network: Class I – Shared-Use Paths, Class II – Bike Lanes, Class IIB – Buffered Bike Lanes, Class III – Bicycle Routes, Class IIIB – Bicycle Boulevard, and Class IV – Separated Bikeway. Class I bikeways provide bicycle travel on a paths completely separated from motor vehicle traffic used by people walking and biking. Class II bikeways provide a dedicated striped lane adjacent to traffic. Class IIB bikeways provide a dedicated striped lane separated from vehicular traffic by a painted buffer. Class III bikeways are shared use pathways with motor vehicle traffic and are identified only by signing. Class IIIB bikeways shares use with motor vehicle traffic but within calm, local streets where cyclists have the priority. Class IV bikeways are on-street bikeway separated from motor-vehicle traffic by a curb, median, planters, parking delineations or other physical barriers. The City has proposed Class II bike lanes on Bonnie Drive in its ATP. Figure 3-3 illustrates the recommended bikeway network within the City of Perris.

3.2.2 Pedestrians

The implementation of enhanced pedestrian linkage with a comprehensive trails system links residential areas, schools, parks, and commercial centers so that residents can travel within the community without driving. Safe and attractive sidewalks and walkways improve the walkability of the City. Typically, sidewalks are generally provided on both sides of the streets within the City. Additionally, standard paved trails and non-standard unpaved trails are frequently used by bicyclists and pedestrians in the City. The existence of trails and sidewalks provides accessible facilities, provides safety features, and improves walkability in the City of Perris. Although there are no current or proposed trails within the study area, paved sidewalks are present on both sides of Trumble Road. Continuous sidewalks are present on westbound direction on Mapes Road within the Study Area.

3.2.3 Transit

Riverside Transit Agency (RTA) provides fixed route and Dial-a-Ride bus service within Perris and neighboring jurisdictions. RTA bus routes 28 and 74 operate within the study area, connecting Perris to neighboring communities.

3.3 LIST OF CHAPTER 3.0 FIGURES

- Figure 3-1: Existing with Project Study Intersection Geometrics and Traffic Control
- Figure 3-2: City of Perris Roadway Classification
- Figure 3-3: City of Perris Recommended Bikeway Network

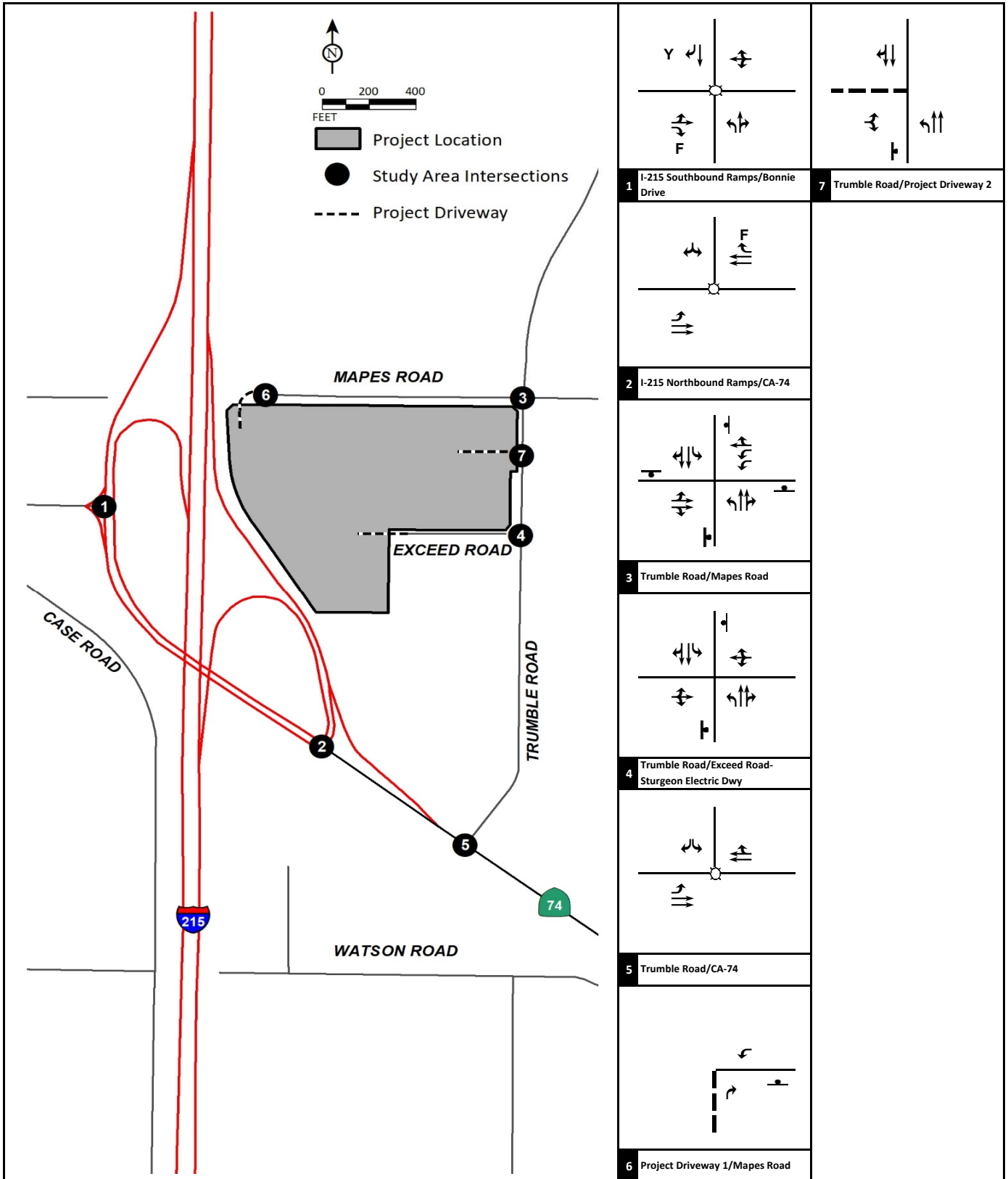
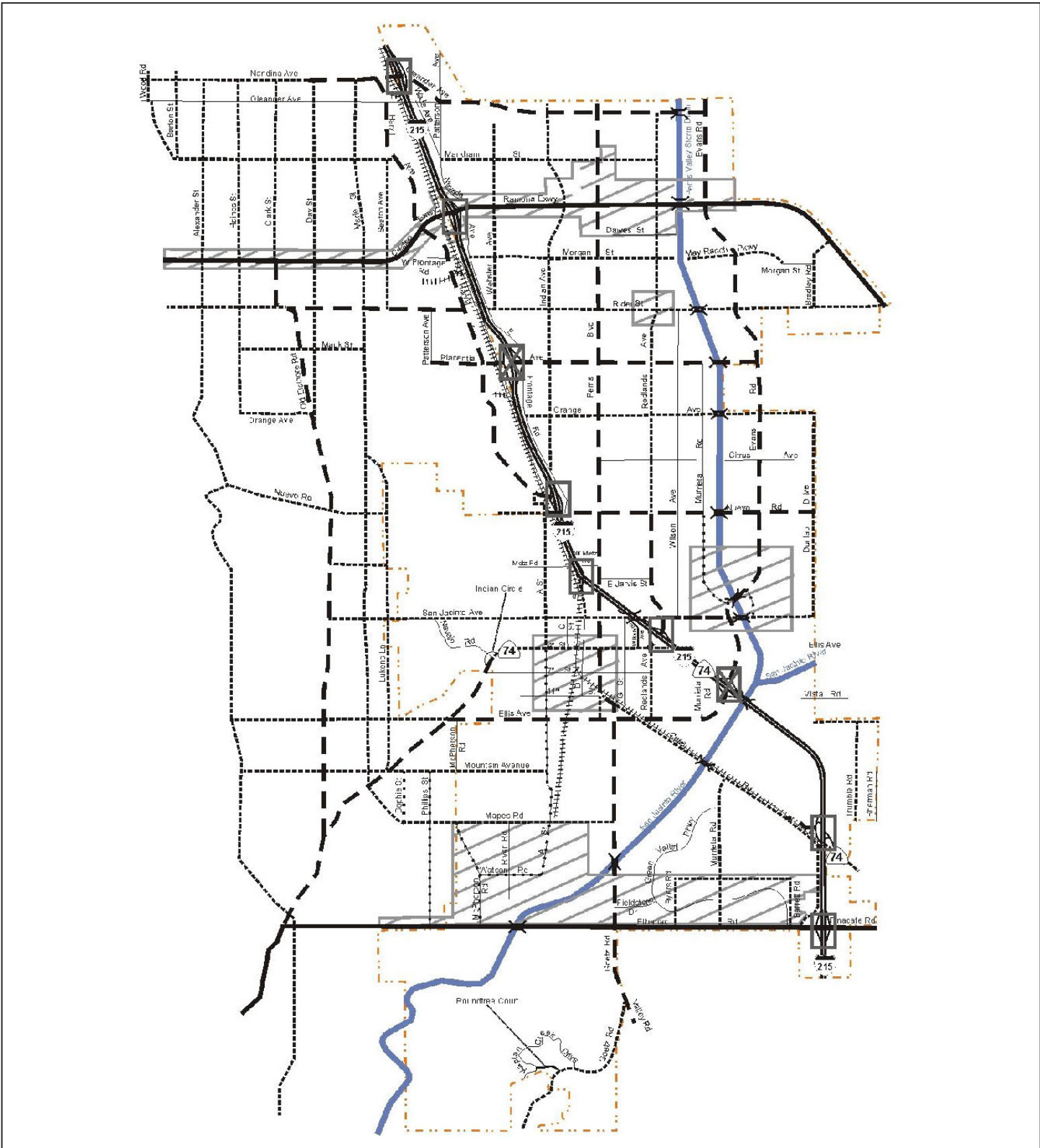


FIGURE 3-1

LSA

Mapes and Trumble Industrial Facility Project
Traffic Study

Existing with Project Study Intersection Geometrics and Traffic Control



LSA



Legend:

	Freeway		Collector (66' ROW)		Existing Interchange with Future Modifications
	Expressway (184' ROW)		Railroad		Proposed Interchange
	Arterial (128' ROW)		Bridge		Corridor Study Areas
	Secondary Arterial (94' ROW)		Water		
	Major Collector (78' ROW)		City Boundary		

FIGURE 3-2

Mapes and Trumble Industrial Facility Project
Traffic Study

City of Perris Future Roadway Network

SOURCE: City of Perris General Plan Circulation Element, January 2022
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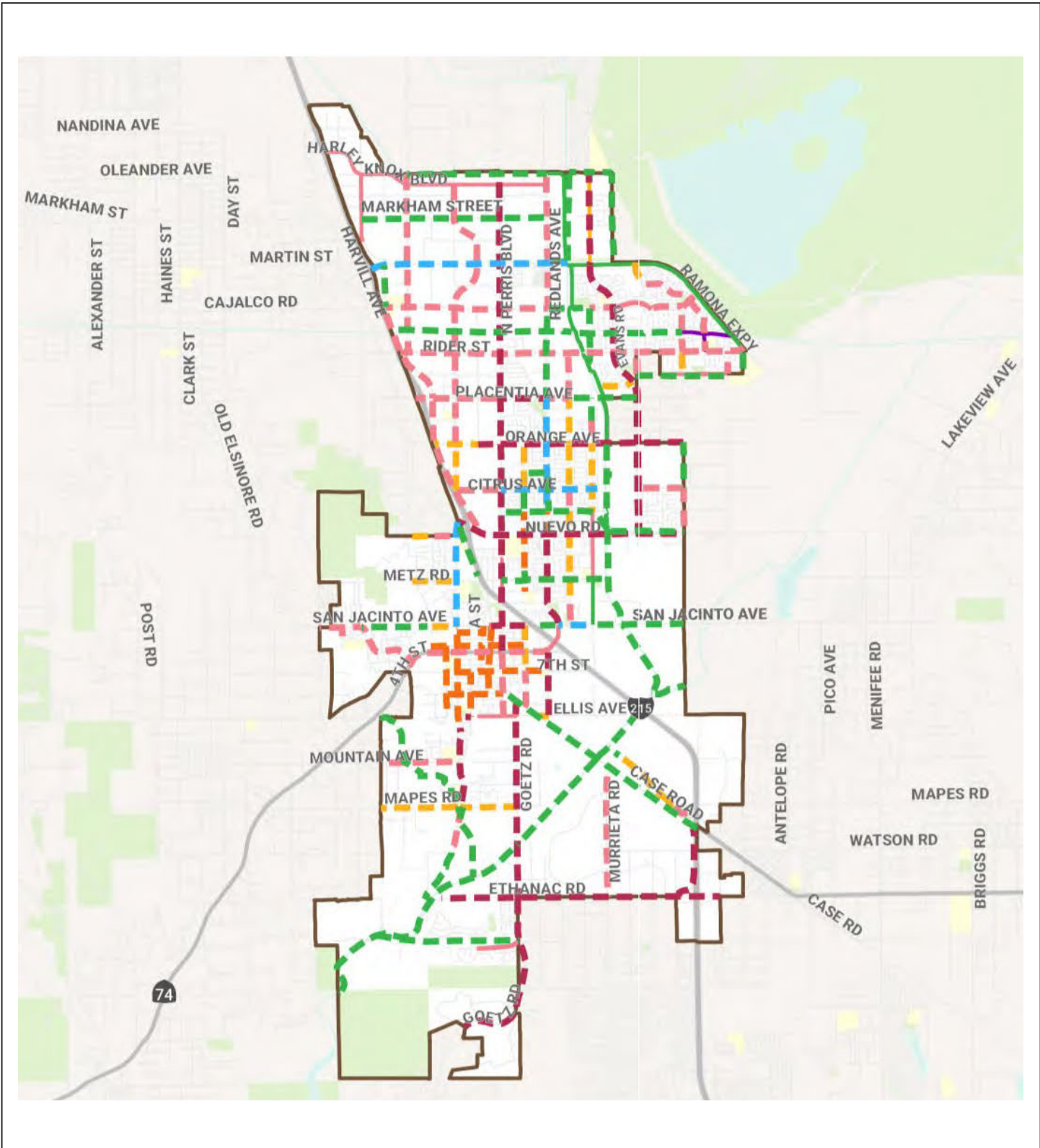
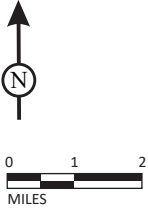


FIGURE 3-3

LSA



- Legend**
- Shared-Use Path (Class I)
 - Bicycle Lane (Class II)
 - Buffered Bike Lane (Class IIB)
 - Bicycle Route (Class III)
 - Bicycle Boulevard (Class IIIB)
 - Separated Bikeway (Class IV)
 - Walking Trail
- Destinations + Boundaries**
- City Boundary
 - School
 - Park or Open Space

Mapes and Trumble Industrial Facility Project
Traffic Study

City of Perris Recommended Bikeway Network

SOURCE: City of Perris Active Transportation Plan, December 2020
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4.0 TRAFFIC VOLUMES FOR WITHOUT PROJECT SCENARIOS

4.1 EXISTING TRAFFIC VOLUMES

Traffic volumes for existing year traffic conditions are based on existing a.m., and p.m. peak hour traffic counts collected by Counts Unlimited in March 2022. A.m. and p.m. peak hour turning movement counts were collected at study intersections. Detailed count sheets are included in Appendix B.

Vehicle classification counts were conducted at the following intersections:

- I-215 Southbound Ramps/Bonnie Drive;
- I-215 Northbound Ramps/CA-74;
- Trumble Road/Mapes Road; and
- Trumble Road/CA-74 (Menifee/Caltrans).

At these locations, counts were converted to Passenger Car Equivalent (PCE) volumes. The concept of PCEs accounts for the larger impact of trucks on traffic operations. It does so by assigning each type of truck a PCE factor that represents the number of passenger vehicles that could travel through an intersection in the same time that a particular type of truck could. PCE volumes at study intersections were computed using a factor of 1.5 for 2-axle trucks, 2.0 for 3-axle trucks, and 3.0 for trucks with four or more axles.

The percentage of trucks at the remaining study intersections without classification counts was determined based on truck percentages derived from adjacent intersections with classification counts. At these locations, truck PCE volumes were computed using a PCE factor of 2.0 for all trucks, consistent with the HCM 6 methodologies.

Figure 4-1 illustrates existing peak hour traffic volumes at study intersections. Detailed counts are included in Appendix B. Detailed Volume development worksheets are included in Appendix C.

4.2 CUMULATIVE (2024) WITHOUT PROJECT TRAFFIC VOLUMES

As approved during the City's scoping agreement process (Appendix A), traffic volumes for Cumulative (2024) conditions were developed by applying a 3.0 percent per annum growth rate to the existing traffic volumes and by adding trips from cumulative projects in the area.

Information concerning cumulative projects in the vicinity of the proposed project was obtained from the City of Perris, City of Menifee, and County of Riverside. Figure 4-2 illustrates the cumulative project locations. The trip generation for cumulative projects was developed using trip generation rates from the Institute of Transportation Engineers (ITE) *Trip Generation Manual* (11th Edition), and/or using information from approved traffic studies where available. Table 4-A lists the cumulative projects included in this analysis, and shows the cumulative projects are expected to generate 4,926 a.m. peak hour trips, 6,023 p.m. peak hour trips, and 59,704 daily trips.

Cumulative project trips were assigned to the roadway network based on their locations in relation to surrounding land uses and regional arterials, and/or using information from approved traffic

studies where available. Figure 4-3 illustrates the peak hour cumulative project trip assignment at the study area intersections. Figure 4-4 illustrates the peak hour traffic volumes at study intersections under cumulative (2024) without project conditions.

Detailed volume development worksheets are included in Appendix C.

4.3 LIST OF CHAPTER 4.0 FIGURES AND TABLES

- Figure 4-1: Existing Peak Hour Traffic Volumes
- Figure 4-2: Cumulative Project Locations
- Figure 4-3: Cumulative Projects Trip Assignment
- Figure 4-4: Cumulative (2024) Peak Hour Traffic Volumes
- Table 4-A: Cumulative Projects Trip Generation

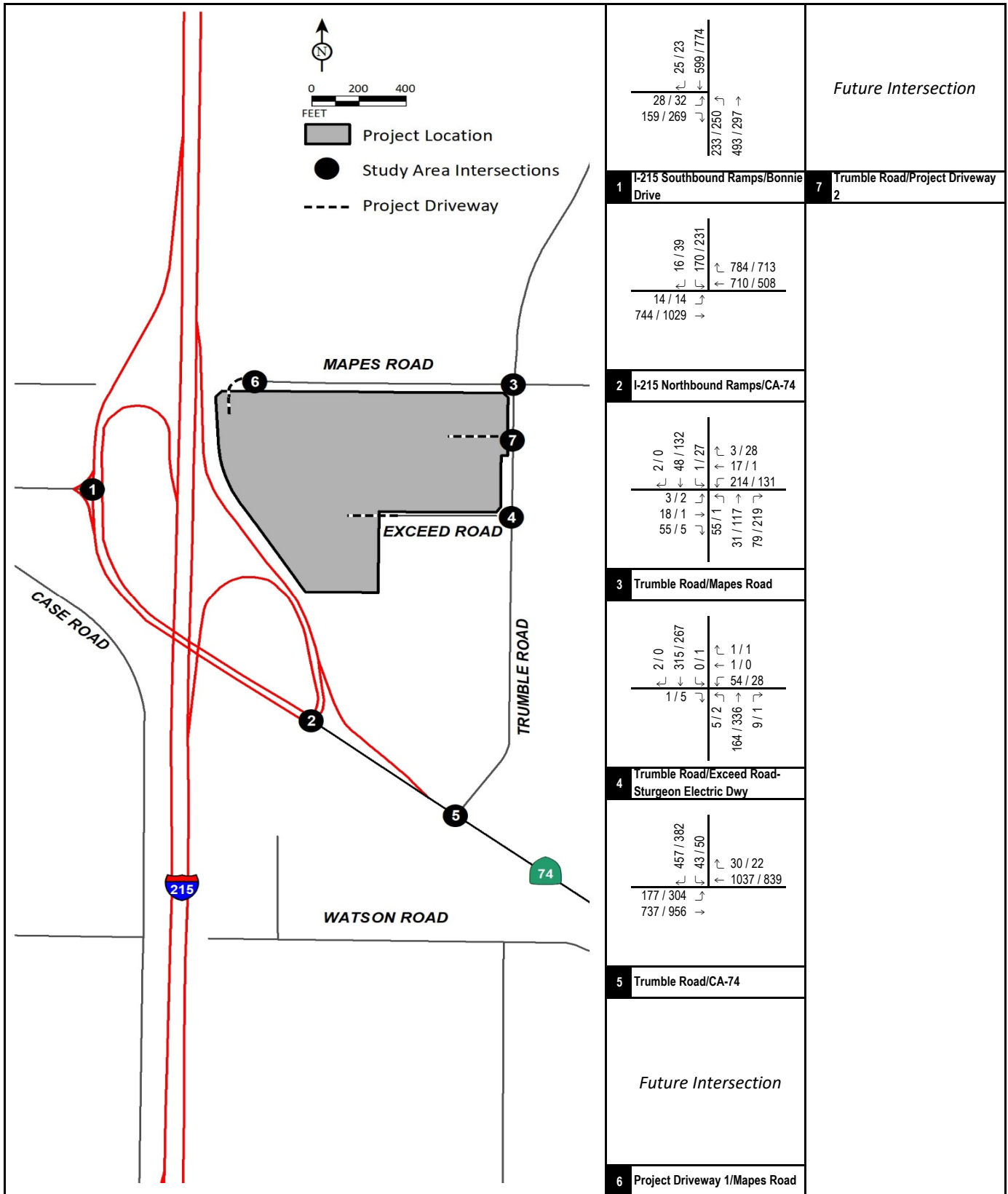


FIGURE 4-1



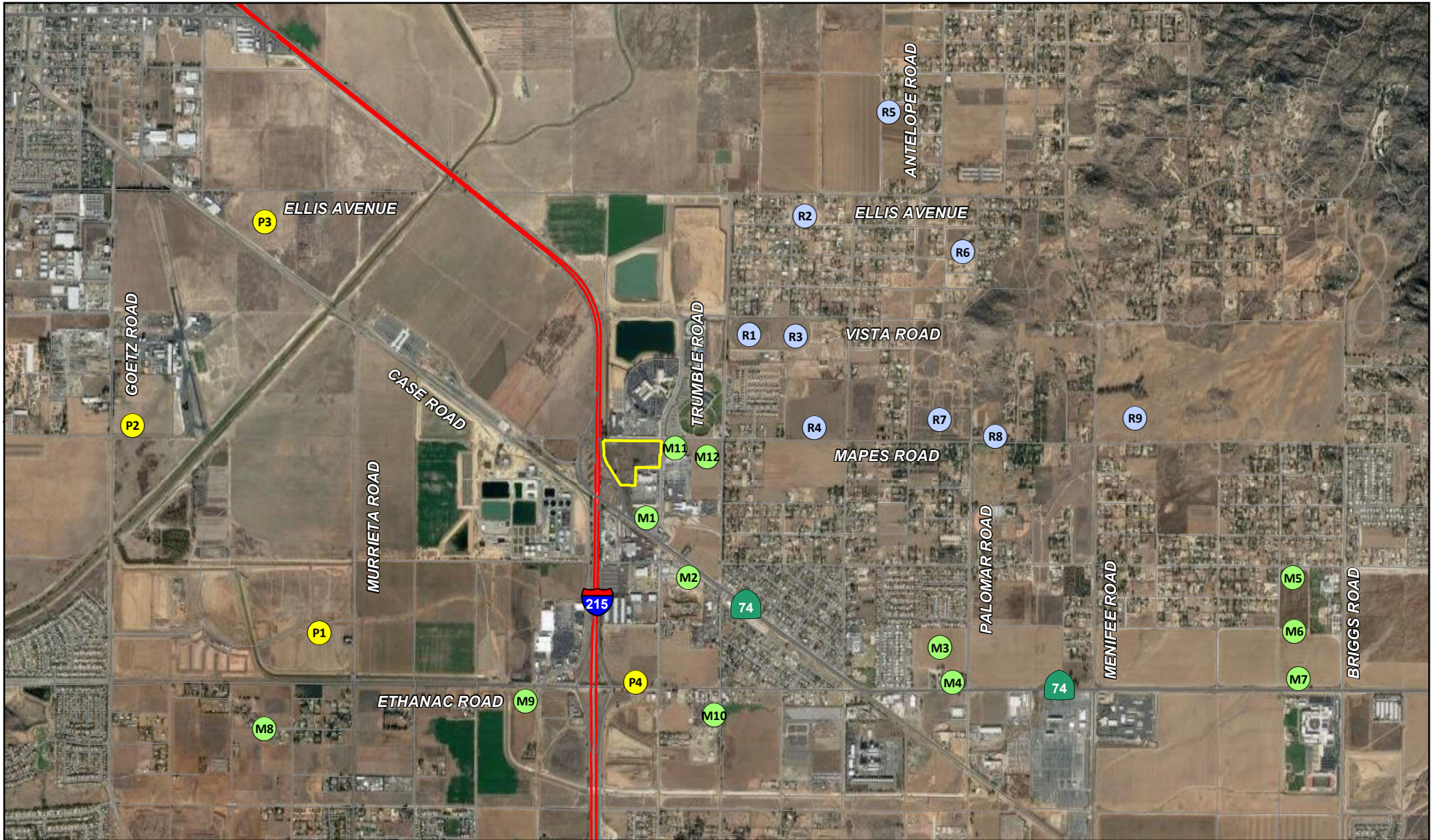
XXXX / YYYY

AM / PM Peak Hour Traffic Volumes (In PCE)

--- Project Driveway

Mapes and Trumble Industrial Facility Project
 Traffic Study

Existing Peak Hour Traffic Volumes



LSA

LEGEND

- Project Location
- City of Perris Cumulative Project
- County of Riverside Cumulative Project
- City of Menifee Cumulative Project



SOURCE: ESRI Streetmap, 2021; Google Earth, 2018.

I:\BAV2101\Traffic\Reports\fig4-2_Cumulative_Projects.mxd (6/6/2022)

FIGURE 4-2

*Mapes and Trumble Industrial Facility Project
Traffic Study
Cumulative Project Locations*

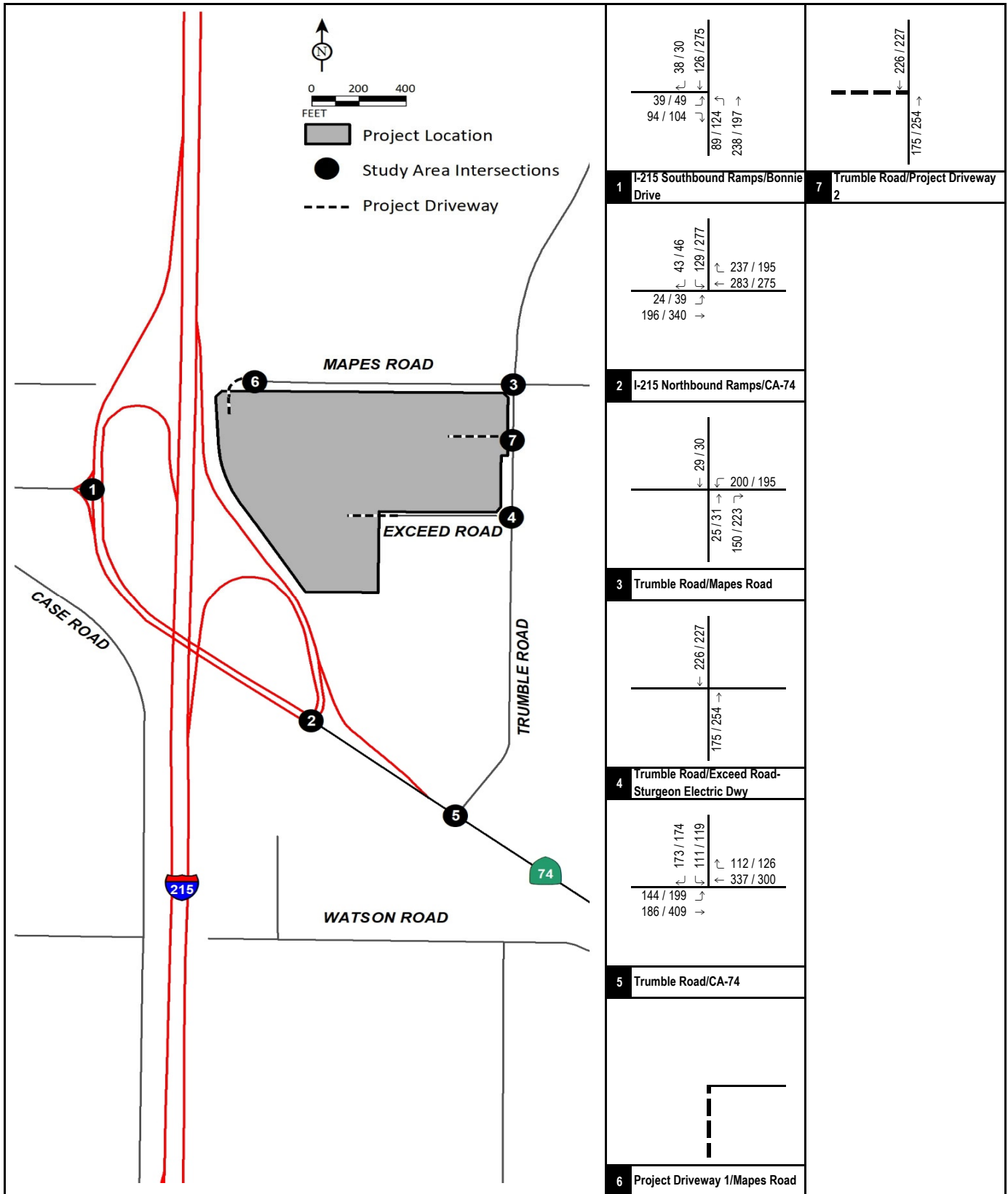


FIGURE 4-3



XXX / YYY

AM / PM Peak Hour Traffic Volumes (In PCE)

----- Project Driveway

Mapes and Trumble Industrial Facility Project
Traffic Study

Cumulative Project Trip Assignment

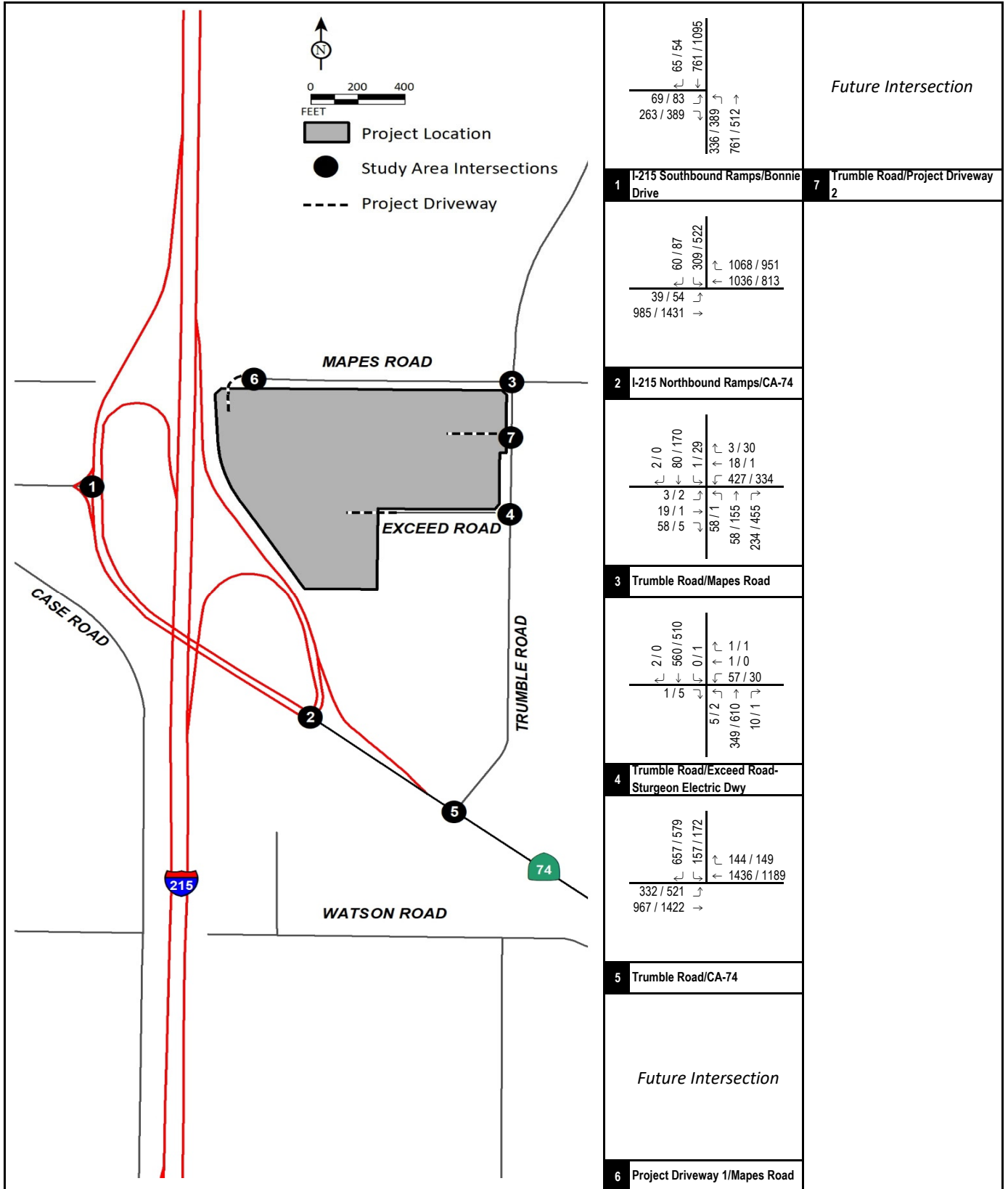


FIGURE 4-4



XXXX / YYYY

AM / PM Peak Hour Traffic Volumes (In PCE)

- - - Project Driveway

Mapes and Trumble Industrial Facility Project
 Traffic Study

Cumulative (2024) Peak Hour Traffic Volumes

Table 4-A - Cumulative Projects Trip Generation

Project No.	Project Name/Land Use/Builder/Applicant	Units	A.M. Peak Hour			P.M. Peak Hour			Daily
			In	Out	Total	In	Out	Total	
M1 . On-Deck									
North corner of Trumble Road and CA-74									
Hotel		108 RM							
	Trips/Unit ¹		0.26	0.20	0.46	0.30	0.29	0.59	7.99
	Trip Generation		28	22	50	32	31	64	863
Convenience Store/Gas Station - GFA (2-4k)		12 VFP							
	Trips/Unit ²		8.03	8.03	16.06	9.21	9.21	18.42	265.12
	Trip Generation		96	96	193	111	111	221	3,181
	Total Pass-By Trips ³		(73)	(73)	(147)	(83)	(83)	(166)	(2402)
	Total Net Trip Generation		23	23	46	28	28	55	779
Fast-Food Restaurant with Drive-Through Window		3.000 TSF							
	Trips/Unit ⁴		22.75	21.86	44.61	17.18	15.85	33.03	467.48
	Trip Generation		68	66	134	52	48	99	1,402
	Total Pass-By Trips ⁵		(34)	(33)	(67)	(29)	(26)	(54)	(736)
	Total Net Trip Generation		34	33	67	23	22	45	666
High-Turnover Sit Down Restaurant		5.500 TSF							
	Trips/Unit ⁶		5.26	4.31	9.57	5.52	3.53	9.05	107.20
	Trip Generation		29	24	53	30	19	50	590
	Total Pass-By Trips ⁷		0	0	0	(13)	(8)	(22)	(254)
	Total Net Trip Generation		29	24	53	17	11	28	336
Carwash		110.00 Li Ft							
	Trips/Unit ⁸		0.38	0.28	0.66	0.62	0.63	1.25	11.87
	Trip Generation		42	31	73	68	69	137	1,306
	Total Pass-By Trips		(4)	(3)	(7)	(23)	(23)	(46)	(131)
	Total Net Trip Generation		38	28	66	45	46	91	1,175
Total Net Project Trip Generation			152	130	282	145	138	283	3,819
M2 . Paragon Framing									
Southeast corner of Trumble Road and Watson Road									
Single Tenant Office Building		5.45 TSF							
	Trips/Unit ⁹		1.65	0.20	1.85	0.26	1.50	1.76	13.07
	Trip Generation		9	1	10	1	8	10	71
Utility		5.00 TSF							
	Trips/Unit ¹⁰		2.03	0.30	2.33	0.39	1.77	2.16	12.29
	Trip Generation		10	2	12	2	9	11	61
	Total Project Trip Generation		19	3	22	3	17	21	132
M3 . MR-27 LLC (Rancon)									
West of Palomar Road between Cider St and CA-74									
Single-Family Detached Housing		85 DU							
	Trips/Unit ¹¹		0.18	0.52	0.70	0.59	0.35	0.94	9.43
	Trip Generation		15	44	60	50	30	80	802
Single-Family Attached Housing		87 DU							
	Trips/Unit ¹²		0.15	0.33	0.48	0.54	0.40	0.94	7.20
	Trip Generation		13	29	42	47	35	82	626
	Total Project Trip Generation		28	73	102	97	65	162	1,428

Table 4-A - Cumulative Projects Trip Generation

Project No.	Project Name/Land Use/Builder/Applicant	Units	A.M. Peak Hour			P.M. Peak Hour			Daily
			In	Out	Total	In	Out	Total	
M4 . Motte Country Plaza	Northwest corner of Palomar Rd and CA-74								
	Convenience Store/Gas Station - GFA (2-4k)	12 VFP							
	Trips/Unit ²		8.03	8.03	16.06	9.21	9.21	18.42	
	Trip Generation		96	96	193	111	111	221	
	Total Pass-By Trips ³		(73)	(73)	(147)	(83)	(83)	(166)	
	Total Net Trip Generation^x		23	23	46	28	28	55	
M5 . MR 56 LLC (Rancon)	Southeast corner of Malone Ave and Watson Rd								
	Single-Family Detached Housing	52 DU							
	Trips/Unit ¹¹		0.18	0.52	0.70	0.59	0.35	0.94	
	Trip Generation		9	27	36	31	18	49	
M6 . MR-27 LLC (Rancon)	Southeast corner of Malone Ave and Varela Ln								
	Single-Family Attached Housing	153 DU							
	Trips/Unit ¹²		0.15	0.33	0.48	0.54	0.40	0.94	
	Trip Generation		23	50	73	83	61	144	
M7 . Harvest Glen Marketplace	Northwest corner of Briggs Rd and CA-74								
	Convenience Store/Gas Station - GFA (4-5.5k)	16 VFP							
	Trips/Unit ¹³		13.52	13.52	27.04	11.38	11.38	22.76	
	Trip Generation		216	216	433	182	182	364	
	Total Pass-By Trips ³		(164)	(164)	(329)	(137)	(137)	(273)	
	Total Net Trip Generation		52	52	104	45	45	91	
	Fast-Food Restaurant with Drive-Through Window	3.268 TSF							
	Trips/Unit ⁴		22.75	21.86	44.61	17.18	15.85	33.03	
	Trip Generation		74	71	146	56	52	108	
	Total Pass-By Trips ⁵		(37)	(36)	(73)	(31)	(29)	(59)	
	Total Net Trip Generation		37	35	73	25	23	49	
	Total Net Project Trip Generation		89	87	177	70	68	140	
M8 . Northern Gateway Commerce Center I	South of Ethanac Rd between Byers Rd and Geary St								
	Warehouse ¹⁴	1,170.871 TSF							
	Total Trip Generation		152	47	199	60	151	211	
	Auto Trips		104	33	137	40	105	145	
	Truck PCE Trips		120	35	155	50	115	165	
	Total PCE Trip Generation		224	68	292	90	220	310	
M9 . Northern Gateway Commerce Center II	South of Ethanac Rd between Hull St and Evans Rd								
	High-Cube Fulfillment Center Warehouse ¹⁵	1,316.754 TSF							
	Total Trip Generation		140	57	197	87	123	210	
	Auto Trips		110	26	136	57	88	145	
	Truck PCE Trips		75	78	153	75	88	163	
	Total PCE Trip Generation		185	104	289	132	176	308	

Table 4-A - Cumulative Projects Trip Generation

Project No.	Project Name/Land Use/Builder/Applicant	Units	A.M. Peak Hour			P.M. Peak Hour			Daily
			In	Out	Total	In	Out	Total	
M10	Menifee Commerce Center (Core 5) ¹⁶								
	South of Ethanac Rd between Trumble Rd and Dawson Rd								
	Building 1- High-Cube Fulfillment Center Warehouse	1,249.317 TSF							
	Total Trip Generation		884	208	1,092	586	918	1,504	8,077
	Auto Trips		871	195	1,066	575	905	1,480	7,839
	Truck PCE Trips		33	33	66	28	33	61	605
	Total PCE Trip Generation		904	228	1,132	603	938	1,541	8,444
	Building 2-Warehouse ¹⁴	385.596 TSF							
	Total Trip Generation		52	15	67	20	53	73	672
	Auto Trips		47	11	58	14	48	62	440
	Truck PCE Trips		13	10	23	16	13	29	590
	Total PCE Trip Generation		60	21	81	30	61	91	1,030
	Total Project Trip Generation		936	223	1,159	606	971	1,577	8,749
	Total Project Auto Trips		918	206	1,124	589	953	1,542	8,279
	Total Project Truck PCE Trips		46	43	89	44	46	90	1,195
	Total Project PCE Trip Generation		964	249	1,213	633	999	1,632	9,474
M11	United Carports Facility								
	Southeast corner of Trumble Rd and Mapes Road								
	Manufacturing ¹⁷	58.640 TSF							
	Total Trip Generation		29	11	40	15	29	44	278
	Auto Trips		24	7	31	11	23	34	219
	Truck PCE Trips		11	10	21	10	14	24	133
	Total PCE Trip Generation		35	17	52	21	37	58	352
M12	Mapes and Sherman Warehouse								
	Southwest corner of Sherman Rd and Mapes Rd								
	High-Cube Fulfillment Center Warehouse ¹⁵	277.578 TSF							
	Total Trip Generation		29	13	42	18	26	44	503
	Auto Trips		23	6	29	12	19	31	347
	Truck PCE Trips		16	17	33	16	17	33	389
	Total PCE Trip Generation		39	23	62	28	36	64	736
R1	Tentative Parcel Map 37727								
	Southeast corner of Sherman Rd and Vista Rd								
	Single-Family Detached Housing	4 DU							
	Trips/Unit ¹¹		0.18	0.52	0.70	0.59	0.35	0.94	9.43
	Trip Generation		1	2	3	2	1	4	38
R2	Tentative Parcel Map 37213								
	Northeast corner of Dawson Rd and Patti Ln								
	Single-Family Detached Housing	4 DU							
	Trips/Unit ¹¹		0.18	0.52	0.70	0.59	0.35	0.94	9.43
	Trip Generation		1	2	3	2	1	4	38
R3	Tentative Tract Map 37728								
	Southwest corner of Antelope Rd and Vista Rd								
	Single-Family Detached Housing	228 DU							
	Trips/Unit ¹¹		0.18	0.52	0.70	0.59	0.35	0.94	9.43
	Trip Generation		41	119	160	135	80	214	2,150

Table 4-A - Cumulative Projects Trip Generation

Project No.	Project Name/Land Use/Builder/Applicant	Units	A.M. Peak Hour			P.M. Peak Hour			Daily
			In	Out	Total	In	Out	Total	
R4 . Tentative Tract Map 37358	Northwest corner of Antelope Rd and Mapes Rd								
	Single-Family Detached Housing	154 DU							
	Trips/Unit ¹¹		0.18	0.52	0.70	0.59	0.35	0.94	
	Trip Generation		28	80	108	91	54	145	
R5 . Tentative Parcel Map 37919	Southwest corner of Antelope Rd and Roan Ranch Rd								
	Single-Family Detached Housing	4 DU							
	Trips/Unit ¹¹		0.18	0.52	0.70	0.59	0.35	0.94	
	Trip Generation		1	2	3	2	1	4	
R6 . Tentative Parcel Map 37589	Northeast corner of Pico Ave and Benigni Ave								
	Single-Family Detached Housing	4 DU							
	Trips/Unit ¹¹		0.18	0.52	0.70	0.59	0.35	0.94	
	Trip Generation		1	2	3	2	1	4	
R7 . Tract 31687	Northeast corner of Tradewinds Dr and Mapes Rd								
	Single-Family Detached Housing	65 DU							
	Trips/Unit ¹¹		0.18	0.52	0.70	0.59	0.35	0.94	
	Trip Generation		12	34	46	38	23	61	
R8 . RV Storage Facility	North of Mapes Road between Palomar Rd and Rocky Peak Rd								
	RV and Boat Storage	121 Stalls							
	Trips/Unit ¹⁸		0.02	0.01	0.03	0.02	0.02	0.04	
	Trip Generation		2	1	3	2	2	4	
R9 . Residential Subdivision	Northeast corner of Menifee Rd and Mapes Rd								
	Single-Family Detached Housing	712 DU							
	Trips/Unit ¹¹		0.18	0.52	0.70	0.59	0.35	0.94	
	Trip Generation		128	370	498	420	249	669	
P1 . Green Valley Specific Plan Phase 1	South of Case Rd, North of Ethanac Rd, West of Green Valley Pkwy, East of Goetz Rd								
	Phase 1								
	Single-Family Detached Housing	542 DU							
	Trips/Unit ¹¹		0.18	0.52	0.70	0.59	0.35	0.94	
	Trip Generation		98	282	379	320	190	509	
	Multifamily Housing (Low-Rise) Not Close to Rail Transit	699 DU							
	Trips/Unit ¹⁹		0.10	0.30	0.40	0.32	0.19	0.51	
	Trip Generation		70	210	280	224	133	356	
	Total Project Trip Generation		168	492	659	544	323	865	
P2 . TPM 38412	Southeast corner of Ellis Ave and Case Rd								
	High-Cube Fulfillment Center Warehouse ¹⁵	742.560 TSF							
	Total Trip Generation		79	33	112	49	70	119	
	Auto Trips		62	15	77	32	50	82	
	Truck PCE Trips		42	45	87	42	50	92	
	Total PCE Trip Generation		104	60	164	74	100	174	

Table 4-A - Cumulative Projects Trip Generation

Project No.	Project Name/Land Use/Builder/Applicant	Units	A.M. Peak Hour			P.M. Peak Hour			Daily
			In	Out	Total	In	Out	Total	
P3	DPR22-00002 Northwest corner of Goetz Rd and Mapes Rd High-Cube Fulfillment Center Warehouse¹⁵	659,000 TSF							
	Total Trip Generation		70	28	98	43	63	106	1,193
	Auto Trips		55	13	68	28	45	73	823
	Truck PCE Trips		38	37	75	38	45	83	923
	Total PCE Trip Generation		93	50	143	66	90	156	1,746
P4	Pilot J Travel center Northwest corner of Ethanac Rd and Trumble Rd Project Net Trip Generation²⁰		241	247	488	258	238	496	8,568
Total Net PCE Trip Generation			2,611	2,315	4,926	2,997	3,026	6,023	59,704

- Notes:
- RM = Rooms; DU = Dwelling Units; VFP = Vehicle Fueling Positions; TSF = Thousand Square Feet; Li ft = Linear Feet; Stalls = Parking Stalls.
 - ¹ Rates based on Land Use 310 - "Hotel" from Institute of Transportation Engineers (ITE) *Trip Generation Manual*, 11th Edition. Setting/Location used is General Urban/Suburban.
 - ² Rates based on Land Use 945 - "Convenience Store/Gas Station - GFA (2-4k)" from Institute of Transportation Engineers (ITE) *Trip Generation Manual*, 11th Edition. Setting/Location used is General Urban/Suburban.
 - ³ Pass-by rates obtained from the ITE *Trip Generation Manual* (11th Edition) for Land Use 945. a.m. peak period pass-by rate for this land use is 76% and p.m. peak period pass-by rate for this land use in the ITE handbook is 75%. No daily pass-by rates are provided. Therefore, the daily pass-by rate was taken to be the average of the a.m. and p.m. peak period pass-by rate.
 - ⁴ Rates based on Land Use 934 - "Fast-Food Restaurant with Drive-Through Window" from Institute of Transportation Engineers (ITE) *Trip Generation Manual*, 11th Edition. Setting/Location used is General Urban/Suburban.
 - ⁵ Pass-by rates obtained from the ITE *Trip Generation Manual* (11th Edition) for Land Use 934. a.m. peak period pass-by rate for this land use is 50% and p.m. peak period pass-by rate for this land use in the ITE handbook is 55%. No daily pass-by rates are provided. Therefore, the daily pass-by rate was taken to be the average of the a.m. and p.m. peak period pass-by rate.
 - ⁶ Rates based on Land Use 932 - "High-Turnover Sit-Down Restaurant" from Institute of Transportation Engineers (ITE) *Trip Generation Manual*, 11th Edition. Setting/Location used is General Urban/Suburban.
 - ⁷ Pass-by rates obtained from the ITE *Trip Generation Manual* (11th Edition) for Land Use 932. P.m. peak period pass-by rate for this land use in the ITE handbook is 43%. No a.m. peak hour or daily pass-by rates are provided. Therefore, the daily pass-by rate was taken to be the p.m. peak period pass-by rate.
 - ⁸ Trip generation rate and pass-by rate were obtained from the *Newport Pointe Traffic Impact Analysis Report*, dated April 2021, by Linscott, Law, and Greenspan, Engineers for a similar car-wash facility.
 - ⁹ Rates based on Land Use 715 - "Single Tenant Office Building" from Institute of Transportation Engineers (ITE) *Trip Generation Manual*, 11th Edition. Setting/Location used is General Urban/Suburban.
 - ¹⁰ Rates based on Land Use 170 - "Utility" from Institute of Transportation Engineers (ITE) *Trip Generation Manual*, 11th Edition. Setting/Location used is General Urban/Suburban.
 - ¹¹ Rates based on Land Use 210 - "Single-Family Detached Housing" from Institute of Transportation Engineers (ITE) *Trip Generation Manual*, 11th Edition. Setting/Location used is General Urban/Suburban.
 - ¹² Rates based on Land Use 215 - "Single-Family Attached Housing" from Institute of Transportation Engineers (ITE) *Trip Generation Manual*, 11th Edition. Setting/Location used is General Urban/Suburban.
 - ¹³ Rates based on Land Use 945 - "Convenience Store/Gas Station - GFA (4-5.5k)" from Institute of Transportation Engineers (ITE) *Trip Generation Manual*, 11th Edition. Setting/Location used is General Urban/Suburban.
 - ¹⁴ The trip generation was developed based on the ITE *Trip Generation Manual* (11th edition) rates for Land Use 150 – "Warehousing." The resulting trips were converted to trucks and passenger vehicles based on the South Coast Air Quality Management District (SCAQMD) recommendations for warehousing projects. As such, 31 percent of project traffic will be trucks. Based on Vehicle Mix from the SCAQMD, Warehouse Truck Trip Study Data Results and Usage, dated December 2014, the truck mix was considered as 18.7% 4-axle, 5.5% 3-axle, and 6.8% 2-axle trucks. All truck trips were converted to passenger car equivalents (PCEs) using a 1.5 PCE factor for 2-axle trucks, 2.0 for 3-axle trucks, and 3.0 for 4- and more axle trucks.
 - ¹⁵ Rates based on the ITE *Trip Generation Manual* (11th Edition) for Land Use 155 - "High-Cube Fulfillment Center Warehouse - Non-Sort", Setting Location - "General Urban/Suburban." Passenger vehicles and truck in/out splits from Institute of Transportation Engineers (ITE) *Trip Generation Manual* (11th Edition) rates for Land Use 155 – "High-Cube Fulfillment Center Warehouse - Non-Sort", Setting/Location - "General Urban/Suburban." The resulting trips were converted to trucks and passenger vehicles based on the South Coast Air Quality Management District (SCAQMD) recommendations for warehousing projects. As such, 31 percent of project traffic will be trucks. Based on Vehicle Mix from the SCAQMD, Warehouse Truck Trip Study Data Results and Usage, dated December 2014, the truck mix was considered as 18.7% 4+ axle, 5.5% 3-axle, and 6.8% 2-axle trucks. All truck trips were converted to passenger car equivalents (PCEs) using a 1.5 PCE factor for 2-axle trucks, 2.0 for 3-axle trucks, and 3.0 for 4- and more axle trucks.
 - ¹⁶ Trip generation and distribution was obtained from the *Menifee Commerce Center Project Traffic Impact Analysis*, dated August 2021, by Albert A. Webb Associates.
 - ¹⁷ The trip generation was developed using rates from the ITE *Trip Generation Manual* (11th Edition) for Land Use 140 – "Manufacturing." The resulting trips were converted to trucks and passenger vehicles based on the vehicle mix from the City of Fontana's Truck Trip Generation Study (August 2003). As such, 78.6% of project traffic will be passenger vehicles and 21.4% of project traffic will be trucks. All truck trips were converted to passenger car equivalents (PCEs) using a 1.5 PCE factor for 2-axle trucks, 2.0 for 3-axle trucks, and 3.0 for 4- and more axle trucks.
 - ¹⁸ Trip generation rates are based on Great park RV Storage Study, Prepared by LSA Associates, 2021.
 - ¹⁹ Rates based on Land Use 220 - "Multifamily Housing (Low-Rise) Not Close to Rail Transit" from Institute of Transportation Engineers (ITE) *Trip Generation Manual*, 11th Edition. Setting/Location used is General Urban/Suburban.
 - ²⁰ Trip generation were obtained from the *Perris Travel Center TIA Scoping Agreement*, dated May 2022, by Kimley-Horn and Associates.

5.0 PROJECT TRAFFIC

5.1 PROJECT TRIP GENERATION

The trip generation for the proposed project was developed using rates from the Institute of Transportation Engineers (ITE) *Trip Generation Manual* (11th Edition) Land Use 155 – “High-Cube Fulfillment Center Warehouse – Non-Sort.” That was because the project is not proposing a “Sort” use at the proposed facility.

As previously mentioned, the project proposes to include 50 percent High-Cube Cold Storage Warehouse as an alternative. However, peak hour trip generation rate for High-Cube Cold Storage Warehouse is lower than corresponding trip generation rates for High Cube Fulfillment Center Warehouse – Non-Sort” land use. Since this study evaluates the project’s effect on adjacent circulation network under a.m. and p.m. peak hours, the project was considered as 100 percent High-Cube Fulfillment Center Warehouse – Non-Sort” as a conservative approach.

The resulting trips were converted to passenger vehicles and trucks based on the South Coast Air Quality Management District (SCAQMD) recommendations for warehousing projects. As such, 31% of project traffic will be trucks. Based on the truck mix from the SCAQMD Warehouse Truck Trip Study Data Results and Usage, dated December 2014, the truck mix was considered as 6.8% 2-axle trucks, 5.5% 3-axle trucks, and 18.7% as 4 or more axle trucks.

Inbound/Outbound splits for passenger vehicles were obtained from the ITE Trip Generation (11th Edition) for Land Use 155 – High-Cube Fulfillment Center Warehouse - Non-Sort, vehicle Trip generation inbound/outbound splits for a.m. peak hour, p.m. peak hour and daily trips. Inbound/outbound splits for trucks were obtained from ITE Trip Generation 11th Edition Supplemental for a.m. peak hour, p.m. peak hour and daily trips.

All truck trips were converted to passenger car equivalents (PCEs) using a 1.5 PCE factor for 2-axle trucks, 2.0 for 3-axle trucks, and 3.0 for 4 or more axle trucks. Table 5-A summarizes the project trip generation. As shown in Table 5-A, the proposed project is estimated to generate 1,054 daily PCE trips, with 86 trips occurring the a.m. peak hour and 92 trips occurring during the p.m. peak hour.

5.2 PROJECT TRIP DISTRIBUTION AND ASSIGNMENT

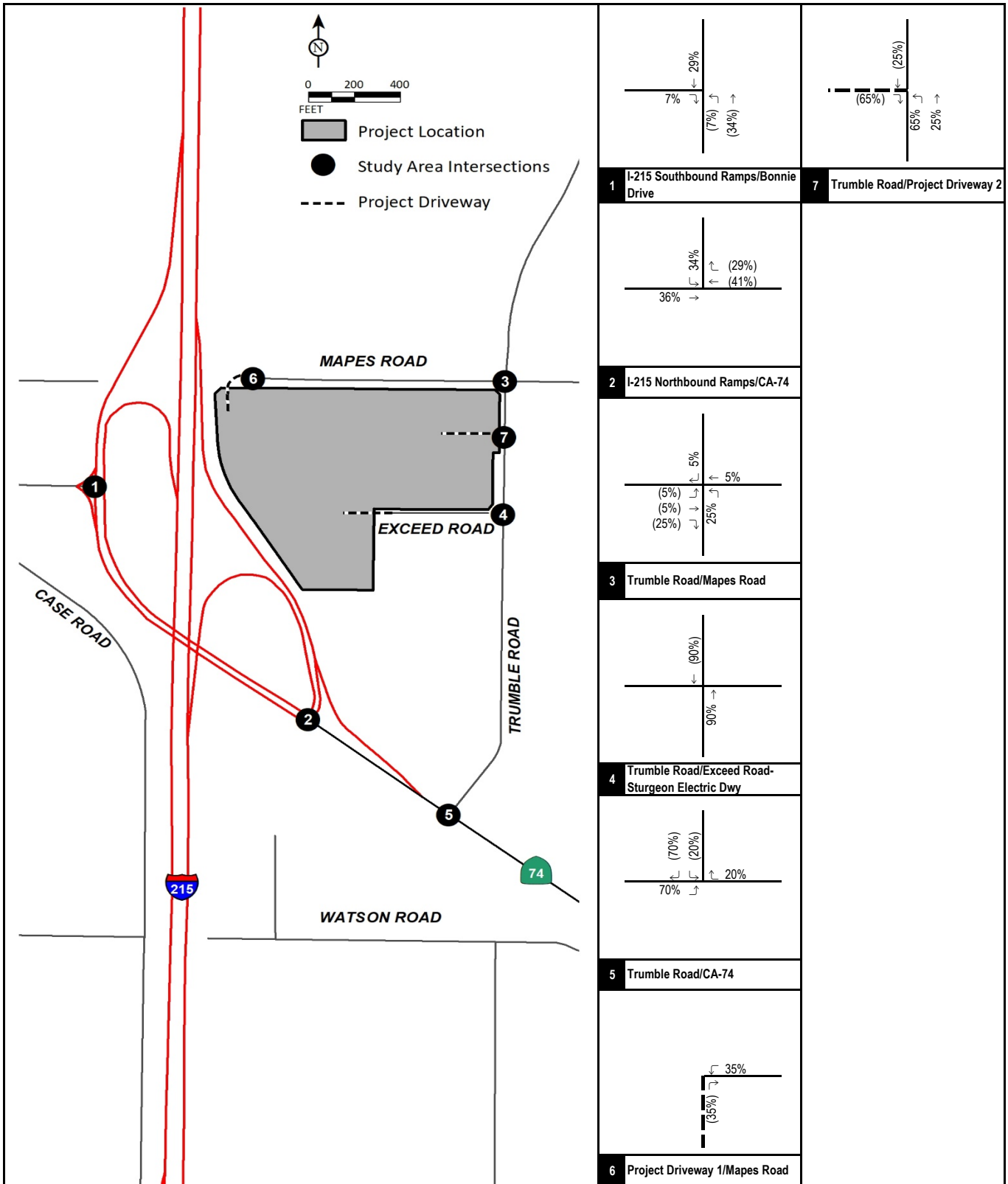
Trip distribution patterns were obtained from select zone model runs of the Riverside County Model version 3.0 (RIVCOM). Separate select zone trip distributions for passenger vehicles and truck trips were obtained from RIVCOM. The select zone distribution plots are included in Appendix D. Project trip distributions were further refined based on the location of the proposed project in relation to surrounding land uses, regional roadway network and truck routes, and in consultation with the City staff. Additionally, as shown in Figure 1-2, all project truck traffic will be accessing the project through the Exceed Road Driveway. Figure 5-1 illustrates the project passenger vehicle trip distribution. Figure 5-2 illustrates the project truck trip distribution.

The project trip assignment is the product of the project trip generation and trip distribution percentages. Figure 5-3 illustrates the project passenger vehicle trip assignment. Figure 5-4

illustrates the project truck trip assignment (in PCE). Figure 5-5 illustrates the total project trip assignment (in PCE) for the proposed project.

5.3 LIST OF CHAPTER 5.0 FIGURES AND TABLES

- Figure 5-1: Project Trip Distribution – Passenger Vehicles
- Figure 5-2: Project Trip Distribution – Trucks
- Figure 5-3: Project Trip Assignment – Passenger Vehicles
- Figure 5-4: Project Trip Assignment – Trucks
- Figure 5-5: Total Project Trip Assignment
- Table 5-A: Project Trip Generation



LSA

XX% (YY%)
 Inbound (Outbound) Trip Distribution
 - - - Project Driveway

FIGURE 5-1

Mapes and Trumble Industrial Facility Project
 Traffic Study

Project Trip Distribution - Passenger Vehicles

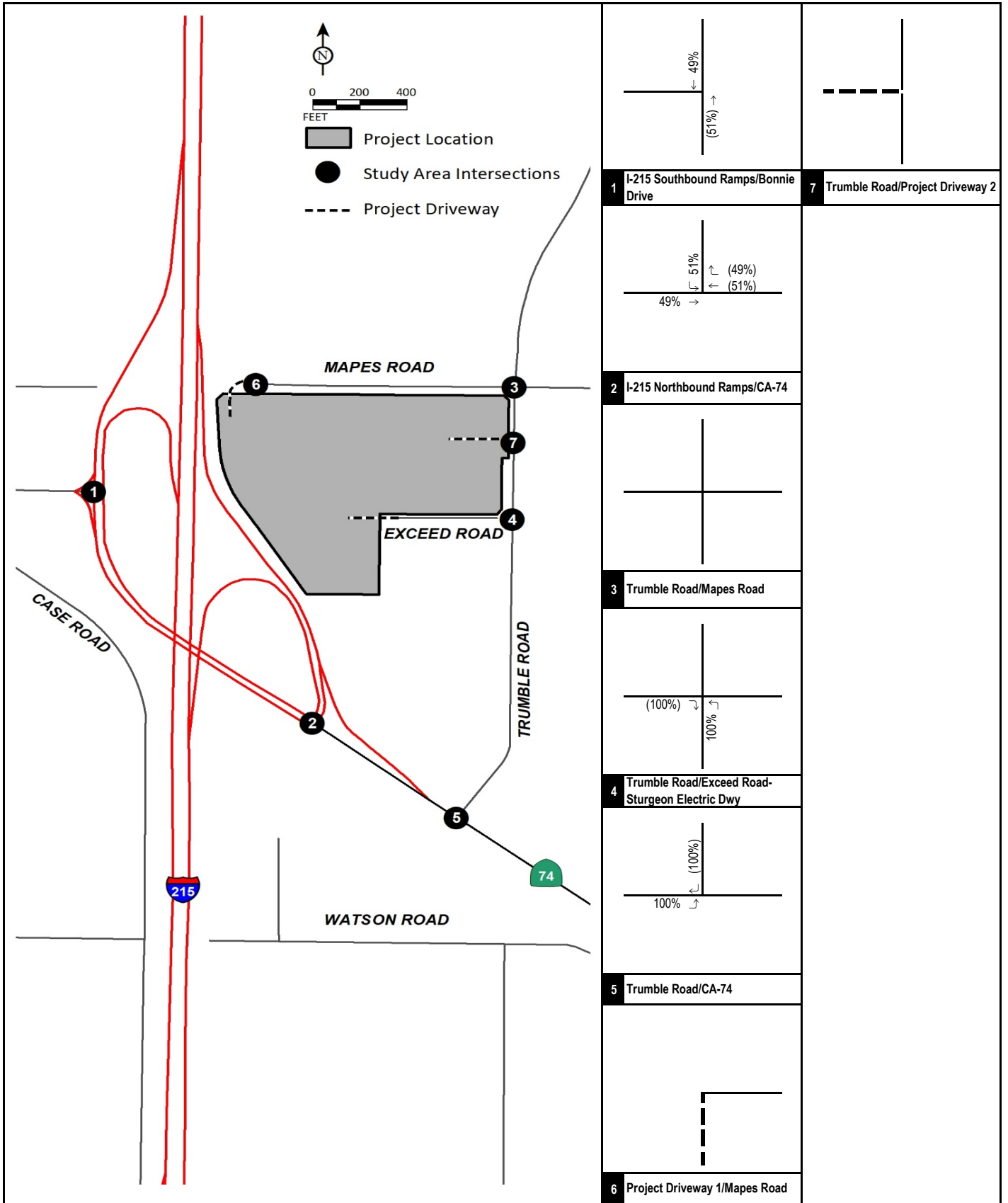


FIGURE 5-2

LSA

XXX% (YYY%)

Inbound (Outbound) Trip Distribution

----- Project Driveway

Mapes and Trumble Industrial Facility Project
Traffic Study

Project Trip Distribution - Trucks

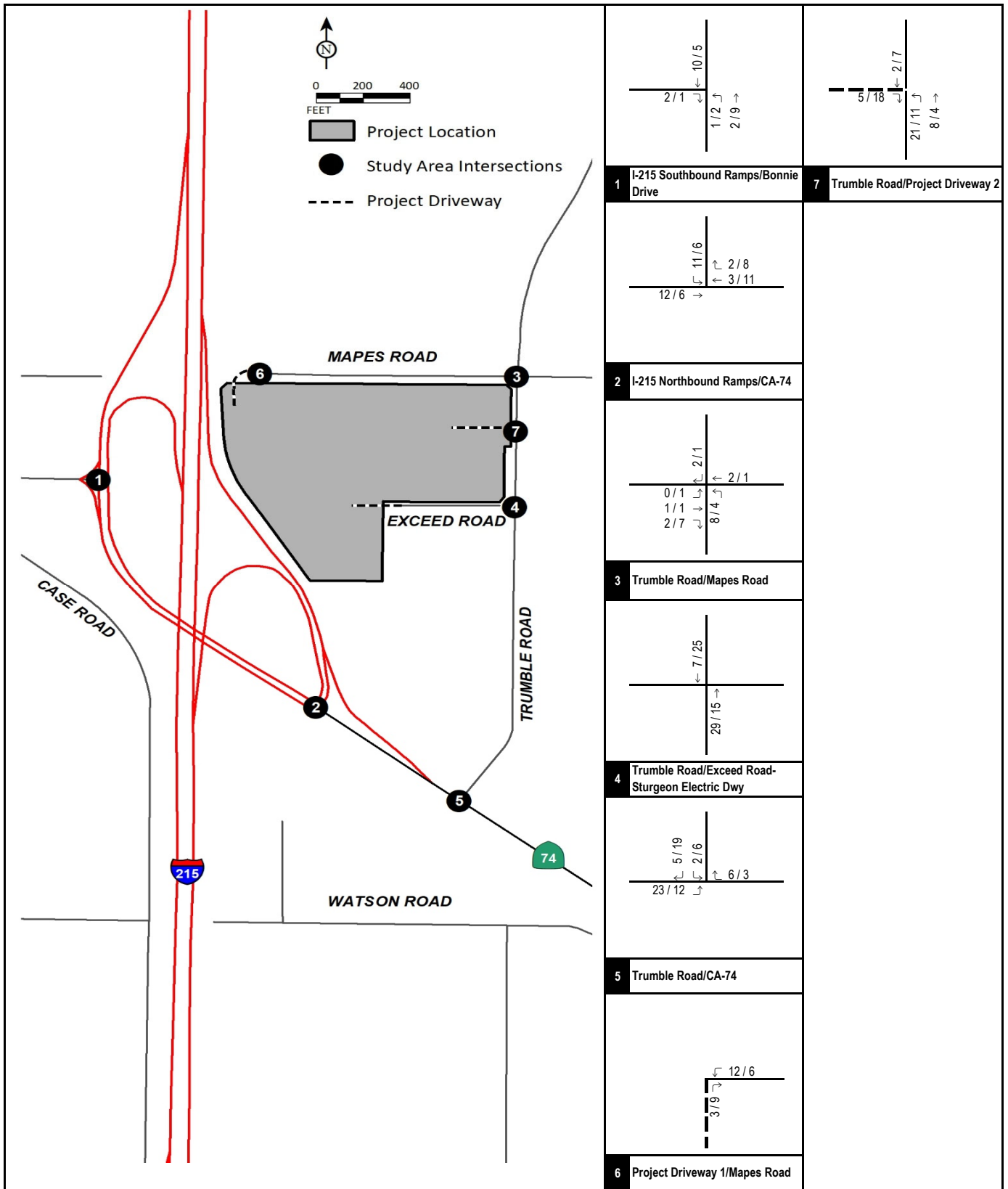


FIGURE 5-3

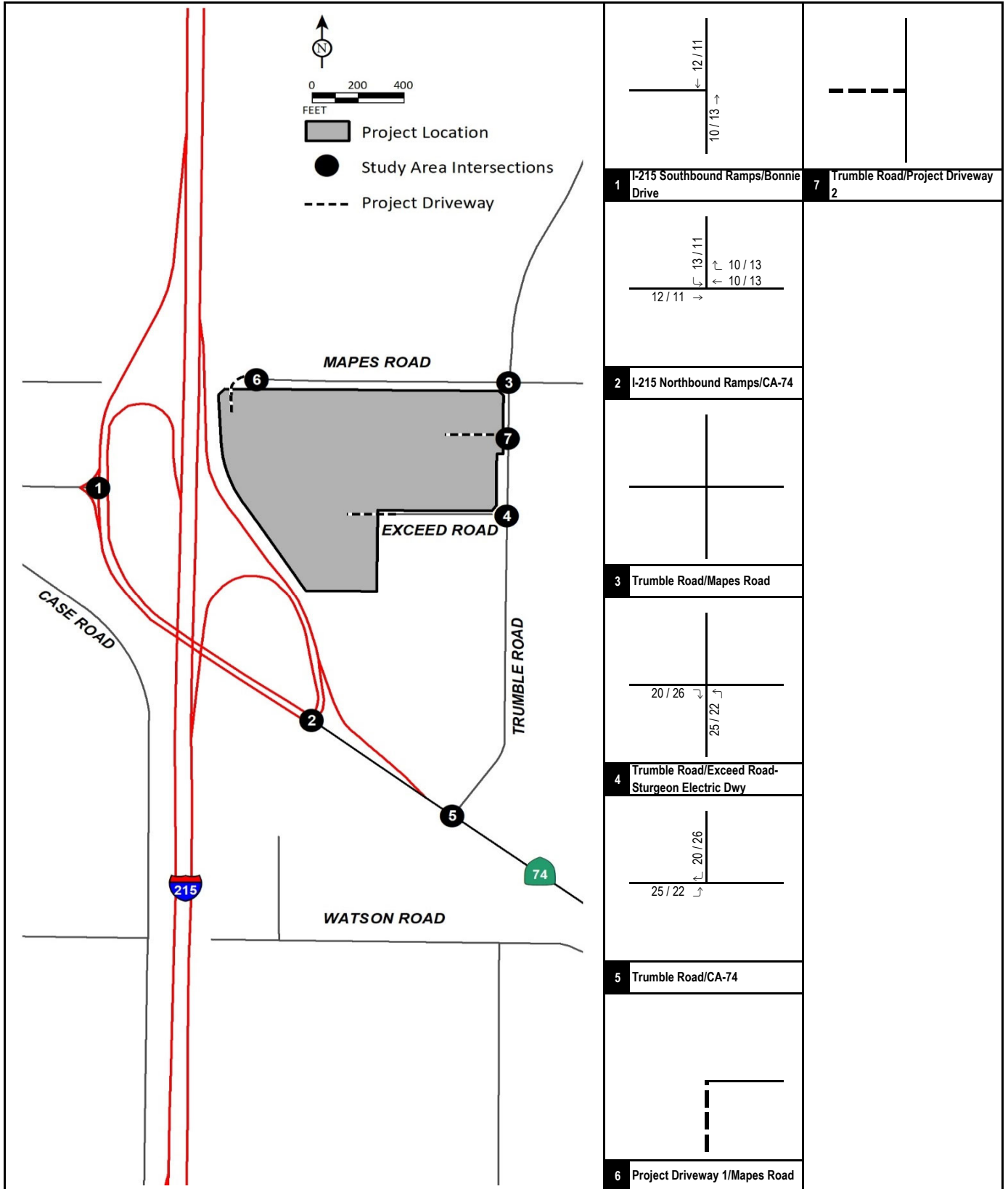
LSA

XX / YY

AM / PM Peak Hour Traffic Volumes

----- Project Driveway

Mapes and Trumble Industrial Facility Project
 Traffic Study
 Project Trip Assignment - Passenger Vehicles



<p>1 I-215 Southbound Ramps/Bonnie Drive</p>	<p>7 Trumble Road/Project Driveway 2</p>
<p>2 I-215 Northbound Ramps/CA-74</p>	
<p>3 Trumble Road/Mapes Road</p>	
<p>4 Trumble Road/Exceed Road-Sturgeon Electric Dwy</p>	
<p>5 Trumble Road/CA-74</p>	
<p>6 Project Driveway 1/Mapes Road</p>	

FIGURE 5-4

LSA

XX / YY

AM / PM Peak Hour PCE Volumes

--- Project Driveway

Mapes and Trumble Industrial Facility Project
Traffic Study

Project Trip Assignment - Trucks

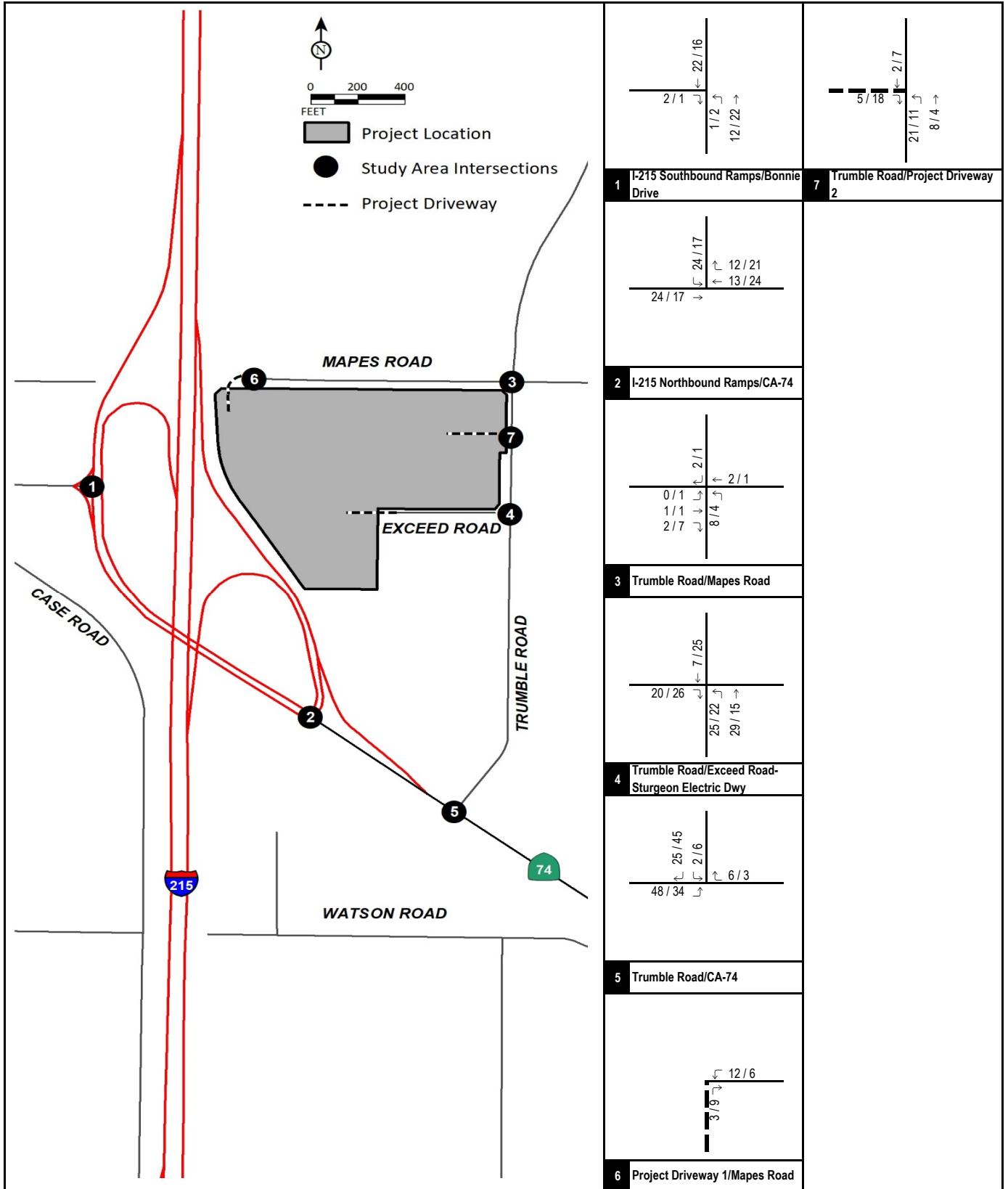


FIGURE 5-5

LSA

XX / YY

AM / PM Peak Hour Traffic Volumes (In PCE)

----- Project Driveway

*Mapes and Trumble Industrial Facility Project
Traffic Study*

Project Total Trip Assignment

Table 5-A - Project Trip Generation

Land Uses	Units	A.M. Peak Hour			P.M. Peak Hour			Daily	
		In	Out	Total	In	Out	Total		
High-Cube Fulfillment Center^{1,2,3}	396	TSF							
Trips/Unit (Cars)			0.084	0.020	0.104	0.043	0.067	0.110	1.249
Trips/Unit (2-Axle Trucks)			0.005	0.005	0.010	0.005	0.006	0.011	0.123
Trips/Unit (3-Axle Trucks)			0.004	0.004	0.008	0.004	0.005	0.009	0.100
Trips/Unit (4+ Axle Trucks)			0.014	0.014	0.028	0.014	0.016	0.030	0.338
Trips/Unit (Total)			0.107	0.043	0.150	0.066	0.094	0.160	1.810
Trip Generation (Cars)			33	8	41	17	27	44	495
Trip Generation (2-Axle Trucks)			2	2	4	2	2	4	49
Trip Generation (3-Axle Trucks)			2	1	3	2	1	3	40
Trip Generation (4+ Axle Trucks)			6	5	11	5	7	12	135
Trip Generation (Total Trucks)			10	8	18	9	10	19	224
Trip Generation (Total)			43	16	59	26	37	63	719
Trip Generation (Cars)			33	8	41	17	27	44	495
PCE Trip Generation (2-Axle Trucks)			3	3	6	3	3	6	74
PCE Trip Generation (3-Axle Trucks)			4	2	6	4	2	6	80
PCE Trip Generation (4+ Axle Trucks)			18	15	33	15	21	36	405
PCE Trip Generation (Total Trucks)			25	20	45	22	26	48	559
PCE Trip Generation (Total)			58	28	86	39	53	92	1,054

Notes:

TSF = thousand square-feet

¹ Rates based on the ITE Trip Generation Manual (11th Edition) for Land Use 155 - "High-Cube Fulfillment Center Warehouse - Non-Sort", Setting Location - "General Urban/Suburban."

² Passenger vehicles and truck in/out splits from Institute of Transportation Engineers (ITE) Trip Generation Manual (11th Edition) rates for Land Use 155 – "High-Cube Fulfillment Center Warehouse - Non-Sort", Setting/Location - "General Urban/Suburban."

³ The resulting trips were converted to trucks and passenger vehicles based on the South Coast Air Quality Management District (SCAQMD) recommendations for warehousing projects. As such, 31 percent of project traffic will be trucks. Based on Vehicle Mix from the SCAQMD, Warehouse Truck Trip Study Data Results and Usage, dated December 2014, the truck mix was considered as 18.7% 4+ axle, 5.5% 3-axle, and 6.8% 2-axle trucks. All truck trips were converted to passenger car equivalents (PCEs) using a 1.5 PCE factor for 2-axle trucks, 2.0 for 3-axle trucks, and 3.0 for 4- and more axle trucks.

6.0 TRAFFIC VOLUMES FOR WITH PROJECT SCENARIOS

Existing and Cumulative (2024) with project traffic volumes were developed by adding project traffic to the corresponding without project scenarios. Figures 6-1 and 6-2 illustrate “with project” peak hour traffic volumes at study intersections under existing and Cumulative (2024) conditions, respectively.

Detailed volume development worksheets are included in Appendix C.

6.1 LIST OF CHAPTER 6.0 FIGURES

- Figure 6-1: Existing with Project Peak Hour Traffic Volumes
- Figure 6-2: Cumulative (2024) with Project Peak Hour Traffic Volumes

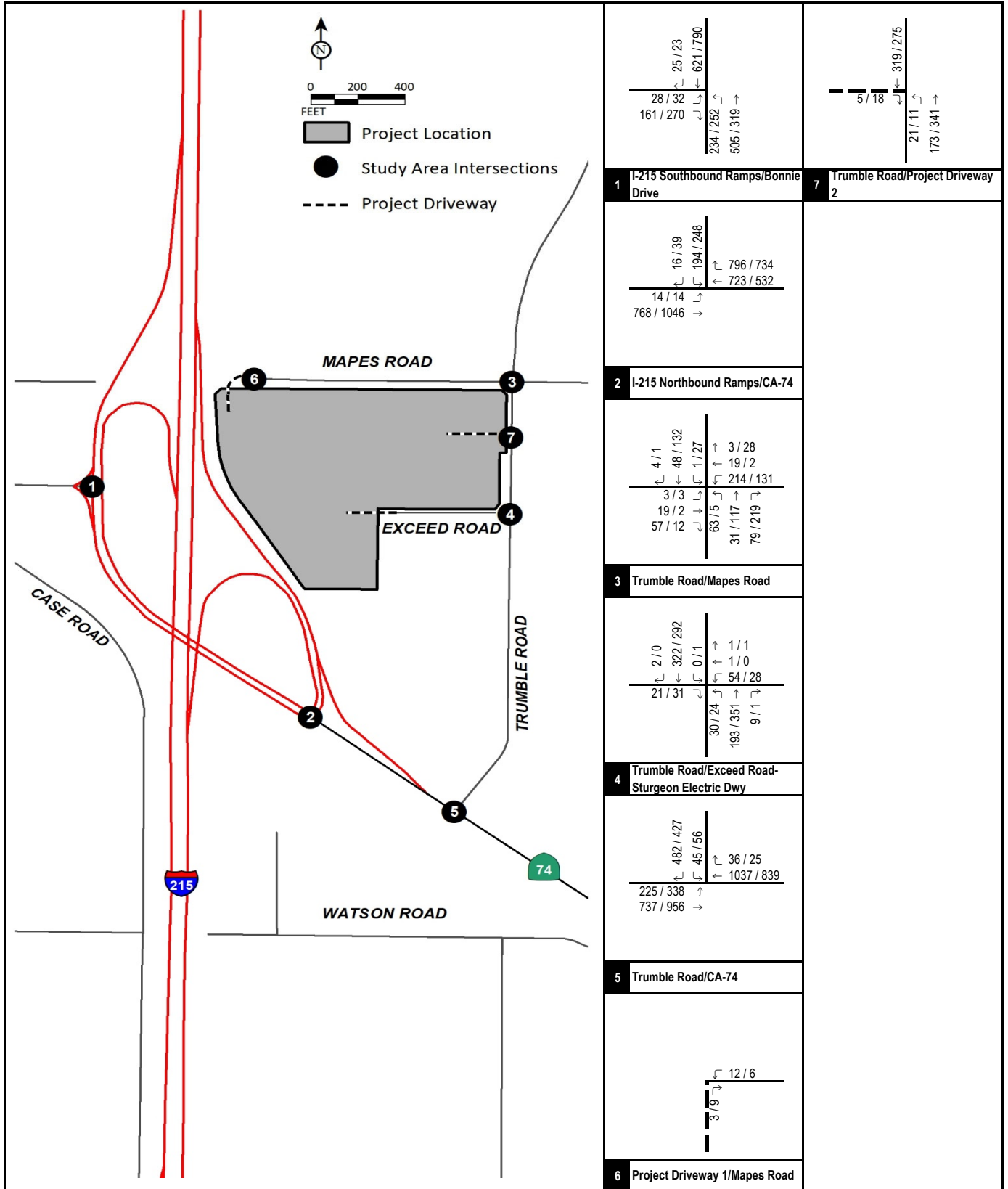


FIGURE 6-1



XXXX / YYYY
 AM / PM Peak Hour Traffic Volumes (In PCE)
 --- Project Driveway

Mapes and Trumble Industrial Facility Project
 Traffic Study

Existing with Project Peak Hour Traffic Volumes

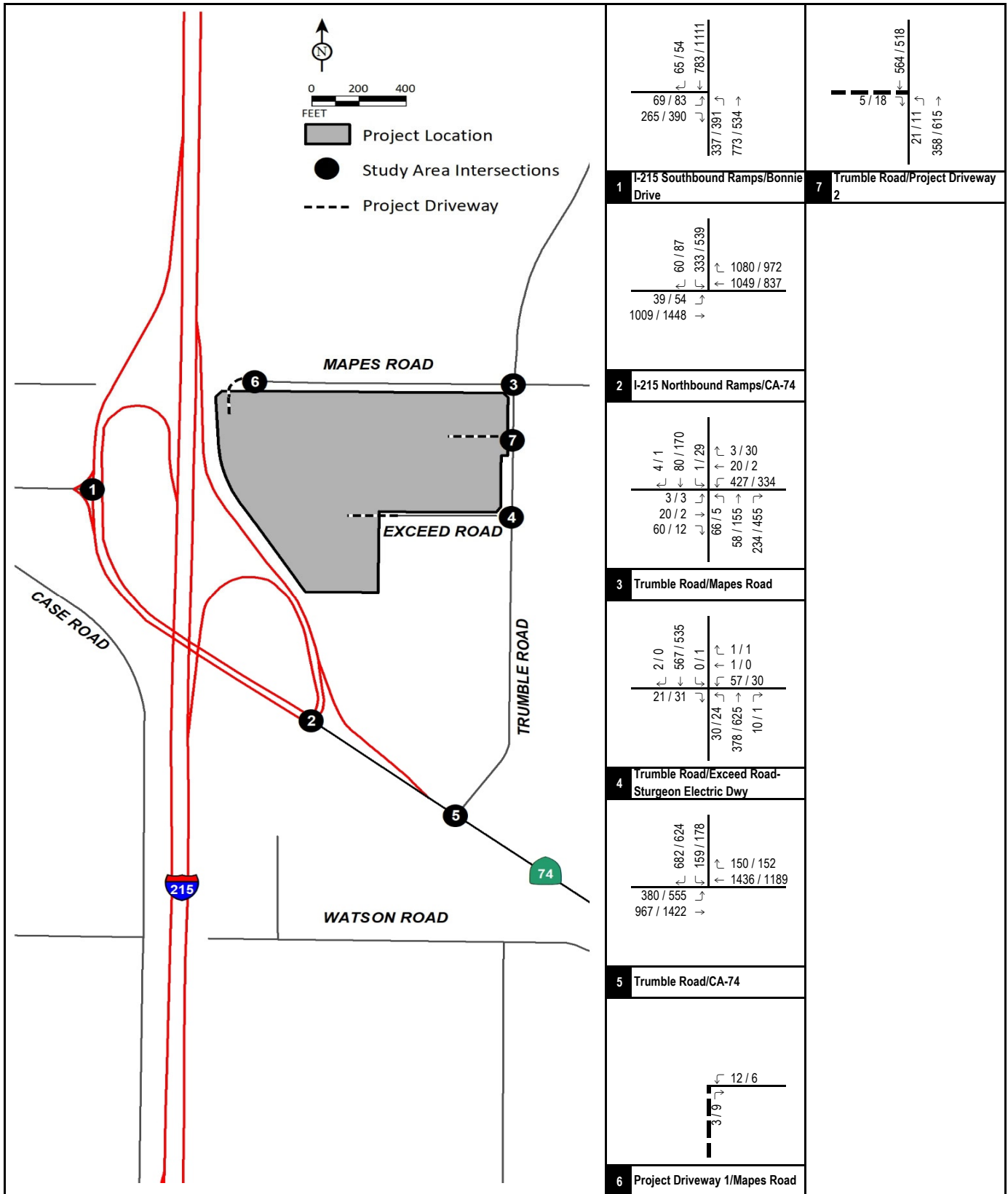


FIGURE 6-2



XXXX / YYYY

AM / PM Peak Hour Traffic Volumes (In PCE)

----- Project Driveway

Mapes and Trumble Industrial Facility Project
 Traffic Study

Cumulative (2024) with Project Peak Hour Traffic Volumes

7.0 INTERSECTION AND ROADWAY SEGMENT LEVELS OF SERVICE

7.1 EXISTING LEVELS OF SERVICE

7.1.1 Study Intersections

Previously referenced Figure 3-1 illustrates existing study intersection geometrics and traffic control. An intersection LOS analysis was conducted for existing conditions using the methodologies previously discussed. Table 7-A summarizes the results of this analysis and shows that all intersections are currently operating at a satisfactory LOS.

Detailed intersection levels of service worksheets are included in Appendix D.

7.2 EXISTING WITH PROJECT LEVELS OF SERVICE

Analysis of the existing with project scenario is provided to identify direct project related operational deficiency if the project were to be built and in operation today. This scenario eliminates the effects of ambient growth and other cumulative projects and deals specifically with operational deficiencies only due to the project traffic.

7.2.1 Study Intersections

An intersection LOS analysis was conducted for existing with project conditions using the methodologies previously discussed. Previously referenced Table 7-A summarizes the results of this analysis and shows that all intersections are forecast to operate at a satisfactory LOS under existing with project conditions.

As such, there is no direct project related operational deficiency caused by the project within the study area.

Detailed intersection levels of service worksheets are included in Appendix D.

7.3 CUMULATIVE (2024) WITHOUT PROJECT LEVELS OF SERVICE

7.3.1 Study Intersections

An intersection LOS analysis was conducted for Cumulative (2024) without project conditions using the methodologies previously discussed. Table 7-B summarizes the results of this analysis and shows that all intersections are forecast to operate at a satisfactory LOS under Cumulative (2024) without project conditions with the exception of the following:

- I-215 Southbound Ramps/Bonnie Drive (p.m. peak hour);
- Trumble Road/Mapes Road (p.m. peak hour); and
- Trumble Road/CA-74 (a.m. and p.m. peak hours).

Detailed intersection levels of service worksheets are included in Appendix D.

7.4 CUMULATIVE (2024) WITH PROJECT LEVELS OF SERVICE

7.4.1 Study Intersections

An intersection LOS analysis was conducted for project completion with project conditions using the methodologies previously discussed. Previously referenced Table 7-B summarizes the results of this analysis and shows that all study intersections are forecast to operate at a satisfactory LOS under Cumulative (2024) with project conditions with the exception of the following:

- I-215 Southbound Ramps/Bonnie Drive (p.m. peak hour);
- Trumble Road/Mapes Road (p.m. peak hour);
- Trumble Road/Exceed Road-Sturgeon Electric Driveway (p.m. peak hour); and
- Trumble Road/CA-74 (a.m. and p.m. peak hours).

It should be noted that except for the intersection of Trumble Road/Exceed Road-Sturgeon Electric Driveway, all other intersections also forecast to operate at a deficient LOS under Cumulative (2024) no project condition. As such, there is a cumulative operational deficiency at these intersections. Additionally, implementation of recommended improvements at the intersection of Trumble Road/Mapes Road alleviates the operational deficiency at the intersection of Trumble Road/Exceed Road-Sturgeon Electric Driveway under Cumulative (2024) with project condition. This has also been described in the Circulation Improvement section of this TS.

Detailed intersection levels of service worksheets are included in Appendix D.

For intersections forecasted to operate at a deficient LOS, corresponding improvements have been identified in Chapter 8 of this report.

7.5 LIST OF CHAPTER 7.0 TABLES

- Table 7-A: Existing Intersection Levels of Service
- Table 7-B: Cumulative (2024) Intersection Levels of Service

Table 7-A - Existing Intersection Levels of Service

Intersection	Control	Without Project				With Project			
		AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour	
		Delay (sec.)	LOS	Delay (sec.)	LOS	Delay (sec.)	LOS	Delay (sec.)	LOS
1 . I-215 Southbound Ramps/Bonnie Drive	Signal	18.3	B	23.8	C	18.3	B	23.8	C
2 . I-215 Northbound Ramps/CA-74	Signal	9.7	A	12.5	B	10.5	B	13.3	B
3 . Trumble Road/Mapes Road	AWSC	9.1	A	10.6	B	9.2	A	11.1	B
4 . Trumble Road/Exceed Road-Sturgeon Electric Dwy	TWSC	12.1	B	15.2	C	13.9	B	17.7	C
5 . Trumble Road/CA-74	Signal	31.4	C	30.9	C	38.5	D	38.6	D
6 . Project Driveway 1/Mapes Road	OWSC	<i>Future Intersection</i>		<i>Future Intersection</i>		8.3	A	8.4	A
7 . Trumble Road/Project Driveway 2	OWSC	<i>Future Intersection</i>		<i>Future Intersection</i>		9.3	A	9.2	A

Notes:

OWSC = One-Way Stop Control; TWSC = Two-Way Stop Control; AWSC = All way Stop Control.

Delay = Average control delay in seconds (For OWSC/TWSC intersections, reported delay is for worst-case movement).

LOS = Level of Service

* Exceeds LOS Standard

Table 7-B - Cumulative (2024) Intersection Levels of Service

Intersection	Control	Without Project				With Project			
		AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour	
		Delay (sec.)	LOS	Delay (sec.)	LOS	Delay (sec.)	LOS	Delay (sec.)	LOS
1 . I-215 Southbound Ramps/Bonnie Drive	Signal	33.0	C	78.0	E *	33.2	C	80.6	F *
2 . I-215 Northbound Ramps/CA-74	Signal	17.8	B	41.6	D	18.9	B	44.8	D
3 . Trumble Road/Mapes Road	AWSC	12.9	B	55.5	F *	12.9	B	60.0	F *
4 . Trumble Road/Exceed Road-Sturgeon Electric Dwy	TWSC	18.8	C	32.4	D	22.9	C	41.9	E *
5 . Trumble Road/CA-74	Signal	>100	F *	>100	F *	>100	F *	>100	F *
6 . Project Driveway 1/Mapes Road	OWSC	<i>Future Intersection</i>		<i>Future Intersection</i>		8.3	A	8.4	A
7 . Trumble Road/Project Driveway 2	OWSC	<i>Future Intersection</i>		<i>Future Intersection</i>		10.3	B	10.2	B

Notes:

OWSC = One-Way Stop Control; TWSC = Two-Way Stop Control; AWSC = All way Stop Control.

Delay = Average control delay in seconds (For OWSC/TWSC intersections, reported delay is for worst-case movement).

LOS = Level of Service

* Exceeds LOS Standard

8.0 CIRCULATION IMPROVEMENTS AND FUNDING SOURCES

8.1 RECOMMENDED IMPROVEMENTS

Improvements have been recommended at study intersections where an operational deficiency has been identified or where the project contributes to an unsatisfactory LOS. The recommended improvements are as follows:

8.1.1 Cumulative (2024) with Project Conditions Intersection Improvements

- I-215 Southbound Ramps/Bonnie Drive: Add a second southbound through lane. Align this lane with the receiving lane of the free eastbound right turn lane in the south leg of the intersection. Convert the free eastbound right turn lane to yield to the southbound through traffic. Optimize the signal timing.
- Trumble Road/Mapes Road: Install a signal. A peak hour signal warrant analysis was conducted to determine the feasibility of this improvement under Cumulative (2024) without and with project conditions. Figure 8-1 illustrates the result of this warrant analysis and shows that this intersection meets the signal warrant for both a.m. and p.m. peak hour for both without and with project condition. As such, installing a signal at this intersection is recommended.
- Trumble Road/Exceed Road-Sturgeon Electric Driveway: Installing a signal at the intersection of Trumble Road/Mapes Road eliminates the operational deficiency at this intersection. No further improvement is required at this intersection. It should be noted that the inclusion of a traffic signal at Trumble Road/Mapes Road improves traffic operations at this intersection to an acceptable LOS, because the nearby traffic signal produces larger gaps in traffic along Trumble Road. This has been further evaluated through a simulation analysis (SimTraffic), and has been discussed in detail in chapter 10.
- Trumble Road/CA-74: Add a second southbound right-turn lane. Optimize the signal timing with southbound right overlap phasing.

Table 8-A summarizes the level of service at these intersections with the recommended improvements under Cumulative (2024) with project conditions. Figure 8-2 illustrates the Cumulative (2024) with project with improvements intersection lane geometrics and traffic control. Figure 8-3 illustrates a conceptual striping plan for the proposed improvements at the intersection of I-215 Southbound Ramps/Bonnie Drive.

Detailed Level of Service Worksheets are included in Appendix D.

8.2 FUNDING SOURCES AND MECHANISMS

Where there is a funding mechanism (fee program) in place for the recommended improvements, payment into the fee program would be considered sufficient project obligation to alleviate project related operational deficiencies. At study locations where the addition of project traffic creates an operational deficiency (existing with project conditions) and there is no funding mechanism in place, the project will be responsible for the implementation of the improvements. At locations where the

project adds to or creates a forecast deficiency and there is no funding mechanism in place, the project is responsible for its fair-share payment.

8.2.1 Transportation Uniform Mitigation Fee (TUMF) Program

The underlying purpose of the TUMF program is “the need to establish a comprehensive funding source to mitigate the cumulative regional transportation impacts of new development on regional arterial highways.” As new development occurs in western Riverside County, the cumulative transportation impacts of this new development are reflected in increased demand for transportation infrastructure leading to decreased levels of service, increased delay and increased congestion on regional transportation facilities, and an overall decline in regional mobility. Therefore, the need to invest in additional transportation infrastructure to meet the increased travel demand and to sustain pre-development traffic conditions to “keep traffic flowing” represents the fundamental premise of the TUMF program. TUMF program includes I-215/SR-74 interchange improvement including recommended improvements at the intersection of I-215 Southbound Ramps/Bonnie Drive. Therefore, the project will be paying into the TUMF program for this improvement.

8.2.2 Project Fair Share

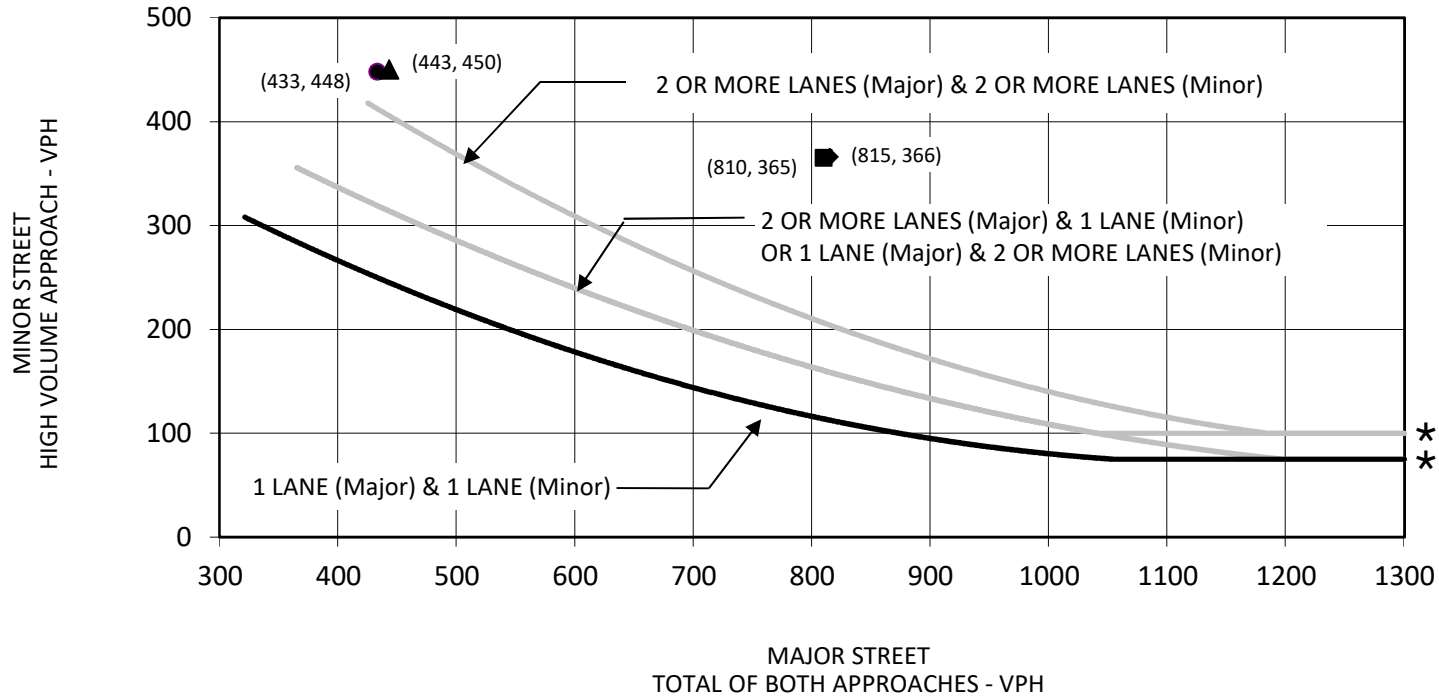
In the absence of a fee program where the project has an impact on the roadway network, the project will pay its respective fair share for the proposed improvements. The project’s fair share has been calculated based on project traffic as a percentage of total growth from existing to cumulative (2024) with project conditions. Table 8-B summarizes the project fair share for the deficient intersections. However, it should be noted that since the project will be paying to the TUMF fee program, a fair share payment will not be required for the intersection of I-215 Southbound Ramps/Bonnie Drive. Since the project has a cumulative operational deficiency at the intersections of Trumble Road/Mapes Road, and Trumble Road/SR-74, the project will be required to pay its fair share.

8.3 LIST OF CHAPTER 8.0 FIGURES AND TABLES

- Figure 8-1: Warrant 3: Peak Hour- Trumble Road/Mapes Road – Cumulative (2024) Conditions
- Figure 8-2: Cumulative (2024) with project with Improvements Intersection Geometrics and Traffic Control
- Figure 8-3: Conceptual Striping Plan - I-215 Southbound Ramps/Bonnie Drive
- Table 8-A: Cumulative (2024) with Project With Improvements Intersection Levels of Service
- Table 8-B: Project Fair-Share

WARRANT 3, PEAK HOUR (70% FACTOR)

(COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 mph ON MAJOR STREET)



★ 100 VPH applies as the lower threshold volume for a minor street approach with two or more lanes and 75 VPH applies as the lower threshold volume for a minor street approaching with one lane.



FIGURE 8-1

- No Project AM Peak Hour
- ▲ With Project AM Peak Hour
- No Project PM Peak Hour
- ◆ With Project PM Peak Hour

Mapes and Trumble Industrial Facility Project
Traffic Study

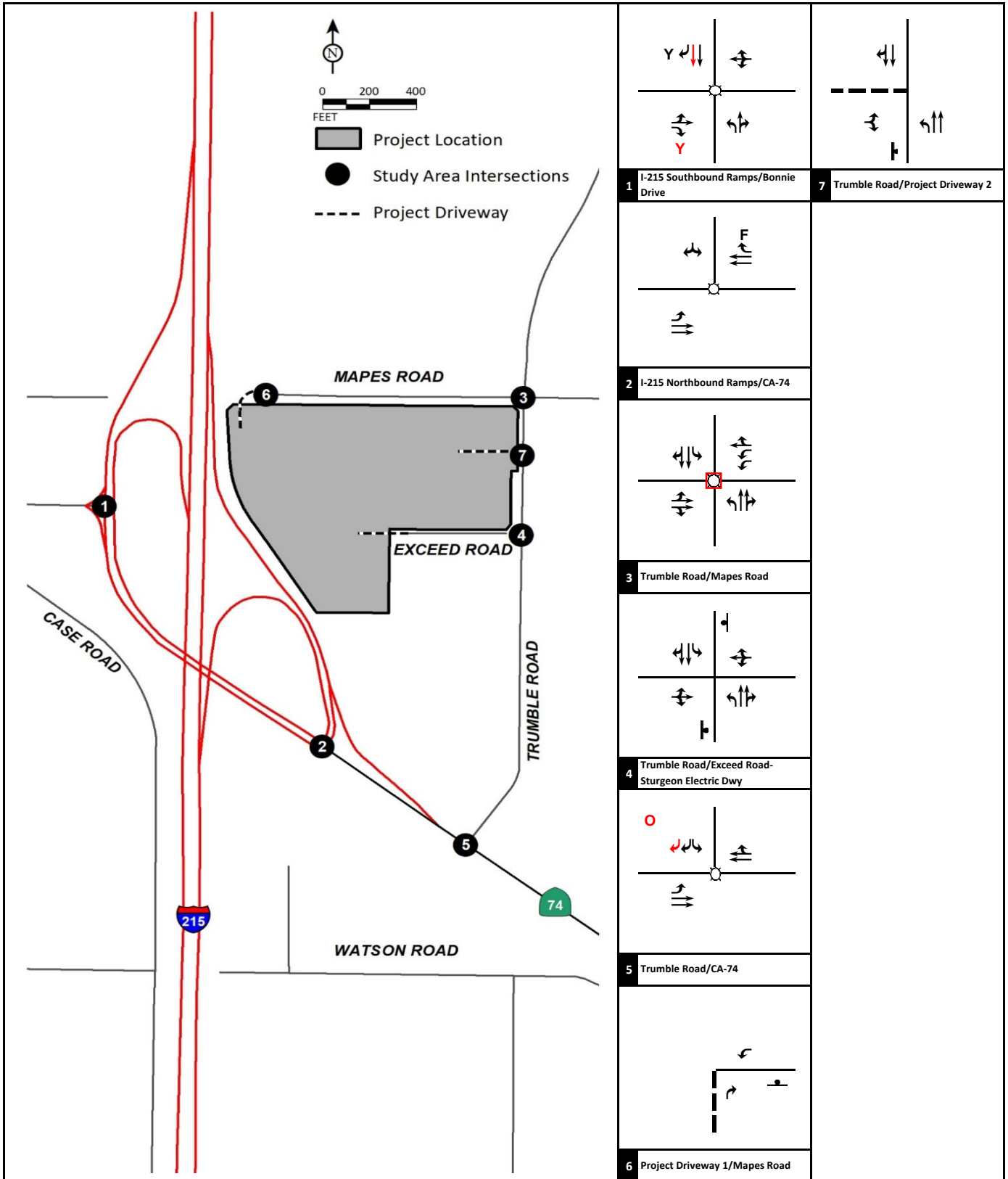


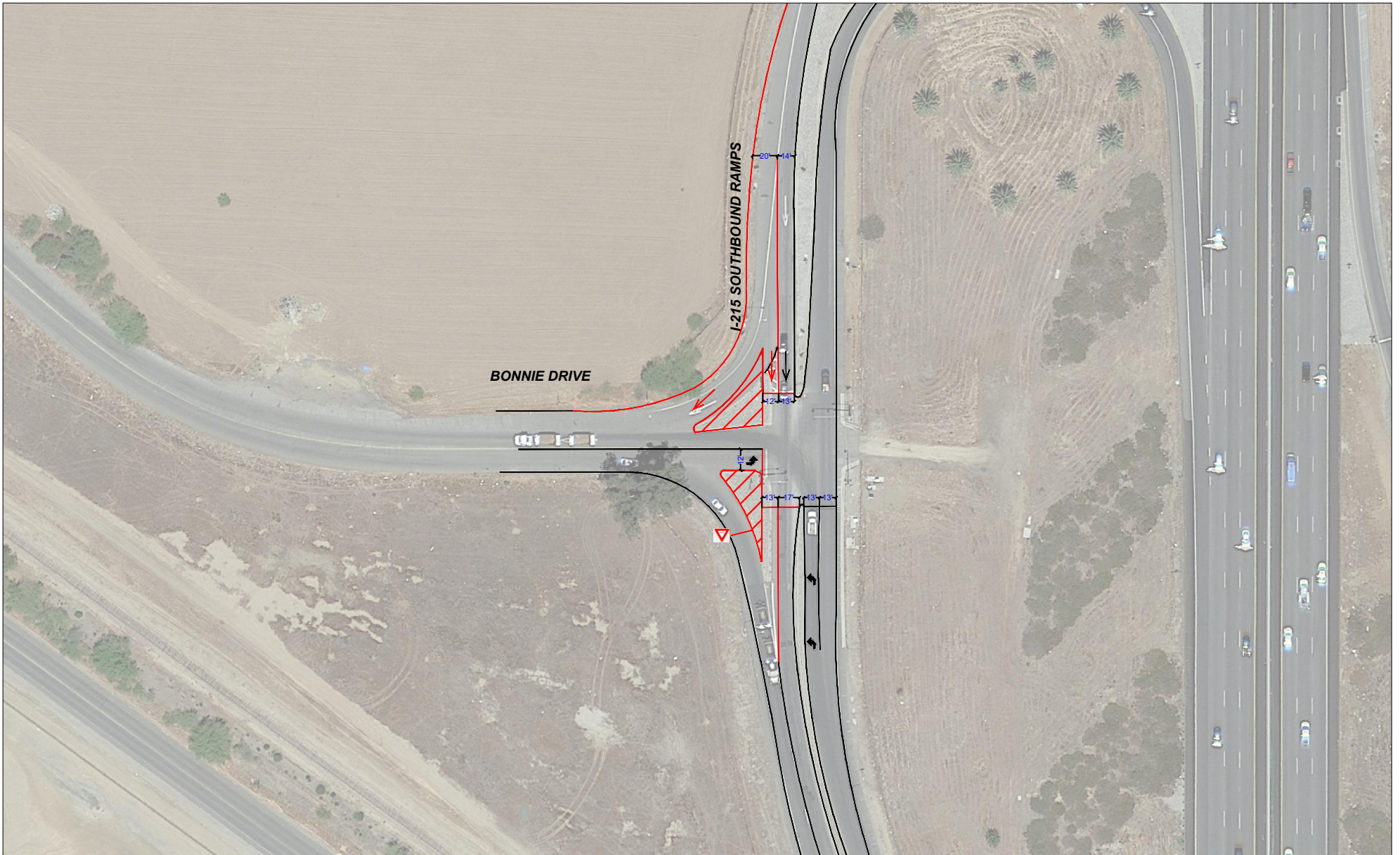
FIGURE 8-2

LSA

- Legend
- Project Driveway
 - Overlap Phasing
 - ↔ Stop Sign
 - Project Driveway
 - Overlap Phasing
 - F Free Right Turn
 - ↔ Proposed Improvements
 - Y Yield to Through Traffic

Mapes and Trumble Industrial Facility Project
Traffic Study

Cumulative (2024) with project with Improvements Intersection Geometrics and Traffic Control



LSA

- LEGEND
- Existing Lane
 - Proposed Lane

FIGURE 8-3

*Mapes and Trumble Industrial Facility Project
Traffic Study*

Conceptual Striping Plan
I-215 southbound Ramps/Bonnie Drive

SOURCE: Google Earth, 2018.
I:\BAV2101-RIV\Traffic\Reports\striping.dwg (06/06/2022)

Table 8-A - Cumulative (2024) With Project with Improvements Intersection Levels of Service

Intersection	With Project Without Improvements					With Project With Improvements					
	Control	AM Peak Hour		PM Peak Hour			Control	AM Peak Hour		PM Peak Hour	
		Delay (sec.)	LOS	Delay (sec.)	LOS	*		Delay (sec.)	LOS	Delay (sec.)	LOS
1 . I-215 Southbound Ramps/Bonnie Drive	Signal	33.2	C	80.6	F	*	Signal	30.1	C	29.8	C
3 . Trumble Road/Mapes Road	AWSC	12.9	B	60.0	F	*	Signal	20.3	C	15.6	B
4 . Trumble Road/Exceed Road-Sturgeon Electric Dwy	TWSC	22.9	C	41.9	E	*	TWSC	22.9	C	25.6	D
5 . Trumble Road/CA-74	Signal	>100	F	>100	F	*	Signal	50.8	D	43.2	D

Notes:

Delay = Average control delay in seconds.

LOS = Level of Service

* Exceeds LOS Standard

Table 8-B - Project Fair-Share

Intersection	A.M. Peak Hour					P.M. Peak Hour					Project Fair Share %
	Total Volume		Total Growth	Project Trips	AM Fair Share %	Total Volume		Total Growth	Project Trips	PM Fair Share %	
	Existing	Cumul + Project				Existing	Cumul + Project				
1 . I-215 Southbound Ramps/Bonnie Drive	1,537	2,292	755	37	N/A	1,645	2,563	918	41	4.47%	4.47%
3 . Trumble Road/Mapes Road	526	976	450	15	N/A	664	1,198	534	15	2.81%	2.81%
4 . Trumble Road/Exceed Road-Sturgeon Electric Dwy	552	1,067	515	81	N/A	641	1,248	607	88	14.50%	14.50%
5 . Trumble Road/CA-74	2,481	3,776	1,295	81	6.25%	2,553	4,124	1,571	88	5.60%	6.25%

Notes:

N/A = Not Applicable

N/A indicates that since the project does not create an operational deficiency for the said peak hour.

Bold = Project Fair Share Percentage is the highest fair share value of the AM and PM peak hour when both peak hours are impacted by the project, or only in the peak hour where the project has an impact.

9.0 VEHICLE MILES TRAVELED ANALYSIS

On December 28, 2018, the California Office of Administrative Law cleared the revised California Environmental Quality Act (CEQA) guidelines for use. Among the changes to the guidelines was removal of vehicle delay and level of service from consideration under CEQA. With the adopted guidelines, transportation impacts are to be evaluated based on a project's effect on vehicle miles traveled (VMT).

For projects within the City, the VMT analysis is based upon the methodology and significance impact criteria set in the *City of Perris Transportation Impact Analysis Guidelines for CEQA* (VMT Guidelines), dated May 2020. The City's Guidelines includes the screening criteria, VMT analysis methodology, VMT impact thresholds, and VMT mitigation measures. Therefore, the City's VMT Guidelines was used in the evaluation of the project's VMT impact.

9.1 METHODOLOGY

9.1.1 Project Screening Criteria

The VMT Guidelines provides multiple screening criteria for land use projects. The project was compared with the screening criteria established in the "Project Screening" section of the VMT Guidelines to check if the project could be screened out from a detailed VMT analysis. Following is a brief description about the project in relation with the project screening criteria:

- **Transit Priority Area (TPA) Screening:** The project is not located within a TPA. Therefore, this screening criteria does not apply to the project.
- **Low Trip Generator:** The VMT Guidelines identifies that projects generating less than 500 daily trips could also be screened out. As discussed in the project trip generation section, the project is estimated to generate 719 daily trips. Therefore, the project does not satisfy this screening criteria.
- **Affordable Housing/Local Serving Land Use:** The project is not an affordable housing project nor it could be classified as retail, institutional/government uses or public service uses. Therefore, these criteria do not apply for the project.
- **Located in a Low VMT Area:** The project is located within RIVCOM TAZ 1847. The project was evaluated using the WRCOG screening tool to determine whether the project is located within a low VMT area. As analyzed in the WRCOG screening tool, the base year (2018) project location/project TAZ VMT per employee is 19.5, that is greater than the citywide average VMT per employee, 17.1. As such, the project TAZ's VMT per employee is 14.26% higher than City's VMT per employee under base year 2018 conditions. Therefore, the project is not located within a low VMT area and cannot be screened out from a detailed VMT analysis. The WRCOG screening tool result is included in Appendix E.

Since the project could not be screened out from a detailed VMT analysis, a detailed VMT analysis for the project was conducted to evaluate whether the project will have any significant VMT impact.

9.1.2 Methodology and Thresholds of Significance for Detailed VMT Analysis

This VMT analysis was conducted using the recommended methodology included in the VMT Impact Analysis section of the VMT Guidelines. As such, RIVCOM was used for VMT calculations for the project. The RIVCOM model was used to calculate the project and Citywide VMT per service population. The VMT Guidelines recommends comparing the project generated VMT per service population for both the base and future year with the City's threshold of significance to identify potential VMT impacts. The VMT Guidelines also establishes City of Perris Base year VMT per service population as the significance threshold for both base and future year. The project VMT was calculated from RIVCOM model run as described below:

9.1.3 Project Traffic Analysis Zone Update

The first step in preparation of this analysis was to update the traffic analysis zone (TAZ) in the model that includes the project area. The project land uses were converted into model socioeconomic categories using appropriate rates from ITE *Trip Generation Manual*, 11th Edition. The socioeconomic data for the project TAZ in both RIVCOM base year and future year model scenario was updated. Project socioeconomic data was added to the project TAZ for both scenarios. Non-project related socioeconomic data was shifted to an adjacent TAZ to isolate project from other land uses. Non-project related socioeconomic data for the future scenario was adjusted to account for the project.

9.1.4 Model Runs and Project VMT Estimation

Model run was conducted for this updated model upon completion of the socioeconomic data update for both base and future year. The outputs from these with project model runs were utilized to calculate the base and future year project VMT per service population. The Citywide VMT per service population for the base year was calculated from the base year no project model run.

9.2 PROJECT VMT ANALYSIS

Table 9-A summarizes the significant threshold and project VMT per service population for the base year. As shown in Table 9-A, the project VMT per service population under base year is 19.17% lower than the Citywide VMT per service population for the base year.

Table 9-B summarizes the significant threshold and project VMT per service population for the future year. As shown in Table 9-B, the project VMT per service population under future year is 27.71% lower than the Citywide VMT per service population for the base year.

Therefore, based on the City's significance threshold as established in the VMT Guidelines, the project will not have any significant VMT impact.

Detailed VMT calculation for the project is included in Appendix E.

9.3 LIST OF CHAPTER 9.0 TABLES

- Table 9-A: Base Year Significant Threshold and Project VMT per Service Population
- Table 9-B: Future Year Significant Threshold and Project VMT per Service Population

Table 9-A: Base Year Significant Threshold and Project VMT per Service Population

Significant Threshold*	Project	Difference	Percentage Difference	Significant Impact
32.4	26.2	6.2	-19.17%	No

* Obtained from RIVCOM Base Year No Project Model Run.

Table 9-B: Future Year Significant Threshold and Project VMT per Service Population

Significant Threshold*	Project	Difference	Percentage Difference	Significant Impact
32.4	23.5	8.9	-27.71%	No

* Obtained from RIVCOM Base Year No Project Model Run.

10.0 ACCESS ANALYSIS

10.1 ACCESS ANALYSIS

As discussed previously and shown in Figure 1-2, access to the project will be provided via three driveways. The driveway located on Mapes Road, and the driveway located on Trumble Road will be used by only passenger vehicles, while the driveway located on Exceed Road will be used by only trucks.

10.1.1 Truck Turning Analysis

As previously mentioned, project related truck traffic will access the project using the driveway and the gated entrance located at the western end of Exceed Road. Additionally, as summarized in the project trip distribution section, project truck traffic are anticipated to access the site using Trumble Road. Therefore, this analysis evaluates the adequacy of truck turning radii at the intersection of Trumble Road/Exceed Road-Sturgeon Electric Driveway for truck ingress and egress. As a conservative estimate, large semitrailer (WB-67) templates were used to evaluate the adequacy of truck turning radii. The truck template is in accordance with the 2018 Edition of the American Association of State Highway and Transportation Officials (AASHTO) Green Book. Figure 10-1 illustrates the ingress truck turning template for the project at this intersection. Figure 10-2 illustrates the egress truck turning template. As illustrated in Figures 10-1 and 10-2, there is adequate roadway width to accommodate both inbound and outbound truck turning radii. It should be noted that currently there is provision for on-street parking on Exceed Road. However, for a smooth maneuver of project truck traffic, the on-street parking provision needs to be removed along Exceed Road as follows:

- Along the southern edge of Exceed Road, on-street parking provision needs to be removed between the intersection of Trumble Road/Exceed Road-Sturgeon Electric Driveway and the easterly driveway for Brenthel Industries project (approximately 100 feet).
- Similarly, along the northern edge of Exceed Road, on-street parking provision needs to be removed for approximately 100 feet from the intersection of Trumble Road/Exceed Road-Sturgeon Electric Driveway.

10.1.2 Truck Ingress Gate Stacking Analysis

As previously mentioned, project truck access will be restricted with an entry gate, located at the west end of Exceed Road. Project truck traffic needs to wait in front of the entry gate before entering the project site. Therefore, it was evaluated whether project ingress traffic will spill over on Trumble Road due to the gated access at the project entrance.

The length of Exceed Road is approximately 520 feet. As such, considering that an average WB-67 truck would take up approximately 100 ft. per truck, Exceed Road can accommodate up to five trucks without spilling over to Trumble Road. As summarized in Table 5-A, the highest project ingress traffic occurs during the a.m. peak hour, with 10 inbound trucks.

As such, considering an even distribution of ingress project truck traffic arriving during the a.m. peak hour, it is anticipated a truck would be arriving in every six minutes. That would provide adequate time for a truck to enter the project site without creating a queue along Exceed Road. Therefore, it is estimated that the ingress project truck traffic would not spill over to Trumble Road.

10.2 INTERSECTION QUEUING AND BLOCKING

As included in Chapter 8.0, a signal has been recommended for the intersection of Trumble Road/Mapes Road as a recommended improvement. Typically, signalization at nearby intersection helps cross street traffic by providing larger gaps in traffic flow along the major travel route (Trumble Road). However, it can also adversely affect unsignalized minor cross streets by creating a long enough queue to restrict the movements of cross street traffic at these nearby unsignalized locations. To identify whether the recommended signal would adversely affect the cross-street traffic at the intersection of Trumble Road/Exceed Road-Sturgeon Electric Driveway, a simulation analysis was prepared using SimTraffic to assess the queuing and whether it would potentially block cross traffic at this intersection.

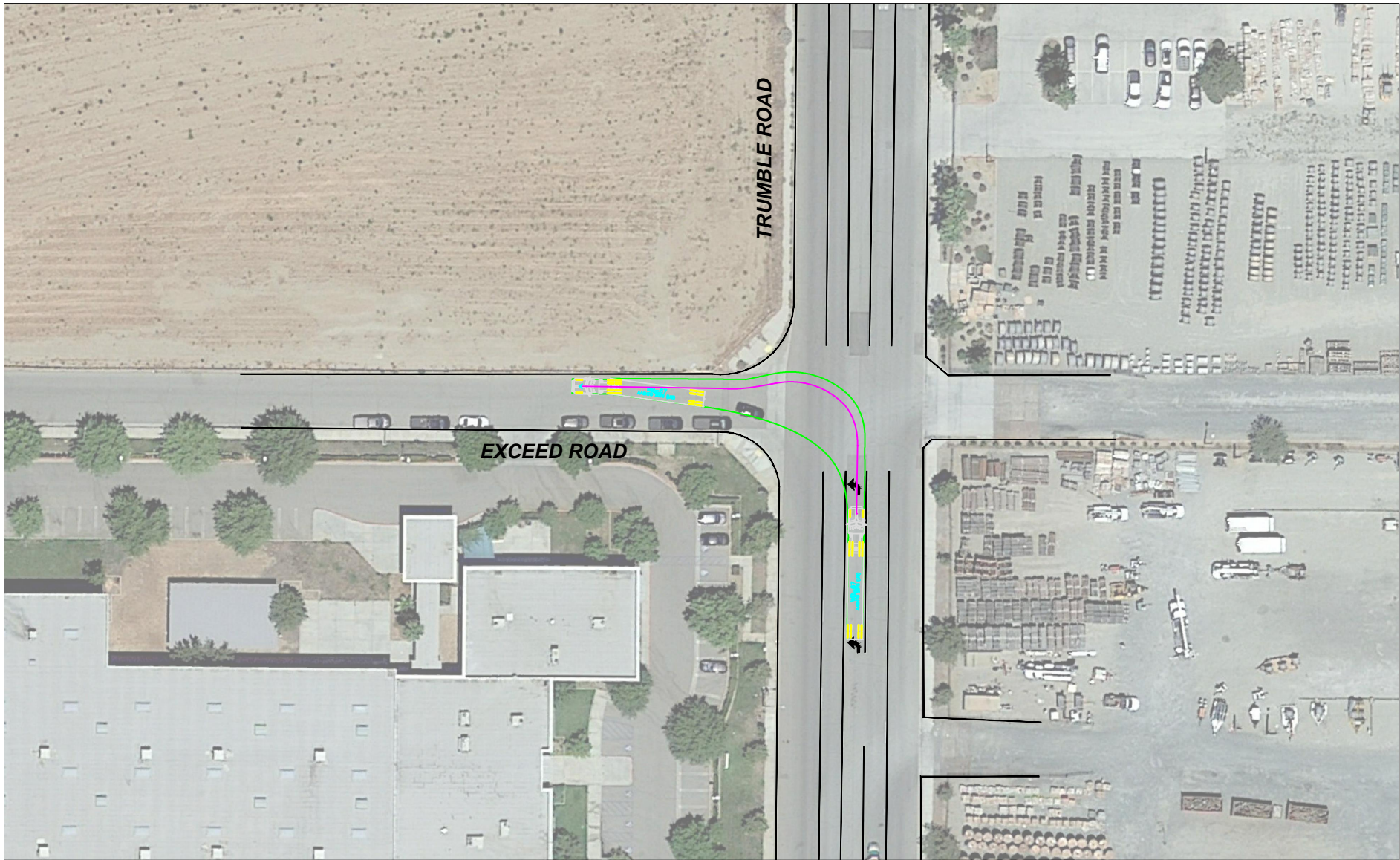
As part of the simulation analysis, a 95th percentile queuing analysis was conducted at the intersection of Trumble Road/Mapes Road to determine whether queues resulting from signalization are estimated to block any other driveways adjacent to the intersection. The highest queue forecast at this intersection after signalization is for westbound left turn movement, 130 feet in the a.m. peak hour, and 105 feet in the p.m. peak hour. All other forecasted queues reported at this intersection is less than 50 feet. The nearest driveway distance from the intersection is currently 390 feet on the north leg, and 475 feet along the south leg. The project proposes to construct a driveway which will be approximately 295 feet from this intersection along the south leg. As such, queues resulting from signalization at this intersection is not estimated to block any of the existing or proposed driveways.

In summary, based on the simulation analysis, signalization of Trumble Road/Mapes Road is not estimated to have any upstream or storage blockage for any movements at this intersection. The signalization at Trumble Road/Mapes Road is not estimated to adversely affect the cross-street traffic. As such, this improvement helps eliminate the forecasted deficiency at Trumble Road/Exceed Road-Sturgeon Electric Driveway.

Detailed queuing analysis worksheets are included in Appendix F.

10.3 LIST OF CHAPTER 10.0 FIGURES

- Figure 10-1: (WB-67) Truck Turning Template- Ingress -Trumble Road/Exceed Road-Sturgeon Electric Driveway
- Figure 10-2: (WB-67) Truck Turning Template- Egress -Trumble Road/Exceed Road-Sturgeon Electric Driveway



LSA



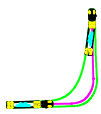
0 30 60
FEET

SOURCE: Google Earth, 2018.

I:\BAV2101\Traffic\Reports\Truck Turning.dwg (06/06/2022)

LEGEND

— Existing Lane

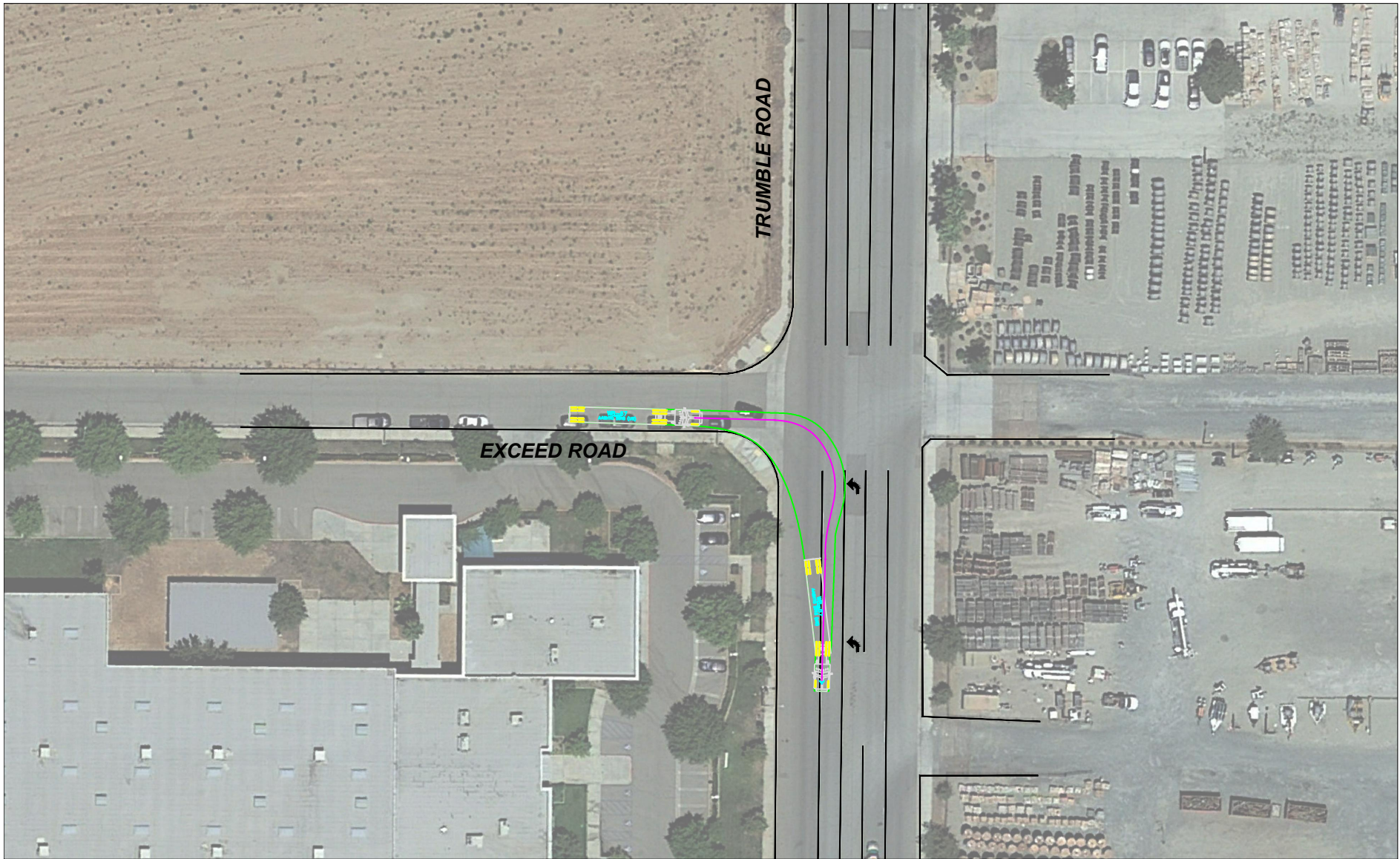


WB-67 Truck Turning Path

FIGURE 10-1

*Mapes and Trumble Industrial Facility Project
Traffic Study*

(WB-67) Truck Turning Template- Ingress
Trumble Road/Exceed Road-Sturgeon Electric Driveway



LSA



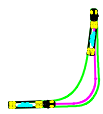
0 30 60
FEET

SOURCE: Google Earth, 2018.

I:\BAV2101\Traffic\Reports\Truck Turning.dwg (06/06/2022)

LEGEND

— Existing Lane



WB-67 Truck Turning Path

FIGURE 10-2

*Mapes and Trumble Industrial Facility Project
Traffic Study*

(WB-67) Truck Turning Template- Egress
Trumble Road/Exceed Road-Sturgeon Electric Driveway

11.0 CALTRANS QUEUING ANALYSIS

Consistent with Caltrans requirements, an off-ramp queuing analysis was performed at the intersections under the jurisdiction of Caltrans, i.e., I-215 Southbound Ramps/Bonnie Drive, and I-215 Northbound Ramps/CA-74 to determine the adequacy of off-ramp storage length. The queuing analysis at these locations has been prepared for disclosure purposes only.

Table 11-A lists the available turn-pocket storage lengths and summarize the 95th percentile back-of-queue lengths at this intersection under existing and Cumulative (2024) conditions. The queues have been reported from Synchro. As shown in Table 11-A, queues for the off-ramp movements are not estimated to exceed the existing available turn-pocket storage lengths under existing and Cumulative (2024) conditions.

Detailed queuing analysis worksheets are included in Appendix F.

11.1 LIST OF CHAPTER 11.0 TABLES

- Table 11-A: Intersection Queuing Analysis

Table 11-A - Intersection Queuing Analysis

Intersection	Movement	Storage Length ¹ (ft/lane)	Queue Lengths ²							
			Existing				Cumulative (2024)			
			No Project		With Project		No Project		With Project	
			AM	PM	AM	PM	AM	PM	AM	PM
1 . I-215 Southbound Ramps/Bonnie Drive Signalized	SBT	1600	445	795	470	825	850	1495	885	1520
	SBR	120	0	0	0	0	0	0	0	0
2 . I-215 Northbound Ramps/CA-74 Signalized	SBLR	1420	130	195	150	205	260	285	60	645

Notes:

ft/lane = feet per lane

EB = Eastbound; WB = Westbound; NB = Northbound; SB = Southbound

L = Left; R = Right

Bold = Queue exceeds available storage.

¹ Storage length for all movements obtained from Google Earth measurements.

² All queues reported are 95th percentile queues. All queues for signalized intersections have been reported from Synchro.

12.0 SUMMARY AND CONCLUSIONS

The proposed project will add one high cube fulfillment center warehouse building with a total area of 395,000 SF. Access to the project will be provided via three driveways. The driveway located on Mapes Road and the driveway located on Trumble Road will be used by only passenger vehicles, while the driveway located on Exceed Road will be used by only trucks. The project will also be improving the project frontage by adding curb, gutter and sidewalk along the project frontage. The project is estimated to generate 1,054 daily PCE trips, with 86 trips occurring the a.m. peak hour and 92 trips occurring during the p.m. peak hour. The project is anticipated to be completed by 2024.

12.1 EXISTING CONDITIONS SUMMARY

All study intersections operate at a satisfactory LOS under existing without and with project conditions.

12.2 CUMULATIVE (2024) CONDITIONS SUMMARY

All study intersections are forecast to operate at a satisfactory LOS under Cumulative (2024) without and with project conditions with the exception of four intersections.

12.3 IMPROVEMENTS SUMMARY

Based on the improvements discussed in Section 8.1 “Recommended Improvements” of this report, the recommended improvements include, lane addition and signal timing optimization at the intersection of I-215 Southbound Ramps/Bonnie Drive, installing signal at the intersection of Trumble Road/Mapes Road, and lane addition with adding a southbound right overlap phasing at the intersection of Trumble Road/CA-74. Improvements at the intersection of I-215 Southbound Ramps/Bonnie Drive is covered under WRCOG TUMF program and the project will be paying fair-share percentages for remaining improvements.

12.4 CEQA VMT ANALYSIS SUMMARY

Though the project could be screened out being located in a low VMT zone, a detailed VMT analysis was performed due to the project’s land use and size. Based on the results of this analysis, the project would not have any significant VMT impact.

12.5 ACCESS ANALYSIS SUMMARY

There is adequate space for truck turning radii for project ingress and egress truck traffic. Though project access for trucks will be restricted, it is estimated that the project traffic would not spill over to Trumble Road. Additionally, installing signal at Trumble Road/Mapes road is not estimated to adversely affect the Exceed Road Driveways

12.6 QUEUING ANALYSIS SUMMARY

The queues for off-ramp movements at the Caltrans ramps are not projected to exceed the available turn-pocket storage lengths under existing and Cumulative (2024) conditions.

APPENDIX A:

SCOPING AGREEMENT



March 13, 2022

Ms. Lupita Garcia
Associate Planner, Planning Division
City of Perris
101 N D Street
Perris, California 92570

Subject: Scope of Work for Mapes Road and Trumble Road Industrial Facility Project Traffic Study
(LSA Project No. BAV2101; City Record ID PLN22-05023)

Dear Ms. Garcia:

LSA is under contract to prepare a Traffic Study (TS) for the proposed Mapes Road and Trumble Road Industrial Facility Project to be located at the southwest corner of Mapes Road and Trumble Road, in the City of Perris (City). The project is bounded by Mapes Road to the north, Exceed Road to the south, Interstate 215 (I-215) to the west, and Trumble Road to the east.

The site is currently undeveloped and the proposed project will add one high cube fulfillment center warehouse building with a total area of 396,000 square feet (SF). Figure 1 (all figures attached) illustrates the regional and project location. Figure 2 illustrates the conceptual site plan. As illustrated in Figure 2, access to the project will be provided via three driveways. Following are detailed access description of each driveway:

- Driveway 1 – This driveway located on Mapes Road will be used by only passenger vehicles.
- Driveway 2 – This driveway located on Trumble Road will be used by only passenger vehicles.
- Driveway 3 – This driveway located on Exceed Road will be used by only trucks.

The City has adopted the *Draft City of Perris Transportation Impact Analysis Guidelines for CEQA* (TIA Guidelines), dated May 2020 (TIA). However, this guideline does not provide guidance for Levels of Service (LOS) Analysis for general Plan consistency. Therefore, as recommended by City staff, the LOS analysis for the project will be prepared in accordance with the *City of Perris LOS Standards and Threshold of Significance*. This document is included under Appendix A as part of this scoping letter.

LSA anticipates that the following scope of work will be required to prepare the TS for the proposed project.

SCOPE OF WORK: LEVELS OF SERVICE ANALYSIS

Study Intersection Analysis

All study intersections will be analyzed during the weekday a.m. and p.m. peak hours. The a.m. peak hour is defined as the one hour of highest traffic volumes occurring between 7:00 and 9:00 a.m. while the p.m. peak hour is defined as the one hour of highest traffic volumes occurring between 4:00 and 6:00 p.m. Intersection levels of service (LOS) will be calculated using the Highway Capacity Manual 6 (HCM 6) analysis methodologies and using Synchro 11 software. The TS will examine the following intersections:

1. Interstate 215 (I-215) Southbound Ramps/Bonnie Drive (Perris/Caltrans);
2. I-215 Northbound Ramps/California State Route 74 (CA-74) (Perris/Menifee/Caltrans);
3. Trumble Road/Mapes Road (Perris/Menifee);
4. Trumble Road/Exceed Road (Perris/Menifee);
5. Trumble Road/CA-74 (Menifee/Caltrans);
6. Project Driveway 1/Mapes Road (Perris);
7. Project Driveway 2/Trumble Road (Perris/Menifee).

Figure 3 illustrates the study area intersections.

Trip Generation, Trip Distribution, and Trip Assignment

The trip generation for the proposed project was developed using rates from the Institute of Transportation Engineers (ITE) *Trip Generation Manual* (11th Edition) Land Use 155 – “High-Cube Fulfillment Center Warehouse – Non-Sort.” The resulting trips were converted to passenger vehicles and trucks based on the South Coast Air Quality Management District (SCAQMD) recommendations for warehousing projects. As such, 31% of project traffic will be trucks. Based on the truck mix from the SCAQMD, Warehouse Truck Trip Study Data Results and Usage, dated December 2014, the truck mix was considered as 6.8% 2-axle trucks, 5.5% 3-axle trucks, and 18.7% as 4 or more axle trucks.

Inbound/Outbound splits for passenger vehicles were obtained from the ITE Trip Generation (11th Edition) for Land Use 155 – High-Cube Fulfillment Center Warehouse - Non-Sort, vehicle Trip generation inbound/outbound splits for a.m. peak hour, p.m. peak hour and daily trips. Inbound/outbound splits for trucks were obtained from ITE Trip Generation 11th Edition Supplemental for a.m. peak hour, p.m. peak hour and daily trips.

All truck trips were converted to passenger car equivalents (PCEs) using a 1.5 PCE factor for 2-axle trucks, 2.0 for 3-axle trucks, and 3.0 for 4 or more axle trucks. Table A summarizes the daily, a.m., and p.m. peak hour project trip. As shown in Table A, the proposed project is estimated to generate 1,054 daily PCE trips, with 86 trips occurring the a.m. peak hour and 92 trips occurring during the p.m. peak hour. The ITE Trip Generation rates is included in Appendix B. The SCAQMD Truck Rates are included in Appendix C.

Trip distribution patterns were obtained from select zone model runs of the Riverside County Model version 3.0 (RIVCOM). Separate select zone trip distributions for passenger car and truck trips were obtained from RIVCOM. The select zone distribution plots are included in Appendix D. Project trip distributions were further refined based on the location of the proposed project in relation to surrounding land uses, regional roadway network and truck routes, and in consultation with the City staff. Additionally, as shown in Figure 2, all project truck traffic will be accessing the project through the Exceed Road Driveway. Figure 4 illustrates the project passenger vehicle trip distribution. Figure 5 illustrates the project truck trip distribution.

The project trip assignment is the product of the project trip generation and trip distribution percentages. Figure 6 illustrates the project passenger vehicle trip assignment. Figure 7 illustrates the project truck trip assignment (in PCE). Figure 8 illustrates the total project trip assignment (in PCE) for the proposed project.

Analysis Scenarios

The Traffic Study will be prepared based on consultation with City staff and to meet the requirements of the City's General Plan Levels of Service Standards. The project opening year is anticipated to be 2024. The TS will examine traffic conditions and project related operational deficiencies under the following scenarios:

- Existing Conditions;
- Existing with Project Conditions;
- Cumulative (2024) without Project Conditions; and
- Cumulative (2024) with Project Conditions.

Volume Development and Analysis Methodology

Traffic volumes for existing year traffic conditions will be based on existing a.m., and p.m. peak hour traffic counts collected for the intersections. Cumulative (2024) conditions traffic volumes will be developed by applying a 3 percent per annum growth rate to existing traffic volumes and by adding trips from other projects in the vicinity to the project completion year volumes. Attached Table B summarizes the list of cumulative projects to be included in this analysis.

Existing and cumulative (2024) with project volumes will be developed by adding project traffic to the corresponding without project scenario traffic volumes.

As previously stated, the TS will analyze study intersections during the a.m. and p.m. peak hours. Intersection LOS will be calculated using HCM 6 analysis methodologies by using the Synchro 11 software.

Analysis of Traffic Operations and Recommended Circulation Improvements

LOS without the project will be compared to LOS with the project for all analysis scenarios to determine potential operational deficiencies. Determination of operational deficiencies will be made based on the City's LOS standards as applicable.

Necessary improvements will be recommended at locations operating at an unsatisfactory LOS or where the project causes operational deficiencies. Improvements may include the addition of intersection turn lanes, and signalization. The LOS with recommended improvements will be calculated and summarized, along with a comparison of the LOS without improvements.

TUMF/DIF/Mitigation Fair Share

LSA will evaluate whether the recommended improvements identified in the TS are included as part of WRCOG Transportation Uniform Mitigation Fee (TUMF), or the City's Development Impact Fee (DIF) programs. If it is determined that an improvement is not covered through the DIF or the TUMF program, the project's fair share contribution will be calculated based on the project traffic as percentage of total growth from existing conditions to cumulative with project conditions.

Signal Warrant Analysis

A signal warrant analysis would be conducted at unsignalized intersections if a signal is recommended as an improvement. Peak hour approach volumes for the study intersections will be examined to determine whether signalization may be warranted per the criteria defined in the California supplement of the *Manual on Uniform traffic Control Devices* (CA-MUTCD).

Queuing Analysis

Caltrans typically requires a queuing analysis for the intersections under its jurisdictions. A queuing analysis will be performed for all the study intersections under the jurisdiction of Caltrans. Additionally, an inbound queuing analysis would be conducted at the gated access for project truck access located the western end of Exceed Road to identify whether the project trucks would be causing a vehicle backup on Exceed Road and any spillover along Trumble Road. 95th percentile queues will be reported from Synchro/SimTraffic to evaluate the adequacy of turn pocket lengths.

Truck Turning Templates

Truck turning templates will be created for the project driveway accessed by trucks. Truck turning template will be based on AASTHO Guidelines.

Concept Striping Plan

A conceptual striping plan for Trumble Road would be created for Trumble in the vicinity of the proposed project to illustrate and identify ingress and egress movements of the project traffic.

SCOPE OF WORK: CEQA VEHICLE MILES TRAVELED (VMT) ANALYSIS

Typically, for projects within the City, the VMT analysis needs to be based upon the methodology and significant impact criteria set in the *Draft City of Perris Transportation Impact Analysis Guidelines for CEQA* (TIA Guidelines), dated May 2020. As shown in attached Table A, the project is estimated to generate 719 daily trips. Therefore, based on the screening criteria mentioned in the TIA Guidelines, the project could be potentially screened out, since it generates less than 2,500 daily trips. However, it is our understanding that the project may require additional analysis to determine whether the project would be eligible to be screened out from a VMT analysis as explained below.

The TIA Guidelines recommends that for projects that does not require a General Plan Amendment and forecast to generate less than 2,500 daily trips, the City's VMT scoping form may be used to determine project's potential VMT impact. As such, if the project VMT rate calculated from the VMT scoping form is higher than the Citywide average, the project would recommend mitigation measures and VMT reduction percentages attributable to these measures to bring the project VMT rate lower than the citywide average. Since the project is forecast to generate less than 2,500 daily trips and does not require a General Plan Amendment, the project may be eligible to use the VMT scoping form for project VMT screening analysis.

LSA reviewed the City's VMT scoping form which states that the baseline VMT data was created using information from the Riverside County Transportation Analysis model (RIVTAM) model. Based on RIVTAM, the project is within TAZ 3893. The baseline VMT per employee for TAZ 3893 is 12.35 compared to the City's VMT per employee which is 11.62. Therefore, as per RIVTAM data, the City's VMT per employee is 5.91% lower than the baseline project TAZ VMT per employee under base year 2012 conditions. It should be noted that the baseline project TAZ VMT per employee do not include the project's influence on VMT per employee with addition of the proposed project.

Currently, RIVCOM is the most recent travel demand model for Riverside County, which replaces RIVTAM for VMT analysis. RIVCOM base year is 2018 compared to 2012 for RIVTAM. Additionally, under RIVCOM, the project is located within RIVCOM TAZ 1847. As calculated from RIVCOM, the base year (2018) project location/project TAZ VMT per employee is 23.9, and the Citywide average VMT per employee for the City of Perris is 24.33. Therefore, the project's VMT per employee is 1.77% lower than City's VMT per employee under base year 2018 conditions using RIVCOM. Therefore, pursuant to the recommended methodology included in the VMT Screening Criteria section of the TIA Guidelines, the project is located within a low VMT zone and could be potentially screened out from further VMT analysis.

It should be noted that, using the baseline TAZ VMT profile, as recommended in the City's TIA guidelines for VMT analysis, may be appropriate when the project size do not affect regional traffic pattern. Given the size of the project, using the baseline TAZ VMT per employee may not be appropriate to conclude that the project would not have a significant VMT impact based on low VMT zone criteria. Therefore, as a conservative approach, LSA proposes to conduct a detailed VMT analysis for the project to evaluate whether the project will have any significant VMT impact. This VMT analysis will be conducted using the recommended methodology included in the VMT Impact Analysis section of the TIA Guidelines.

As such, LSA recommends using RIVCOM for VMT calculations for the project. The RIVCOM model will be used to calculate the project and Citywide VMT per service population. The TIA Guidelines recommends comparing the project generated VMT per service population for both the base and future year with the City's threshold of significance to identify potential VMT impacts. The scope of work for the detailed VMT analysis is as follows:

Project Traffic Analysis Zone Update

The first step in preparation of this analysis will be to update the traffic analysis zone (TAZ) in the model that includes the project area. LSA will convert the project land use into model

socioeconomic categories using regional conversion factors. The socioeconomic data for the project TAZ in both RIVCOM base year and future year model scenario will be updated. Project socioeconomic data will be added to the project TAZ for both scenarios. Non-project related socioeconomic data will be shifted to an adjacent TAZ to isolate project from other land uses. Non-project related socioeconomic data for the future scenario will be adjusted to account for the project.

Project VMT Analysis

Upon completion of the socioeconomic data update, LSA will conduct model runs for both base and future scenarios. The outputs from these with project model runs will be utilized to calculate the base and future year project VMT per service population. The Citywide VMT per service population for the base year will be calculated from the base year no project run. If the project VMT per service population under either the base year or future year scenario exceeds the citywide base year VMT per service population, then the project would create significant VMT impact.

Should you have any questions, please do not hesitate to contact me at (951) 781-9310 or email me at Ambarish.Mukherjee@lsa.net.

Sincerely,

LSA ASSOCIATES, INC.



Ambarish Mukherjee, AICP, PE
Principal

Attachments:

Table A: Project Trip Generation

Table B: Cumulative Projects

Figure 1: Regional and Project Location

Figure 2: Conceptual Site Plan

Figure 3: Study Area Intersections

Figure 4: Project Trip Distribution - Passenger Vehicles

Figure 5: Project Trip Distribution - Trucks

Figure 6: Project Trip Assignment - Passenger Vehicles

Figure 7: Project Trip Assignment - Trucks

Figure 8: Project Total Trip Assignment

Appendix A: City of Perris LOS Standards and Threshold of Significance

Appendix B: ITE Trip Generation Rates for Land Use 155 – High-Cube Fulfillment Center Warehouse - Non-Sort

Appendix C: South Coast Air Quality Management District Truck Trip Generation Rates

Appendix D: RIVCOM Select Zone Plots

Table A - Project Trip Generation

Land Uses	Units		A.M. Peak Hour			P.M. Peak Hour			Daily
			In	Out	Total	In	Out	Total	
High-Cube Fulfillment Center^{1,2,3}	396	TSF							
Trips/Unit (Cars)			0.084	0.020	0.104	0.043	0.067	0.110	1.249
Trips/Unit (2-Axle Trucks)			0.005	0.005	0.010	0.005	0.006	0.011	0.123
Trips/Unit (3-Axle Trucks)			0.004	0.004	0.008	0.004	0.005	0.009	0.100
Trips/Unit (4+ Axle Trucks)			0.014	0.014	0.028	0.014	0.016	0.030	0.338
Trips/Unit (Total)			0.107	0.043	0.150	0.066	0.094	0.160	1.810
Trip Generation (Cars)			33	8	41	17	27	44	495
Trip Generation (2-Axle Trucks)			2	2	4	2	2	4	49
Trip Generation (3-Axle Trucks)			2	1	3	2	1	3	40
Trip Generation (4+ Axle Trucks)			6	5	11	5	7	12	135
Trip Generation (Total Trucks)			10	8	18	9	10	19	224
Trip Generation (Total)			43	16	59	26	37	63	719
Trip Generation (Cars)			33	8	41	17	27	44	495
PCE Trip Generation (2-Axle Trucks)			3	3	6	3	3	6	74
PCE Trip Generation (3-Axle Trucks)			4	2	6	4	2	6	80
PCE Trip Generation (4+ Axle Trucks)			18	15	33	15	21	36	405
PCE Trip Generation (Total Trucks)			25	20	45	22	26	48	559
PCE Trip Generation (Total)			58	28	86	39	53	92	1,054

Notes:

TSF = thousand square-feet

¹ Rates based on the ITE Trip Generation Manual (11th Edition) for Land Use 155 - "High-Cube Fulfillment Center Warehouse - Non-Sort", Setting Location - "General Urban/Suburban."

² Passenger vehicles and truck in/out splits from Institute of Transportation Engineers (ITE) Trip Generation Manual (11th Edition) rates for Land Use 155 – "High-Cube Fulfillment Center Warehouse - Non-Sort", Setting/Location - "General Urban/Suburban."

³ The resulting trips were converted to trucks and passenger vehicles based on the South Coast Air Quality Management District (SCAQMD) recommendations for warehousing projects. As such, 31 percent of project traffic will be trucks. Based on Vehicle Mix from the SCAQMD, Warehouse Truck Trip Study Data Results and Usage, dated December 2014, the truck mix was considered as 18.7% 4+ axle, 5.5% 3-axle, and 6.8% 2-axle trucks. All truck trips were converted to passenger car equivalents (PCEs) using a 1.5 PCE factor for 2-axle trucks, 2.0 for 3-axle trucks, and 3.0 for 4- and more axle trucks.

Table B - Cumulative Projects

Sr. No.	Project No.	Project Name/Reference	Address/Location	Project Description	Project Units/Area/Other
M1	CUP 2016-290, PM 2016-291	On-Deck	North corner of Trumble Road and CA-74	120 Hotel Rooms, sit down restaurant, fast food restaurant with drive through and a gas station/convenience store/car wash	120 RM Hotel, sit down restaurant, fast food restaurant with drive through and a gas station with convenience store/car wash
M2	CUP 2018-126	Paragon Framing	Southeast corner of Trumble Road and Watson Road	3.76 acres - 5,454 sf office building - 5,000 sf workshop/storage building	5,454 TSF Office Building, 5.0 TSF Workshop/Storage Building
M3	TTM 34118	MR-27 LLC (Rancon)	West of Palomar Road between Cider St and CA-74	27.58 acres - 85 residential lots - 4,000 sq ft min. and 87 attached garden court condos (6 du/ac) with rec center and pool	85 Detached SFDU, 87 Attached SFDU
M4	PP 2018-300, TPM 2018-320	Motte Country Plaza	Northwest corner of Palomar Rd and CA-74	3.8 acres - Redevelopment of the Motte Country Plaza - Relocation of the train car restaurant; new 4,700 sq. ft. 6-dual pump gas station; new 3,838 sf Convenience store with drive-thru restaurant; new 1,050 sf car wash	Train car restaurant, 12 VFP Gas Station w/ 3.838 TSF Convenience Store, and 1.050 TSF Car Wash
M5	SP 2018-181 /TM37573	Menifee Valley SP (Brookfield)	South of CA-74 between Menifee Road and Briggs Road	205.2 acres - 559 residential lots -	559 Detached SFDU
M6	TTM 33738	MR 56 LLC (Rancon)	Southeast corner of Malone Ave and Watson Rd	11.37 acres - 52 residential lots - 5,000 sq ft min. lot size	52 Detached SFDU
M7	TTM 34600	MR-27 LLC (Rancon)	Southeast corner of Malone Ave and Varela Ln	18.3 acres - 153 condominium units (8.36 du/ac) with rec center and pool	153 MFUDU
M8	PP 2017-225	Harvest Glen Marketplace	Northwest corner of Briggs Rd and CA-74	5.04 acres - 18,501 sq. ft. commercial buildings (gas station with canopy, convenience store, two (2) drive thru restaurants and a car wash with vacuums	18,501 Square Ft Commercial with gas station with convenience store and car wash, two drive-through restaurants.
R1	TPM37727	Tentative Parcel Map 37727	Southeast corner of Sherman Rd and Vista Rd	4 Lot Parcel Map	4 Detached SFDU
R2	TPM37213	Tentative Parcel Map 37213	Northeast corner of Dawson Rd and Patti Ln	4 Lot Subdivision	4 Detached SFDU
R3	TTM37728	Tentative Tract Map 37728	Southwest corner of Antelope Rd and Vista Rd	228 Lot Tentative Tract Map	228 Detached SFDU
R4	TTM37358	Tentative Tract Map 37358	Northwest corner of Antelope Rd and Mapes Rd	154 Lot Tract Map	154 Detached SFDU
R5	TPM37919	Tentative Parcel Map 37919	Southwest corner of Antelope Rd and Roan Ranch Rd	Subdivide 5 Acre Parcel into 4 half acre developable lots	4 Detached SFDU
R6	TPM37589	Tentative Parcel Map 37589	Northeast corner of Pico Ave and Benigni Ave	Subdivide 4.72 acre lot into 4 parcels	4 Detached SFDU
R7	TR31687	Tract 31687	Northeast corner of Tradewinds Dr and Mapes Rd	Subdivide 40.17 acre lot into 65 SFR lots and 2 open space lots	65 Detached SFDU
R8	CUP180002	RV Storage Facility	North of Mapes Road between Palomar Rd and Rocky Peak Rd	121 Storage/Parking Stalls for RV Storage	121 RV Parking Stalls
R9	TR35045	Residential Subdivision	Northeast corner of Menifee Rd and Mapes Rd	712 Single Family Lots Subdivision Lots, 1 School and 1 Park	712 Detached SFDU
P1		Green Valley Specific Plan Phase 2	South of Case Rd, North of Ethanac Rd, West of Green Valley Pkwy, East of Goetz Rd	Single Family Residential, Multifamily Residential, and Commercial Development within the Green Valley Specific Plan Phase 2	385 Detached SFDU, 1294 MFUDU, 278.8 TSF Retail

Notes:

SFDU = Single-Family Dwelling Units; DU = Dwelling Units; TSF = Thousand Square Feet; RM = Rooms.

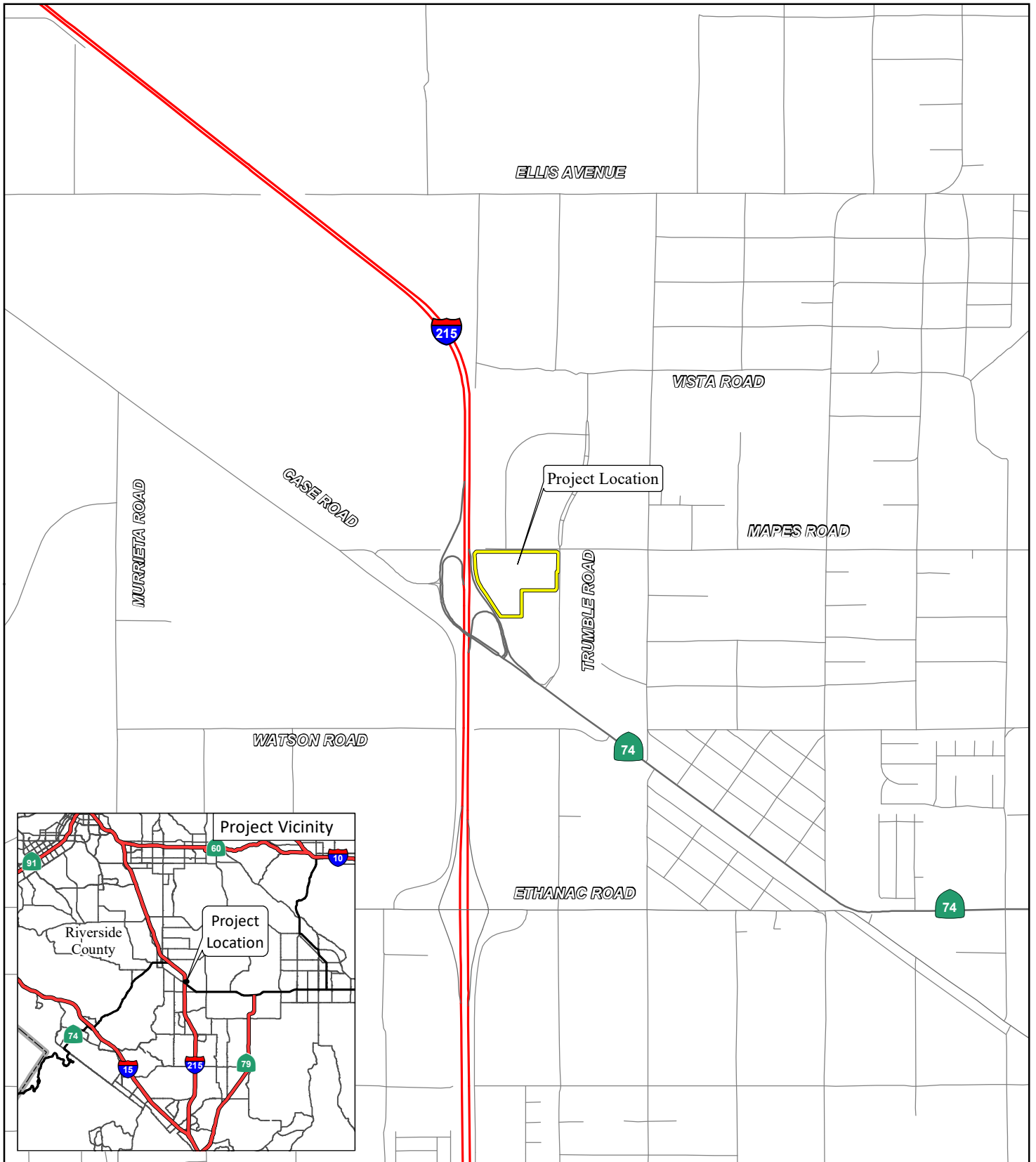
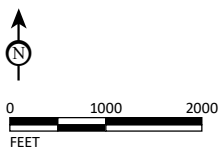


FIGURE 1

LSA



SOURCE: ESRI Streetmap, 2021; Google Earth, 2019.

R:\CIM2105_Riverwalk Townhomes\Traffic\GIS\Reports\fig1_Reg_ProjLoc.mxd (10/7/2021)

Mapes and Trumble Industrial Facility Project
Traffic Study

Regional and Project Location

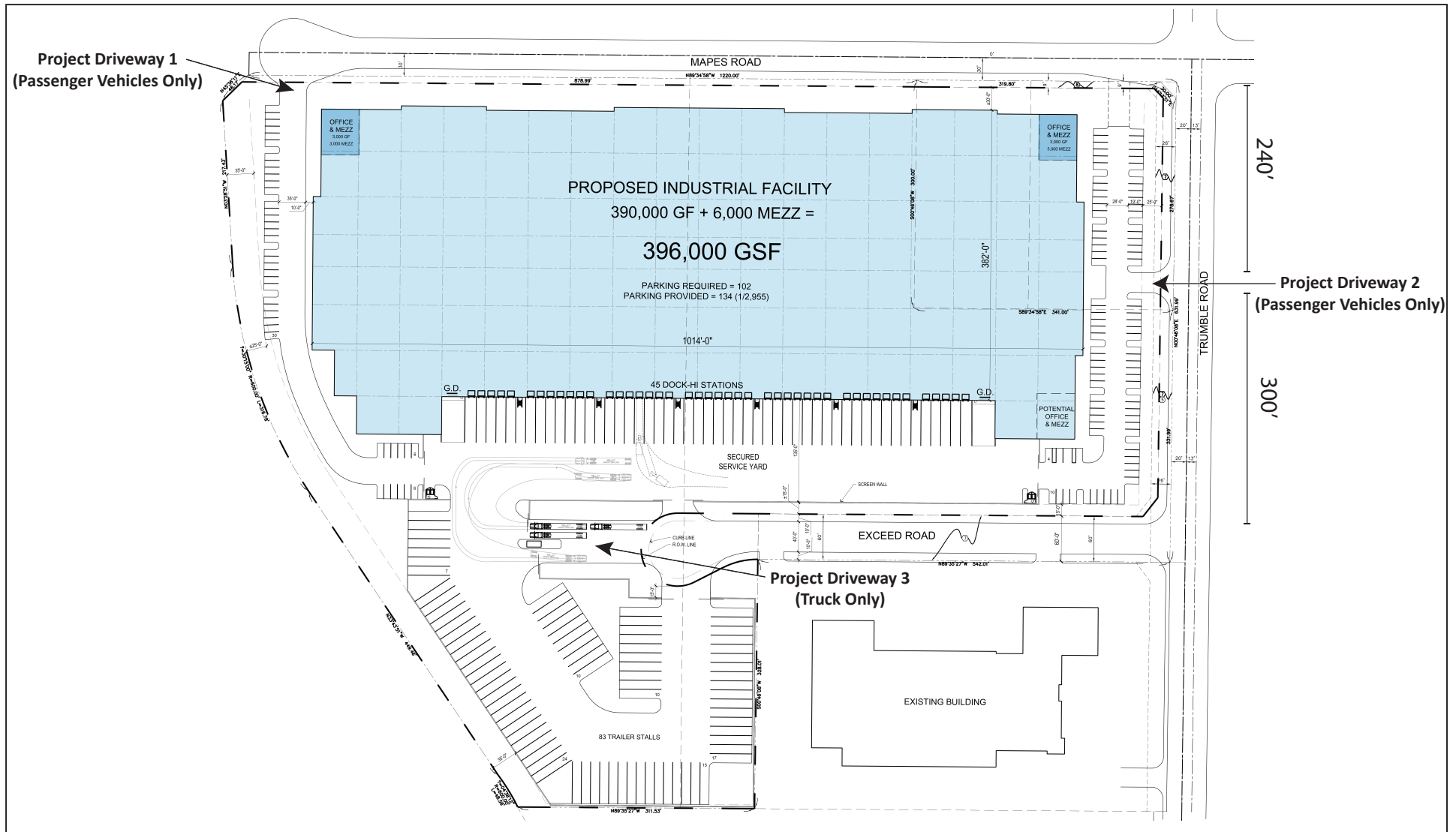
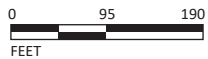


FIGURE 2

LSA



Trumble and Mapes Warehouse Project
Traffic Study

Conceptual Site Plan

SOURCE: AO Architects, October 2021

R:\BAV2101\Traffic\GIS_Graphics\Reports\fig2_Siteplan_Landscape.ai (02/23/2022)

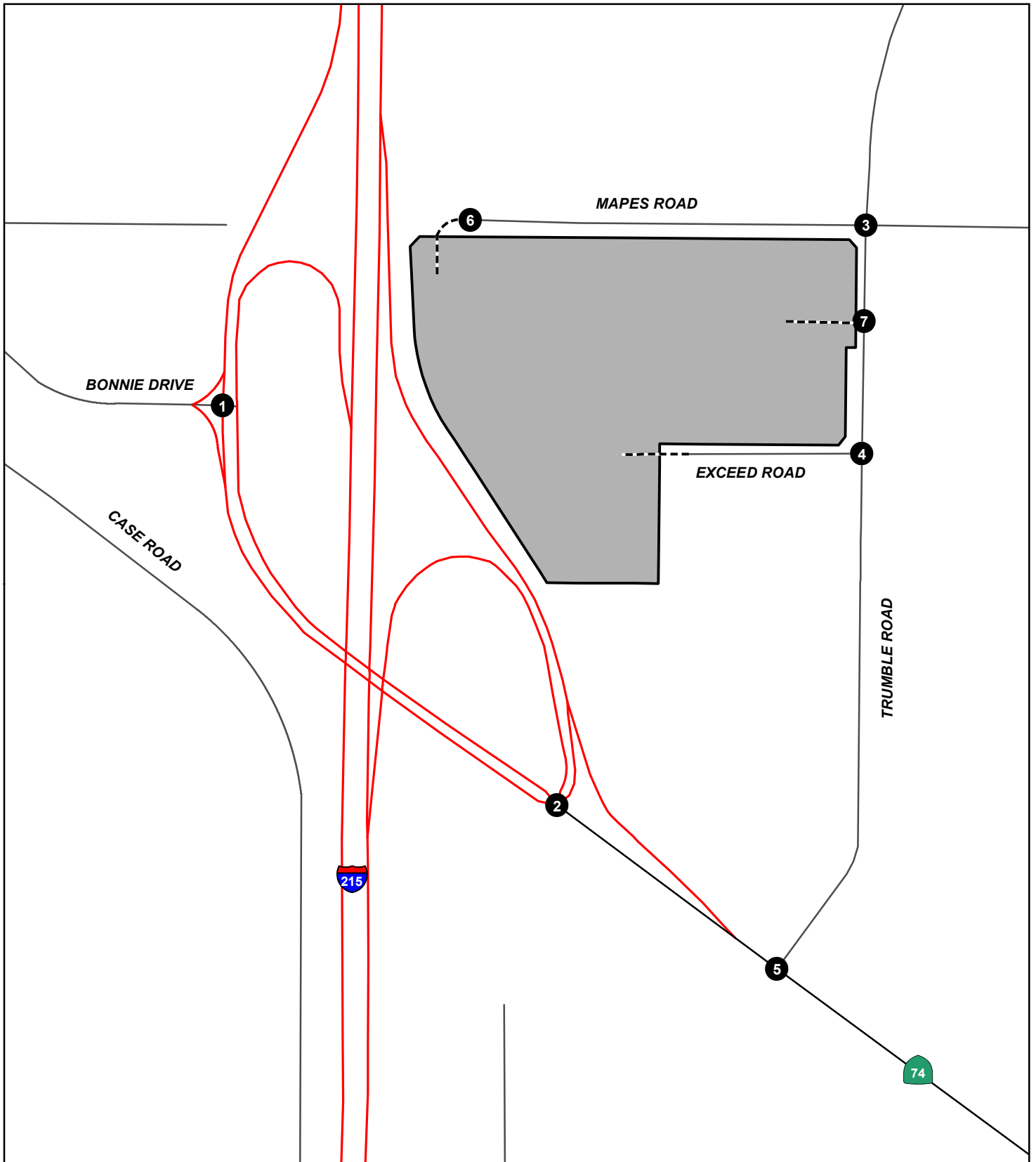
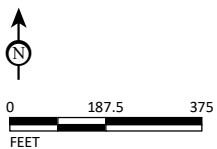


FIGURE 3

LSA

LEGEND

- Project Location
- Study Area Intersections
- Project Driveway



*Horizon Business Park Warehouse Project
Traffic Study
Study Area Intersections*

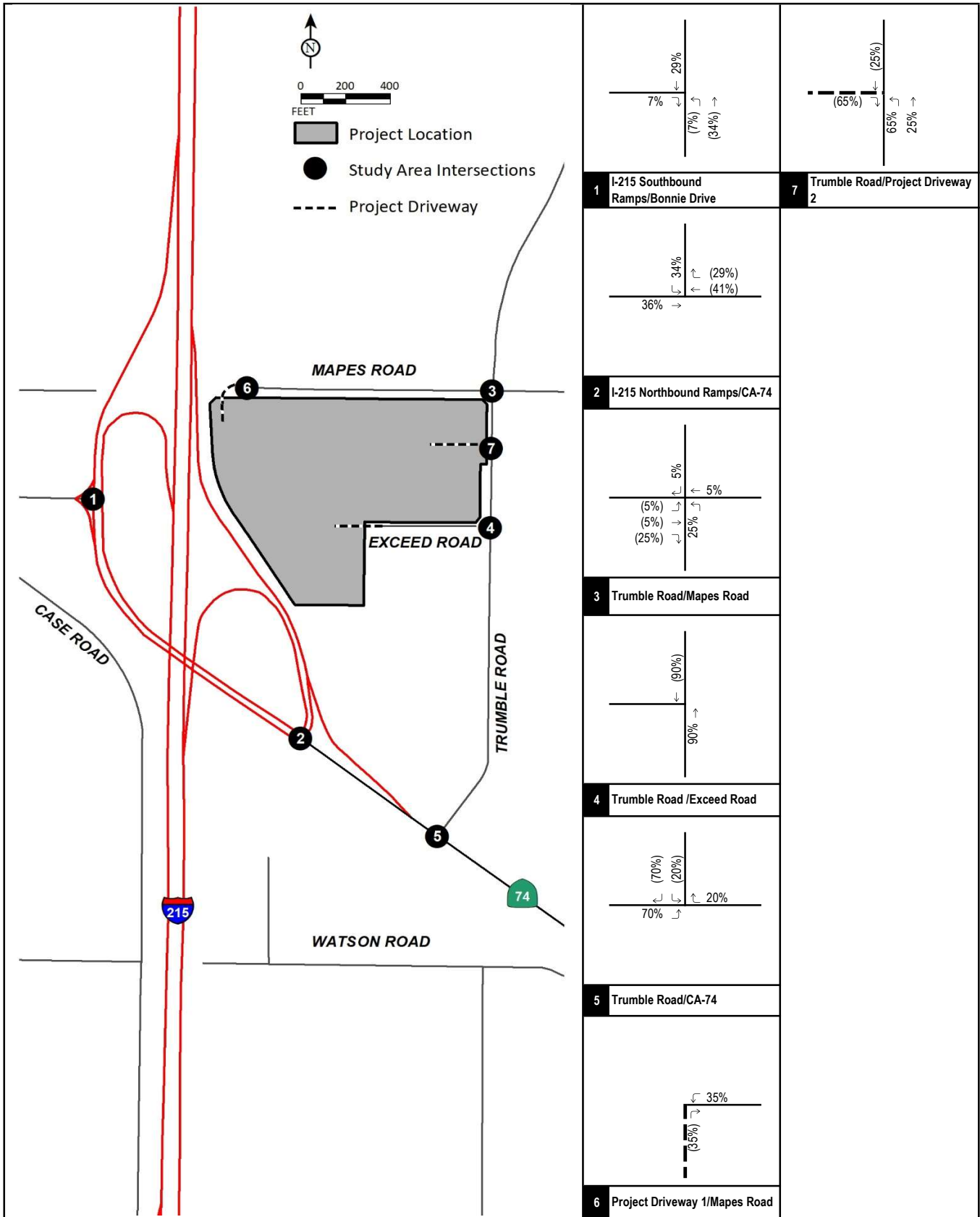


FIGURE 4

LSA

XX% (YY%)
Inbound (Outbound) Trip Distribution
----- Project Driveway

Mapes and Trumble Industrial Facility Project
Traffic Study

Project Trip Distribution - Passenger Vehicles

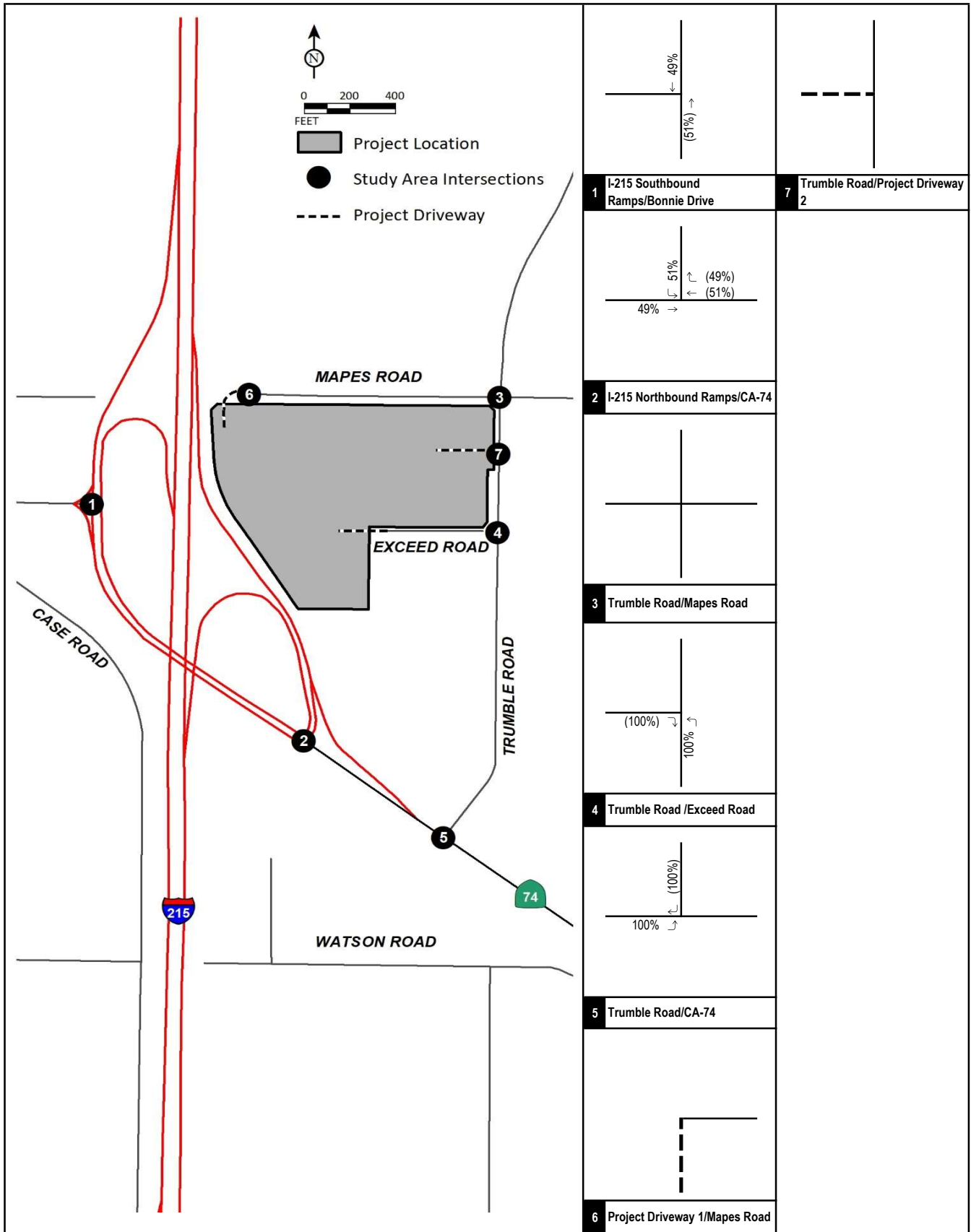


FIGURE 5

LSA

XXX% (YYY%)

Inbound (Outbound) Trip Distribution

----- Project Driveway

Mapes and Trumble Industrial Facility Project
Traffic Study

Project Trip Distribution - Trucks

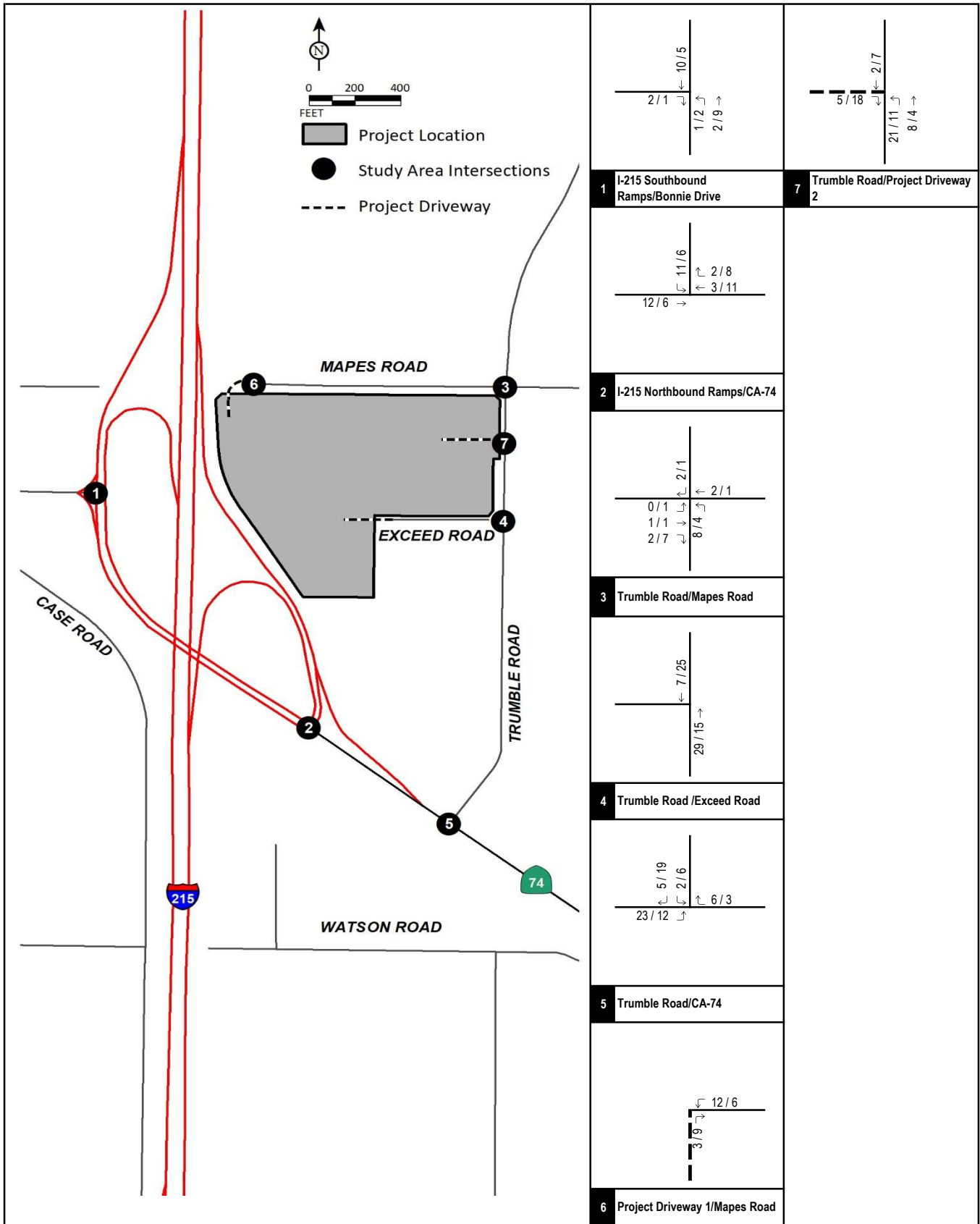


FIGURE 6

LSA

XX / YY

AM / PM Peak Hour Traffic Volumes

----- Project Driveway

Mapes and Trumble Industrial Facility Project
Traffic Study
Project Trip Assignment - Passenger Vehicles

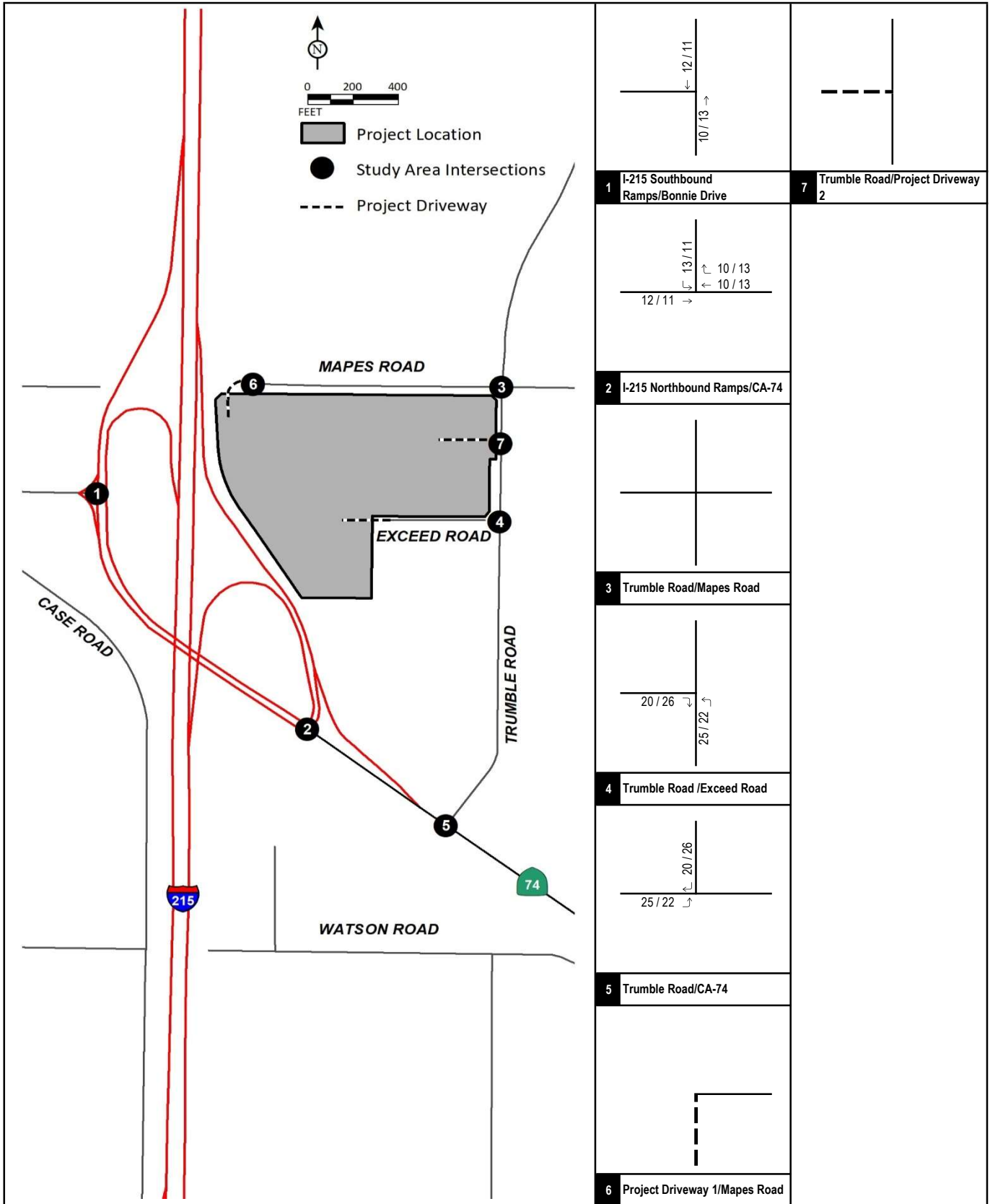


FIGURE 7

LSA

XX / YY

AM / PM Peak Hour PCE Volumes

----- Project Driveway

Mapes and Trumble Industrial Facility Project
Traffic Study

Project Trip Assignment - Trucks

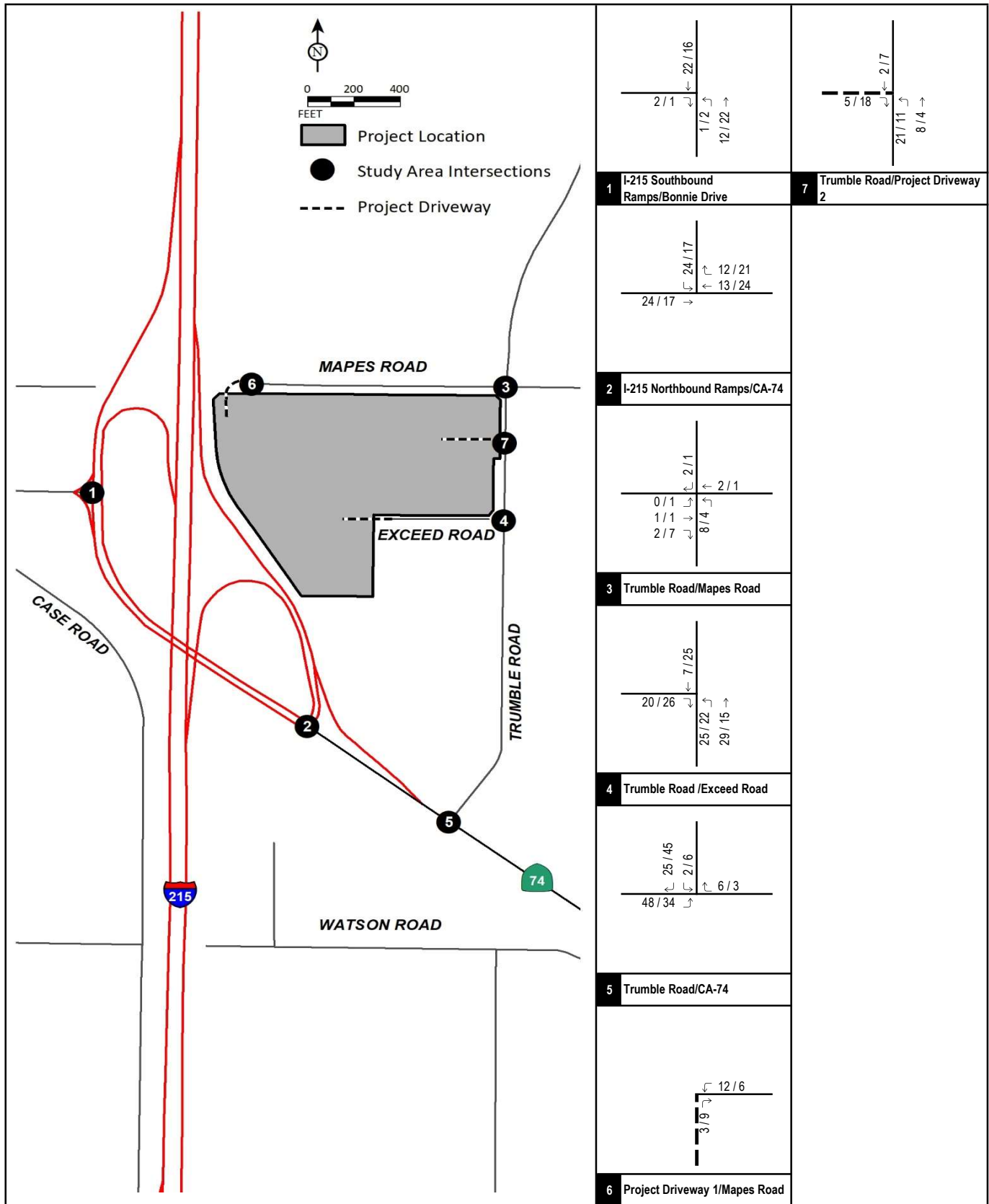


FIGURE 8

LSA

XX / YY
 AM / PM Peak Hour Traffic Volumes (In PCE)
 - - - - Project Driveway

Mapes and Trumble Industrial Facility Project
 Traffic Study
 Project Total Trip Assignment

**City of Perris
LOS Standards**



- I.A.5 Consider ancillary parking facilities with transit connections to activity centers such as downtown.
- I.A.6 Require parking facility design that minimizes visual and physical impacts while maintaining pedestrian and motorist safety and supporting adjacent activities.

Policy I.B

Support development of a variety of transportation options for major employment and activity centers including direct access to commuter facilities, primary arterial highways, bikeways, park-n-ride facilities, and pedestrian facilities.

- I.B.1 Require on-site improvements that accommodate public transit vehicles (i.e. bus pullouts and transit stops and cueing lanes, bus turnarounds and other improvements) at major trip attractions (i.e. community centers, tourist and employment centers, etc.).

Policy I.C

Cooperate with local, regional, State and federal agencies to establish an efficient multi-modal circulation system.

Policy I.D

Encourage and support the development of projects that facilitate and enhance the use of alternative modes of transportation.

Goal II

A well planned, designed, constructed and maintained street and highway system that facilitates the movement of vehicles and provides safe and convenient access to surrounding developments.

Policy II.A

Maintain the following target Levels of Service:

- ❖ LOS “D” along all City maintained roads (including intersections) and LOS “D” along I-215 and SR 74 (including intersections with local streets and roads). An exception to the local road standard is LOS “E”, at intersections of any Arterials and Expressways with SR 74, the Ramona-Cajalco Expressway or at I-215 freeway ramps.
- ❖ LOS “E” may be allowed within the boundaries of the Downtown Specific Plan Area to the extent that it would support transit-oriented development and walkable communities. Increased congestion in this area will facilitate an increase in transit ridership and encourage development of a complementary mix of land uses within a comfortable walking distance from light rail stations.

Implementation Measures

- II.A.1 Utilize existing infrastructure (lanes, median islands, turn lanes, available right-of-way) and rights-of-way to the maximum extent practicable.

Threshold of Significance

**Table 1
LOS & Delay Ranges**

LOS	Delay (seconds/vehicle)	
	Signalized Intersections	Unsignalized Intersections
A	≤ 10.0	≤ 10.0
B	> 10.0 to ≤ 20.0	> 10.0 to ≤ 15.0
C	> 20.0 to ≤ 35.0	> 15.0 to ≤ 25.0
D	> 35.0 to ≤ 55.0	> 25.0 to ≤ 35.0
E	> 55.0 to ≤ 80.0	> 35.0 to ≤ 50.0
F	> 80.0	> 50.0

Source: 2000 Highway Capacity Manual

Level of service is based on the average stopped delay per vehicle for all movements of signalized intersections and all-way stop-controlled intersections; for one-way or two-way stop-controlled intersections, LOS is based on the worst stop-controlled approach.

Peak Hour Performance Criteria

Level of Service (LOS) D or better is generally considered acceptable based on City of Perris, Caltrans, and County of Riverside performance criteria, except at the following intersections for which LOS E or better shall be considered acceptable based the City of Perris General Plan Circulation Element Policy II.A:

- I-215 Southbound Ramps/Harley Knox Boulevard;
- I-215 Northbound Ramps/Harley Knox Boulevard;
- I-215 Southbound Ramps/Cajalco Expressway;
- I-215 Northbound Ramps/Ramona Expressway;
- Webster Avenue/Ramona Expressway;
- Indian Street/Ramona Expressway;
- Perris Boulevard/Ramona Expressway;
- Redlands Avenue/Ramona Expressway; and
- Evans Road/Ramona Expressway.

Thresholds of Significance

To determine whether the addition of project-generated trips (or alternative-generated trips) results in a significant impact, and thus requires mitigation, the analysis shall evaluate significant impacts based on the following criteria:

- A project-related impact is considered direct and significant when a study intersection operates at an acceptable Level of Service for existing conditions (without the project) and the addition of 50 or more a.m. or p.m. peak hour project trips causes the intersection to operate at an unacceptable Level of Service for existing plus project conditions.

- A project-related impact is considered direct and significant when a study intersection operates at an unacceptable Level of Service for existing conditions (without the project) and the addition of 50 or more a.m. or p.m. peak hour project trips causes the intersection delay to increase by 2 seconds or more.
- A cumulative impact is considered significant when a study intersection is forecast to operate at an unacceptable Level of Service with the addition of cumulative/background traffic and 50 or more a.m. or p.m. peak hour project trips.

EXISTING CONDITIONS

Roadway Description

The characteristics of the roadway system in the vicinity of the project site are described below:

I-215 Freeway provides regional access for the project site as a freeway facility, traversing the Inland Empire in a north-south direction. I-215 begins at its southern terminus at the junction with I-15 in Murrieta and continues northbound through Perris. I-215 merges with SR-60 from Moreno Valley to Riverside, splits at SR-90, continues northbound through San Bernardino and terminates at the junction with I-15 just before the Cajon Pass. In the project vicinity, I-215 is a six-lane freeway providing access to the project site via the Harley Knox Boulevard and Cajalco/Ramona Expressway interchanges.

Day Street is a two-lane undivided roadway trending in a north-south direction within the project vicinity. Day Street is unpaved south of Cajalco Expressway. The posted speed limit on Day Street is 45 miles per hour; on-street parking is permitted.

Decker Road is a two-lane undivided roadway trending in a north-south direction. There is no posted speed limit on Decker Road within the project vicinity; on-street parking is permitted.

Seaton Avenue is a two-lane undivided roadway trending in a north-south direction. There is no posted speed limit on Seaton Avenue within the project vicinity; on-street parking is permitted.

Harvill Avenue is a four-lane divided roadway with a painted median trending in a north-south direction. The posted speed limit on Harvill Avenue is 50 miles per hour within the project vicinity; on-street parking is permitted.

Webster Avenue is a two-lane undivided roadway north of Ramona Expressway and a four-lane divided roadway with a painted median trending in a north-south direction. The posted speed limit on Webster Avenue is 35 miles per hour within the project vicinity; on-street parking is permitted north of Ramona Expressway only.

Western Way is a two-lane undivided roadway trending in a north-south direction. There is no visible posted speed limit on Western Way within the project vicinity; on-street parking is permitted.

Indian Street is a four-lane divided roadway with a raised median trending in a north-south direction. The posted speed limit on Indian Street is 40 miles per hour within the project vicinity; on-street parking is permitted.

Land Use: 155

High-Cube Fulfillment Center Warehouse

Description

A high-cube warehouse (HCW) is a building that typically has at least 200,000 gross square feet of floor area, has a ceiling height of 24 feet or more, and is used primarily for the storage and/or consolidation of manufactured goods (and to a lesser extent, raw materials) prior to their distribution to retail locations or other warehouses. A typical HCW has a high level of on-site automation and logistics management. The automation and logistics enable highly-efficient processing of goods through the HCW. A high-cube warehouse can be free-standing or located in an industrial park.

Warehousing (Land Use 150), high-cube transload and short-term storage warehouse (Land Use 154), high-cube parcel hub warehouse (Land Use 156), and high-cube cold storage warehouse (Land Use 157) are related land uses.

Land Use Subcategory

Each fulfillment center in the ITE database has been categorized as either a sort or non-sort facility. A sort facility is a fulfillment center that ships out smaller items, requiring extensive sorting, typically by manual means. A non-sort facility is a fulfillment center that ships large box items that are processed primarily with automation rather than through manual means. Separate sets of data plots are presented for the sort and non-sort fulfillment centers. Some limited assembly and repackaging may occur within the facility.

Additional Data

A high-cube warehouse may contain a mezzanine. In a HCW setting, a mezzanine is a free-standing, semi-permanent structure that is commonly supported by structural steel columns and that is lined with racks or shelves. The gross floor area (GFA) values for the study sites in the database for this land use do NOT include the floor area of the mezzanine. The GFA values represent only the permanent ground-floor square footage.

The amount of office/employee welfare space that is provided within a HCW can be highly variable but is typically an insignificant portion of the overall building square footage. Within the trip generation database, common values are between 3,000 and 5,000 square feet for a Cold Storage HCW and between 5,000 and 10,000 square feet for Transload, Fulfillment Center, and Parcel Hub HCW (all of which are less than one percent of total GFA for a site). Therefore, for the trip generation data plots, any office space that is part of the normal operation of the warehouse is included in the total GFA.

The High-Cube Warehouse/Distribution Center-related land uses underwent specialized consideration through a commissioned study titled "High-Cube Warehouse Vehicle Trip Generation Analysis," published in October 2016. The results of this study are posted on the ITE website at <http://library.ite.org/pub/a3e6679a-e3a8-bf38-7f29-2961becdd498>.

The sites were surveyed in the 2000s and the 2010s in California, New Jersey, and Texas.

Source Numbers

752, 941, 1001, 1002, 1011

High-Cube Fulfillment Center Warehouse - Non-Sort (155)

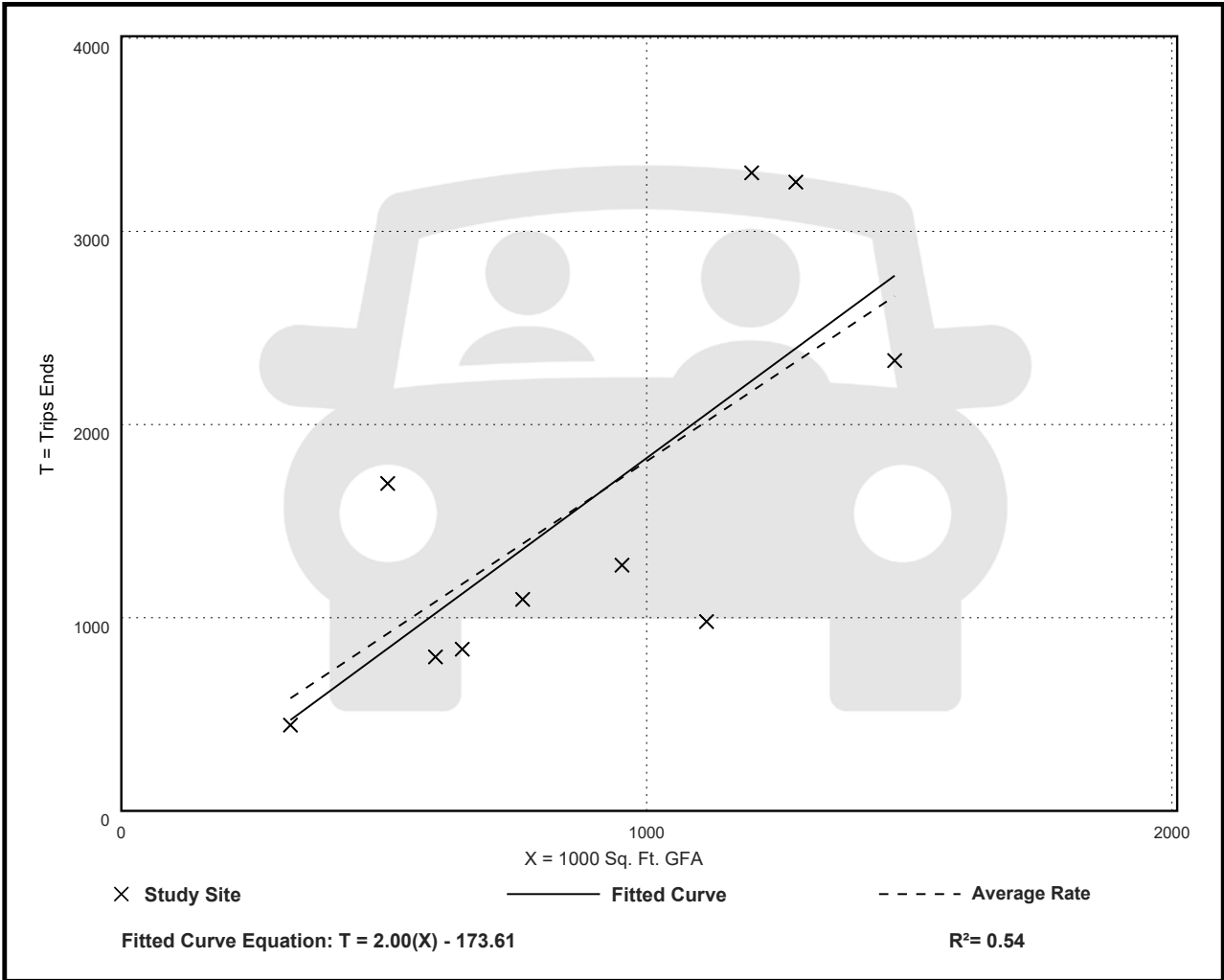
Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
 On a: Weekday

Setting/Location: General Urban/Suburban
 Number of Studies: 10
 Avg. 1000 Sq. Ft. GFA: 886
 Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
1.81	0.88 - 3.34	0.76

Data Plot and Equation



High-Cube Fulfillment Center Warehouse - Non-Sort (155)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 22

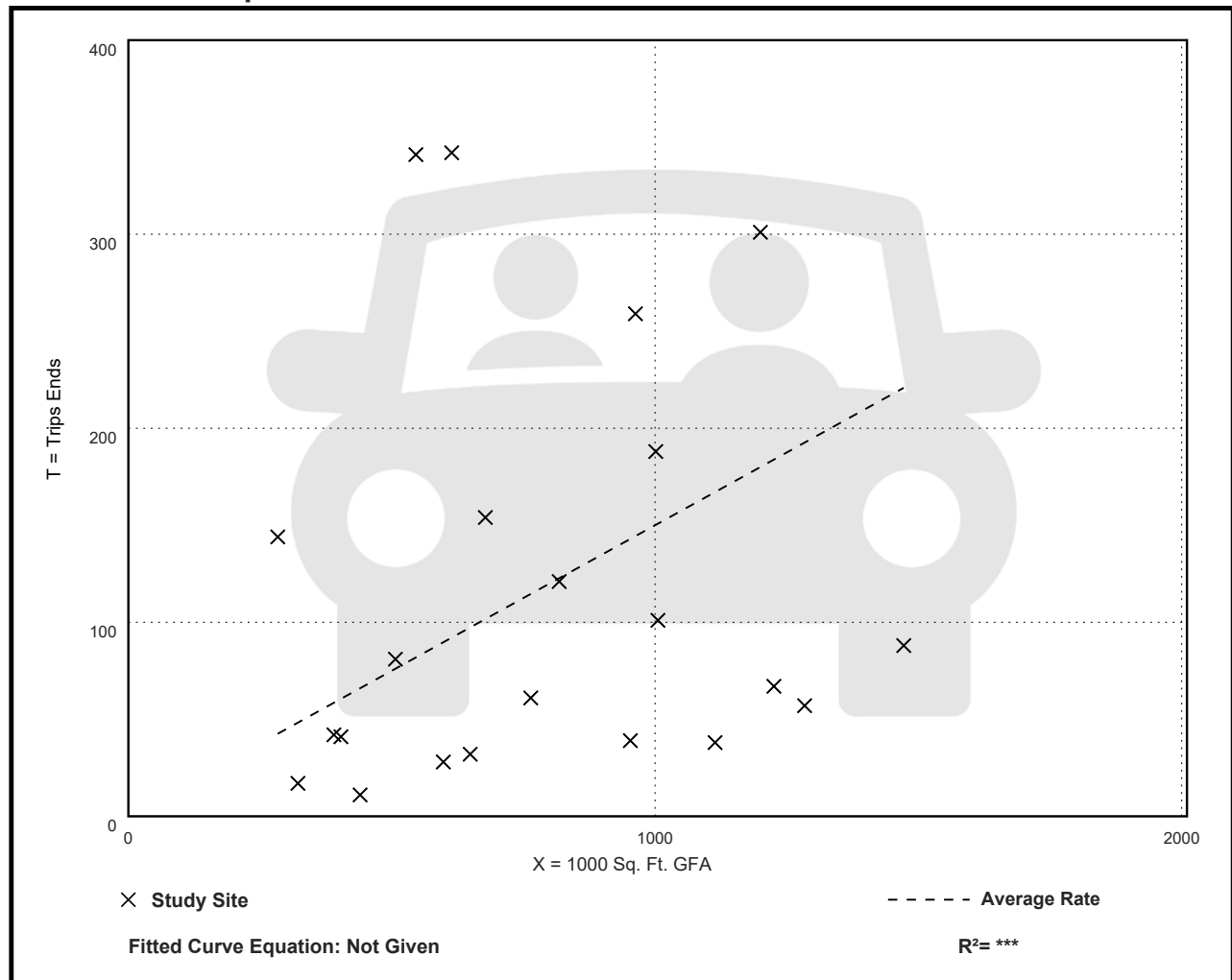
Avg. 1000 Sq. Ft. GFA: 783

Directional Distribution: 81% entering, 19% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
0.15	0.03 - 0.62	0.15

Data Plot and Equation



High-Cube Fulfillment Center Warehouse - Non-Sort (155)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 4 and 6 p.m.

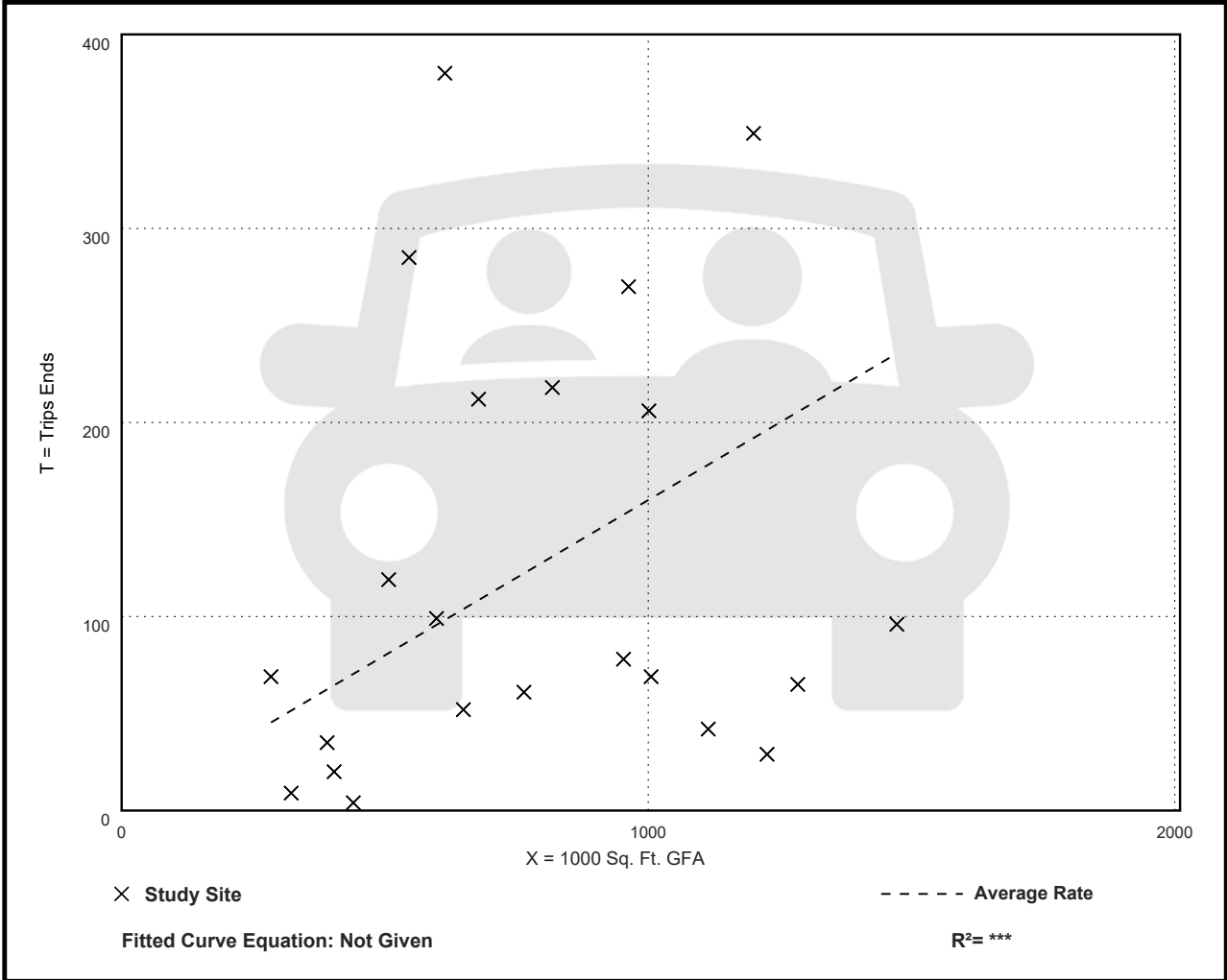
Setting/Location: General Urban/Suburban

Number of Studies: 22
Avg. 1000 Sq. Ft. GFA: 783
Directional Distribution: 39% entering, 61% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
0.16	0.01 - 0.62	0.15

Data Plot and Equation



High-Cube Fulfillment Center Warehouse - Non-Sort (155)

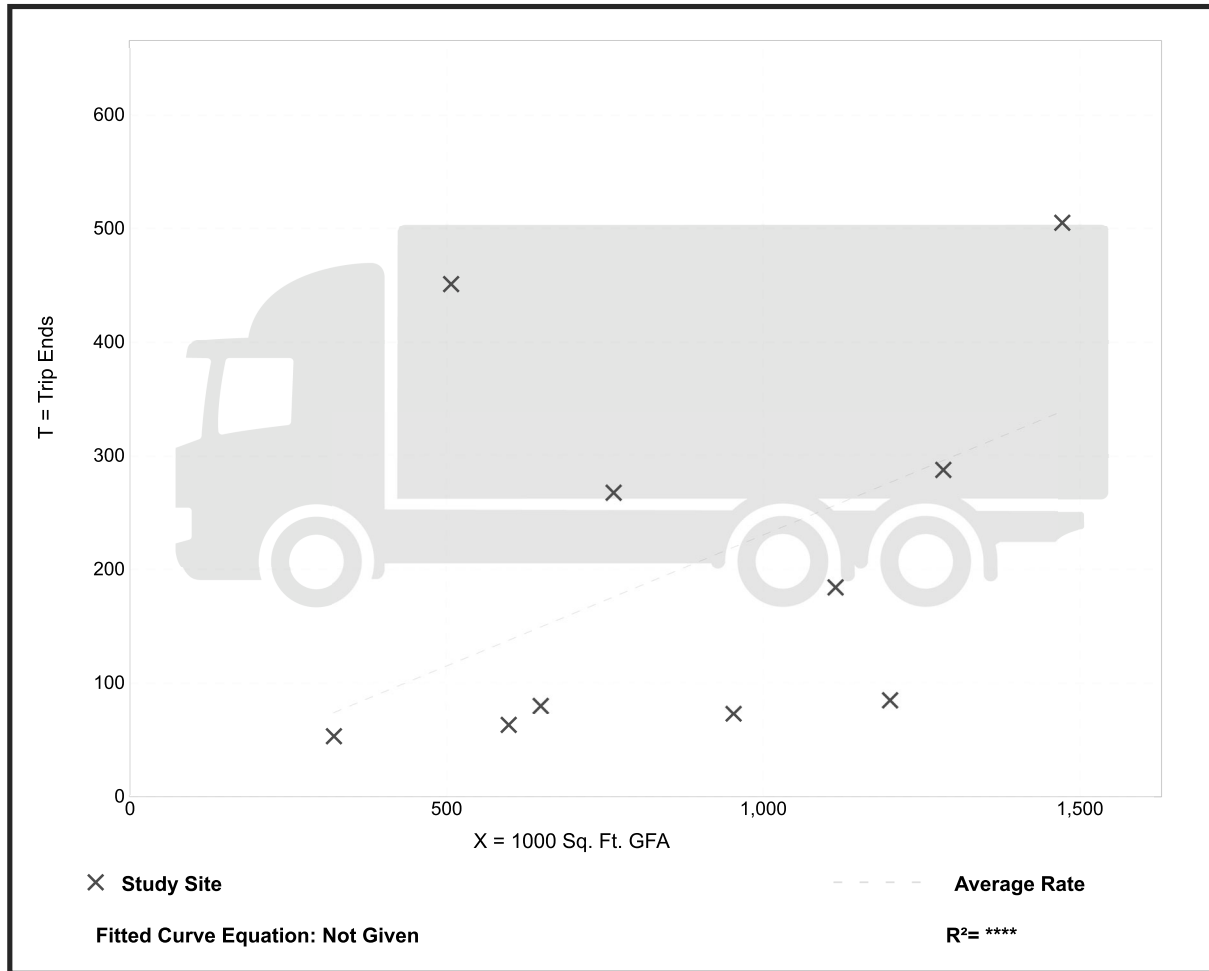
Truck Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday

Setting/Location: General Urban/Suburban
Number of Studies: 10
Avg. 1000 Sq. Ft. GFA: 886
Directional Distribution: 50% entering, 50% exiting

Truck Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
0.23	0.07 - 0.89	0.20

Data Plot and Equation



High-Cube Fulfillment Center Warehouse - Non-Sort (155)

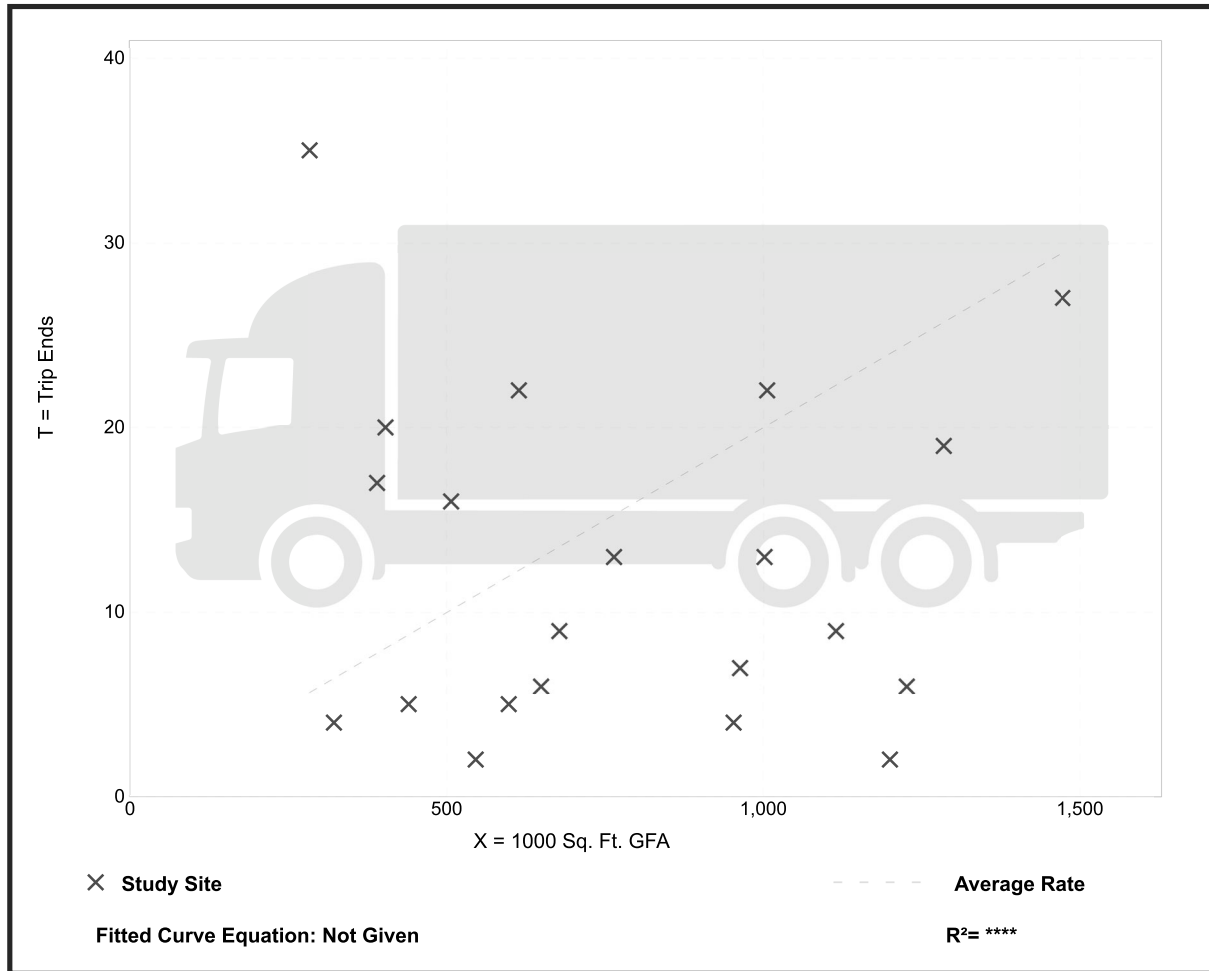
Truck Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban
 Number of Studies: 21
 Avg. 1000 Sq. Ft. GFA: 782
 Directional Distribution: 50% entering, 50% exiting

Truck Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
0.02	0.00 - 0.12	0.02

Data Plot and Equation



High-Cube Fulfillment Center Warehouse - Non-Sort (155)

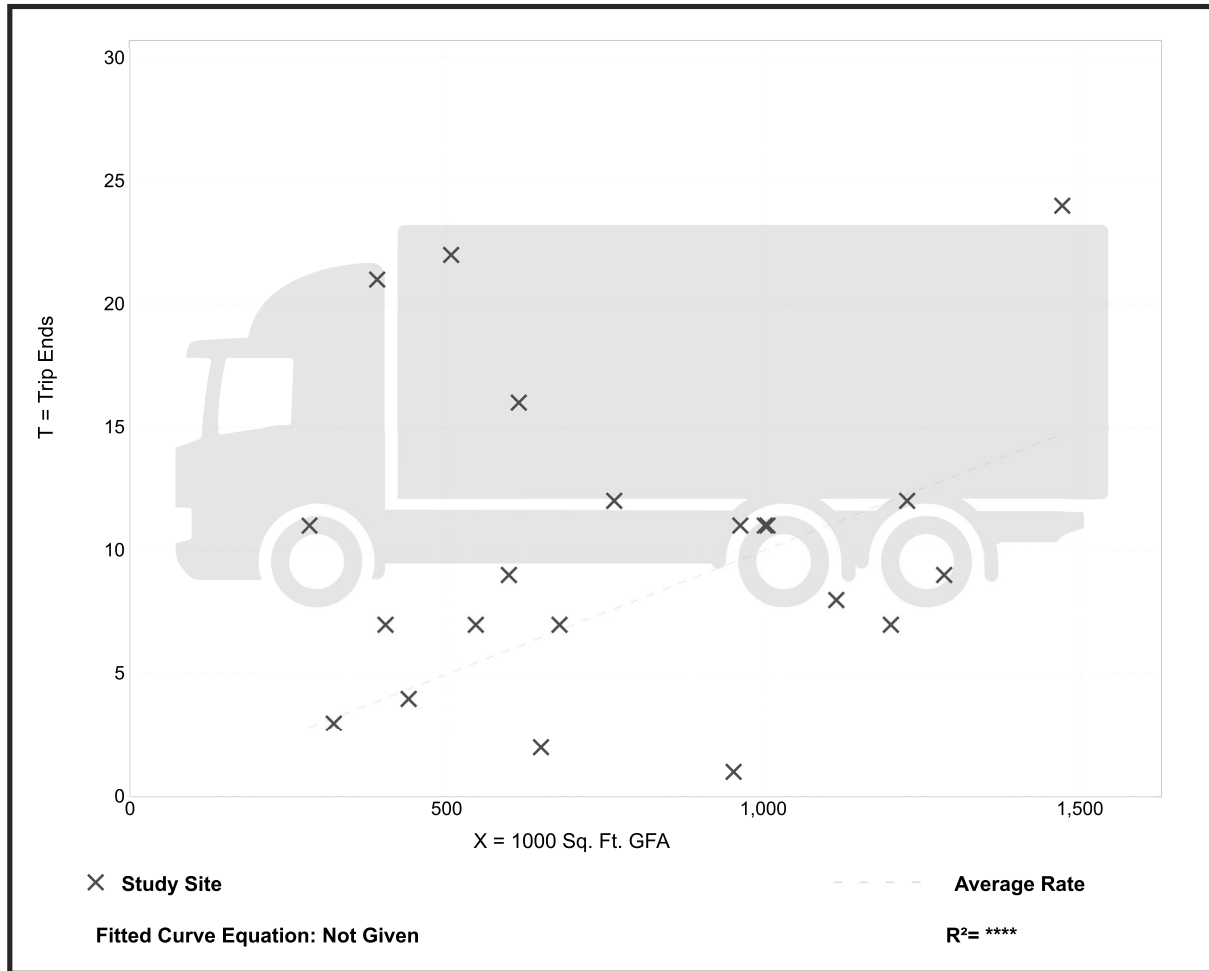
Truck Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban
 Number of Studies: 21
 Avg. 1000 Sq. Ft. GFA: 782
 Directional Distribution: 46% entering, 54% exiting

Truck Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
0.01	0.00 - 0.05	0.01

Data Plot and Equation



BACK UP SLIDES

Warehouse Truck Trip Study Data Results and Usage

Stakeholder Working Group
July 17, 2014



Cleaning the Air That We Breathe...

SCAQMD Warehouse Truck Study – Peaking Factor from Business Survey

- Business Survey Question
 - *“How much more trucking occurs at your facility during peak months compared to average months?”*
- Response
 - 30 high cube warehouse facilities responded

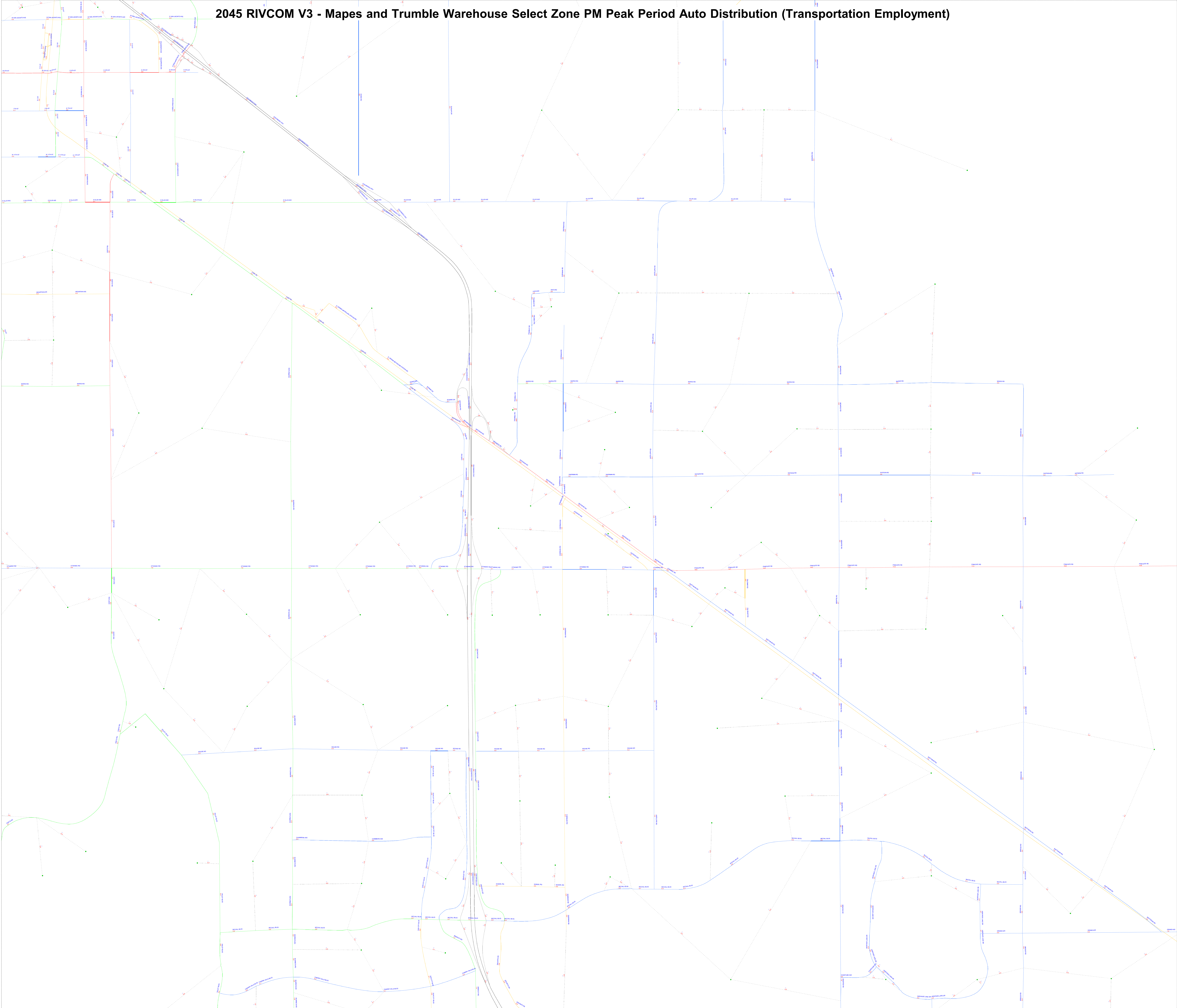
Statistical Measure	Percent Increase
Minimum	5%
Maximum	100%
Overall Average	27%
Cold Storage Average (N=14)	20%
Non Cold Storage Average (N=16)	33%

SCAQMD Warehouse Truck Study Truck Fleet Mix

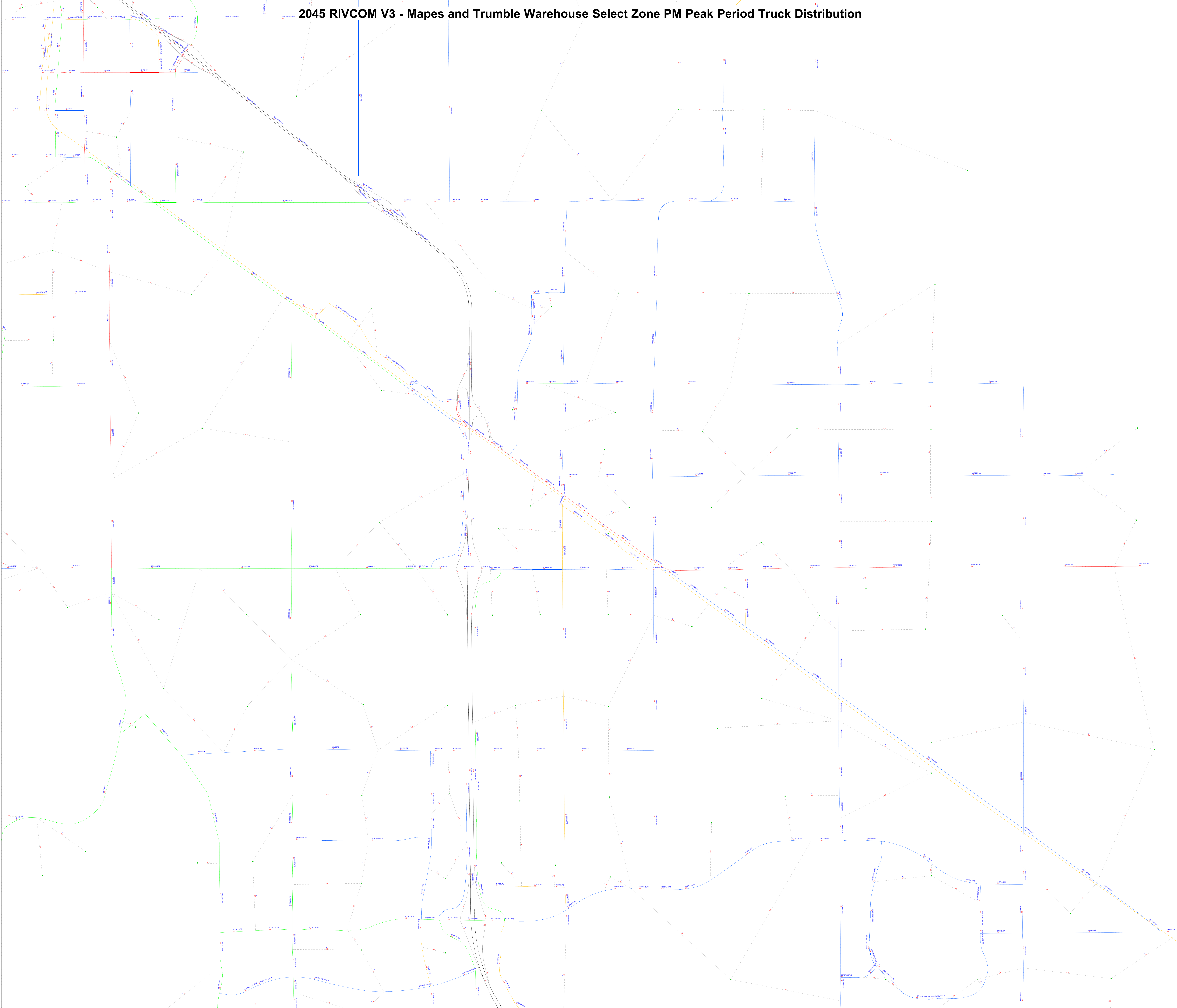
Grouping	All Trucks	Actual %		
		2-Axle	3-Axle	4+ Axle
SCAQMD Composite	31.0%	6.8%	5.5%	18.7%
With Cold Storage	44.7%	15.5%	4.9%	24.3%
Without Cold Storage	27.5%	4.6%	5.7%	17.2%
Fontana Study	20.4%	3.5%	4.6%	12.3%

Grouping	All Trucks	Normalized %		
		2-Axle	3-Axle	4+ Axle
SCAQMD Composite	31.0%	21.9%	17.7%	60.3%
With Cold Storage	44.7%	34.7%	11.0%	54.4%
Without Cold Storage	27.5%	16.7%	20.7%	62.5%
Fontana Study	20.4%	17.2%	22.5%	60.3%

2045 RIVCOM V3 - Mapes and Trumble Warehouse Select Zone PM Peak Period Auto Distribution (Transportation Employment)



2045 RIVCOM V3 - Maps and Trumble Warehouse Select Zone PM Peak Period Truck Distribution



April 13, 2022

Ms. Lupita Garcia
CITY OF PERRIS (Planning Division)
135 North "D" Street
Perris, CA 92570

**Subject: Mapes Road and Trumble Road Industrial Facility Project
(PLN 22 - 05023) Traffic Study Scoping Agreement and VMT
Analysis Review #2, City of Perris**

Introduction

RK ENGINEERING GROUP, INC. (RK) has reviewed the traffic study scoping agreement and VMT analysis #2 for the Mapes Road and Trumble Road Industrial Facility Project in the City of Perris. The project is located south of Mapes Road, west of Trumble Road, and east of the I-215 freeway in the City of Perris. The project will have three (3) access points including a driveway for passenger vehicles on Mapes Road, a driveway on Trumble Road for passenger cars, and a driveway located on Exceed Road that will be used by trucks.

RK has reviewed the traffic study scoping agreement and VMT analysis for the Mapes and Trumble Industrial Facility Project traffic study prepared by LSA Consultants, dated March 13, 2022, and it is acceptable as revised. LSA has adequately addressed all of the comments provided by RK in the February 16, 2022 Review #1 comment letter.

Comments

RK has the following comments on the traffic study scoping agreement and VMT analysis #2.

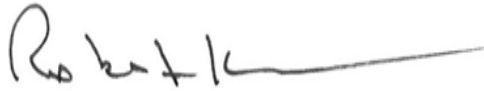
1. The traffic study scoping agreement and VMT analysis are acceptable from a technical standpoint. **As part of the Traffic Study and VMT Analysis submittal, please include details on the project's socioeconomic data and how the project TAZ is updated as part of the model runs.**

Conclusions

RK has reviewed the Traffic Study Scoping Agreement and VMT Analysis #2 for the Mapes and Trumble Industrial Facility Project (PLN 22 – 05023) prepared by LSA Consultants and it is acceptable as revised. LSA may proceed with the preparation of the traffic study.

RK appreciates this opportunity to work with the City of Perris on this project and if you have any questions, please contact me at (949) 474-0809.

Sincerely,
RK ENGINEERING GROUP, INC.



Robert Kahn, P.E.
Founding Principal



Justin Tucker, P.E.
Principal

Registered Civil Engineer 20285
Registered Traffic Engineer 0555

XC: Kenneth Phung, City of Perris,
Nathan Perez, City of Perris
Stuart McKibben, City of Peris
John Pourkazemi, Tri-Lake Consultants

Attachment

RK17305.DOC
JN: 2126-2022-01



APPENDIX B:

TRAFFIC COUNT SHEETS AND SIGNAL TIMING SHEETS

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- Passenger Vehicles - Large 2 Axle Vehicles - 3 Axle Vehicles - 4+ Axle Trucks

Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
07:00 AM	130	7	137	38	103	141	6	34	40	318
07:15 AM	145	2	147	58	109	167	9	45	54	368
07:30 AM	124	3	127	51	104	155	3	34	37	319
07:45 AM	121	9	130	66	117	183	10	32	42	355
Total	520	21	541	213	433	646	28	145	173	1360
08:00 AM	125	9	134	40	95	135	4	35	39	308
08:15 AM	124	5	129	41	82	123	7	34	41	293
08:30 AM	127	10	137	42	61	103	3	27	30	270
08:45 AM	118	5	123	38	62	100	5	22	27	250
Total	494	29	523	161	300	461	19	118	137	1121
Grand Total	1014	50	1064	374	733	1107	47	263	310	2481
Apprch %	95.3	4.7		33.8	66.2		15.2	84.8		
Total %	40.9	2	42.9	15.1	29.5	44.6	1.9	10.6	12.5	
Passenger Vehicles	864	38	902	336	633	969	46	232	278	2149
% Passenger Vehicles	85.2	76	84.8	89.8	86.4	87.5	97.9	88.2	89.7	86.6
Large 2 Axle Vehicles	83	9	92	29	85	114	1	21	22	228
% Large 2 Axle Vehicles	8.2	18	8.6	7.8	11.6	10.3	2.1	8	7.1	9.2
3 Axle Vehicles	27	2	29	7	12	19	0	8	8	56
% 3 Axle Vehicles	2.7	4	2.7	1.9	1.6	1.7	0	3	2.6	2.3
4+ Axle Trucks	40	1	41	2	3	5	0	2	2	48
% 4+ Axle Trucks	3.9	2	3.9	0.5	0.4	0.5	0	0.8	0.6	1.9

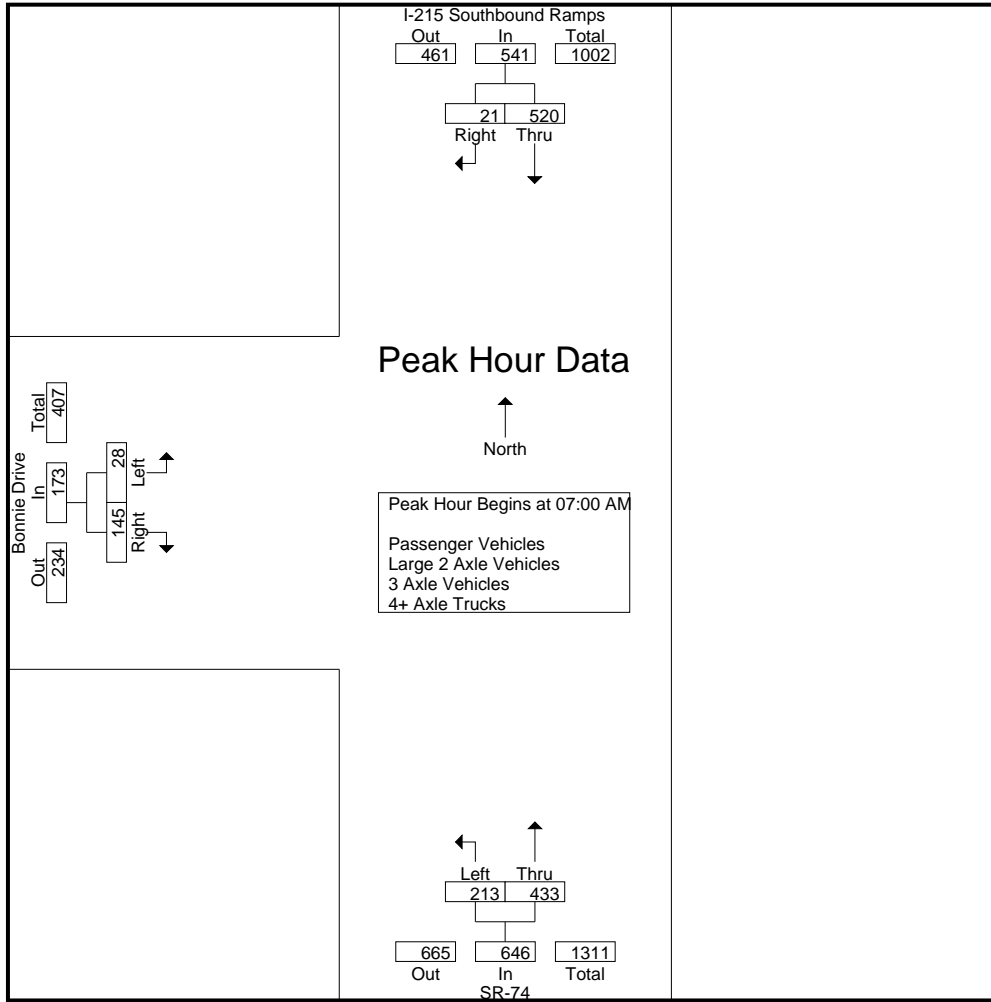
Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
07:00 AM	130	7	137	38	103	141	6	34	40	318
07:15 AM	145	2	147	58	109	167	9	45	54	368
07:30 AM	124	3	127	51	104	155	3	34	37	319
07:45 AM	121	9	130	66	117	183	10	32	42	355
Total Volume	520	21	541	213	433	646	28	145	173	1360
% App. Total	96.1	3.9		33	67		16.2	83.8		
PHF	.897	.583	.920	.807	.925	.883	.700	.806	.801	.924

Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 07:00 AM

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:00 AM			07:00 AM			07:00 AM		
+0 mins.	130	7	137	38	103	141	6	34	40
+15 mins.	145	2	147	58	109	167	9	45	54
+30 mins.	124	3	127	51	104	155	3	34	37
+45 mins.	121	9	130	66	117	183	10	32	42
Total Volume	520	21	541	213	433	646	28	145	173
% App. Total	96.1	3.9		33	67		16.2	83.8	
PHF	.897	.583	.920	.807	.925	.883	.700	.806	.801

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- Passenger Vehicles

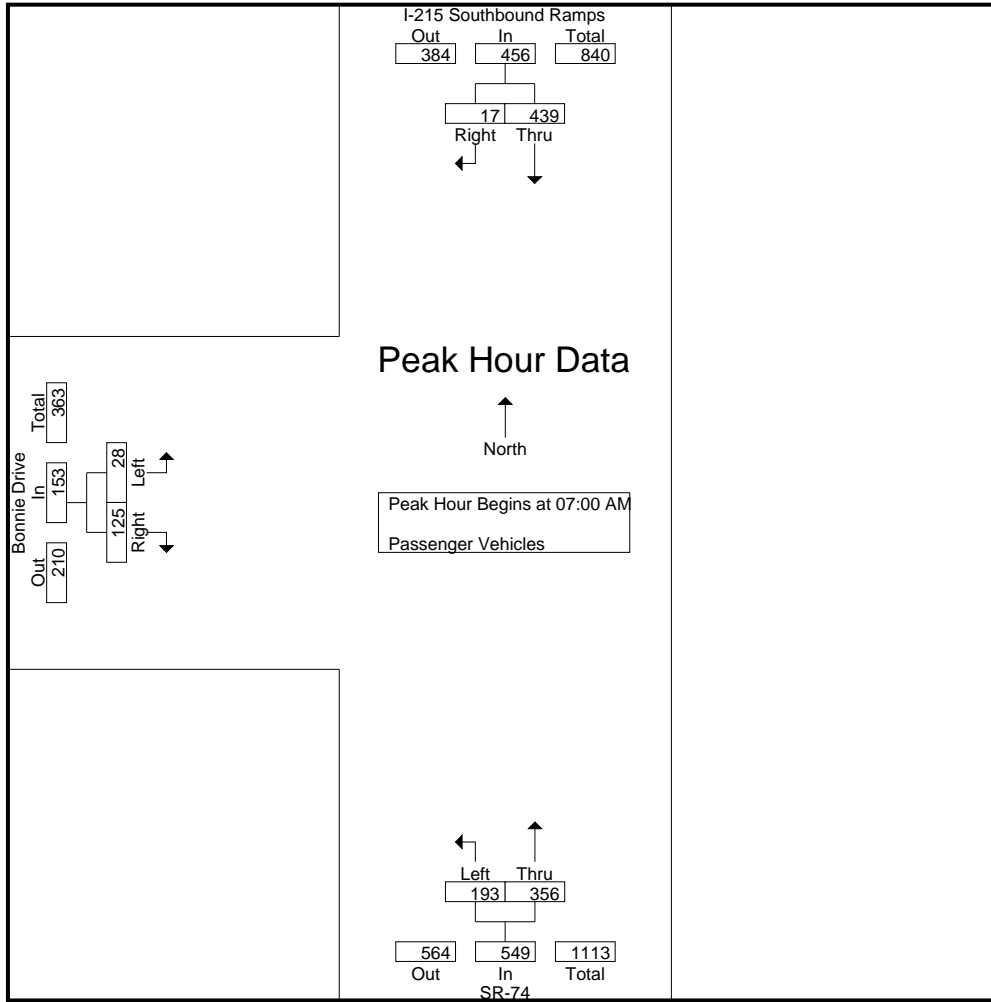
Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
07:00 AM	106	6	112	35	70	105	6	29	35	252
07:15 AM	127	2	129	52	89	141	9	38	47	317
07:30 AM	100	1	101	47	87	134	3	31	34	269
07:45 AM	106	8	114	59	110	169	10	27	37	320
Total	439	17	456	193	356	549	28	125	153	1158
08:00 AM	103	8	111	35	90	125	4	34	38	274
08:15 AM	111	3	114	37	74	111	7	31	38	263
08:30 AM	111	7	118	36	55	91	2	24	26	235
08:45 AM	100	3	103	35	58	93	5	18	23	219
Total	425	21	446	143	277	420	18	107	125	991
Grand Total	864	38	902	336	633	969	46	232	278	2149
Apprch %	95.8	4.2		34.7	65.3		16.5	83.5		
Total %	40.2	1.8	42	15.6	29.5	45.1	2.1	10.8	12.9	

Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
07:00 AM	106	6	112	35	70	105	6	29	35	252
07:15 AM	127	2	129	52	89	141	9	38	47	317
07:30 AM	100	1	101	47	87	134	3	31	34	269
07:45 AM	106	8	114	59	110	169	10	27	37	320
Total Volume	439	17	456	193	356	549	28	125	153	1158
% App. Total	96.3	3.7		35.2	64.8		18.3	81.7		
PHF	.864	.531	.884	.818	.809	.812	.700	.822	.814	.905

Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1
 Peak Hour for Entire Intersection Begins at 07:00 AM

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:00 AM			07:00 AM			07:00 AM		
+0 mins.	106	6	112	35	70	105	6	29	35
+15 mins.	127	2	129	52	89	141	9	38	47
+30 mins.	100	1	101	47	87	134	3	31	34
+45 mins.	106	8	114	59	110	169	10	27	37
Total Volume	439	17	456	193	356	549	28	125	153
% App. Total	96.3	3.7		35.2	64.8		18.3	81.7	
PHF	.864	.531	.884	.818	.809	.812	.700	.822	.814

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- Large 2 Axle Vehicles

Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
07:00 AM	11	1	12	2	28	30	0	4	4	46
07:15 AM	12	0	12	5	17	22	0	5	5	39
07:30 AM	13	1	14	4	15	19	0	2	2	35
07:45 AM	9	1	10	6	7	13	0	2	2	25
Total	45	3	48	17	67	84	0	13	13	145
08:00 AM	13	0	13	3	4	7	0	1	1	21
08:15 AM	7	2	9	3	7	10	0	2	2	21
08:30 AM	7	3	10	5	3	8	1	1	2	20
08:45 AM	11	1	12	1	4	5	0	4	4	21
Total	38	6	44	12	18	30	1	8	9	83
Grand Total	83	9	92	29	85	114	1	21	22	228
Apprch %	90.2	9.8		25.4	74.6		4.5	95.5		
Total %	36.4	3.9	40.4	12.7	37.3	50	0.4	9.2	9.6	

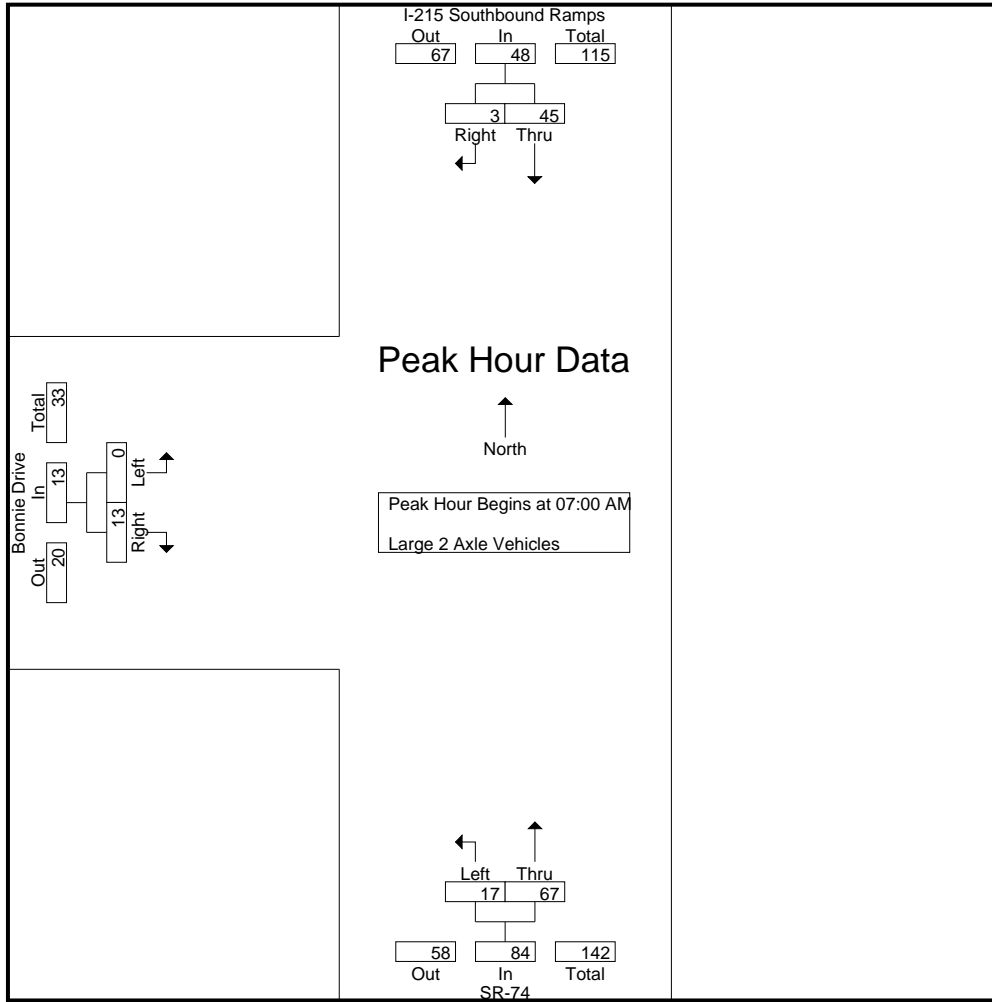
Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
07:00 AM	11	1	12	2	28	30	0	4	4	46
07:15 AM	12	0	12	5	17	22	0	5	5	39
07:30 AM	13	1	14	4	15	19	0	2	2	35
07:45 AM	9	1	10	6	7	13	0	2	2	25
Total Volume	45	3	48	17	67	84	0	13	13	145
% App. Total	93.8	6.2		20.2	79.8		0	100		
PHF	.865	.750	.857	.708	.598	.700	.000	.650	.650	.788

Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 07:00 AM

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:00 AM			07:00 AM			07:00 AM		
+0 mins.	11	1	12	2	28	30	0	4	4
+15 mins.	12	0	12	5	17	22	0	5	5
+30 mins.	13	1	14	4	15	19	0	2	2
+45 mins.	9	1	10	6	7	13	0	2	2
Total Volume	45	3	48	17	67	84	0	13	13
% App. Total	93.8	6.2		20.2	79.8		0	100	
PHF	.865	.750	.857	.708	.598	.700	.000	.650	.650

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- 3 Axle Vehicles

Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
07:00 AM	7	0	7	1	4	5	0	1	1	13
07:15 AM	2	0	2	1	2	3	0	2	2	7
07:30 AM	2	0	2	0	2	2	0	1	1	5
07:45 AM	1	0	1	0	0	0	0	2	2	3
Total	12	0	12	2	8	10	0	6	6	28
08:00 AM	3	1	4	2	1	3	0	0	0	7
08:15 AM	3	0	3	1	1	2	0	1	1	6
08:30 AM	5	0	5	0	2	2	0	1	1	8
08:45 AM	4	1	5	2	0	2	0	0	0	7
Total	15	2	17	5	4	9	0	2	2	28
Grand Total	27	2	29	7	12	19	0	8	8	56
Apprch %	93.1	6.9		36.8	63.2		0	100		
Total %	48.2	3.6	51.8	12.5	21.4	33.9	0	14.3	14.3	

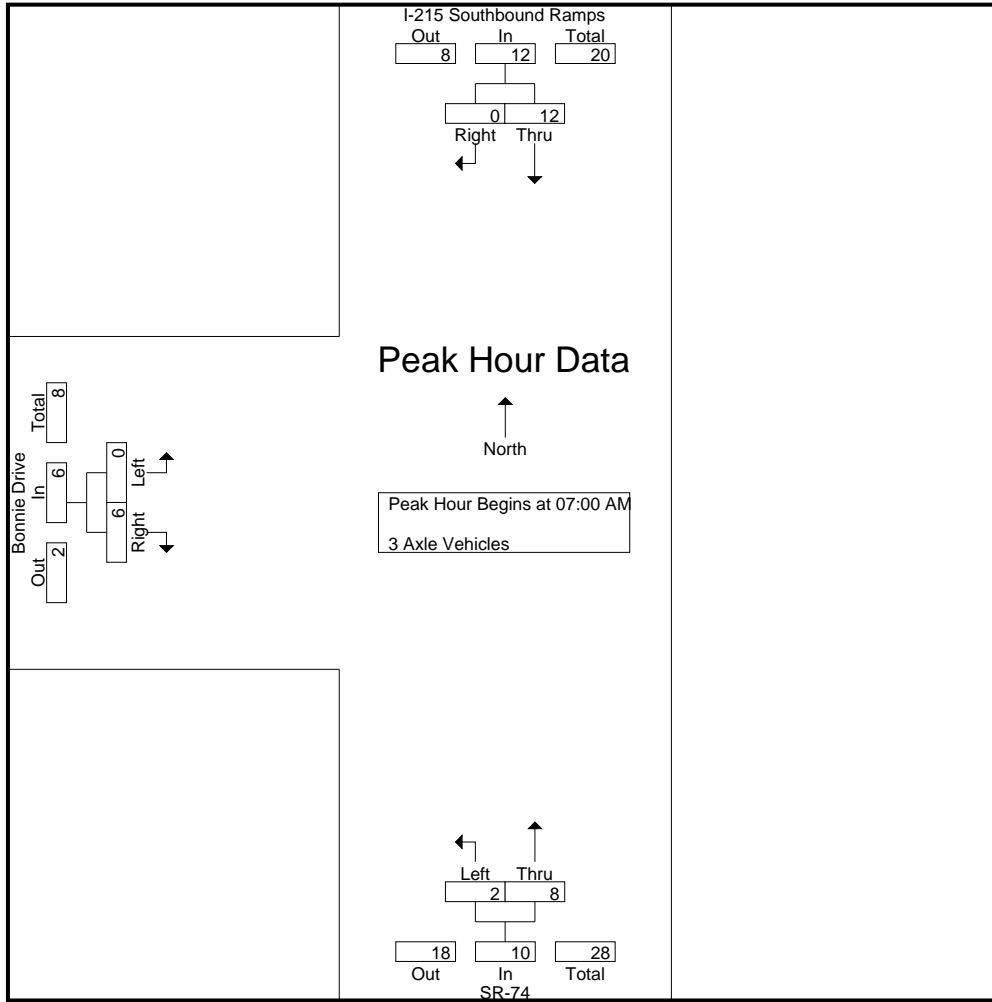
Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
07:00 AM	7	0	7	1	4	5	0	1	1	13
07:15 AM	2	0	2	1	2	3	0	2	2	7
07:30 AM	2	0	2	0	2	2	0	1	1	5
07:45 AM	1	0	1	0	0	0	0	2	2	3
Total Volume	12	0	12	2	8	10	0	6	6	28
% App. Total	100	0		20	80		0	100		
PHF	.429	.000	.429	.500	.500	.500	.000	.750	.750	.538

Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 07:00 AM

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:00 AM			07:00 AM			07:00 AM		
+0 mins.	7	0	7	1	4	5	0	1	1
+15 mins.	2	0	2	1	2	3	0	2	2
+30 mins.	2	0	2	0	2	2	0	1	1
+45 mins.	1	0	1	0	0	0	0	2	2
Total Volume	12	0	12	2	8	10	0	6	6
% App. Total	100	0		20	80		0	100	
PHF	.429	.000	.429	.500	.500	.500	.000	.750	.750

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- 4+ Axle Trucks

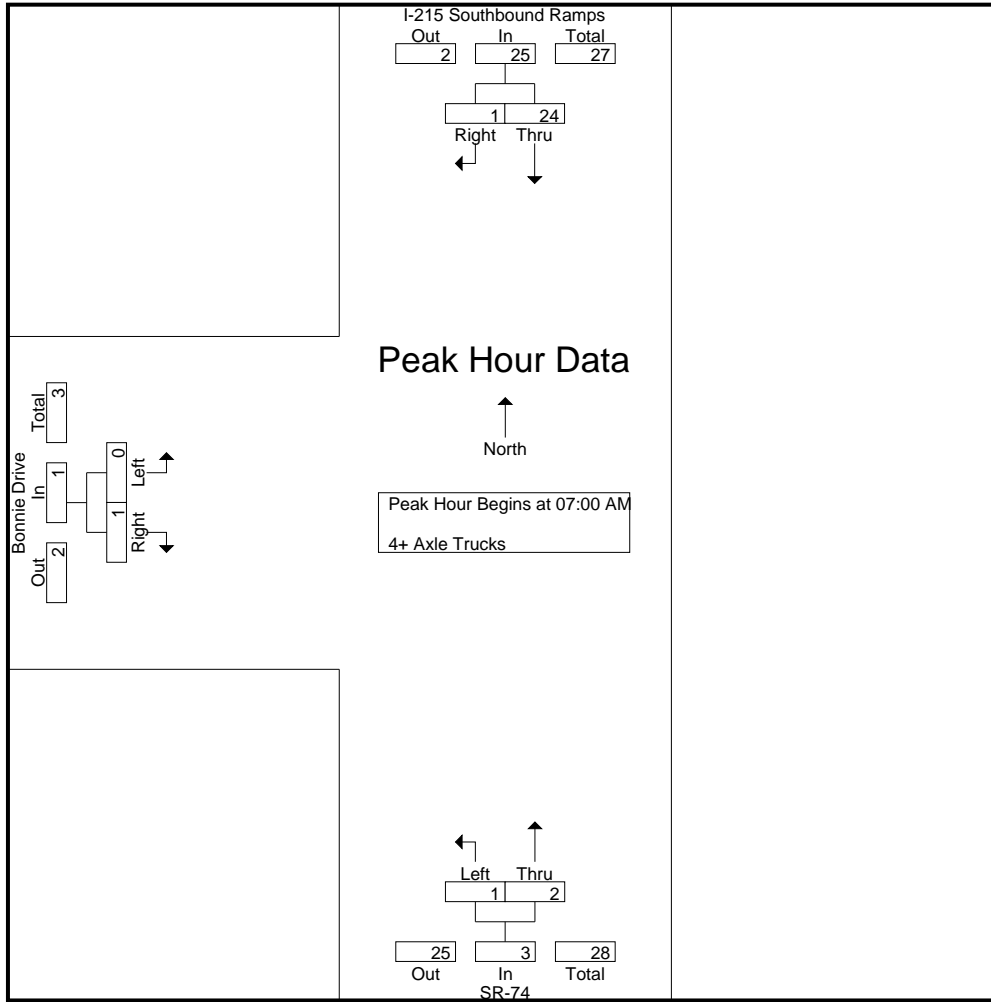
Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
07:00 AM	6	0	6	0	1	1	0	0	0	7
07:15 AM	4	0	4	0	1	1	0	0	0	5
07:30 AM	9	1	10	0	0	0	0	0	0	10
07:45 AM	5	0	5	1	0	1	0	1	1	7
Total	24	1	25	1	2	3	0	1	1	29
08:00 AM	6	0	6	0	0	0	0	0	0	6
08:15 AM	3	0	3	0	0	0	0	0	0	3
08:30 AM	4	0	4	1	1	2	0	1	1	7
08:45 AM	3	0	3	0	0	0	0	0	0	3
Total	16	0	16	1	1	2	0	1	1	19
Grand Total	40	1	41	2	3	5	0	2	2	48
Apprch %	97.6	2.4		40	60		0	100		
Total %	83.3	2.1	85.4	4.2	6.2	10.4	0	4.2	4.2	

Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
07:00 AM	6	0	6	0	1	1	0	0	0	7
07:15 AM	4	0	4	0	1	1	0	0	0	5
07:30 AM	9	1	10	0	0	0	0	0	0	10
07:45 AM	5	0	5	1	0	1	0	1	1	7
Total Volume	24	1	25	1	2	3	0	1	1	29
% App. Total	96	4		33.3	66.7		0	100		
PHF	.667	.250	.625	.250	.500	.750	.000	.250	.250	.725

Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1
 Peak Hour for Entire Intersection Begins at 07:00 AM

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:00 AM			07:00 AM			07:00 AM		
+0 mins.	6	0	6	0	1	1	0	0	0
+15 mins.	4	0	4	0	1	1	0	0	0
+30 mins.	9	1	10	0	0	0	0	0	0
+45 mins.	5	0	5	1	0	1	0	1	1
Total Volume	24	1	25	1	2	3	0	1	1
% App. Total	96	4		33.3	66.7		0	100	
PHF	.667	.250	.625	.250	.500	.750	.000	.250	.250

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

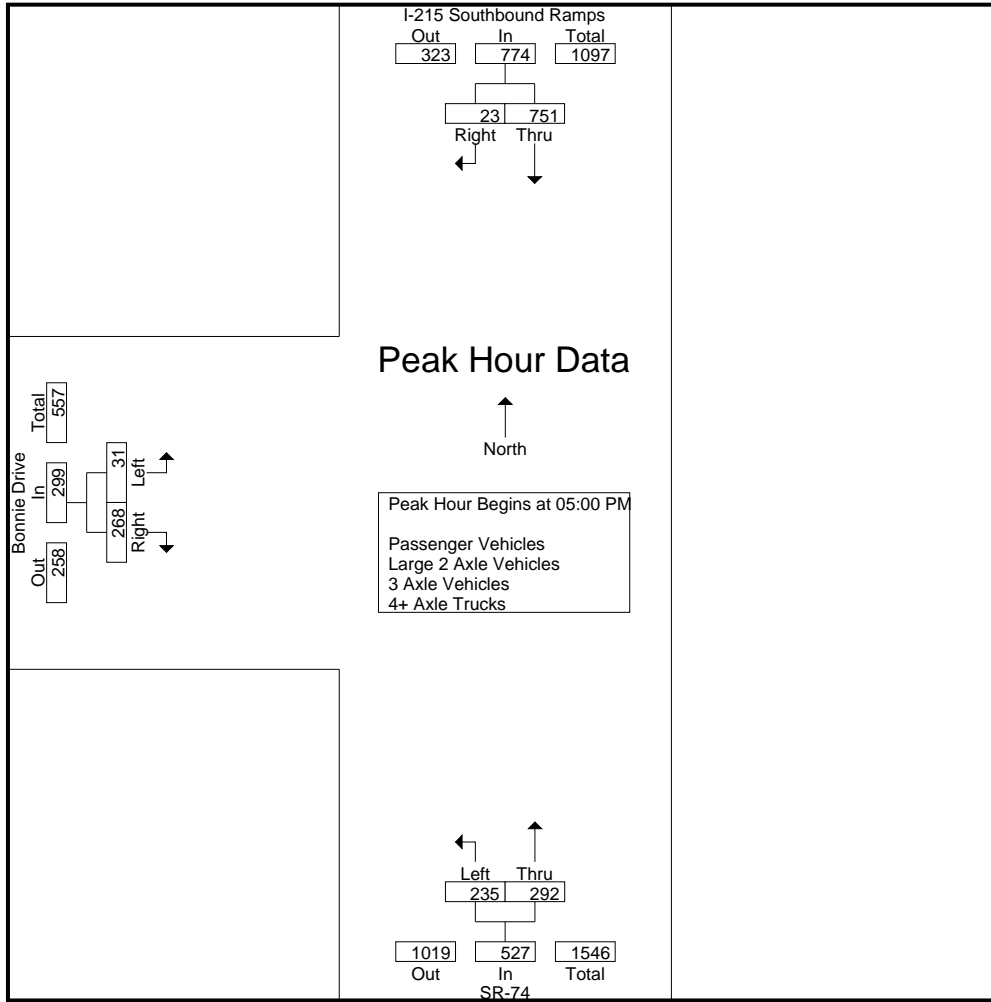
Groups Printed- Passenger Vehicles - Large 2 Axle Vehicles - 3 Axle Vehicles - 4+ Axle Trucks

Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
04:00 PM	193	5	198	61	76	137	9	73	82	417
04:15 PM	177	8	185	63	71	134	7	69	76	395
04:30 PM	177	9	186	46	90	136	10	71	81	403
04:45 PM	178	4	182	44	54	98	3	64	67	347
Total	725	26	751	214	291	505	29	277	306	1562
05:00 PM	189	8	197	71	109	180	4	66	70	447
05:15 PM	177	7	184	45	69	114	7	64	71	369
05:30 PM	178	5	183	52	58	110	4	72	76	369
05:45 PM	207	3	210	67	56	123	16	66	82	415
Total	751	23	774	235	292	527	31	268	299	1600
Grand Total	1476	49	1525	449	583	1032	60	545	605	3162
Apprch %	96.8	3.2		43.5	56.5		9.9	90.1		
Total %	46.7	1.5	48.2	14.2	18.4	32.6	1.9	17.2	19.1	
Passenger Vehicles	1418	49	1467	411	568	979	57	538	595	3041
% Passenger Vehicles	96.1	100	96.2	91.5	97.4	94.9	95	98.7	98.3	96.2
Large 2 Axle Vehicles	48	0	48	22	13	35	3	5	8	91
% Large 2 Axle Vehicles	3.3	0	3.1	4.9	2.2	3.4	5	0.9	1.3	2.9
3 Axle Vehicles	3	0	3	16	1	17	0	1	1	21
% 3 Axle Vehicles	0.2	0	0.2	3.6	0.2	1.6	0	0.2	0.2	0.7
4+ Axle Trucks	7	0	7	0	1	1	0	1	1	9
% 4+ Axle Trucks	0.5	0	0.5	0	0.2	0.1	0	0.2	0.2	0.3

Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 05:00 PM										
05:00 PM	189	8	197	71	109	180	4	66	70	447
05:15 PM	177	7	184	45	69	114	7	64	71	369
05:30 PM	178	5	183	52	58	110	4	72	76	369
05:45 PM	207	3	210	67	56	123	16	66	82	415
Total Volume	751	23	774	235	292	527	31	268	299	1600
% App. Total	97	3		44.6	55.4		10.4	89.6		
PHF	.907	.719	.921	.827	.670	.732	.484	.931	.912	.895

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	05:00 PM			04:15 PM			04:00 PM		
+0 mins.	189	8	197	63	71	134	9	73	82
+15 mins.	177	7	184	46	90	136	7	69	76
+30 mins.	178	5	183	44	54	98	10	71	81
+45 mins.	207	3	210	71	109	180	3	64	67
Total Volume	751	23	774	224	324	548	29	277	306
% App. Total	97	3		40.9	59.1		9.5	90.5	
PHF	.907	.719	.921	.789	.743	.761	.725	.949	.933

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- Passenger Vehicles

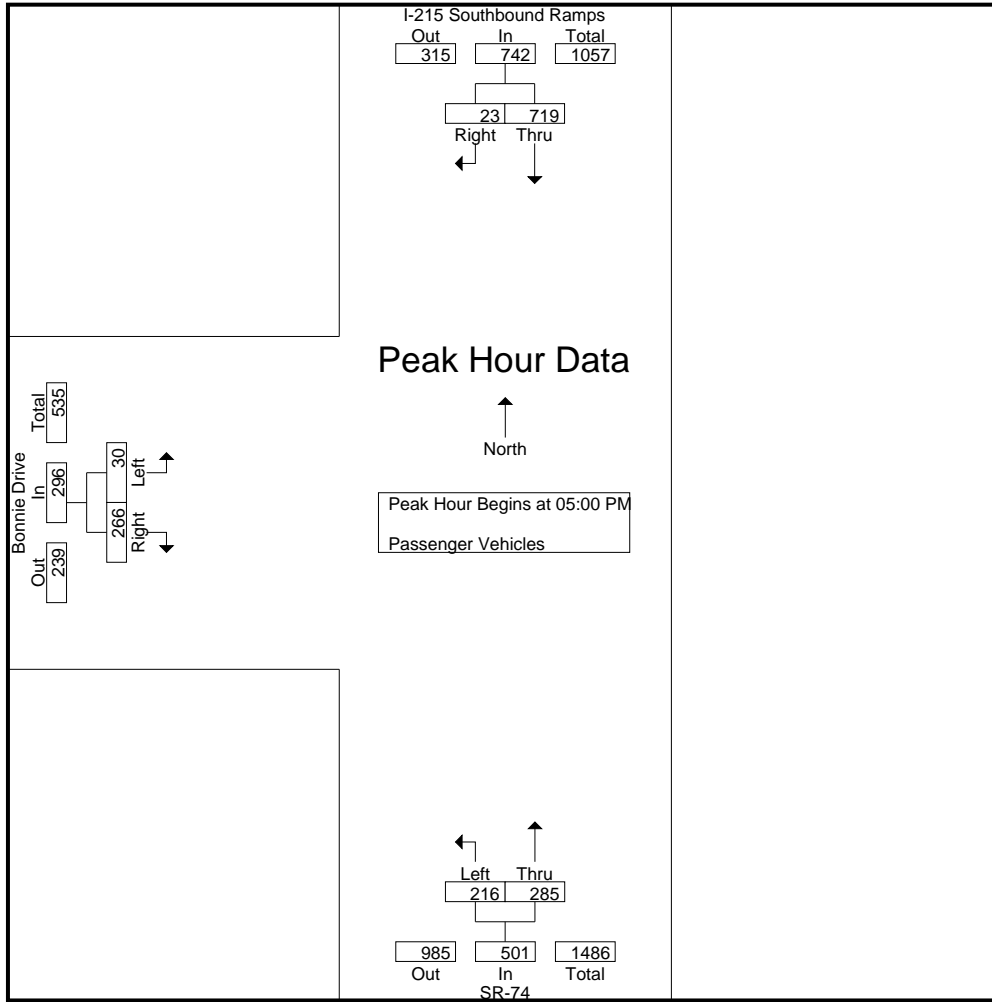
Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
04:00 PM	187	5	192	57	73	130	9	70	79	401
04:15 PM	168	8	176	58	70	128	7	68	75	379
04:30 PM	172	9	181	40	88	128	8	70	78	387
04:45 PM	172	4	176	40	52	92	3	64	67	335
Total	699	26	725	195	283	478	27	272	299	1502
05:00 PM	181	8	189	65	108	173	3	66	69	431
05:15 PM	166	7	173	40	68	108	7	64	71	352
05:30 PM	168	5	173	49	58	107	4	71	75	355
05:45 PM	204	3	207	62	51	113	16	65	81	401
Total	719	23	742	216	285	501	30	266	296	1539
Grand Total	1418	49	1467	411	568	979	57	538	595	3041
Apprch %	96.7	3.3		42	58		9.6	90.4		
Total %	46.6	1.6	48.2	13.5	18.7	32.2	1.9	17.7	19.6	

Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
05:00 PM	181	8	189	65	108	173	3	66	69	431
05:15 PM	166	7	173	40	68	108	7	64	71	352
05:30 PM	168	5	173	49	58	107	4	71	75	355
05:45 PM	204	3	207	62	51	113	16	65	81	401
Total Volume	719	23	742	216	285	501	30	266	296	1539
% App. Total	96.9	3.1		43.1	56.9		10.1	89.9		
PHF	.881	.719	.896	.831	.660	.724	.469	.937	.914	.893

Peak Hour Analysis From 05:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Entire Intersection Begins at 05:00 PM

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 05:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	05:00 PM			05:00 PM			05:00 PM		
+0 mins.	181	8	189	65	108	173	3	66	69
+15 mins.	166	7	173	40	68	108	7	64	71
+30 mins.	168	5	173	49	58	107	4	71	75
+45 mins.	204	3	207	62	51	113	16	65	81
Total Volume	719	23	742	216	285	501	30	266	296
% App. Total	96.9	3.1		43.1	56.9		10.1	89.9	
PHF	.881	.719	.896	.831	.660	.724	.469	.937	.914

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- Large 2 Axle Vehicles

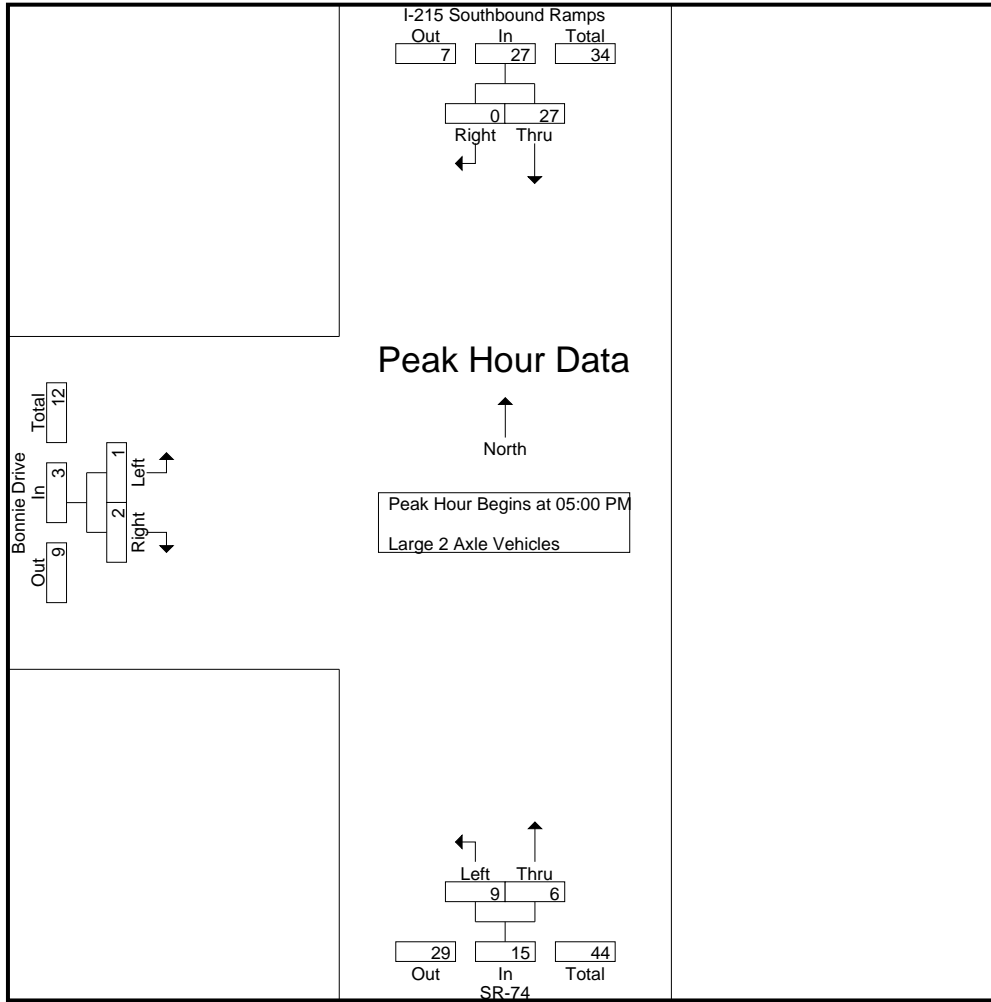
Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
04:00 PM	3	0	3	4	3	7	0	2	2	12
04:15 PM	8	0	8	3	1	4	0	0	0	12
04:30 PM	4	0	4	4	2	6	2	1	3	13
04:45 PM	6	0	6	2	1	3	0	0	0	9
Total	21	0	21	13	7	20	2	3	5	46
05:00 PM	8	0	8	3	1	4	1	0	1	13
05:15 PM	9	0	9	3	1	4	0	0	0	13
05:30 PM	8	0	8	1	0	1	0	1	1	10
05:45 PM	2	0	2	2	4	6	0	1	1	9
Total	27	0	27	9	6	15	1	2	3	45
Grand Total	48	0	48	22	13	35	3	5	8	91
Apprch %	100	0		62.9	37.1		37.5	62.5		
Total %	52.7	0	52.7	24.2	14.3	38.5	3.3	5.5	8.8	

Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
05:00 PM	8	0	8	3	1	4	1	0	1	13
05:15 PM	9	0	9	3	1	4	0	0	0	13
05:30 PM	8	0	8	1	0	1	0	1	1	10
05:45 PM	2	0	2	2	4	6	0	1	1	9
Total Volume	27	0	27	9	6	15	1	2	3	45
% App. Total	100	0		60	40		33.3	66.7		
PHF	.750	.000	.750	.750	.375	.625	.250	.500	.750	.865

Peak Hour Analysis From 05:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Entire Intersection Begins at 05:00 PM

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 05:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	05:00 PM			05:00 PM			05:00 PM		
+0 mins.	8	0	8	3	1	4	1	0	1
+15 mins.	9	0	9	3	1	4	0	0	0
+30 mins.	8	0	8	1	0	1	0	1	1
+45 mins.	2	0	2	2	4	6	0	1	1
Total Volume	27	0	27	9	6	15	1	2	3
% App. Total	100	0		60	40		33.3	66.7	
PHF	.750	.000	.750	.750	.375	.625	.250	.500	.750

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- 3 Axle Vehicles

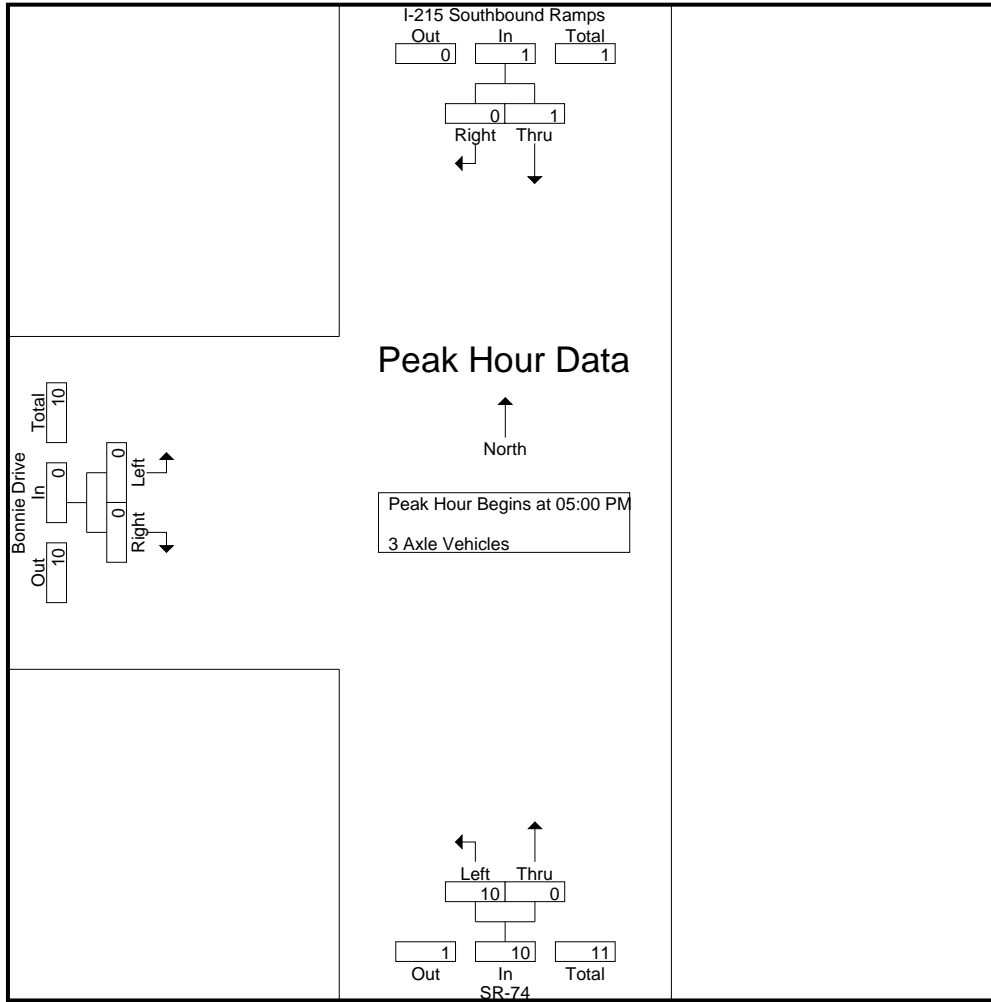
Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
04:00 PM	1	0	1	0	0	0	0	0	0	1
04:15 PM	0	0	0	2	0	2	0	1	1	3
04:30 PM	1	0	1	2	0	2	0	0	0	3
04:45 PM	0	0	0	2	1	3	0	0	0	3
Total	2	0	2	6	1	7	0	1	1	10
05:00 PM	0	0	0	3	0	3	0	0	0	3
05:15 PM	0	0	0	2	0	2	0	0	0	2
05:30 PM	0	0	0	2	0	2	0	0	0	2
05:45 PM	1	0	1	3	0	3	0	0	0	4
Total	1	0	1	10	0	10	0	0	0	11
Grand Total	3	0	3	16	1	17	0	1	1	21
Apprch %	100	0		94.1	5.9		0	100		
Total %	14.3	0	14.3	76.2	4.8	81	0	4.8	4.8	

Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
05:00 PM	0	0	0	3	0	3	0	0	0	3
05:15 PM	0	0	0	2	0	2	0	0	0	2
05:30 PM	0	0	0	2	0	2	0	0	0	2
05:45 PM	1	0	1	3	0	3	0	0	0	4
Total Volume	1	0	1	10	0	10	0	0	0	11
% App. Total	100	0		100	0		0	0		
PHF	.250	.000	.250	.833	.000	.833	.000	.000	.000	.688

Peak Hour Analysis From 05:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Entire Intersection Begins at 05:00 PM

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 05:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	05:00 PM			05:00 PM			05:00 PM		
+0 mins.	0	0	0	3	0	3	0	0	0
+15 mins.	0	0	0	2	0	2	0	0	0
+30 mins.	0	0	0	2	0	2	0	0	0
+45 mins.	1	0	1	3	0	3	0	0	0
Total Volume	1	0	1	10	0	10	0	0	0
% App. Total	100	0		100	0		0	0	
PHF	.250	.000	.250	.833	.000	.833	.000	.000	.000

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- 4+ Axle Trucks

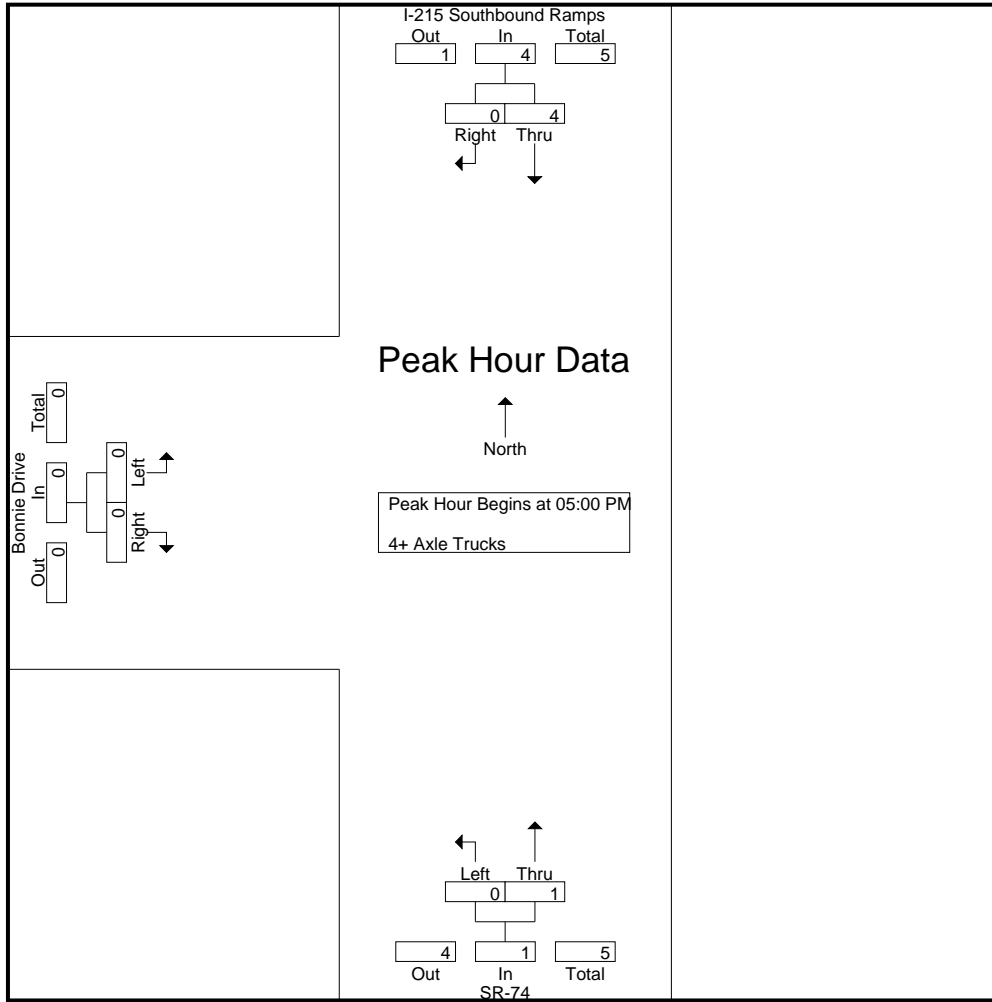
Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
04:00 PM	2	0	2	0	0	0	0	1	1	3
04:15 PM	1	0	1	0	0	0	0	0	0	1
04:30 PM	0	0	0	0	0	0	0	0	0	0
04:45 PM	0	0	0	0	0	0	0	0	0	0
Total	3	0	3	0	0	0	0	1	1	4
05:00 PM	0	0	0	0	0	0	0	0	0	0
05:15 PM	2	0	2	0	0	0	0	0	0	2
05:30 PM	2	0	2	0	0	0	0	0	0	2
05:45 PM	0	0	0	0	1	1	0	0	0	1
Total	4	0	4	0	1	1	0	0	0	5
Grand Total	7	0	7	0	1	1	0	1	1	9
Apprch %	100	0		0	100		0	100		
Total %	77.8	0	77.8	0	11.1	11.1	0	11.1	11.1	

Start Time	I-215 Southbound Ramps Southbound			SR-74 Northbound			Bonnie Drive Eastbound			Int. Total
	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	
05:00 PM	0	0	0	0	0	0	0	0	0	0
05:15 PM	2	0	2	0	0	0	0	0	0	2
05:30 PM	2	0	2	0	0	0	0	0	0	2
05:45 PM	0	0	0	0	1	1	0	0	0	1
Total Volume	4	0	4	0	1	1	0	0	0	5
% App. Total	100	0		0	100		0	0		
PHF	.500	.000	.500	.000	.250	.250	.000	.000	.000	.625

Peak Hour Analysis From 05:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Entire Intersection Begins at 05:00 PM

City of Perris
 N/S: I-215 Southbound Ramps/SR-74
 E/W: Bonnie Drive
 Weather: Clear

File Name : 01_PER_215S_Bonnie PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 05:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	05:00 PM			05:00 PM			05:00 PM		
+0 mins.	0	0	0	0	0	0	0	0	0
+15 mins.	2	0	2	0	0	0	0	0	0
+30 mins.	2	0	2	0	0	0	0	0	0
+45 mins.	0	0	0	0	1	1	0	0	0
Total Volume	4	0	4	0	1	1	0	0	0
% App. Total	100	0		0	100		0	0	
PHF	.500	.000	.500	.000	.250	.250	.000	.000	.000

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

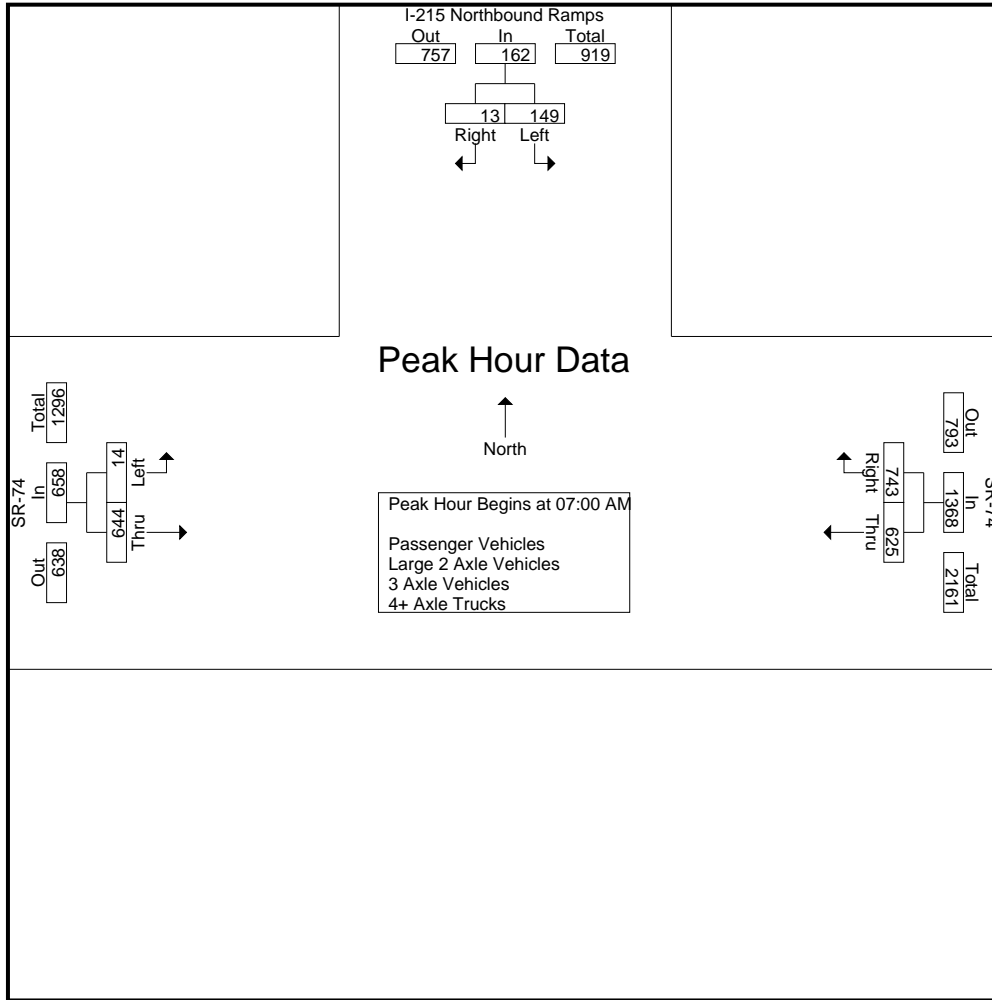
Groups Printed- Passenger Vehicles - Large 2 Axle Vehicles - 3 Axle Vehicles - 4+ Axle Trucks

Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
07:00 AM	32	5	37	145	175	320	8	155	163	520
07:15 AM	48	4	52	150	199	349	2	179	181	582
07:30 AM	41	2	43	153	185	338	3	150	153	534
07:45 AM	28	2	30	177	184	361	1	160	161	552
Total	149	13	162	625	743	1368	14	644	658	2188
08:00 AM	27	4	31	131	198	329	3	147	150	510
08:15 AM	26	0	26	119	150	269	1	159	160	455
08:30 AM	30	5	35	103	150	253	2	149	151	439
08:45 AM	24	2	26	107	145	252	3	133	136	414
Total	107	11	118	460	643	1103	9	588	597	1818
Grand Total	256	24	280	1085	1386	2471	23	1232	1255	4006
Apprch %	91.4	8.6		43.9	56.1		1.8	98.2		
Total %	6.4	0.6	7	27.1	34.6	61.7	0.6	30.8	31.3	
Passenger Vehicles	231	18	249	971	1300	2271	22	1062	1084	3604
% Passenger Vehicles	90.2	75	88.9	89.5	93.8	91.9	95.7	86.2	86.4	90
Large 2 Axle Vehicles	12	2	14	68	42	110	0	86	86	210
% Large 2 Axle Vehicles	4.7	8.3	5	6.3	3	4.5	0	7	6.9	5.2
3 Axle Vehicles	2	1	3	18	12	30	0	37	37	70
% 3 Axle Vehicles	0.8	4.2	1.1	1.7	0.9	1.2	0	3	2.9	1.7
4+ Axle Trucks	11	3	14	28	32	60	1	47	48	122
% 4+ Axle Trucks	4.3	12.5	5	2.6	2.3	2.4	4.3	3.8	3.8	3

Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 07:00 AM										
07:00 AM	32	5	37	145	175	320	8	155	163	520
07:15 AM	48	4	52	150	199	349	2	179	181	582
07:30 AM	41	2	43	153	185	338	3	150	153	534
07:45 AM	28	2	30	177	184	361	1	160	161	552
Total Volume	149	13	162	625	743	1368	14	644	658	2188
% App. Total	92	8		45.7	54.3		2.1	97.9		
PHF	.776	.650	.779	.883	.933	.947	.438	.899	.909	.940

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:00 AM			07:15 AM			07:00 AM		
+0 mins.	32	5	37	150	199	349	8	155	163
+15 mins.	48	4	52	153	185	338	2	179	181
+30 mins.	41	2	43	177	184	361	3	150	153
+45 mins.	28	2	30	131	198	329	1	160	161
Total Volume	149	13	162	611	766	1377	14	644	658
% App. Total	92	8		44.4	55.6		2.1	97.9	
PHF	.776	.650	.779	.863	.962	.954	.438	.899	.909

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- Passenger Vehicles

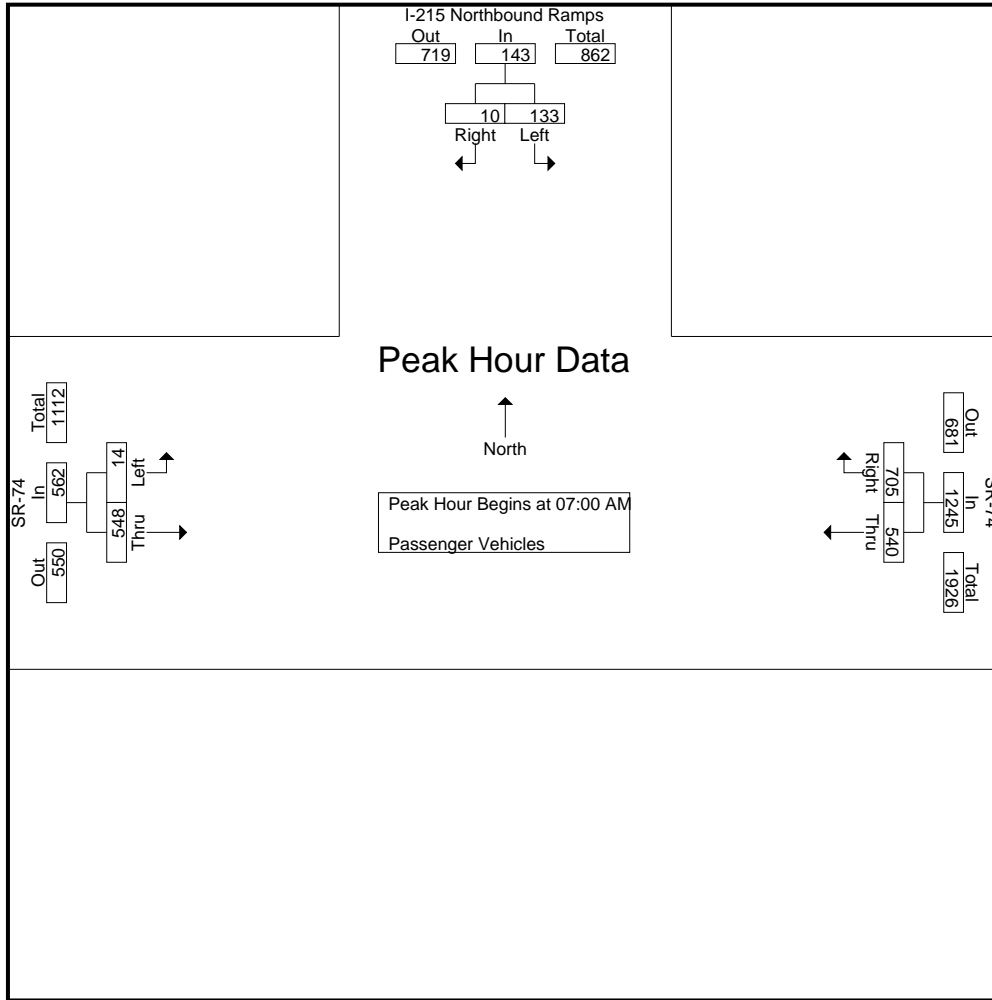
Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
07:00 AM	30	5	35	105	162	267	8	127	135	437
07:15 AM	44	3	47	131	188	319	2	156	158	524
07:30 AM	35	1	36	133	177	310	3	126	129	475
07:45 AM	24	1	25	171	178	349	1	139	140	514
Total	133	10	143	540	705	1245	14	548	562	1950
08:00 AM	24	3	27	126	180	306	3	125	128	461
08:15 AM	23	0	23	109	138	247	1	142	143	413
08:30 AM	28	4	32	95	144	239	2	131	133	404
08:45 AM	23	1	24	101	133	234	2	116	118	376
Total	98	8	106	431	595	1026	8	514	522	1654
Grand Total	231	18	249	971	1300	2271	22	1062	1084	3604
Apprch %	92.8	7.2		42.8	57.2		2	98		
Total %	6.4	0.5	6.9	26.9	36.1	63	0.6	29.5	30.1	

Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
07:00 AM	30	5	35	105	162	267	8	127	135	437
07:15 AM	44	3	47	131	188	319	2	156	158	524
07:30 AM	35	1	36	133	177	310	3	126	129	475
07:45 AM	24	1	25	171	178	349	1	139	140	514
Total Volume	133	10	143	540	705	1245	14	548	562	1950
% App. Total	93	7		43.4	56.6		2.5	97.5		
PHF	.756	.500	.761	.789	.938	.892	.438	.878	.889	.930

Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1
 Peak Hour for Entire Intersection Begins at 07:00 AM

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:00 AM			07:00 AM			07:00 AM		
+0 mins.	30	5	35	105	162	267	8	127	135
+15 mins.	44	3	47	131	188	319	2	156	158
+30 mins.	35	1	36	133	177	310	3	126	129
+45 mins.	24	1	25	171	178	349	1	139	140
Total Volume	133	10	143	540	705	1245	14	548	562
% App. Total	93	7		43.4	56.6		2.5	97.5	
PHF	.756	.500	.761	.789	.938	.892	.438	.878	.889

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- Large 2 Axle Vehicles

Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
07:00 AM	1	0	1	22	6	28	0	12	12	41
07:15 AM	2	0	2	10	6	16	0	14	14	32
07:30 AM	2	1	3	13	5	18	0	10	10	31
07:45 AM	2	1	3	5	3	8	0	13	13	24
Total	7	2	9	50	20	70	0	49	49	128
08:00 AM	1	0	1	1	10	11	0	11	11	23
08:15 AM	3	0	3	7	6	13	0	9	9	25
08:30 AM	1	0	1	5	0	5	0	6	6	12
08:45 AM	0	0	0	5	6	11	0	11	11	22
Total	5	0	5	18	22	40	0	37	37	82
Grand Total	12	2	14	68	42	110	0	86	86	210
Apprch %	85.7	14.3		61.8	38.2		0	100		
Total %	5.7	1	6.7	32.4	20	52.4	0	41	41	

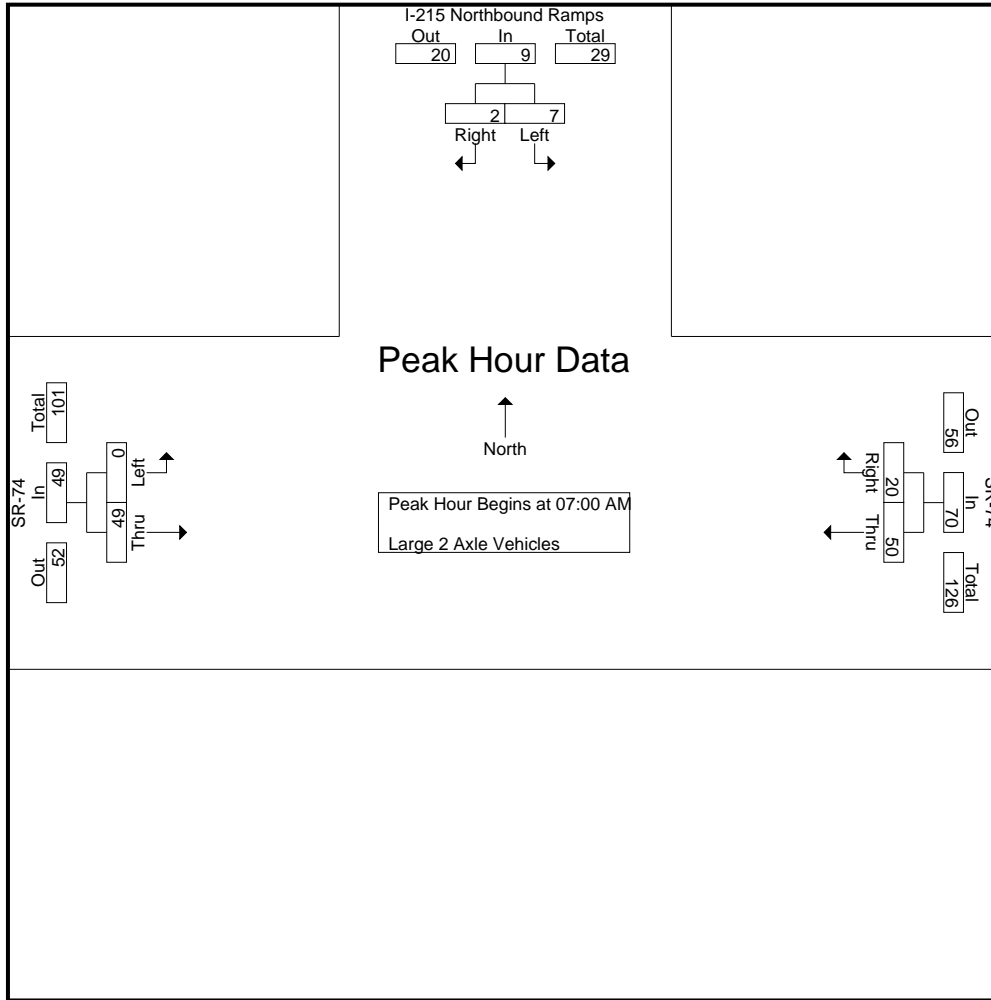
Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
07:00 AM	1	0	1	22	6	28	0	12	12	41
07:15 AM	2	0	2	10	6	16	0	14	14	32
07:30 AM	2	1	3	13	5	18	0	10	10	31
07:45 AM	2	1	3	5	3	8	0	13	13	24
Total Volume	7	2	9	50	20	70	0	49	49	128
% App. Total	77.8	22.2		71.4	28.6		0	100		
PHF	.875	.500	.750	.568	.833	.625	.000	.875	.875	.780

Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 07:00 AM

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:00 AM			07:00 AM			07:00 AM		
+0 mins.	1	0	1	22	6	28	0	12	12
+15 mins.	2	0	2	10	6	16	0	14	14
+30 mins.	2	1	3	13	5	18	0	10	10
+45 mins.	2	1	3	5	3	8	0	13	13
Total Volume	7	2	9	50	20	70	0	49	49
% App. Total	77.8	22.2		71.4	28.6		0	100	
PHF	.875	.500	.750	.568	.833	.625	.000	.875	.875

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- 3 Axle Vehicles

Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
07:00 AM	0	0	0	5	2	7	0	8	8	15
07:15 AM	0	0	0	3	0	3	0	5	5	8
07:30 AM	1	0	1	2	2	4	0	4	4	9
07:45 AM	0	0	0	0	1	1	0	2	2	3
Total	1	0	1	10	5	15	0	19	19	35
08:00 AM	0	0	0	3	3	6	0	4	4	10
08:15 AM	0	0	0	2	1	3	0	4	4	7
08:30 AM	1	0	1	2	1	3	0	6	6	10
08:45 AM	0	1	1	1	2	3	0	4	4	8
Total	1	1	2	8	7	15	0	18	18	35
Grand Total	2	1	3	18	12	30	0	37	37	70
Apprch %	66.7	33.3		60	40		0	100		
Total %	2.9	1.4	4.3	25.7	17.1	42.9	0	52.9	52.9	

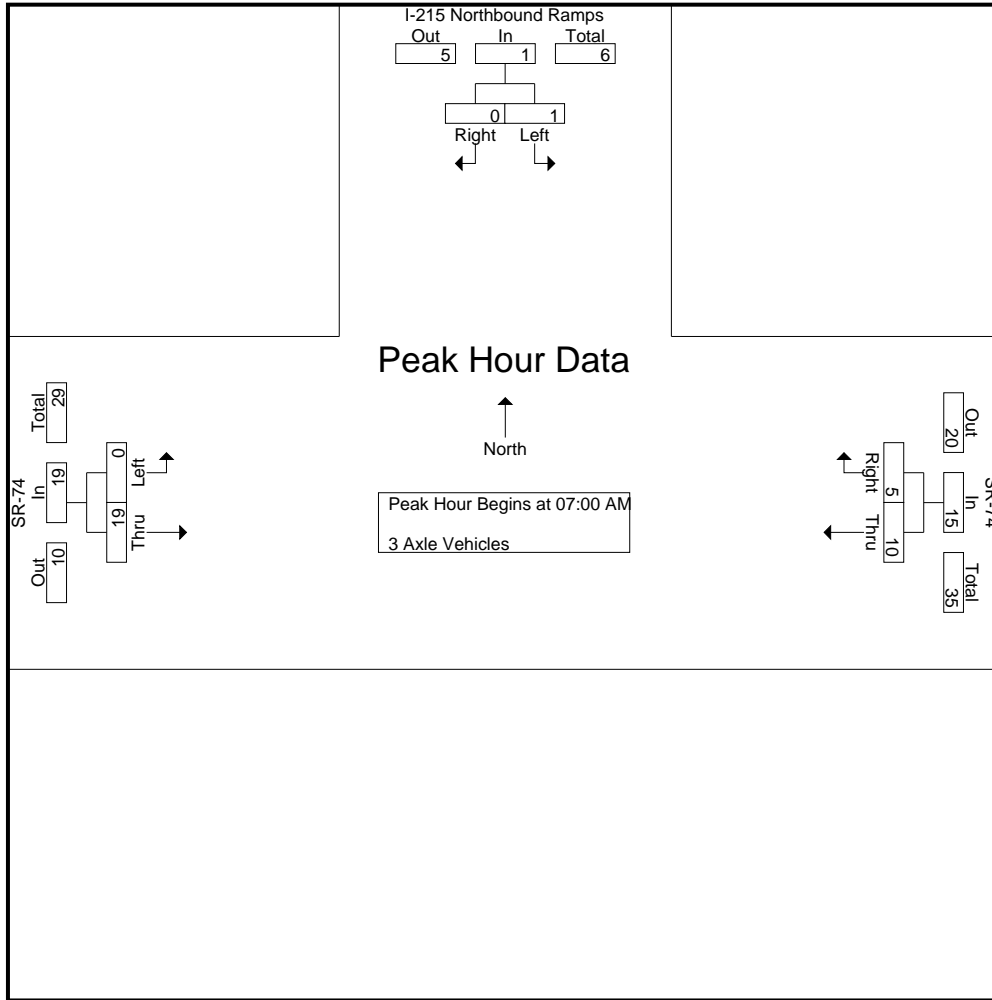
Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
07:00 AM	0	0	0	5	2	7	0	8	8	15
07:15 AM	0	0	0	3	0	3	0	5	5	8
07:30 AM	1	0	1	2	2	4	0	4	4	9
07:45 AM	0	0	0	0	1	1	0	2	2	3
Total Volume	1	0	1	10	5	15	0	19	19	35
% App. Total	100	0		66.7	33.3		0	100		
PHF	.250	.000	.250	.500	.625	.536	.000	.594	.594	.583

Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 07:00 AM

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:00 AM			07:00 AM			07:00 AM		
+0 mins.	0	0	0	5	2	7	0	8	8
+15 mins.	0	0	0	3	0	3	0	5	5
+30 mins.	1	0	1	2	2	4	0	4	4
+45 mins.	0	0	0	0	1	1	0	2	2
Total Volume	1	0	1	10	5	15	0	19	19
% App. Total	100	0		66.7	33.3		0	100	
PHF	.250	.000	.250	.500	.625	.536	.000	.594	.594

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- 4+ Axle Trucks

Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
07:00 AM	1	0	1	13	5	18	0	8	8	27
07:15 AM	2	1	3	6	5	11	0	4	4	18
07:30 AM	3	0	3	5	1	6	0	10	10	19
07:45 AM	2	0	2	1	2	3	0	6	6	11
Total	8	1	9	25	13	38	0	28	28	75
08:00 AM	2	1	3	1	5	6	0	7	7	16
08:15 AM	0	0	0	1	5	6	0	4	4	10
08:30 AM	0	1	1	1	5	6	0	6	6	13
08:45 AM	1	0	1	0	4	4	1	2	3	8
Total	3	2	5	3	19	22	1	19	20	47
Grand Total	11	3	14	28	32	60	1	47	48	122
Apprch %	78.6	21.4		46.7	53.3		2.1	97.9		
Total %	9	2.5	11.5	23	26.2	49.2	0.8	38.5	39.3	

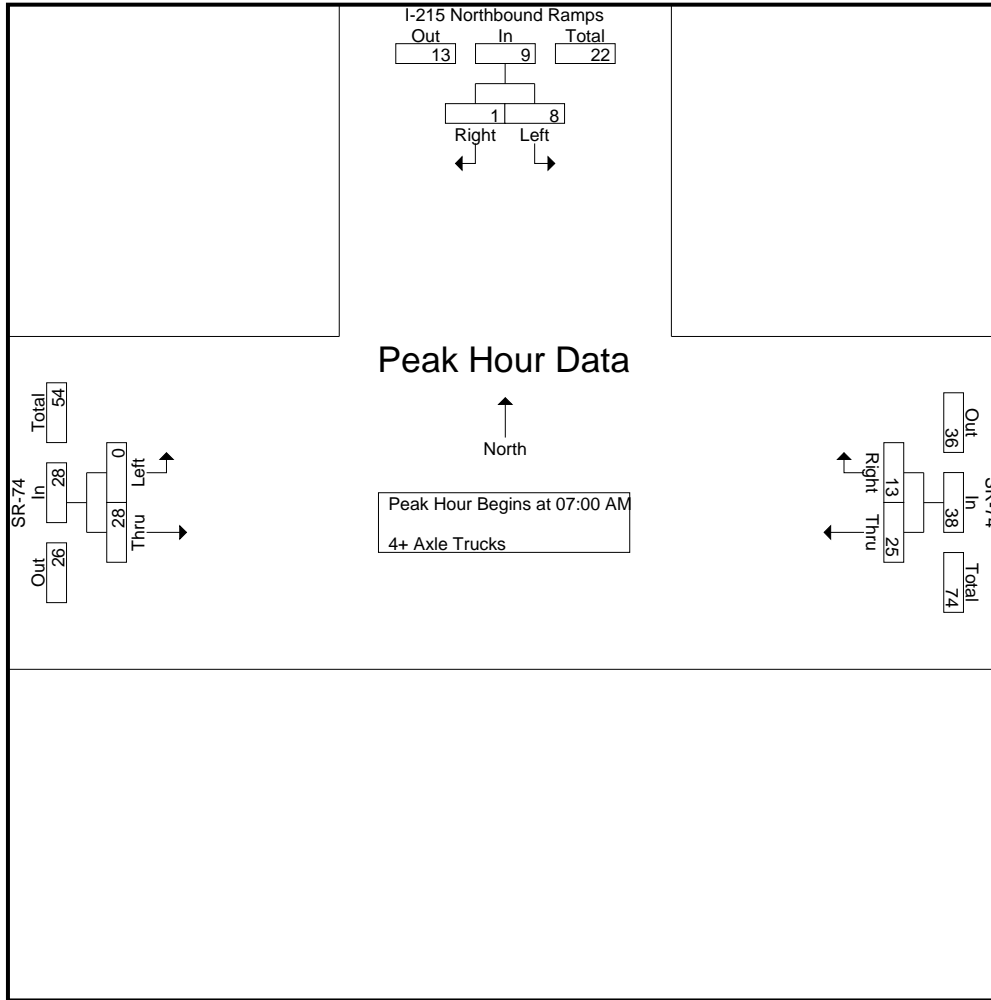
Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
07:00 AM	1	0	1	13	5	18	0	8	8	27
07:15 AM	2	1	3	6	5	11	0	4	4	18
07:30 AM	3	0	3	5	1	6	0	10	10	19
07:45 AM	2	0	2	1	2	3	0	6	6	11
Total Volume	8	1	9	25	13	38	0	28	28	75
% App. Total	88.9	11.1		65.8	34.2		0	100		
PHF	.667	.250	.750	.481	.650	.528	.000	.700	.700	.694

Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 07:00 AM

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:00 AM			07:00 AM			07:00 AM		
+0 mins.	1	0	1	13	5	18	0	8	8
+15 mins.	2	1	3	6	5	11	0	4	4
+30 mins.	3	0	3	5	1	6	0	10	10
+45 mins.	2	0	2	1	2	3	0	6	6
Total Volume	8	1	9	25	13	38	0	28	28
% App. Total	88.9	11.1		65.8	34.2		0	100	
PHF	.667	.250	.750	.481	.650	.528	.000	.700	.700

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

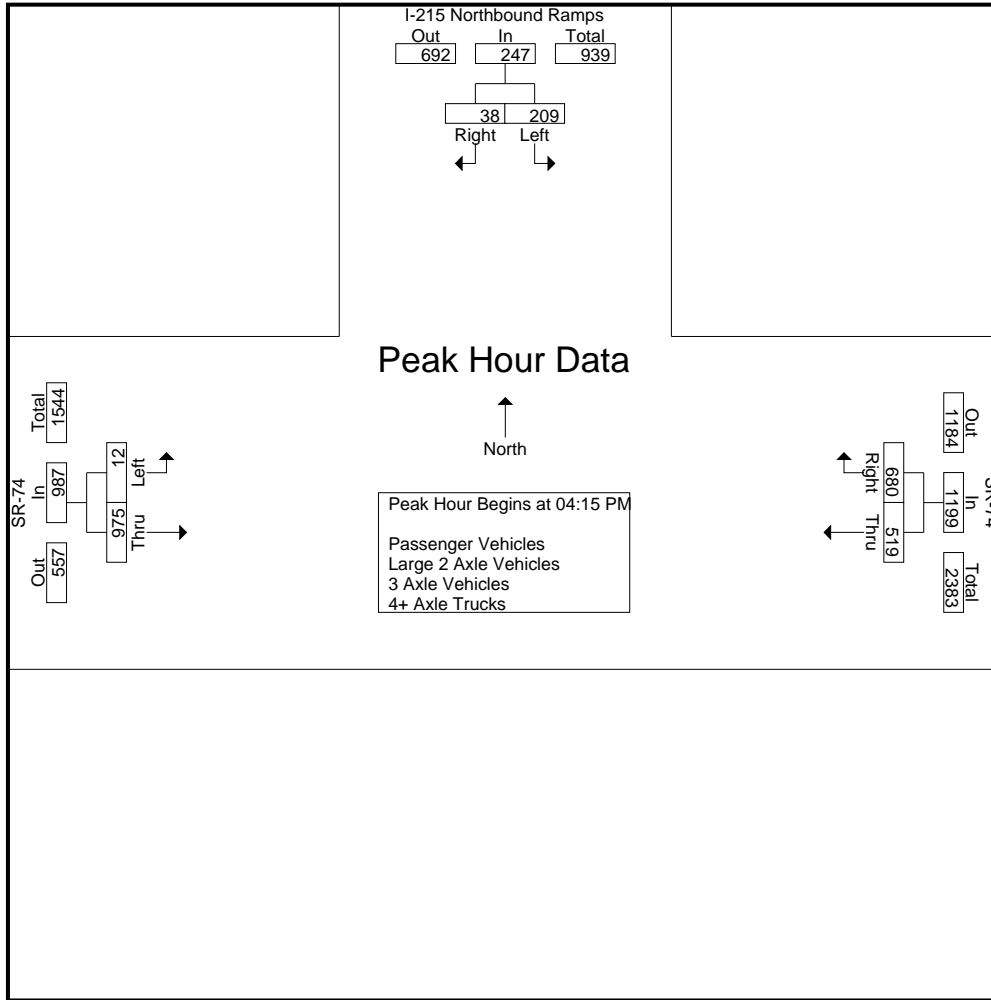
Groups Printed- Passenger Vehicles - Large 2 Axle Vehicles - 3 Axle Vehicles - 4+ Axle Trucks

Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
04:00 PM	52	5	57	126	169	295	4	268	272	624
04:15 PM	54	9	63	132	178	310	3	246	249	622
04:30 PM	44	9	53	125	169	294	3	236	239	586
04:45 PM	65	7	72	94	158	252	3	246	249	573
Total	215	30	245	477	674	1151	13	996	1009	2405
05:00 PM	46	13	59	168	175	343	3	247	250	652
05:15 PM	49	12	61	100	162	262	2	243	245	568
05:30 PM	65	11	76	96	179	275	4	248	252	603
05:45 PM	83	28	111	99	126	225	2	265	267	603
Total	243	64	307	463	642	1105	11	1003	1014	2426
Grand Total	458	94	552	940	1316	2256	24	1999	2023	4831
Apprch %	83	17		41.7	58.3		1.2	98.8		
Total %	9.5	1.9	11.4	19.5	27.2	46.7	0.5	41.4	41.9	
Passenger Vehicles	404	90	494	901	1253	2154	23	1951	1974	4622
% Passenger Vehicles	88.2	95.7	89.5	95.9	95.2	95.5	95.8	97.6	97.6	95.7
Large 2 Axle Vehicles	42	3	45	20	43	63	0	34	34	142
% Large 2 Axle Vehicles	9.2	3.2	8.2	2.1	3.3	2.8	0	1.7	1.7	2.9
3 Axle Vehicles	5	0	5	17	7	24	1	2	3	32
% 3 Axle Vehicles	1.1	0	0.9	1.8	0.5	1.1	4.2	0.1	0.1	0.7
4+ Axle Trucks	7	1	8	2	13	15	0	12	12	35
% 4+ Axle Trucks	1.5	1.1	1.4	0.2	1	0.7	0	0.6	0.6	0.7

Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 04:15 PM										
04:15 PM	54	9	63	132	178	310	3	246	249	622
04:30 PM	44	9	53	125	169	294	3	236	239	586
04:45 PM	65	7	72	94	158	252	3	246	249	573
05:00 PM	46	13	59	168	175	343	3	247	250	652
Total Volume	209	38	247	519	680	1199	12	975	987	2433
% App. Total	84.6	15.4		43.3	56.7		1.2	98.8		
PHF	.804	.731	.858	.772	.955	.874	1.00	.987	.987	.933

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	05:00 PM			04:15 PM			05:00 PM		
+0 mins.	46	13	59	132	178	310	3	247	250
+15 mins.	49	12	61	125	169	294	2	243	245
+30 mins.	65	11	76	94	158	252	4	248	252
+45 mins.	83	28	111	168	175	343	2	265	267
Total Volume	243	64	307	519	680	1199	11	1003	1014
% App. Total	79.2	20.8		43.3	56.7		1.1	98.9	
PHF	.732	.571	.691	.772	.955	.874	.688	.946	.949

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- Passenger Vehicles

Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
04:00 PM	46	5	51	121	156	277	4	258	262	590
04:15 PM	48	9	57	126	165	291	2	241	243	591
04:30 PM	37	8	45	119	164	283	3	231	234	562
04:45 PM	57	7	64	90	150	240	3	242	245	549
Total	188	29	217	456	635	1091	12	972	984	2292
05:00 PM	41	12	53	164	165	329	3	243	246	628
05:15 PM	43	12	55	95	157	252	2	234	236	543
05:30 PM	59	11	70	94	174	268	4	241	245	583
05:45 PM	73	26	99	92	122	214	2	261	263	576
Total	216	61	277	445	618	1063	11	979	990	2330
Grand Total	404	90	494	901	1253	2154	23	1951	1974	4622
Apprch %	81.8	18.2		41.8	58.2		1.2	98.8		
Total %	8.7	1.9	10.7	19.5	27.1	46.6	0.5	42.2	42.7	

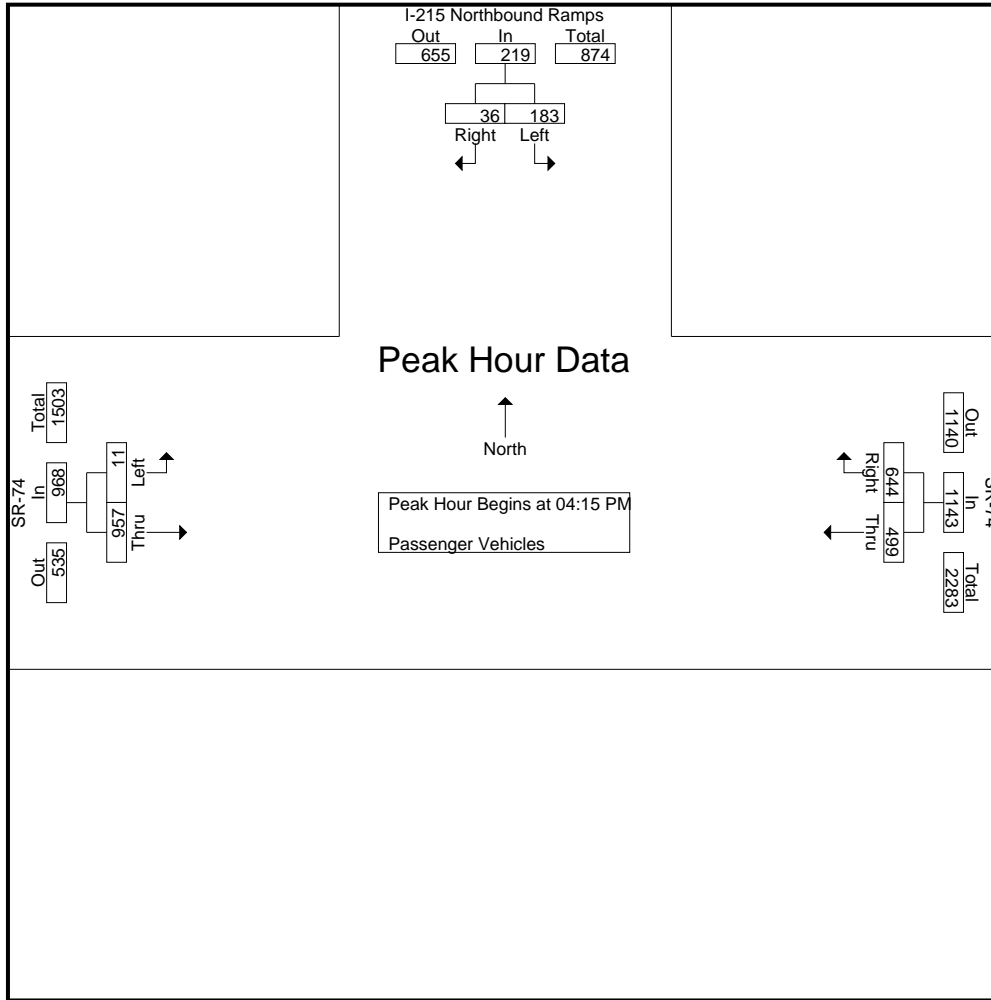
Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
04:15 PM	48	9	57	126	165	291	2	241	243	591
04:30 PM	37	8	45	119	164	283	3	231	234	562
04:45 PM	57	7	64	90	150	240	3	242	245	549
05:00 PM	41	12	53	164	165	329	3	243	246	628
Total Volume	183	36	219	499	644	1143	11	957	968	2330
% App. Total	83.6	16.4		43.7	56.3		1.1	98.9		
PHF	.803	.750	.855	.761	.976	.869	.917	.985	.984	.928

Peak Hour Analysis From 04:15 PM to 05:00 PM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 04:15 PM

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 04:15 PM to 05:00 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	04:15 PM			04:15 PM			04:15 PM		
+0 mins.	48	9	57	126	165	291	2	241	243
+15 mins.	37	8	45	119	164	283	3	231	234
+30 mins.	57	7	64	90	150	240	3	242	245
+45 mins.	41	12	53	164	165	329	3	243	246
Total Volume	183	36	219	499	644	1143	11	957	968
% App. Total	83.6	16.4		43.7	56.3		1.1	98.9	
PHF	.803	.750	.855	.761	.976	.869	.917	.985	.984

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- Large 2 Axle Vehicles

Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
04:00 PM	6	0	6	5	10	15	0	5	5	26
04:15 PM	4	0	4	3	8	11	0	4	4	19
04:30 PM	6	0	6	4	3	7	0	3	3	16
04:45 PM	5	0	5	1	4	5	0	4	4	14
Total	21	0	21	13	25	38	0	16	16	75
05:00 PM	4	1	5	1	10	11	0	4	4	20
05:15 PM	6	0	6	3	3	6	0	6	6	18
05:30 PM	5	0	5	0	4	4	0	5	5	14
05:45 PM	6	2	8	3	1	4	0	3	3	15
Total	21	3	24	7	18	25	0	18	18	67
Grand Total	42	3	45	20	43	63	0	34	34	142
Apprch %	93.3	6.7		31.7	68.3		0	100		
Total %	29.6	2.1	31.7	14.1	30.3	44.4	0	23.9	23.9	

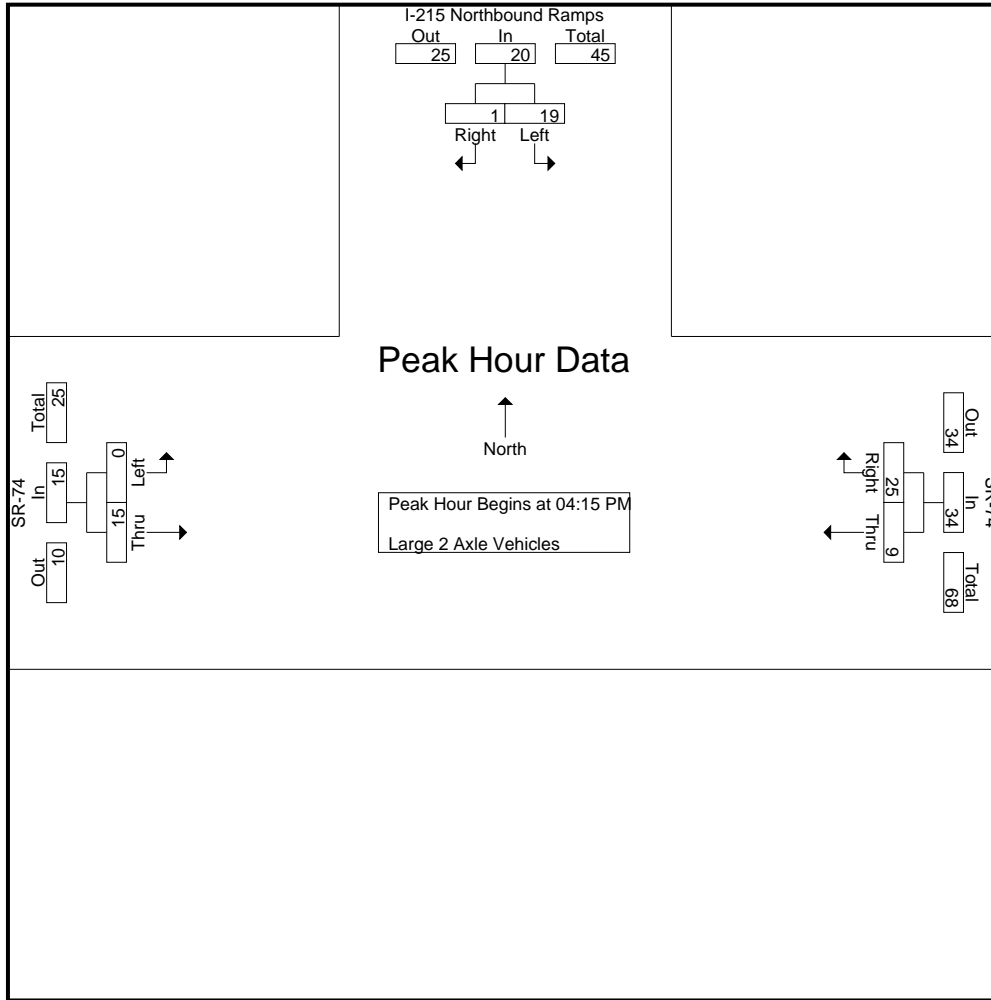
Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
04:15 PM	4	0	4	3	8	11	0	4	4	19
04:30 PM	6	0	6	4	3	7	0	3	3	16
04:45 PM	5	0	5	1	4	5	0	4	4	14
05:00 PM	4	1	5	1	10	11	0	4	4	20
Total Volume	19	1	20	9	25	34	0	15	15	69
% App. Total	95	5		26.5	73.5		0	100		
PHF	.792	.250	.833	.563	.625	.773	.000	.938	.938	.863

Peak Hour Analysis From 04:15 PM to 05:00 PM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 04:15 PM

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 04:15 PM to 05:00 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	04:15 PM			04:15 PM			04:15 PM		
+0 mins.	4	0	4	3	8	11	0	4	4
+15 mins.	6	0	6	4	3	7	0	3	3
+30 mins.	5	0	5	1	4	5	0	4	4
+45 mins.	4	1	5	1	10	11	0	4	4
Total Volume	19	1	20	9	25	34	0	15	15
% App. Total	95	5		26.5	73.5		0	100	
PHF	.792	.250	.833	.563	.625	.773	.000	.938	.938

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- 3 Axle Vehicles

Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
04:00 PM	0	0	0	0	2	2	0	1	1	3
04:15 PM	0	0	0	2	1	3	1	0	1	4
04:30 PM	0	0	0	2	0	2	0	0	0	2
04:45 PM	2	0	2	3	1	4	0	0	0	6
Total	2	0	2	7	4	11	1	1	2	15
05:00 PM	0	0	0	3	0	3	0	0	0	3
05:15 PM	0	0	0	2	2	4	0	0	0	4
05:30 PM	0	0	0	2	0	2	0	0	0	2
05:45 PM	3	0	3	3	1	4	0	1	1	8
Total	3	0	3	10	3	13	0	1	1	17
Grand Total	5	0	5	17	7	24	1	2	3	32
Apprch %	100	0		70.8	29.2		33.3	66.7		
Total %	15.6	0	15.6	53.1	21.9	75	3.1	6.2	9.4	

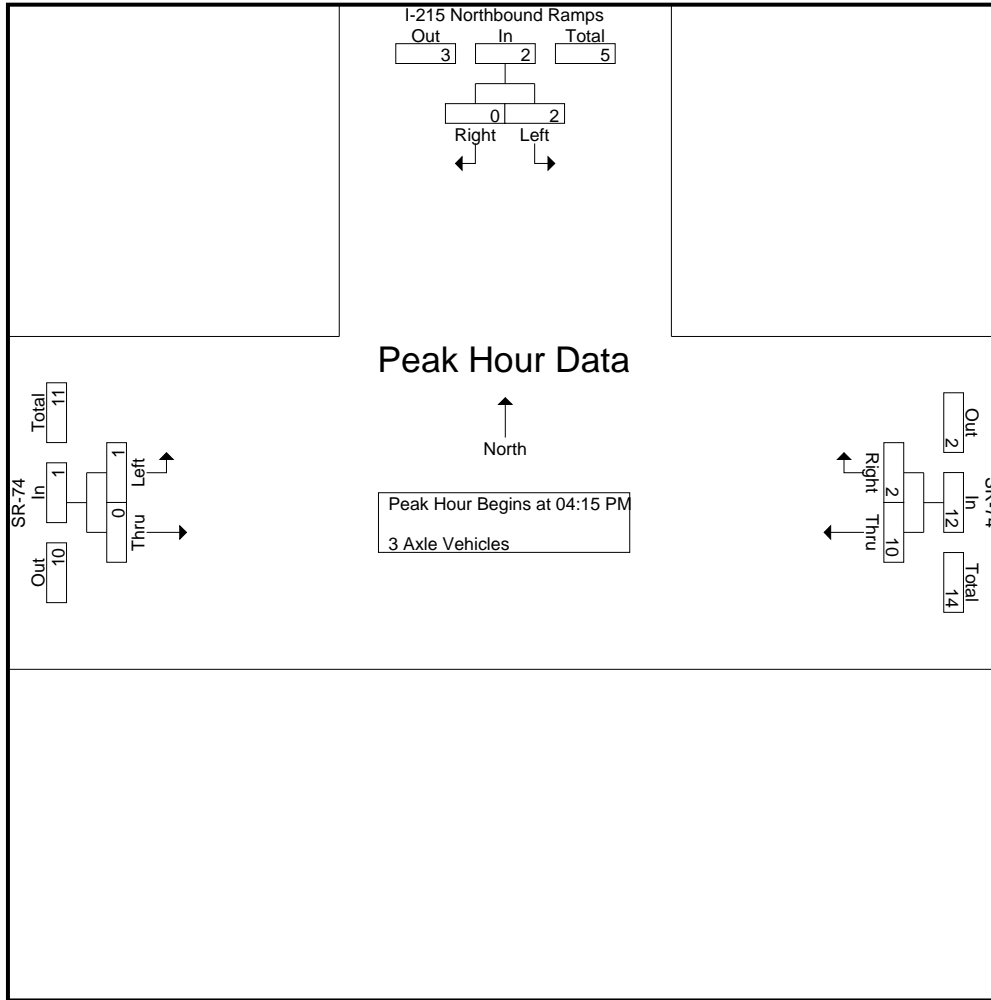
Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
04:15 PM	0	0	0	2	1	3	1	0	1	4
04:30 PM	0	0	0	2	0	2	0	0	0	2
04:45 PM	2	0	2	3	1	4	0	0	0	6
05:00 PM	0	0	0	3	0	3	0	0	0	3
Total Volume	2	0	2	10	2	12	1	0	1	15
% App. Total	100	0		83.3	16.7		100	0		
PHF	.250	.000	.250	.833	.500	.750	.250	.000	.250	.625

Peak Hour Analysis From 04:15 PM to 05:00 PM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 04:15 PM

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 04:15 PM to 05:00 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	04:15 PM			04:15 PM			04:15 PM		
+0 mins.	0	0	0	2	1	3	1	0	1
+15 mins.	0	0	0	2	0	2	0	0	0
+30 mins.	2	0	2	3	1	4	0	0	0
+45 mins.	0	0	0	3	0	3	0	0	0
Total Volume	2	0	2	10	2	12	1	0	1
% App. Total	100	0		83.3	16.7		100	0	
PHF	.250	.000	.250	.833	.500	.750	.250	.000	.250

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- 4+ Axle Trucks

Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
04:00 PM	0	0	0	0	1	1	0	4	4	5
04:15 PM	2	0	2	1	4	5	0	1	1	8
04:30 PM	1	1	2	0	2	2	0	2	2	6
04:45 PM	1	0	1	0	3	3	0	0	0	4
Total	4	1	5	1	10	11	0	7	7	23
05:00 PM	1	0	1	0	0	0	0	0	0	1
05:15 PM	0	0	0	0	0	0	0	3	3	3
05:30 PM	1	0	1	0	1	1	0	2	2	4
05:45 PM	1	0	1	1	2	3	0	0	0	4
Total	3	0	3	1	3	4	0	5	5	12
Grand Total	7	1	8	2	13	15	0	12	12	35
Apprch %	87.5	12.5		13.3	86.7		0	100		
Total %	20	2.9	22.9	5.7	37.1	42.9	0	34.3	34.3	

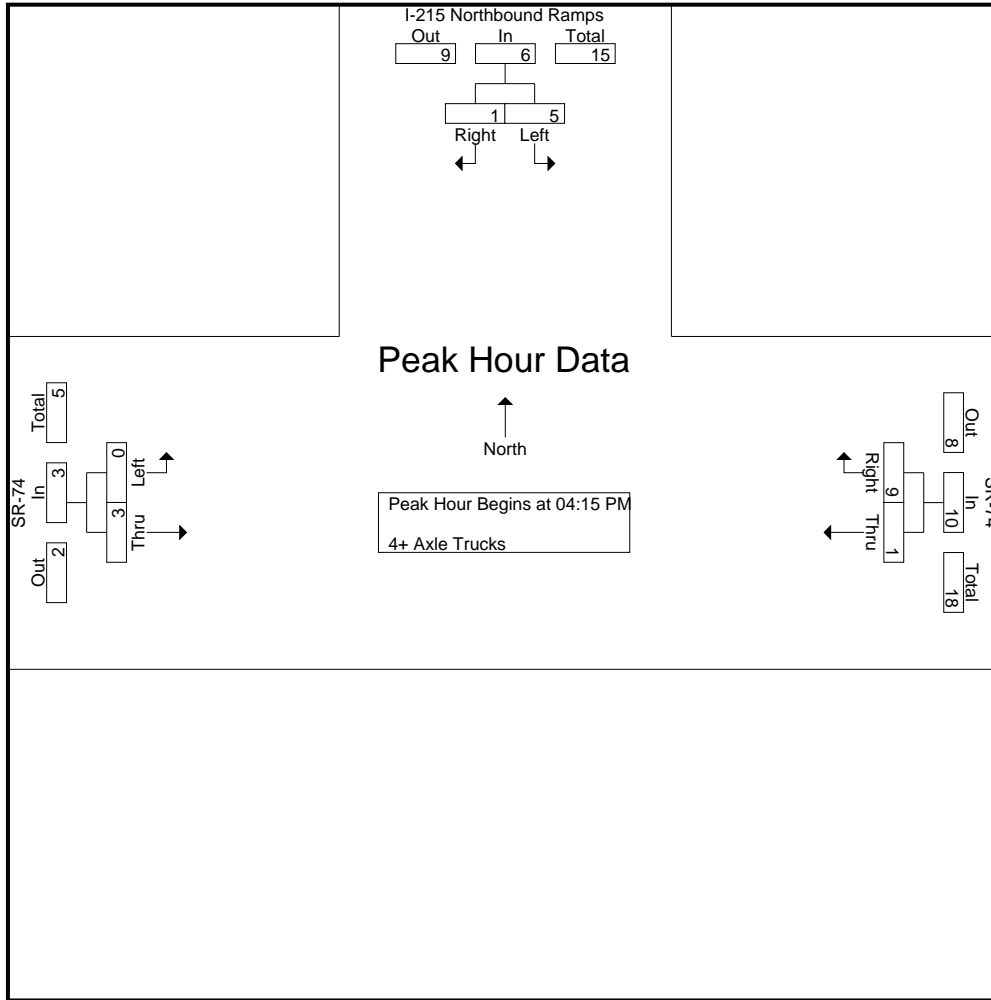
Start Time	I-215 Northbound Ramps Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
04:15 PM	2	0	2	1	4	5	0	1	1	8
04:30 PM	1	1	2	0	2	2	0	2	2	6
04:45 PM	1	0	1	0	3	3	0	0	0	4
05:00 PM	1	0	1	0	0	0	0	0	0	1
Total Volume	5	1	6	1	9	10	0	3	3	19
% App. Total	83.3	16.7		10	90		0	100		
PHF	.625	.250	.750	.250	.563	.500	.000	.375	.375	.594

Peak Hour Analysis From 04:15 PM to 05:00 PM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 04:15 PM

City of Perris
 N/S: I-215 Northbound Ramps
 E/W: SR-74
 Weather: Clear

File Name : 02_PER_215N_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 04:15 PM to 05:00 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	04:15 PM			04:15 PM			04:15 PM		
+0 mins.	2	0	2	1	4	5	0	1	1
+15 mins.	1	1	2	0	2	2	0	2	2
+30 mins.	1	0	1	0	3	3	0	0	0
+45 mins.	1	0	1	0	0	0	0	0	0
Total Volume	5	1	6	1	9	10	0	3	3
% App. Total	83.3	16.7		10	90		0	100	
PHF	.625	.250	.750	.250	.563	.500	.000	.375	.375

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

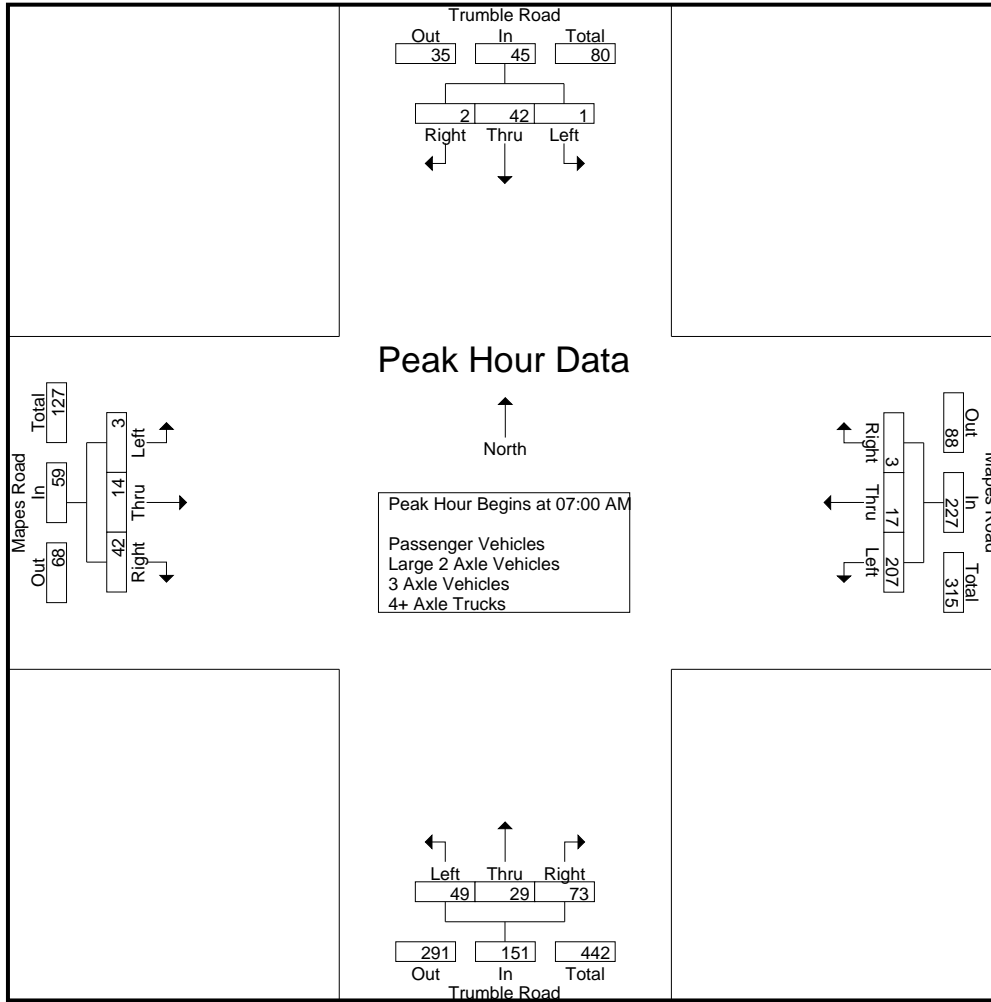
Groups Printed- Passenger Vehicles - Large 2 Axle Vehicles - 3 Axle Vehicles - 4+ Axle Trucks

Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
07:00 AM	0	14	1	15	46	10	1	57	10	6	17	33	0	4	6	10	115
07:15 AM	0	9	1	10	51	5	2	58	23	4	17	44	0	4	13	17	129
07:30 AM	1	11	0	12	54	1	0	55	13	8	17	38	2	4	10	16	121
07:45 AM	0	8	0	8	56	1	0	57	3	11	22	36	1	2	13	16	117
Total	1	42	2	45	207	17	3	227	49	29	73	151	3	14	42	59	482
08:00 AM	0	7	0	7	63	1	0	64	5	6	13	24	0	3	6	9	104
08:15 AM	0	8	0	8	28	1	1	30	4	9	35	48	1	0	10	11	97
08:30 AM	1	4	1	6	44	1	1	46	7	4	30	41	0	1	3	4	97
08:45 AM	1	5	0	6	32	1	2	35	2	4	21	27	0	0	6	6	74
Total	2	24	1	27	167	4	4	175	18	23	99	140	1	4	25	30	372
Grand Total	3	66	3	72	374	21	7	402	67	52	172	291	4	18	67	89	854
Apprch %	4.2	91.7	4.2		93	5.2	1.7		23	17.9	59.1		4.5	20.2	75.3		
Total %	0.4	7.7	0.4	8.4	43.8	2.5	0.8	47.1	7.8	6.1	20.1	34.1	0.5	2.1	7.8	10.4	
Passenger Vehicles	2	61	3	66	364	20	7	391	56	49	161	266	4	10	37	51	774
% Passenger Vehicles	66.7	92.4	100	91.7	97.3	95.2	100	97.3	83.6	94.2	93.6	91.4	100	55.6	55.2	57.3	90.6
Large 2 Axle Vehicles	1	3	0	4	7	1	0	8	10	3	7	20	0	8	28	36	68
% Large 2 Axle Vehicles	33.3	4.5	0	5.6	1.9	4.8	0	2	14.9	5.8	4.1	6.9	0	44.4	41.8	40.4	8
3 Axle Vehicles	0	1	0	1	0	0	0	0	1	0	3	4	0	0	2	2	7
% 3 Axle Vehicles	0	1.5	0	1.4	0	0	0	0	1.5	0	1.7	1.4	0	0	3	2.2	0.8
4+ Axle Trucks	0	1	0	1	3	0	0	3	0	0	1	1	0	0	0	0	5
% 4+ Axle Trucks	0	1.5	0	1.4	0.8	0	0	0.7	0	0	0.6	0.3	0	0	0	0	0.6

Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:00 AM																	
07:00 AM	0	14	1	15	46	10	1	57	10	6	17	33	0	4	6	10	115
07:15 AM	0	9	1	10	51	5	2	58	23	4	17	44	0	4	13	17	129
07:30 AM	1	11	0	12	54	1	0	55	13	8	17	38	2	4	10	16	121
07:45 AM	0	8	0	8	56	1	0	57	3	11	22	36	1	2	13	16	117
Total Volume	1	42	2	45	207	17	3	227	49	29	73	151	3	14	42	59	482
% App. Total	2.2	93.3	4.4		91.2	7.5	1.3		32.5	19.2	48.3		5.1	23.7	71.2		
PHF	.250	.750	.500	.750	.924	.425	.375	.978	.533	.659	.830	.858	.375	.875	.808	.868	.934

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes AM
 Site Code : 00322221
 Start Date : 3/10/2022
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Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:00 AM				07:15 AM				07:00 AM				07:00 AM			
+0 mins.	0	14	1	15	51	5	2	58	10	6	17	33	0	4	6	10
+15 mins.	0	9	1	10	54	1	0	55	23	4	17	44	0	4	13	17
+30 mins.	1	11	0	12	56	1	0	57	13	8	17	38	2	4	10	16
+45 mins.	0	8	0	8	63	1	0	64	3	11	22	36	1	2	13	16
Total Volume	1	42	2	45	224	8	2	234	49	29	73	151	3	14	42	59
% App. Total	2.2	93.3	4.4		95.7	3.4	0.9		32.5	19.2	48.3		5.1	23.7	71.2	
PHF	.250	.750	.500	.750	.889	.400	.250	.914	.533	.659	.830	.858	.375	.875	.808	.868

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- Passenger Vehicles

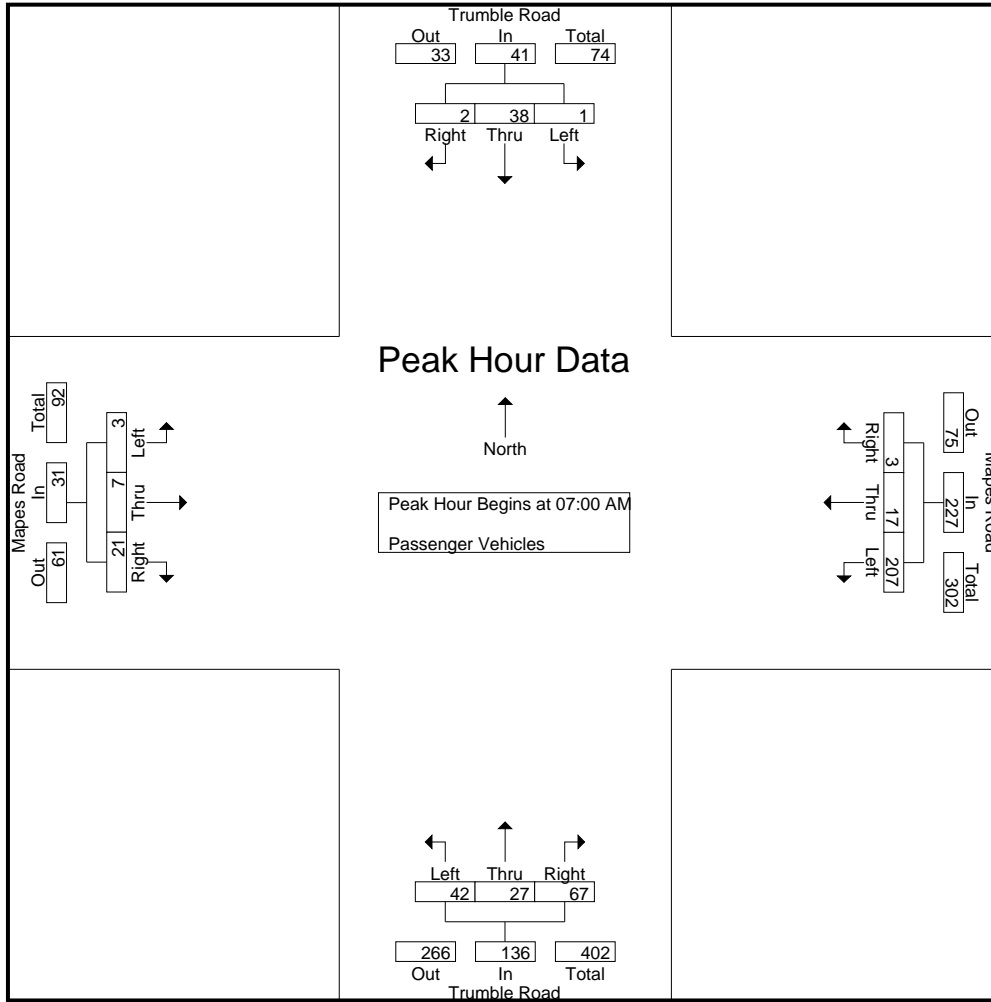
Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
07:00 AM	0	13	1	14	46	10	1	57	6	4	16	26	0	3	3	6	103
07:15 AM	0	7	1	8	51	5	2	58	21	4	16	41	0	2	4	6	113
07:30 AM	1	10	0	11	54	1	0	55	12	8	15	35	2	1	6	9	110
07:45 AM	0	8	0	8	56	1	0	57	3	11	20	34	1	1	8	10	109
Total	1	38	2	41	207	17	3	227	42	27	67	136	3	7	21	31	435
08:00 AM	0	7	0	7	58	1	0	59	3	6	11	20	0	2	5	7	93
08:15 AM	0	7	0	7	27	1	1	29	4	8	34	46	1	0	4	5	87
08:30 AM	0	4	1	5	41	0	1	42	5	4	28	37	0	1	1	2	86
08:45 AM	1	5	0	6	31	1	2	34	2	4	21	27	0	0	6	6	73
Total	1	23	1	25	157	3	4	164	14	22	94	130	1	3	16	20	339
Grand Total	2	61	3	66	364	20	7	391	56	49	161	266	4	10	37	51	774
Apprch %	3	92.4	4.5		93.1	5.1	1.8		21.1	18.4	60.5		7.8	19.6	72.5		
Total %	0.3	7.9	0.4	8.5	47	2.6	0.9	50.5	7.2	6.3	20.8	34.4	0.5	1.3	4.8	6.6	

Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
07:00 AM	0	13	1	14	46	10	1	57	6	4	16	26	0	3	3	6	103
07:15 AM	0	7	1	8	51	5	2	58	21	4	16	41	0	2	4	6	113
07:30 AM	1	10	0	11	54	1	0	55	12	8	15	35	2	1	6	9	110
07:45 AM	0	8	0	8	56	1	0	57	3	11	20	34	1	1	8	10	109
Total Volume	1	38	2	41	207	17	3	227	42	27	67	136	3	7	21	31	435
% App. Total	2.4	92.7	4.9		91.2	7.5	1.3		30.9	19.9	49.3		9.7	22.6	67.7		
PHF	.250	.731	.500	.732	.924	.425	.375	.978	.500	.614	.838	.829	.375	.583	.656	.775	.962

Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1
 Peak Hour for Entire Intersection Begins at 07:00 AM

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes AM
 Site Code : 00322221
 Start Date : 3/10/2022
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Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:00 AM				07:00 AM				07:00 AM				07:00 AM			
+0 mins.	0	13	1	14	46	10	1	57	6	4	16	26	0	3	3	6
+15 mins.	0	7	1	8	51	5	2	58	21	4	16	41	0	2	4	6
+30 mins.	1	10	0	11	54	1	0	55	12	8	15	35	2	1	6	9
+45 mins.	0	8	0	8	56	1	0	57	3	11	20	34	1	1	8	10
Total Volume	1	38	2	41	207	17	3	227	42	27	67	136	3	7	21	31
% App. Total	2.4	92.7	4.9		91.2	7.5	1.3		30.9	19.9	49.3		9.7	22.6	67.7	
PHF	.250	.731	.500	.732	.924	.425	.375	.978	.500	.614	.838	.829	.375	.583	.656	.775

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- Large 2 Axle Vehicles

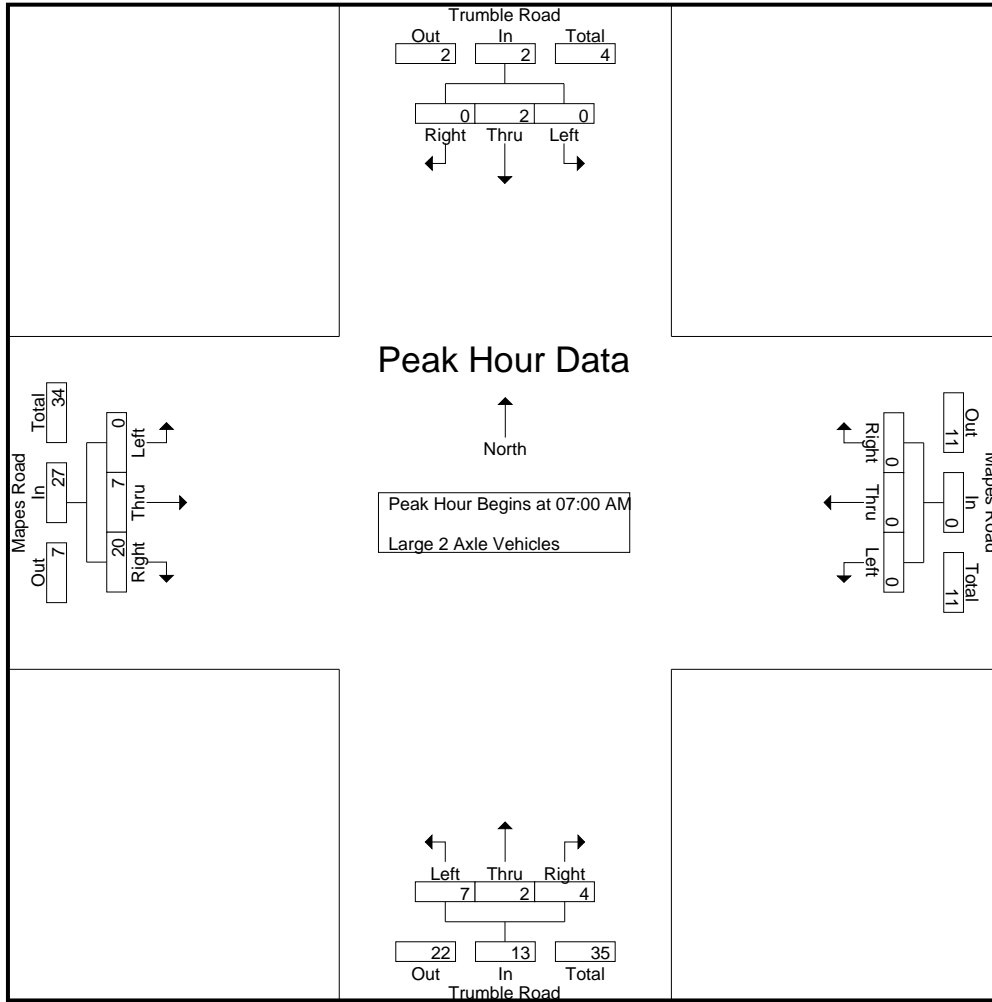
Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
07:00 AM	0	0	0	0	0	0	0	0	4	2	0	6	0	1	3	4	10
07:15 AM	0	1	0	1	0	0	0	0	2	0	1	3	0	2	9	11	15
07:30 AM	0	1	0	1	0	0	0	0	1	0	1	2	0	3	3	6	9
07:45 AM	0	0	0	0	0	0	0	0	0	0	2	2	0	1	5	6	8
Total	0	2	0	2	0	0	0	0	7	2	4	13	0	7	20	27	42
08:00 AM	0	0	0	0	2	0	0	2	1	0	1	2	0	1	1	2	6
08:15 AM	0	1	0	1	1	0	0	1	0	1	1	2	0	0	5	5	9
08:30 AM	1	0	0	1	3	1	0	4	2	0	1	3	0	0	2	2	10
08:45 AM	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	1
Total	1	1	0	2	7	1	0	8	3	1	3	7	0	1	8	9	26
Grand Total	1	3	0	4	7	1	0	8	10	3	7	20	0	8	28	36	68
Apprch %	25	75	0		87.5	12.5	0		50	15	35		0	22.2	77.8		
Total %	1.5	4.4	0	5.9	10.3	1.5	0	11.8	14.7	4.4	10.3	29.4	0	11.8	41.2	52.9	

Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
07:00 AM	0	0	0	0	0	0	0	0	4	2	0	6	0	1	3	4	10
07:15 AM	0	1	0	1	0	0	0	0	2	0	1	3	0	2	9	11	15
07:30 AM	0	1	0	1	0	0	0	0	1	0	1	2	0	3	3	6	9
07:45 AM	0	0	0	0	0	0	0	0	0	0	2	2	0	1	5	6	8
Total Volume	0	2	0	2	0	0	0	0	7	2	4	13	0	7	20	27	42
% App. Total	0	100	0		0	0	0		53.8	15.4	30.8		0	25.9	74.1		
PHF	.000	.500	.000	.500	.000	.000	.000	.000	.438	.250	.500	.542	.000	.583	.556	.614	.700

Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1
 Peak Hour for Entire Intersection Begins at 07:00 AM

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:00 AM				07:00 AM				07:00 AM				07:00 AM			
+0 mins.	0	0	0	0	0	0	0	0	4	2	0	6	0	1	3	4
+15 mins.	0	1	0	1	0	0	0	0	2	0	1	3	0	2	9	11
+30 mins.	0	1	0	1	0	0	0	0	1	0	1	2	0	3	3	6
+45 mins.	0	0	0	0	0	0	0	0	0	0	2	2	0	1	5	6
Total Volume	0	2	0	2	0	0	0	0	7	2	4	13	0	7	20	27
% App. Total	0	100	0	0	0	0	0	0	53.8	15.4	30.8		0	25.9	74.1	
PHF	.000	.500	.000	.500	.000	.000	.000	.000	.438	.250	.500	.542	.000	.583	.556	.614

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- 3 Axle Vehicles

Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total	
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total		
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	1
07:15 AM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	1	1	2
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	1	0	1	0	0	0	0	0	0	0	2	2	0	0	1	1	4
08:00 AM	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	1
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	1
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	1	0	1	2	0	0	1	1	3
Grand Total	0	1	0	1	0	0	0	0	1	0	3	4	0	0	2	2	7	
Apprch %	0	100	0		0	0	0		25	0	75		0	0	100			
Total %	0	14.3	0	14.3	0	0	0	0	14.3	0	42.9	57.1	0	0	28.6	28.6		

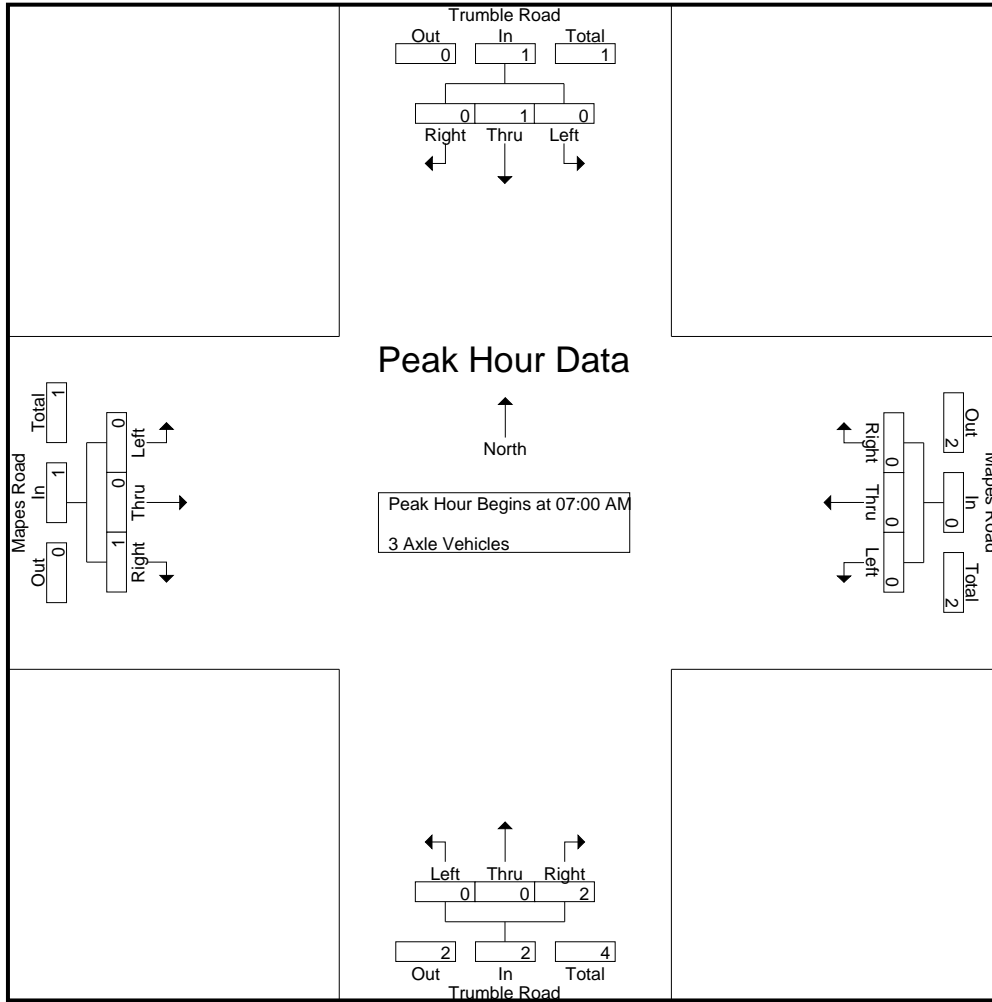
Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total	
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total		
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	1
07:15 AM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	1	1	2
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	1	0	1	0	0	0	0	0	0	0	2	2	0	0	1	1	4
% App. Total	0	100	0		0	0	0		0	0	100		0	0	100			
PHF	.000	.250	.000	.250	.000	.000	.000	.000	.000	.000	.500	.500	.000	.000	.250	.250	.500	

Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 07:00 AM

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:00 AM				07:00 AM				07:00 AM				07:00 AM			
+0 mins.	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0
+15 mins.	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0	0	1	1	0	0	1	1
+45 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	1	0	1	0	0	0	0	0	0	2	2	0	0	1	1
% App. Total	0	100	0	0	0	0	0	0	0	0	100	0	0	0	100	0
PHF	.000	.250	.000	.250	.000	.000	.000	.000	.000	.000	.500	.500	.000	.000	.250	.250

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- 4+ Axle Trucks

Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total	
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total		
07:00 AM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1
08:00 AM	0	0	0	0	3	0	0	3	0	0	1	1	0	0	0	0	0	4
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	3	0	0	3	0	0	1	1	0	0	0	0	0	4
Grand Total	0	1	0	1	3	0	0	3	0	0	1	1	0	0	0	0	0	5
Apprch %	0	100	0		100	0	0		0	0	100		0	0	0			
Total %	0	20	0	20	60	0	0	60	0	0	20	20	0	0	0	0		

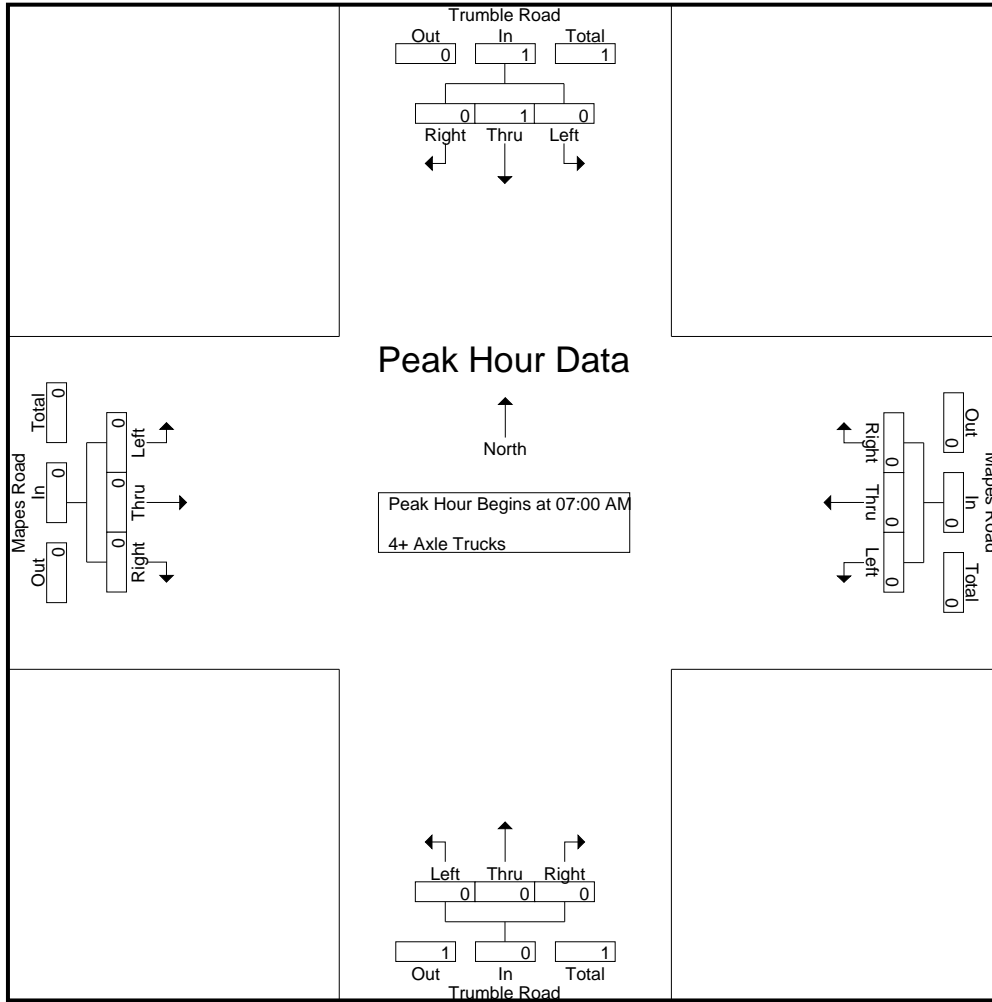
Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total	
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total		
07:00 AM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1
% App. Total	0	100	0		0	0	0		0	0	0		0	0	0			
PHF	.000	.250	.000	.250	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.250

Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 07:00 AM

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 07:00 AM to 07:45 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:00 AM				07:00 AM				07:00 AM				07:00 AM			
+0 mins.	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+45 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0
% App. Total	0	100	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PHF	.000	.250	.000	.250	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

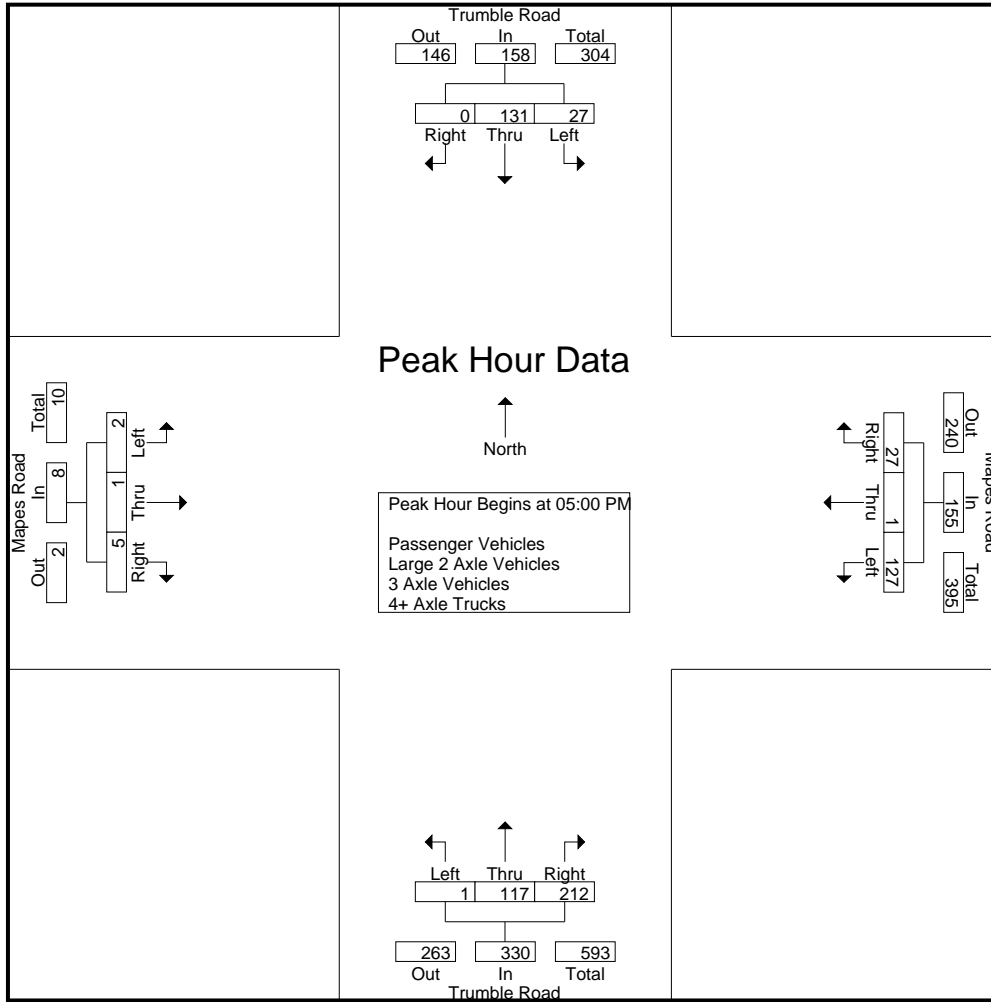
Groups Printed- Passenger Vehicles - Large 2 Axle Vehicles - 3 Axle Vehicles - 4+ Axle Trucks

Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
04:00 PM	15	38	1	54	38	0	0	38	1	11	40	52	0	1	1	2	146
04:15 PM	4	17	0	21	47	2	2	51	6	11	48	65	0	0	6	6	143
04:30 PM	14	51	0	65	37	0	0	37	5	16	37	58	0	3	4	7	167
04:45 PM	3	14	0	17	39	0	0	39	0	15	48	63	0	1	3	4	123
Total	36	120	1	157	161	2	2	165	12	53	173	238	0	5	14	19	579
05:00 PM	20	84	0	104	30	0	4	34	0	19	52	71	0	1	1	2	211
05:15 PM	3	18	0	21	35	1	5	41	1	21	52	74	1	0	0	1	137
05:30 PM	4	22	0	26	23	0	11	34	0	36	51	87	1	0	3	4	151
05:45 PM	0	7	0	7	39	0	7	46	0	41	57	98	0	0	1	1	152
Total	27	131	0	158	127	1	27	155	1	117	212	330	2	1	5	8	651
Grand Total	63	251	1	315	288	3	29	320	13	170	385	568	2	6	19	27	1230
Apprch %	20	79.7	0.3		90	0.9	9.1		2.3	29.9	67.8		7.4	22.2	70.4		
Total %	5.1	20.4	0.1	25.6	23.4	0.2	2.4	26	1.1	13.8	31.3	46.2	0.2	0.5	1.5	2.2	
Passenger Vehicles	63	250	1	314	273	3	28	304	8	169	367	544	2	6	15	23	1185
% Passenger Vehicles	100	99.6	100	99.7	94.8	100	96.6	95	61.5	99.4	95.3	95.8	100	100	78.9	85.2	96.3
Large 2 Axle Vehicles	0	1	0	1	12	0	1	13	5	1	12	18	0	0	3	3	35
% Large 2 Axle Vehicles	0	0.4	0	0.3	4.2	0	3.4	4.1	38.5	0.6	3.1	3.2	0	0	15.8	11.1	2.8
3 Axle Vehicles	0	0	0	0	3	0	0	3	0	0	3	3	0	0	1	1	7
% 3 Axle Vehicles	0	0	0	0	1	0	0	0.9	0	0	0.8	0.5	0	0	5.3	3.7	0.6
4+ Axle Trucks	0	0	0	0	0	0	0	0	0	0	3	3	0	0	0	0	3
% 4+ Axle Trucks	0	0	0	0	0	0	0	0	0	0	0.8	0.5	0	0	0	0	0.2

Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 05:00 PM																	
05:00 PM	20	84	0	104	30	0	4	34	0	19	52	71	0	1	1	2	211
05:15 PM	3	18	0	21	35	1	5	41	1	21	52	74	1	0	0	1	137
05:30 PM	4	22	0	26	23	0	11	34	0	36	51	87	1	0	3	4	151
05:45 PM	0	7	0	7	39	0	7	46	0	41	57	98	0	0	1	1	152
Total Volume	27	131	0	158	127	1	27	155	1	117	212	330	2	1	5	8	651
% App. Total	17.1	82.9	0		81.9	0.6	17.4		0.3	35.5	64.2		25	12.5	62.5		
PHF	.338	.390	.000	.380	.814	.250	.614	.842	.250	.713	.930	.842	.500	.250	.417	.500	.771

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	04:15 PM				04:00 PM				05:00 PM				04:00 PM			
+0 mins.	4	17	0	21	38	0	0	38	0	19	52	71	0	1	1	2
+15 mins.	14	51	0	65	47	2	2	51	1	21	52	74	0	0	0	6
+30 mins.	3	14	0	17	37	0	0	37	0	36	51	87	0	3	4	7
+45 mins.	20	84	0	104	39	0	0	39	0	41	57	98	0	1	3	4
Total Volume	41	166	0	207	161	2	2	165	1	117	212	330	0	5	14	19
% App. Total	19.8	80.2	0		97.6	1.2	1.2		0.3	35.5	64.2		0	26.3	73.7	
PHF	.513	.494	.000	.498	.856	.250	.250	.809	.250	.713	.930	.842	.000	.417	.583	.679

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

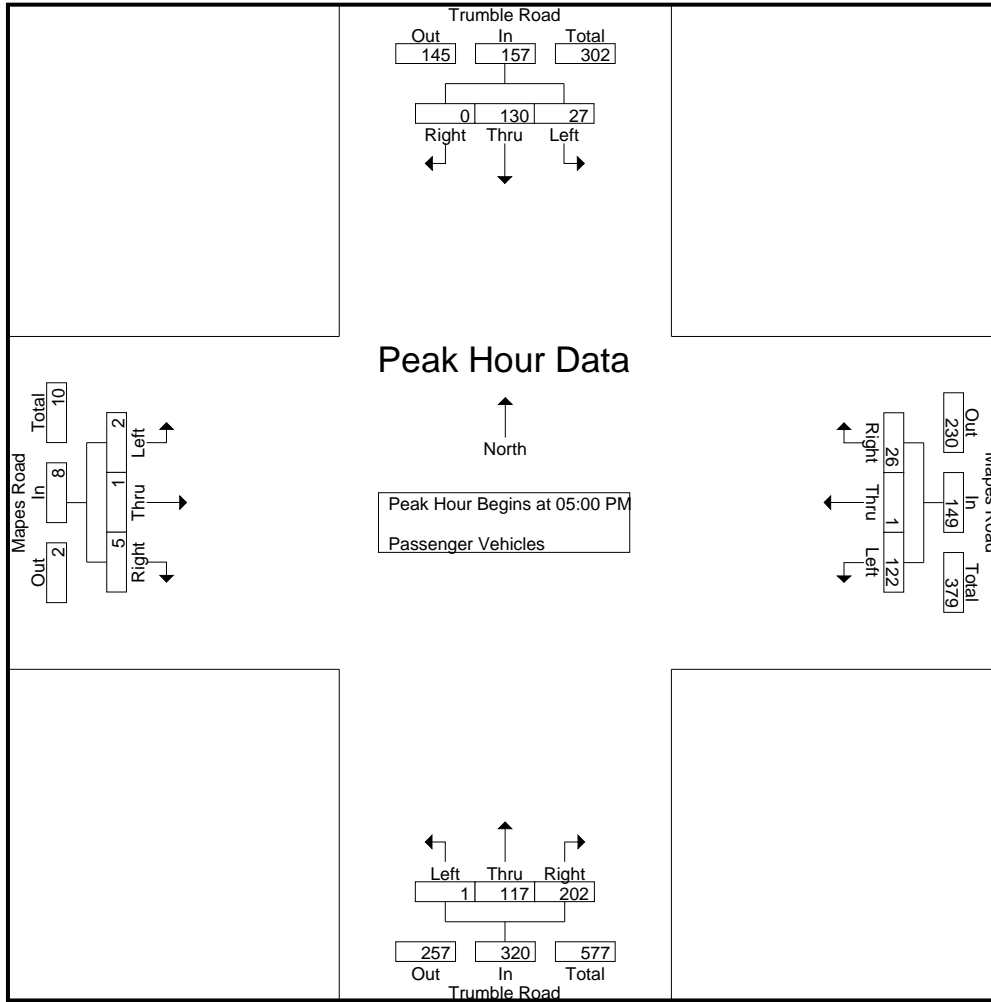
Groups Printed- Passenger Vehicles

Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
04:00 PM	15	38	1	54	31	0	0	31	0	10	38	48	0	1	1	2	135
04:15 PM	4	17	0	21	46	2	2	50	4	11	48	63	0	0	4	4	138
04:30 PM	14	51	0	65	36	0	0	36	3	16	35	54	0	3	4	7	162
04:45 PM	3	14	0	17	38	0	0	38	0	15	44	59	0	1	1	2	116
Total	36	120	1	157	151	2	2	155	7	52	165	224	0	5	10	15	551
05:00 PM	20	83	0	103	29	0	4	33	0	19	50	69	0	1	1	2	207
05:15 PM	3	18	0	21	33	1	5	39	1	21	50	72	1	0	0	1	133
05:30 PM	4	22	0	26	23	0	10	33	0	36	48	84	1	0	3	4	147
05:45 PM	0	7	0	7	37	0	7	44	0	41	54	95	0	0	1	1	147
Total	27	130	0	157	122	1	26	149	1	117	202	320	2	1	5	8	634
Grand Total	63	250	1	314	273	3	28	304	8	169	367	544	2	6	15	23	1185
Apprch %	20.1	79.6	0.3		89.8	1	9.2		1.5	31.1	67.5		8.7	26.1	65.2		
Total %	5.3	21.1	0.1	26.5	23	0.3	2.4	25.7	0.7	14.3	31	45.9	0.2	0.5	1.3	1.9	

Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
Peak Hour Analysis From 05:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 05:00 PM																	
05:00 PM	20	83	0	103	29	0	4	33	0	19	50	69	0	1	1	2	207
05:15 PM	3	18	0	21	33	1	5	39	1	21	50	72	1	0	0	1	133
05:30 PM	4	22	0	26	23	0	10	33	0	36	48	84	1	0	3	4	147
05:45 PM	0	7	0	7	37	0	7	44	0	41	54	95	0	0	1	1	147
Total Volume	27	130	0	157	122	1	26	149	1	117	202	320	2	1	5	8	634
% App. Total	17.2	82.8	0		81.9	0.7	17.4		0.3	36.6	63.1		25	12.5	62.5		
PHF	.338	.392	.000	.381	.824	.250	.650	.847	.250	.713	.935	.842	.500	.250	.417	.500	.766

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 05:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	05:00 PM				05:00 PM				05:00 PM				05:00 PM			
+0 mins.	20	83	0	103	29	0	4	33	0	19	50	69	0	1	1	2
+15 mins.	3	18	0	21	33	1	5	39	1	21	50	72	1	0	0	1
+30 mins.	4	22	0	26	23	0	10	33	0	36	48	84	1	0	3	4
+45 mins.	0	7	0	7	37	0	7	44	0	41	54	95	0	0	1	1
Total Volume	27	130	0	157	122	1	26	149	1	117	202	320	2	1	5	8
% App. Total	17.2	82.8	0		81.9	0.7	17.4		0.3	36.6	63.1		25	12.5	62.5	
PHF	.338	.392	.000	.381	.824	.250	.650	.847	.250	.713	.935	.842	.500	.250	.417	.500

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- Large 2 Axle Vehicles

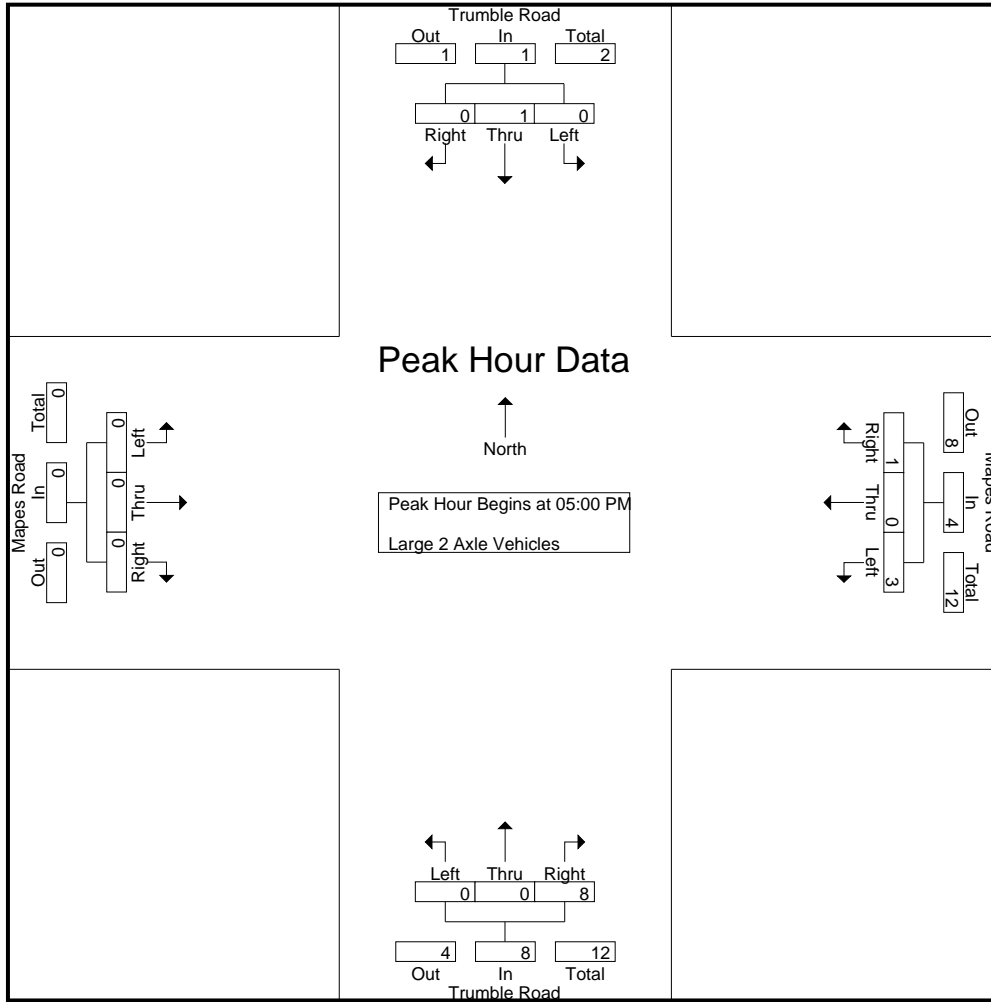
Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
04:00 PM	0	0	0	0	7	0	0	7	1	1	1	3	0	0	0	0	10
04:15 PM	0	0	0	0	0	0	0	0	2	0	0	2	0	0	1	1	3
04:30 PM	0	0	0	0	1	0	0	1	2	0	1	3	0	0	0	0	4
04:45 PM	0	0	0	0	1	0	0	1	0	0	2	2	0	0	2	2	5
Total	0	0	0	0	9	0	0	9	5	1	4	10	0	0	3	3	22
05:00 PM	0	1	0	1	1	0	0	1	0	0	2	2	0	0	0	0	4
05:15 PM	0	0	0	0	0	0	0	0	0	0	2	2	0	0	0	0	2
05:30 PM	0	0	0	0	0	0	1	1	0	0	2	2	0	0	0	0	3
05:45 PM	0	0	0	0	2	0	0	2	0	0	2	2	0	0	0	0	4
Total	0	1	0	1	3	0	1	4	0	0	8	8	0	0	0	0	13
Grand Total	0	1	0	1	12	0	1	13	5	1	12	18	0	0	3	3	35
Apprch %	0	100	0		92.3	0	7.7		27.8	5.6	66.7		0	0	100		
Total %	0	2.9	0	2.9	34.3	0	2.9	37.1	14.3	2.9	34.3	51.4	0	0	8.6	8.6	

Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
05:00 PM	0	1	0	1	1	0	0	1	0	0	2	2	0	0	0	0	4
05:15 PM	0	0	0	0	0	0	0	0	0	0	2	2	0	0	0	0	2
05:30 PM	0	0	0	0	0	0	1	1	0	0	2	2	0	0	0	0	3
05:45 PM	0	0	0	0	2	0	0	2	0	0	2	2	0	0	0	0	4
Total Volume	0	1	0	1	3	0	1	4	0	0	8	8	0	0	0	0	13
% App. Total	0	100	0		75	0	25		0	0	100		0	0	0		
PHF	.000	.250	.000	.250	.375	.000	.250	.500	.000	.000	1.00	1.00	.000	.000	.000	.000	.813

Peak Hour Analysis From 05:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Entire Intersection Begins at 05:00 PM

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 05:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	05:00 PM				05:00 PM				05:00 PM				05:00 PM			
+0 mins.	0	1	0	1	1	0	0	1	0	0	2	2	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0	0	2	2	0	0	0	0
+30 mins.	0	0	0	0	0	0	1	1	0	0	2	2	0	0	0	0
+45 mins.	0	0	0	0	2	0	0	2	0	0	2	2	0	0	0	0
Total Volume	0	1	0	1	3	0	1	4	0	0	8	8	0	0	0	0
% App. Total	0	100	0	0	75	0	25	0	0	0	100	0	0	0	0	0
PHF	.000	.250	.000	.250	.375	.000	.250	.500	.000	.000	1.000	1.000	.000	.000	.000	.000

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- 3 Axle Vehicles

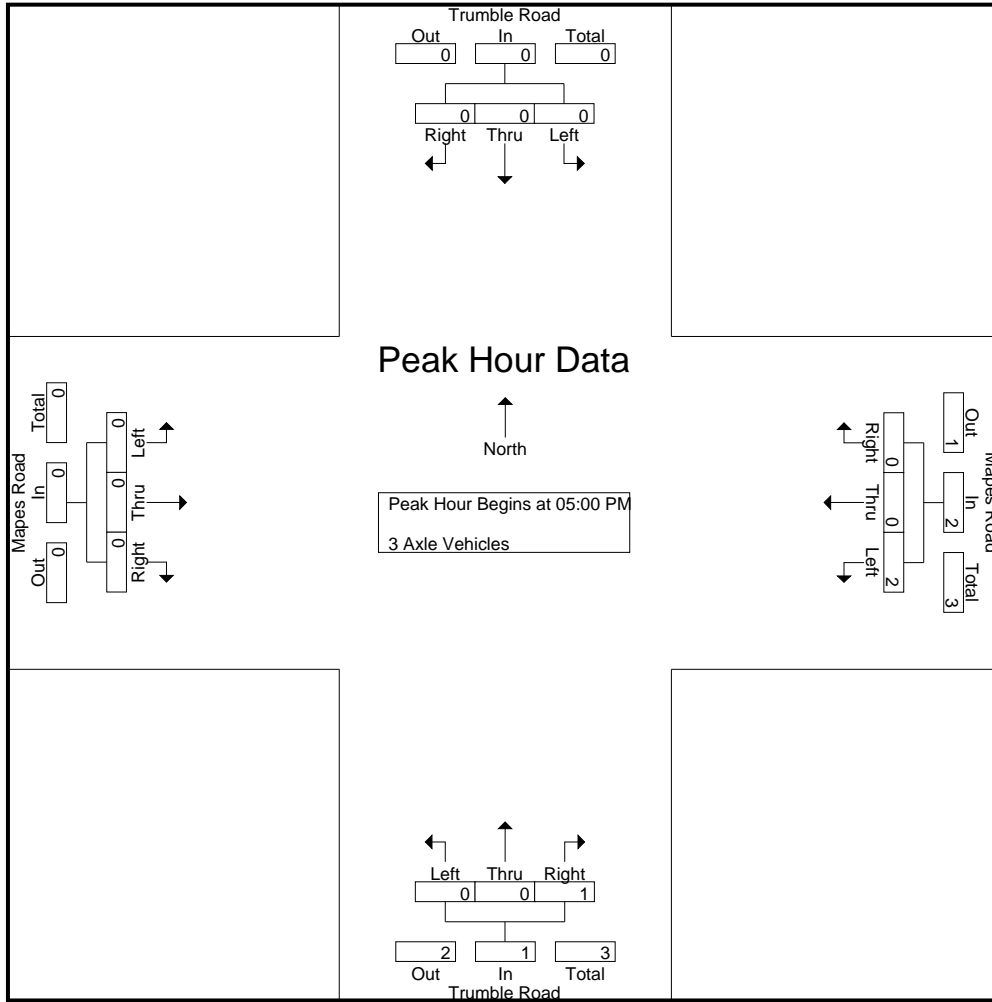
Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 PM	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	1	1
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:45 PM	0	0	0	0	0	0	0	0	0	0	2	2	0	0	0	0	2
Total	0	0	0	0	1	0	0	1	0	0	2	2	0	0	1	1	4
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	0	0	0	2	0	0	2	0	0	0	0	0	0	0	0	2
05:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:45 PM	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	1
Total	0	0	0	0	2	0	0	2	0	0	1	1	0	0	0	0	3
Grand Total	0	0	0	0	3	0	0	3	0	0	3	3	0	0	1	1	7
Apprch %	0	0	0		100	0	0		0	0	100		0	0	100		
Total %	0	0	0	0	42.9	0	0	42.9	0	0	42.9	42.9	0	0	14.3	14.3	

Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	0	0	0	2	0	0	2	0	0	0	0	0	0	0	0	2
05:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:45 PM	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	1
Total Volume	0	0	0	0	2	0	0	2	0	0	1	1	0	0	0	0	3
% App. Total	0	0	0		100	0	0		0	0	100		0	0	0		
PHF	.000	.000	.000	.000	.250	.000	.000	.250	.000	.000	.250	.250	.000	.000	.000	.000	.375

Peak Hour Analysis From 05:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Entire Intersection Begins at 05:00 PM

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 05:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	05:00 PM				05:00 PM				05:00 PM				05:00 PM			
+0 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	2	0	0	2	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+45 mins.	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0
Total Volume	0	0	0	0	2	0	0	2	0	0	1	1	0	0	0	0
% App. Total	0	0	0	0	100	0	0	0	0	0	100	0	0	0	0	0
PHF	.000	.000	.000	.000	.250	.000	.000	.250	.000	.000	.250	.250	.000	.000	.000	.000

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- 4+ Axle Trucks

Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
04:00 PM	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	0
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 PM	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	0
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	2	2	0	0	0	0	2
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:30 PM	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	0
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	1
Grand Total	0	0	0	0	0	0	0	0	0	0	3	3	0	0	0	0	3
Apprch %	0	0	0		0	0	0		0	0	100		0	0	0		
Total %	0	0	0		0	0	0		0	0	100	100	0	0	0		

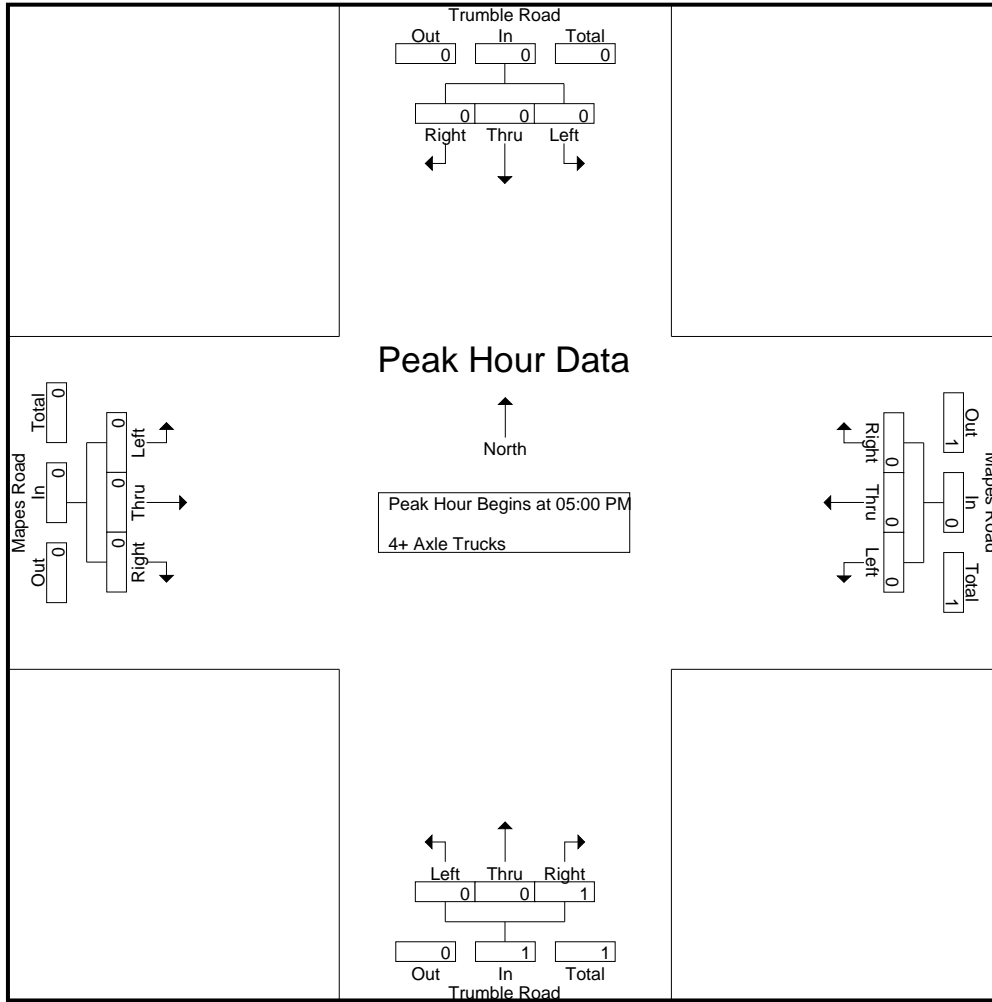
Start Time	Trumble Road Southbound				Mapes Road Westbound				Trumble Road Northbound				Mapes Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:30 PM	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	0
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	1
% App. Total	0	0	0		0	0	0		0	0	100		0	0	0		
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.250	.250	.000	.000	.000	.000	.250

Peak Hour Analysis From 05:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 05:00 PM

City of Perris
 N/S: Trumble Road
 E/W: Mapes Road
 Weather: Clear

File Name : 03_PER_Trumble_Mapes PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 05:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	05:00 PM				05:00 PM				05:00 PM				05:00 PM			
+0 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0
+45 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0
% App. Total	0	0	0	0	0	0	0	0	0	0	100		0	0	0	0
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.250	.250	.000	.000	.000	.000

City of Perris
 N/S: Trumble Road
 E/W: Exceed Road
 Weather: Clear

File Name : 04_PER_Trumble_Exceed AM
 Site Code : 00322221
 Start Date : 3/10/2022
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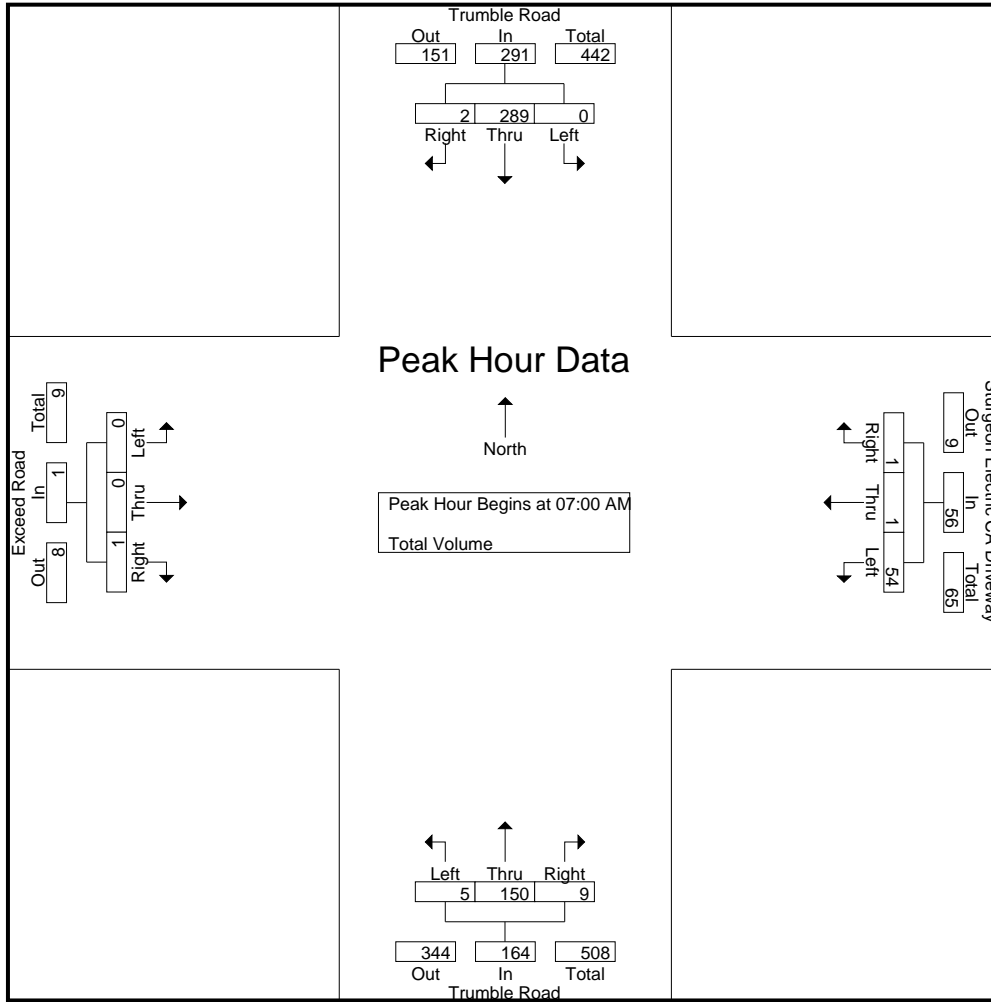
Groups Printed- Total Volume

Start Time	Trumble Road Southbound				Sturgeon Electric CA Driveway Westbound				Trumble Road Northbound				Exceed Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
07:00 AM	0	66	0	66	19	0	1	20	1	33	1	35	0	0	0	0	121
07:15 AM	0	72	0	72	18	0	0	18	0	44	6	50	0	0	0	0	140
07:30 AM	0	76	0	76	11	1	0	12	1	37	1	39	0	0	1	1	128
07:45 AM	0	75	2	77	6	0	0	6	3	36	1	40	0	0	0	0	123
Total	0	289	2	291	54	1	1	56	5	150	9	164	0	0	1	1	512
08:00 AM	0	76	0	76	2	0	0	2	1	30	0	31	0	0	0	0	109
08:15 AM	0	45	0	45	2	0	0	2	2	43	0	45	0	0	0	0	92
08:30 AM	0	52	0	52	1	0	0	1	0	39	0	39	0	0	1	1	93
08:45 AM	1	42	0	43	0	0	0	0	0	29	2	31	0	0	0	0	74
Total	1	215	0	216	5	0	0	5	3	141	2	146	0	0	1	1	368
Grand Total	1	504	2	507	59	1	1	61	8	291	11	310	0	0	2	2	880
Apprch %	0.2	99.4	0.4		96.7	1.6	1.6		2.6	93.9	3.5		0	0	100		
Total %	0.1	57.3	0.2	57.6	6.7	0.1	0.1	6.9	0.9	33.1	1.2	35.2	0	0	0.2	0.2	

Start Time	Trumble Road Southbound				Sturgeon Electric CA Driveway Westbound				Trumble Road Northbound				Exceed Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:00 AM																	
07:00 AM	0	66	0	66	19	0	1	20	1	33	1	35	0	0	0	0	121
07:15 AM	0	72	0	72	18	0	0	18	0	44	6	50	0	0	0	0	140
07:30 AM	0	76	0	76	11	1	0	12	1	37	1	39	0	0	1	1	128
07:45 AM	0	75	2	77	6	0	0	6	3	36	1	40	0	0	0	0	123
Total Volume	0	289	2	291	54	1	1	56	5	150	9	164	0	0	1	1	512
% App. Total	0	99.3	0.7		96.4	1.8	1.8		3	91.5	5.5		0	0	100		
PHF	.000	.951	.250	.945	.711	.250	.250	.700	.417	.852	.375	.820	.000	.000	.250	.250	.914

City of Perris
 N/S: Trumble Road
 E/W: Exceed Road
 Weather: Clear

File Name : 04_PER_Trumble_Exceed AM
 Site Code : 00322221
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Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:15 AM				07:00 AM				07:00 AM				07:00 AM			
+0 mins.	0	72	0	72	19	0	1	20	1	33	1	35	0	0	0	0
+15 mins.	0	76	0	76	18	0	0	18	0	44	6	50	0	0	0	0
+30 mins.	0	75	2	77	11	1	0	12	1	37	1	39	0	0	1	1
+45 mins.	0	76	0	76	6	0	0	6	3	36	1	40	0	0	0	0
Total Volume	0	299	2	301	54	1	1	56	5	150	9	164	0	0	1	1
% App. Total	0	99.3	0.7		96.4	1.8	1.8		3	91.5	5.5		0	0	100	
PHF	.000	.984	.250	.977	.711	.250	.250	.700	.417	.852	.375	.820	.000	.000	.250	.250

City of Perris
 N/S: Trumble Road
 E/W: Exceed Road
 Weather: Clear

File Name : 04_PER_Trumble_Exceed PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

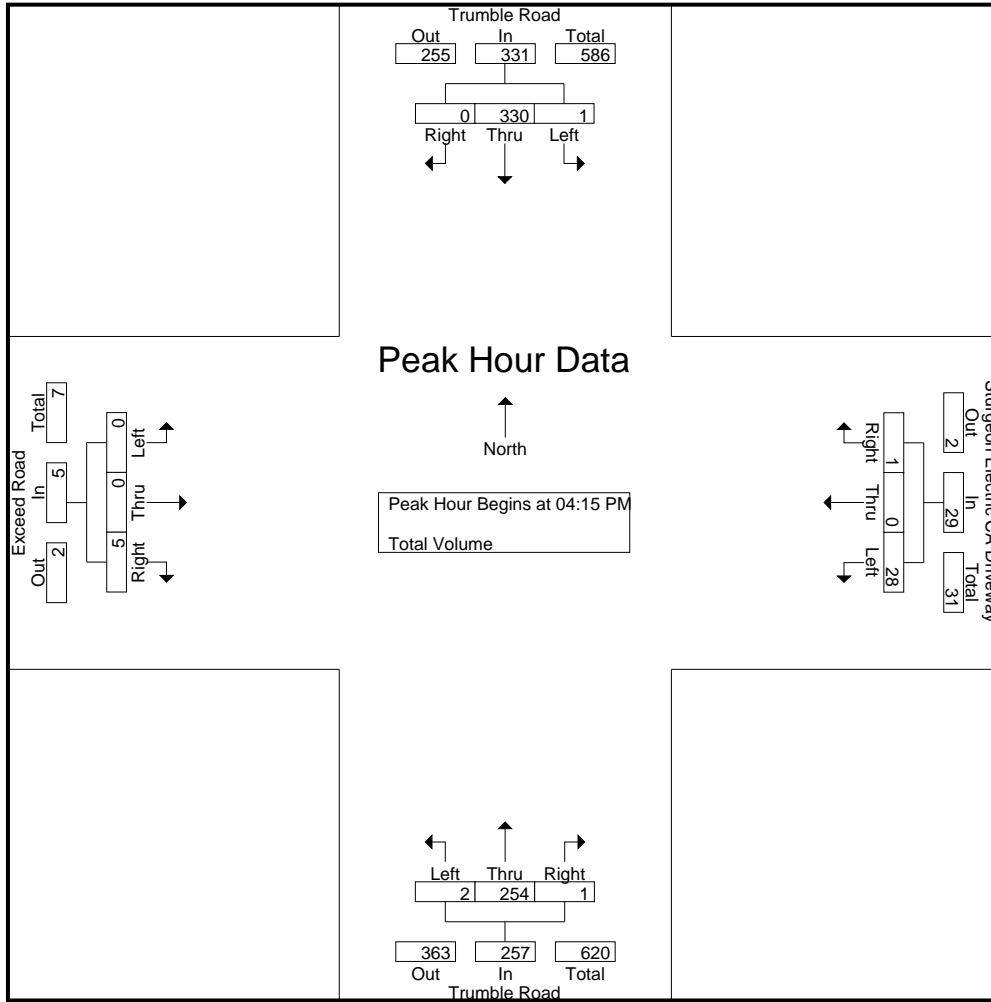
Groups Printed- Total Volume

Start Time	Trumble Road Southbound				Sturgeon Electric CA Driveway Westbound				Trumble Road Northbound				Exceed Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
04:00 PM	0	75	1	76	8	0	0	8	1	53	1	55	1	0	4	5	144
04:15 PM	0	67	0	67	5	0	0	5	0	63	1	64	0	0	1	1	137
04:30 PM	0	93	0	93	4	0	1	5	1	58	0	59	0	0	2	2	159
04:45 PM	1	55	0	56	12	0	0	12	0	61	0	61	0	0	0	0	129
Total	1	290	1	292	29	0	1	30	2	235	2	239	1	0	7	8	569
05:00 PM	0	115	0	115	7	0	0	7	1	72	0	73	0	0	2	2	197
05:15 PM	0	53	0	53	2	0	0	2	0	69	0	69	0	0	1	1	125
05:30 PM	0	48	0	48	2	0	0	2	0	90	0	90	0	0	0	0	140
05:45 PM	0	46	0	46	0	0	0	0	0	96	6	102	0	0	0	0	148
Total	0	262	0	262	11	0	0	11	1	327	6	334	0	0	3	3	610
Grand Total	1	552	1	554	40	0	1	41	3	562	8	573	1	0	10	11	1179
Apprch %	0.2	99.6	0.2		97.6	0	2.4		0.5	98.1	1.4		9.1	0	90.9		
Total %	0.1	46.8	0.1	47	3.4	0	0.1	3.5	0.3	47.7	0.7	48.6	0.1	0	0.8	0.9	

Start Time	Trumble Road Southbound				Sturgeon Electric CA Driveway Westbound				Trumble Road Northbound				Exceed Road Eastbound				Int. Total
	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:15 PM																	
04:15 PM	0	67	0	67	5	0	0	5	0	63	1	64	0	0	1	1	137
04:30 PM	0	93	0	93	4	0	1	5	1	58	0	59	0	0	2	2	159
04:45 PM	1	55	0	56	12	0	0	12	0	61	0	61	0	0	0	0	129
05:00 PM	0	115	0	115	7	0	0	7	1	72	0	73	0	0	2	2	197
Total Volume	1	330	0	331	28	0	1	29	2	254	1	257	0	0	5	5	622
% App. Total	0.3	99.7	0		96.6	0	3.4		0.8	98.8	0.4		0	0	100		
PHF	.250	.717	.000	.720	.583	.000	.250	.604	.500	.882	.250	.880	.000	.000	.625	.625	.789

City of Perris
 N/S: Trumble Road
 E/W: Exceed Road
 Weather: Clear

File Name : 04_PER_Trumble_Exceed PM
 Site Code : 00322221
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Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	04:15 PM				04:00 PM				05:00 PM				04:00 PM			
+0 mins.	0	67	0	67	8	0	0	8	1	72	0	73	1	0	4	5
+15 mins.	0	93	0	93	5	0	0	5	0	69	0	69	0	0	1	1
+30 mins.	1	55	0	56	4	0	1	5	0	90	0	90	0	0	2	2
+45 mins.	0	115	0	115	12	0	0	12	0	96	6	102	0	0	0	0
Total Volume	1	330	0	331	29	0	1	30	1	327	6	334	1	0	7	8
% App. Total	0.3	99.7	0		96.7	0	3.3		0.3	97.9	1.8		12.5	0	87.5	
PHF	.250	.717	.000	.720	.604	.000	.250	.625	.250	.852	.250	.819	.250	.000	.438	.400

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

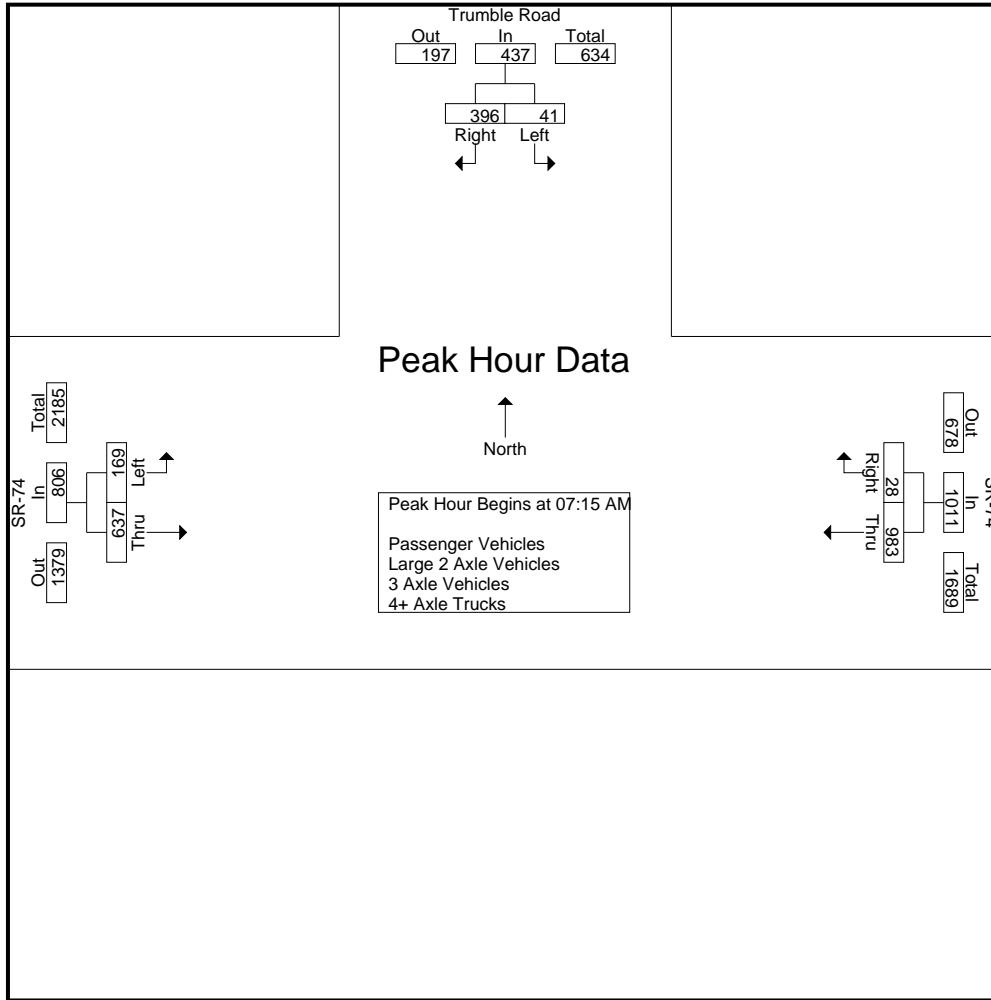
Groups Printed- Passenger Vehicles - Large 2 Axle Vehicles - 3 Axle Vehicles - 4+ Axle Trucks

Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
07:00 AM	6	117	123	211	8	219	39	152	191	533
07:15 AM	13	107	120	236	5	241	52	190	242	603
07:30 AM	5	101	106	231	4	235	38	154	192	533
07:45 AM	16	94	110	267	11	278	37	142	179	567
Total	40	419	459	945	28	973	166	638	804	2236
08:00 AM	7	94	101	249	8	257	42	151	193	551
08:15 AM	5	68	73	187	7	194	45	142	187	454
08:30 AM	6	57	63	193	3	196	35	140	175	434
08:45 AM	1	55	56	180	4	184	36	130	166	406
Total	19	274	293	809	22	831	158	563	721	1845
Grand Total	59	693	752	1754	50	1804	324	1201	1525	4081
Apprch %	7.8	92.2		97.2	2.8		21.2	78.8		
Total %	1.4	17	18.4	43	1.2	44.2	7.9	29.4	37.4	
Passenger Vehicles	51	582	633	1668	45	1713	293	1041	1334	3680
% Passenger Vehicles	86.4	84	84.2	95.1	90	95	90.4	86.7	87.5	90.2
Large 2 Axle Vehicles	8	76	84	46	4	50	23	75	98	232
% Large 2 Axle Vehicles	13.6	11	11.2	2.6	8	2.8	7.1	6.2	6.4	5.7
3 Axle Vehicles	0	8	8	23	1	24	4	35	39	71
% 3 Axle Vehicles	0	1.2	1.1	1.3	2	1.3	1.2	2.9	2.6	1.7
4+ Axle Trucks	0	27	27	17	0	17	4	50	54	98
% 4+ Axle Trucks	0	3.9	3.6	1	0	0.9	1.2	4.2	3.5	2.4

Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 07:15 AM										
07:15 AM	13	107	120	236	5	241	52	190	242	603
07:30 AM	5	101	106	231	4	235	38	154	192	533
07:45 AM	16	94	110	267	11	278	37	142	179	567
08:00 AM	7	94	101	249	8	257	42	151	193	551
Total Volume	41	396	437	983	28	1011	169	637	806	2254
% App. Total	9.4	90.6		97.2	2.8		21	79		
PHF	.641	.925	.910	.920	.636	.909	.813	.838	.833	.934

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 AM
 Site Code : 00322221
 Start Date : 3/10/2022
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Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:00 AM			07:15 AM			07:15 AM		
+0 mins.	6	117	123	236	5	241	52	190	242
+15 mins.	13	107	120	231	4	235	38	154	192
+30 mins.	5	101	106	267	11	278	37	142	179
+45 mins.	16	94	110	249	8	257	42	151	193
Total Volume	40	419	459	983	28	1011	169	637	806
% App. Total	8.7	91.3		97.2	2.8		21	79	
PHF	.625	.895	.933	.920	.636	.909	.813	.838	.833

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- Passenger Vehicles

Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
07:00 AM	3	75	78	195	7	202	33	130	163	443
07:15 AM	12	86	98	228	5	233	50	165	215	546
07:30 AM	5	85	90	220	4	224	35	128	163	477
07:45 AM	14	87	101	259	10	269	35	120	155	525
Total	34	333	367	902	26	928	153	543	696	1991
08:00 AM	7	84	91	240	7	247	36	130	166	504
08:15 AM	4	59	63	175	6	181	38	130	168	412
08:30 AM	5	52	57	186	2	188	31	125	156	401
08:45 AM	1	54	55	165	4	169	35	113	148	372
Total	17	249	266	766	19	785	140	498	638	1689
Grand Total	51	582	633	1668	45	1713	293	1041	1334	3680
Apprch %	8.1	91.9		97.4	2.6		22	78		
Total %	1.4	15.8	17.2	45.3	1.2	46.5	8	28.3	36.2	

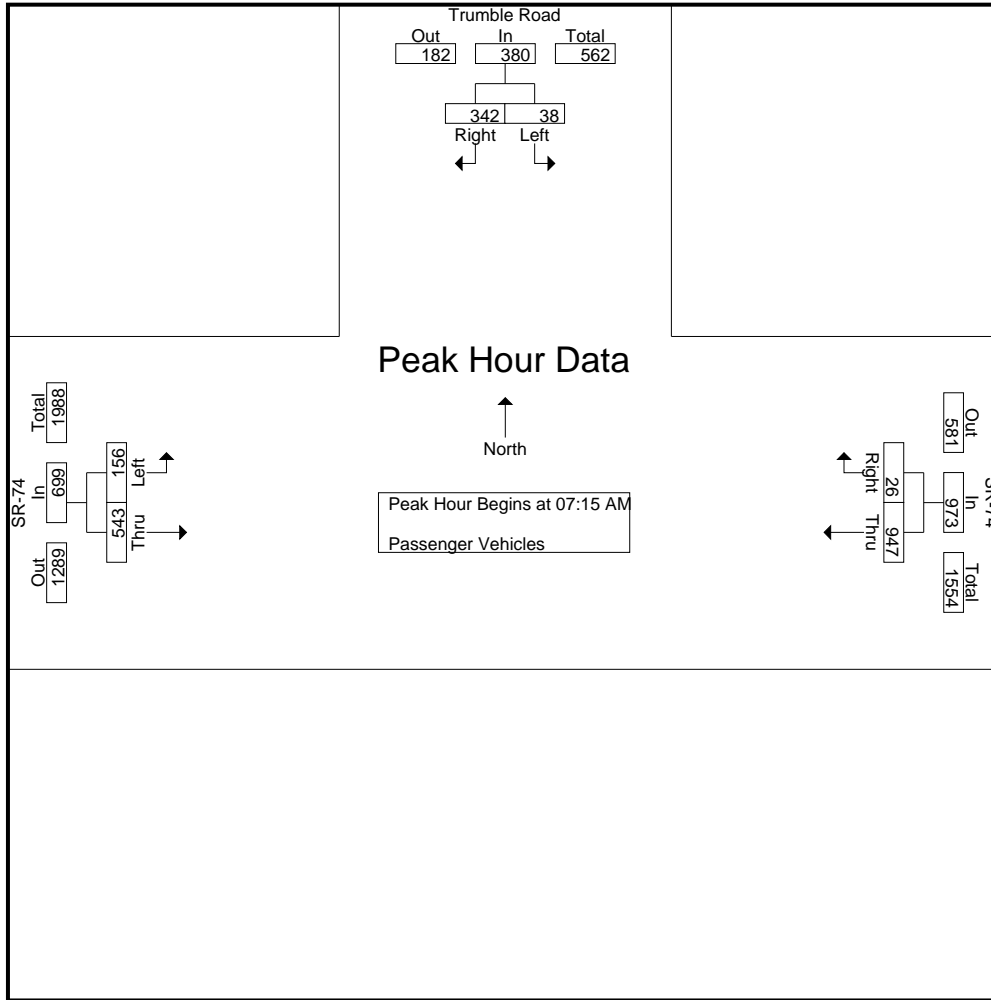
Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
07:15 AM	12	86	98	228	5	233	50	165	215	546
07:30 AM	5	85	90	220	4	224	35	128	163	477
07:45 AM	14	87	101	259	10	269	35	120	155	525
08:00 AM	7	84	91	240	7	247	36	130	166	504
Total Volume	38	342	380	947	26	973	156	543	699	2052
% App. Total	10	90		97.3	2.7		22.3	77.7		
PHF	.679	.983	.941	.914	.650	.904	.780	.823	.813	.940

Peak Hour Analysis From 07:15 AM to 08:00 AM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 07:15 AM

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 AM
 Site Code : 00322221
 Start Date : 3/10/2022
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Peak Hour Analysis From 07:15 AM to 08:00 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:15 AM			07:15 AM			07:15 AM		
+0 mins.	12	86	98	228	5	233	50	165	215
+15 mins.	5	85	90	220	4	224	35	128	163
+30 mins.	14	87	101	259	10	269	35	120	155
+45 mins.	7	84	91	240	7	247	36	130	166
Total Volume	38	342	380	947	26	973	156	543	699
% App. Total	10	90		97.3	2.7		22.3	77.7	
PHF	.679	.983	.941	.914	.650	.904	.780	.823	.813

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 AM
 Site Code : 00322221
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Groups Printed- Large 2 Axle Vehicles

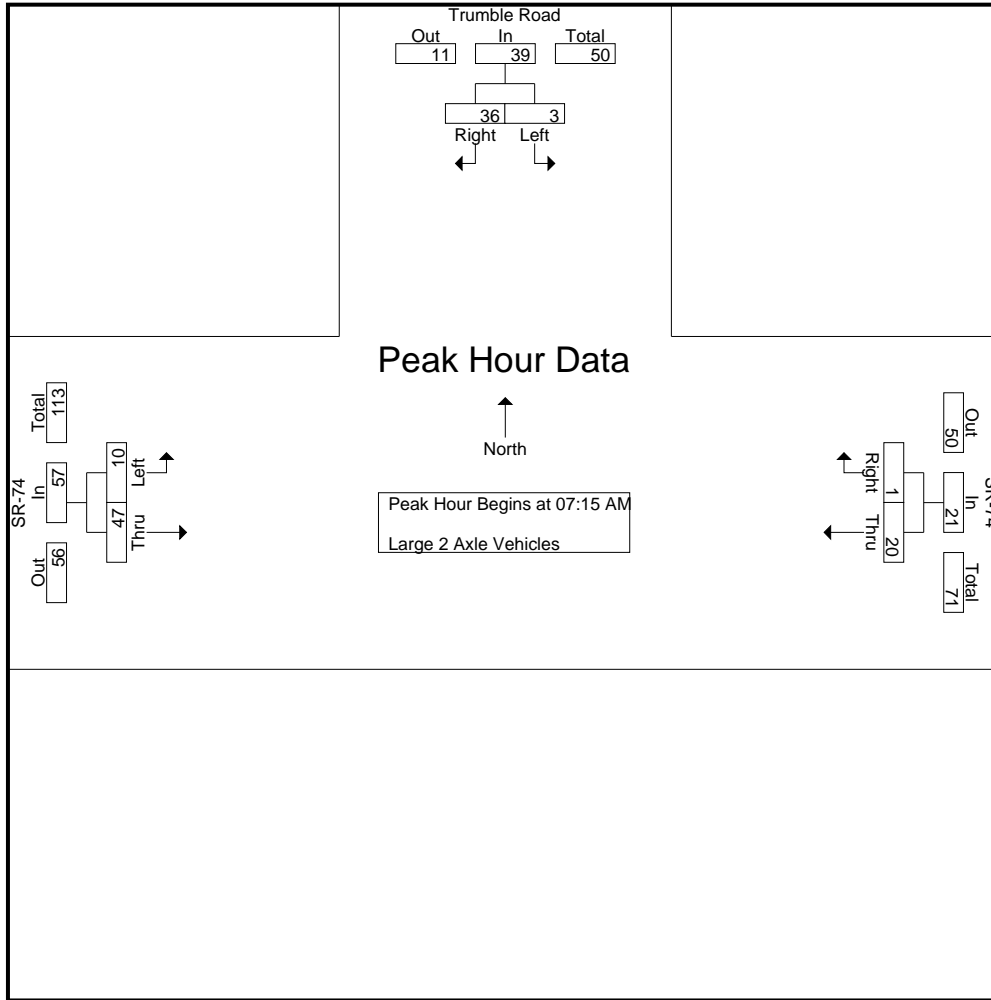
Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
07:00 AM	3	28	31	7	1	8	4	7	11	50
07:15 AM	1	12	13	5	0	5	2	14	16	34
07:30 AM	0	12	12	7	0	7	2	10	12	31
07:45 AM	2	5	7	5	0	5	2	12	14	26
Total	6	57	63	24	1	25	10	43	53	141
08:00 AM	0	7	7	3	1	4	4	11	15	26
08:15 AM	1	8	9	7	1	8	6	6	12	29
08:30 AM	1	3	4	2	1	3	2	5	7	14
08:45 AM	0	1	1	10	0	10	1	10	11	22
Total	2	19	21	22	3	25	13	32	45	91
Grand Total	8	76	84	46	4	50	23	75	98	232
Apprch %	9.5	90.5		92	8		23.5	76.5		
Total %	3.4	32.8	36.2	19.8	1.7	21.6	9.9	32.3	42.2	

Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
07:15 AM	1	12	13	5	0	5	2	14	16	34
07:30 AM	0	12	12	7	0	7	2	10	12	31
07:45 AM	2	5	7	5	0	5	2	12	14	26
08:00 AM	0	7	7	3	1	4	4	11	15	26
Total Volume	3	36	39	20	1	21	10	47	57	117
% App. Total	7.7	92.3		95.2	4.8		17.5	82.5		
PHF	.375	.750	.750	.714	.250	.750	.625	.839	.891	.860

Peak Hour Analysis From 07:15 AM to 08:00 AM - Peak 1 of 1
 Peak Hour for Entire Intersection Begins at 07:15 AM

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 AM
 Site Code : 00322221
 Start Date : 3/10/2022
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Peak Hour Analysis From 07:15 AM to 08:00 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:15 AM			07:15 AM			07:15 AM		
+0 mins.	1	12	13	5	0	5	2	14	16
+15 mins.	0	12	12	7	0	7	2	10	12
+30 mins.	2	5	7	5	0	5	2	12	14
+45 mins.	0	7	7	3	1	4	4	11	15
Total Volume	3	36	39	20	1	21	10	47	57
% App. Total	7.7	92.3		95.2	4.8		17.5	82.5	
PHF	.375	.750	.750	.714	.250	.750	.625	.839	.891

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 AM
 Site Code : 00322221
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Groups Printed- 3 Axle Vehicles

Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
07:00 AM	0	3	3	4	0	4	1	7	8	15
07:15 AM	0	1	1	2	0	2	0	5	5	8
07:30 AM	0	1	1	3	0	3	1	4	5	9
07:45 AM	0	1	1	1	1	2	0	2	2	5
Total	0	6	6	10	1	11	2	18	20	37
08:00 AM	0	0	0	5	0	5	1	3	4	9
08:15 AM	0	0	0	3	0	3	0	4	4	7
08:30 AM	0	2	2	2	0	2	1	6	7	11
08:45 AM	0	0	0	3	0	3	0	4	4	7
Total	0	2	2	13	0	13	2	17	19	34
Grand Total	0	8	8	23	1	24	4	35	39	71
Apprch %	0	100		95.8	4.2		10.3	89.7		
Total %	0	11.3	11.3	32.4	1.4	33.8	5.6	49.3	54.9	

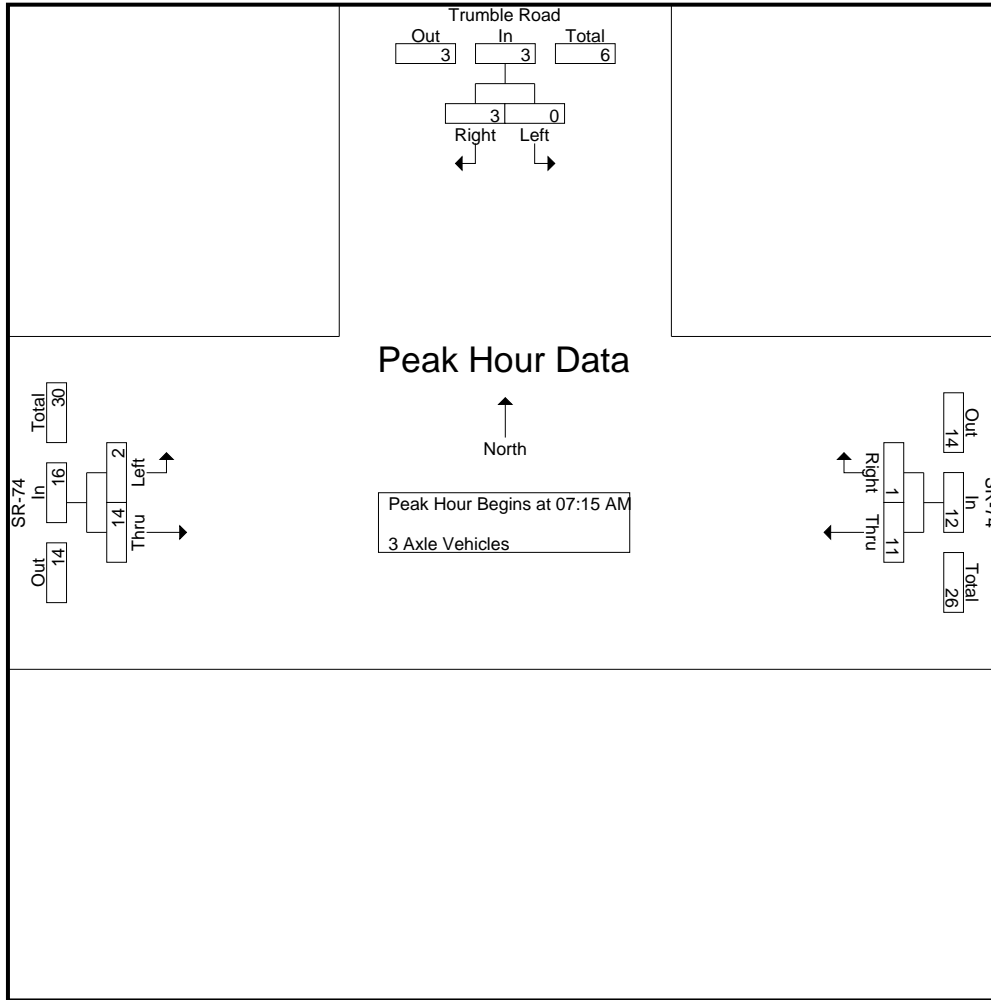
Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
07:15 AM	0	1	1	2	0	2	0	5	5	8
07:30 AM	0	1	1	3	0	3	1	4	5	9
07:45 AM	0	1	1	1	1	2	0	2	2	5
08:00 AM	0	0	0	5	0	5	1	3	4	9
Total Volume	0	3	3	11	1	12	2	14	16	31
% App. Total	0	100		91.7	8.3		12.5	87.5		
PHF	.000	.750	.750	.550	.250	.600	.500	.700	.800	.861

Peak Hour Analysis From 07:15 AM to 08:00 AM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 07:15 AM

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 07:15 AM to 08:00 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:15 AM			07:15 AM			07:15 AM		
+0 mins.	0	1	1	2	0	2	0	5	5
+15 mins.	0	1	1	3	0	3	1	4	5
+30 mins.	0	1	1	1	1	2	0	2	2
+45 mins.	0	0	0	5	0	5	1	3	4
Total Volume	0	3	3	11	1	12	2	14	16
% App. Total	0	100		91.7	8.3		12.5	87.5	
PHF	.000	.750	.750	.550	.250	.600	.500	.700	.800

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- 4+ Axle Trucks

Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
07:00 AM	0	11	11	5	0	5	1	8	9	25
07:15 AM	0	8	8	1	0	1	0	6	6	15
07:30 AM	0	3	3	1	0	1	0	12	12	16
07:45 AM	0	1	1	2	0	2	0	8	8	11
Total	0	23	23	9	0	9	1	34	35	67
08:00 AM	0	3	3	1	0	1	1	7	8	12
08:15 AM	0	1	1	2	0	2	1	2	3	6
08:30 AM	0	0	0	3	0	3	1	4	5	8
08:45 AM	0	0	0	2	0	2	0	3	3	5
Total	0	4	4	8	0	8	3	16	19	31
Grand Total	0	27	27	17	0	17	4	50	54	98
Apprch %	0	100		100	0		7.4	92.6		
Total %	0	27.6	27.6	17.3	0	17.3	4.1	51	55.1	

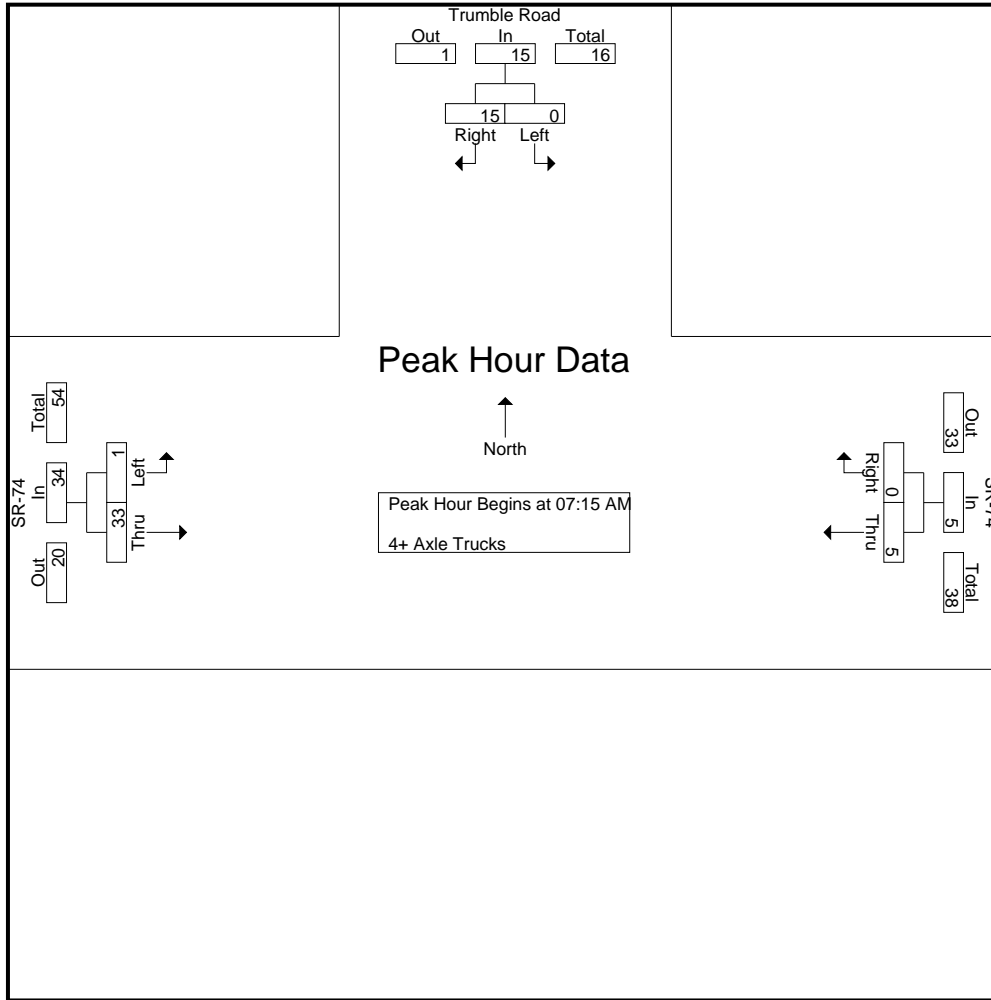
Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
07:15 AM	0	8	8	1	0	1	0	6	6	15
07:30 AM	0	3	3	1	0	1	0	12	12	16
07:45 AM	0	1	1	2	0	2	0	8	8	11
08:00 AM	0	3	3	1	0	1	1	7	8	12
Total Volume	0	15	15	5	0	5	1	33	34	54
% App. Total	0	100		100	0		2.9	97.1		
PHF	.000	.469	.469	.625	.000	.625	.250	.688	.708	.844

Peak Hour Analysis From 07:15 AM to 08:00 AM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 07:15 AM

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 AM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 07:15 AM to 08:00 AM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	07:15 AM			07:15 AM			07:15 AM		
+0 mins.	0	8	8	1	0	1	0	6	6
+15 mins.	0	3	3	1	0	1	0	12	12
+30 mins.	0	1	1	2	0	2	0	8	8
+45 mins.	0	3	3	1	0	1	1	7	8
Total Volume	0	15	15	5	0	5	1	33	34
% App. Total	0	100		100	0		2.9	97.1	
PHF	.000	.469	.469	.625	.000	.625	.250	.688	.708

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

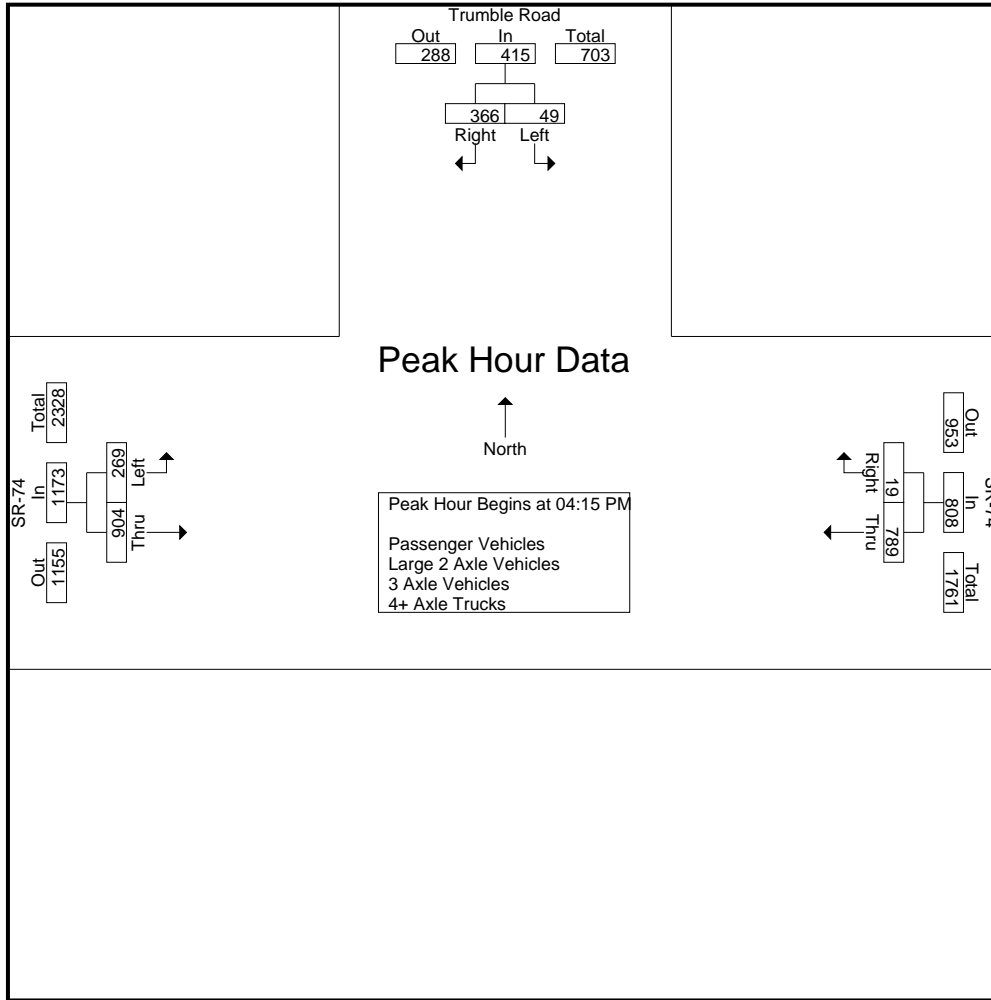
Groups Printed- Passenger Vehicles - Large 2 Axle Vehicles - 3 Axle Vehicles - 4+ Axle Trucks

Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
04:00 PM	17	87	104	200	2	202	57	263	320	626
04:15 PM	12	79	91	216	6	222	60	220	280	593
04:30 PM	12	103	115	178	7	185	62	237	299	599
04:45 PM	11	61	72	180	4	184	70	240	310	566
Total	52	330	382	774	19	793	249	960	1209	2384
05:00 PM	14	123	137	215	2	217	77	207	284	638
05:15 PM	10	60	70	187	4	191	80	219	299	560
05:30 PM	11	53	64	213	11	224	90	227	317	605
05:45 PM	5	53	58	162	11	173	101	249	350	581
Total	40	289	329	777	28	805	348	902	1250	2384
Grand Total	92	619	711	1551	47	1598	597	1862	2459	4768
Apprch %	12.9	87.1		97.1	2.9		24.3	75.7		
Total %	1.9	13	14.9	32.5	1	33.5	12.5	39.1	51.6	
Passenger Vehicles	89	595	684	1477	41	1518	535	1825	2360	4562
% Passenger Vehicles	96.7	96.1	96.2	95.2	87.2	95	89.6	98	96	95.7
Large 2 Axle Vehicles	3	20	23	43	6	49	45	30	75	147
% Large 2 Axle Vehicles	3.3	3.2	3.2	2.8	12.8	3.1	7.5	1.6	3.1	3.1
3 Axle Vehicles	0	2	2	22	0	22	6	0	6	30
% 3 Axle Vehicles	0	0.3	0.3	1.4	0	1.4	1	0	0.2	0.6
4+ Axle Trucks	0	2	2	9	0	9	11	7	18	29
% 4+ Axle Trucks	0	0.3	0.3	0.6	0	0.6	1.8	0.4	0.7	0.6

Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 04:15 PM										
04:15 PM	12	79	91	216	6	222	60	220	280	593
04:30 PM	12	103	115	178	7	185	62	237	299	599
04:45 PM	11	61	72	180	4	184	70	240	310	566
05:00 PM	14	123	137	215	2	217	77	207	284	638
Total Volume	49	366	415	789	19	808	269	904	1173	2396
% App. Total	11.8	88.2		97.6	2.4		22.9	77.1		
PHF	.875	.744	.757	.913	.679	.910	.873	.942	.946	.939

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	04:15 PM			04:45 PM			05:00 PM		
+0 mins.	12	79	91	180	4	184	77	207	284
+15 mins.	12	103	115	215	2	217	80	219	299
+30 mins.	11	61	72	187	4	191	90	227	317
+45 mins.	14	123	137	213	11	224	101	249	350
Total Volume	49	366	415	795	21	816	348	902	1250
% App. Total	11.8	88.2		97.4	2.6		27.8	72.2	
PHF	.875	.744	.757	.924	.477	.911	.861	.906	.893

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- Passenger Vehicles

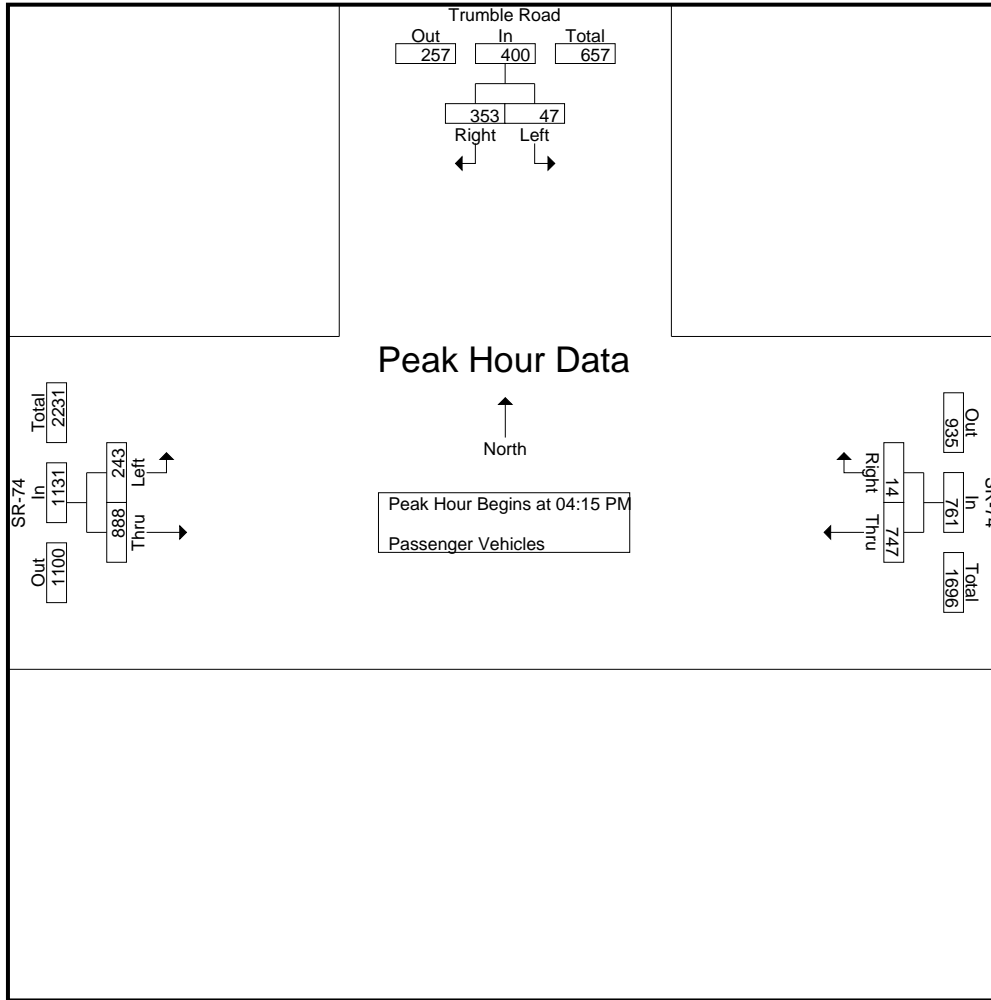
Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
04:00 PM	17	83	100	187	2	189	50	255	305	594
04:15 PM	11	74	85	204	3	207	55	215	270	562
04:30 PM	12	100	112	170	5	175	54	233	287	574
04:45 PM	10	60	70	169	4	173	62	237	299	542
Total	50	317	367	730	14	744	221	940	1161	2272
05:00 PM	14	119	133	204	2	206	72	203	275	614
05:15 PM	10	58	68	181	4	185	71	215	286	539
05:30 PM	11	51	62	208	10	218	81	222	303	583
05:45 PM	4	50	54	154	11	165	90	245	335	554
Total	39	278	317	747	27	774	314	885	1199	2290
Grand Total	89	595	684	1477	41	1518	535	1825	2360	4562
Apprch %	13	87		97.3	2.7		22.7	77.3		
Total %	2	13	15	32.4	0.9	33.3	11.7	40	51.7	

Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
04:15 PM	11	74	85	204	3	207	55	215	270	562
04:30 PM	12	100	112	170	5	175	54	233	287	574
04:45 PM	10	60	70	169	4	173	62	237	299	542
05:00 PM	14	119	133	204	2	206	72	203	275	614
Total Volume	47	353	400	747	14	761	243	888	1131	2292
% App. Total	11.8	88.2		98.2	1.8		21.5	78.5		
PHF	.839	.742	.752	.915	.700	.919	.844	.937	.946	.933

Peak Hour Analysis From 04:15 PM to 05:00 PM - Peak 1 of 1
 Peak Hour for Entire Intersection Begins at 04:15 PM

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 04:15 PM to 05:00 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	04:15 PM			04:15 PM			04:15 PM		
+0 mins.	11	74	85	204	3	207	55	215	270
+15 mins.	12	100	112	170	5	175	54	233	287
+30 mins.	10	60	70	169	4	173	62	237	299
+45 mins.	14	119	133	204	2	206	72	203	275
Total Volume	47	353	400	747	14	761	243	888	1131
% App. Total	11.8	88.2		98.2	1.8		21.5	78.5	
PHF	.839	.742	.752	.915	.700	.919	.844	.937	.946

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- Large 2 Axle Vehicles

Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
04:00 PM	0	4	4	10	0	10	6	5	11	25
04:15 PM	1	3	4	7	3	10	4	4	8	22
04:30 PM	0	3	3	4	2	6	6	3	9	18
04:45 PM	1	1	2	5	0	5	5	3	8	15
Total	2	11	13	26	5	31	21	15	36	80
05:00 PM	0	4	4	8	0	8	4	4	8	20
05:15 PM	0	1	1	4	0	4	8	3	11	16
05:30 PM	0	1	1	3	1	4	6	4	10	15
05:45 PM	1	3	4	2	0	2	6	4	10	16
Total	1	9	10	17	1	18	24	15	39	67
Grand Total	3	20	23	43	6	49	45	30	75	147
Apprch %	13	87		87.8	12.2		60	40		
Total %	2	13.6	15.6	29.3	4.1	33.3	30.6	20.4	51	

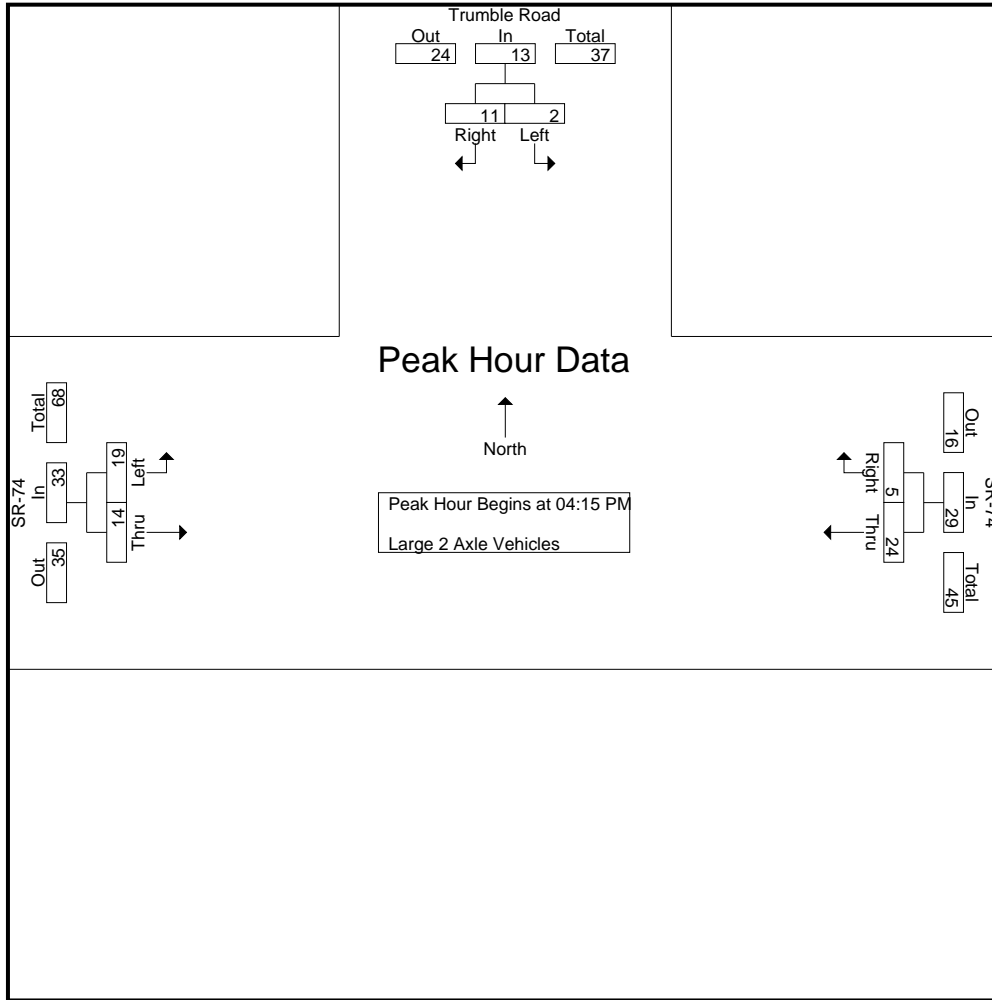
Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
04:15 PM	1	3	4	7	3	10	4	4	8	22
04:30 PM	0	3	3	4	2	6	6	3	9	18
04:45 PM	1	1	2	5	0	5	5	3	8	15
05:00 PM	0	4	4	8	0	8	4	4	8	20
Total Volume	2	11	13	24	5	29	19	14	33	75
% App. Total	15.4	84.6		82.8	17.2		57.6	42.4		
PHF	.500	.688	.813	.750	.417	.725	.792	.875	.917	.852

Peak Hour Analysis From 04:15 PM to 05:00 PM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 04:15 PM

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 04:15 PM to 05:00 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	04:15 PM			04:15 PM			04:15 PM		
+0 mins.	1	3	4	7	3	10	4	4	8
+15 mins.	0	3	3	4	2	6	6	3	9
+30 mins.	1	1	2	5	0	5	5	3	8
+45 mins.	0	4	4	8	0	8	4	4	8
Total Volume	2	11	13	24	5	29	19	14	33
% App. Total	15.4	84.6		82.8	17.2		57.6	42.4	
PHF	.500	.688	.813	.750	.417	.725	.792	.875	.917

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- 3 Axle Vehicles

Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
04:00 PM	0	0	0	2	0	2	0	0	0	2
04:15 PM	0	1	1	2	0	2	0	0	0	3
04:30 PM	0	0	0	3	0	3	0	0	0	3
04:45 PM	0	0	0	4	0	4	2	0	2	6
Total	0	1	1	11	0	11	2	0	2	14
05:00 PM	0	0	0	3	0	3	0	0	0	3
05:15 PM	0	1	1	2	0	2	0	0	0	3
05:30 PM	0	0	0	2	0	2	0	0	0	2
05:45 PM	0	0	0	4	0	4	4	0	4	8
Total	0	1	1	11	0	11	4	0	4	16
Grand Total	0	2	2	22	0	22	6	0	6	30
Apprch %	0	100		100	0		100	0		
Total %	0	6.7	6.7	73.3	0	73.3	20	0	20	

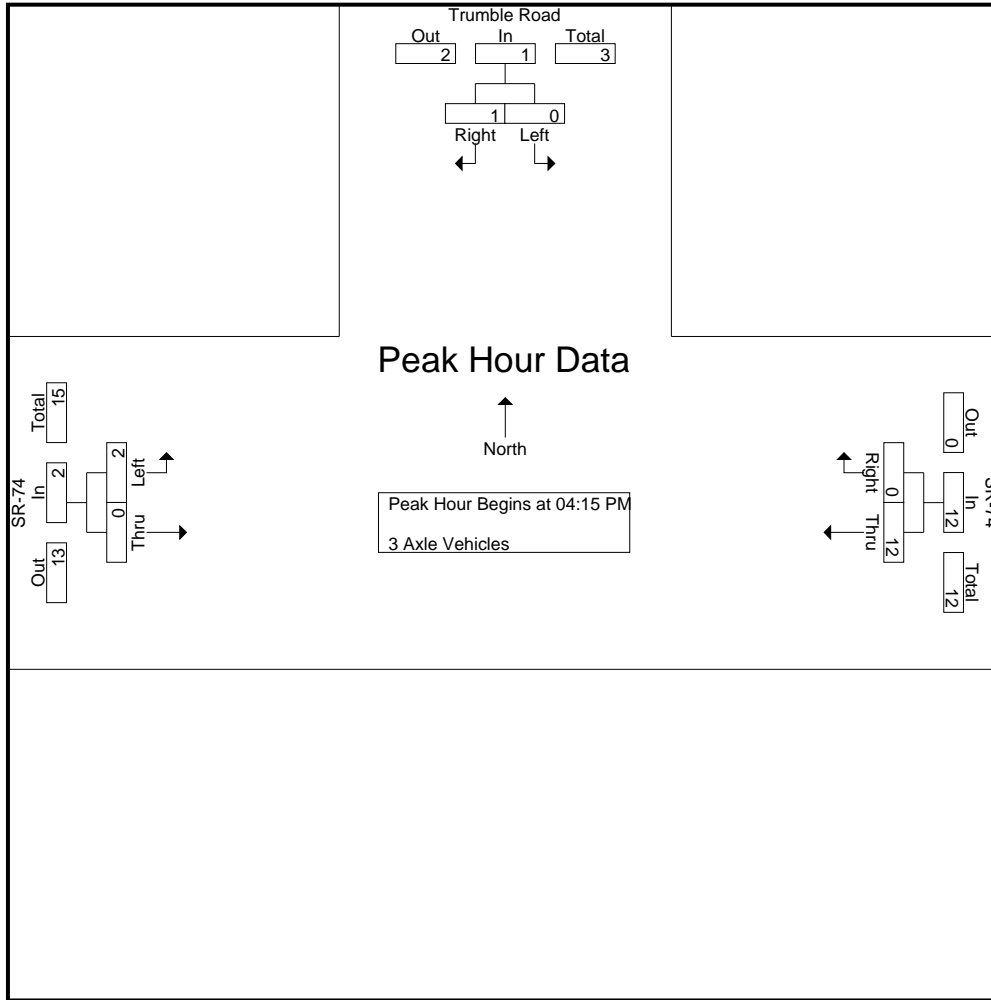
Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
04:15 PM	0	1	1	2	0	2	0	0	0	3
04:30 PM	0	0	0	3	0	3	0	0	0	3
04:45 PM	0	0	0	4	0	4	2	0	2	6
05:00 PM	0	0	0	3	0	3	0	0	0	3
Total Volume	0	1	1	12	0	12	2	0	2	15
% App. Total	0	100		100	0		100	0		
PHF	.000	.250	.250	.750	.000	.750	.250	.000	.250	.625

Peak Hour Analysis From 04:15 PM to 05:00 PM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 04:15 PM

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 04:15 PM to 05:00 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	04:15 PM			04:15 PM			04:15 PM		
+0 mins.	0	1	1	2	0	2	0	0	0
+15 mins.	0	0	0	3	0	3	0	0	0
+30 mins.	0	0	0	4	0	4	2	0	2
+45 mins.	0	0	0	3	0	3	0	0	0
Total Volume	0	1	1	12	0	12	2	0	2
% App. Total	0	100		100	0		100	0	
PHF	.000	.250	.250	.750	.000	.750	.250	.000	.250

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 1

Groups Printed- 4+ Axle Trucks

Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
04:00 PM	0	0	0	1	0	1	1	3	4	5
04:15 PM	0	1	1	3	0	3	1	1	2	6
04:30 PM	0	0	0	1	0	1	2	1	3	4
04:45 PM	0	0	0	2	0	2	1	0	1	3
Total	0	1	1	7	0	7	5	5	10	18
05:00 PM	0	0	0	0	0	0	1	0	1	1
05:15 PM	0	0	0	0	0	0	1	1	2	2
05:30 PM	0	1	1	0	0	0	3	1	4	5
05:45 PM	0	0	0	2	0	2	1	0	1	3
Total	0	1	1	2	0	2	6	2	8	11
Grand Total	0	2	2	9	0	9	11	7	18	29
Apprch %	0	100		100	0		61.1	38.9		
Total %	0	6.9	6.9	31	0	31	37.9	24.1	62.1	

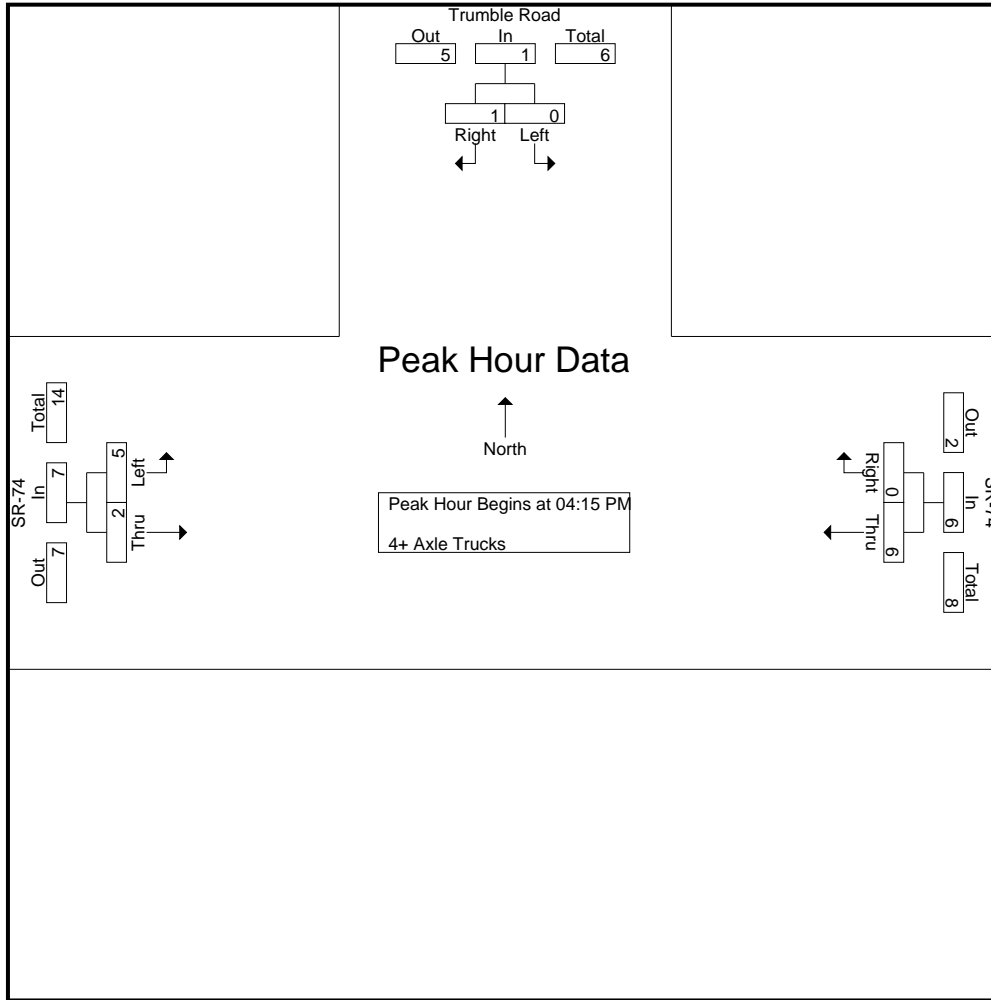
Start Time	Trumble Road Southbound			SR-74 Westbound			SR-74 Eastbound			Int. Total
	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	
04:15 PM	0	1	1	3	0	3	1	1	2	6
04:30 PM	0	0	0	1	0	1	2	1	3	4
04:45 PM	0	0	0	2	0	2	1	0	1	3
05:00 PM	0	0	0	0	0	0	1	0	1	1
Total Volume	0	1	1	6	0	6	5	2	7	14
% App. Total	0	100		100	0		71.4	28.6		
PHF	.000	.250	.250	.500	.000	.500	.625	.500	.583	.583

Peak Hour Analysis From 04:15 PM to 05:00 PM - Peak 1 of 1

Peak Hour for Entire Intersection Begins at 04:15 PM

City of Perris
 N/S: Trumble Road
 E/W: SR-74
 Weather: Clear

File Name : 05_PER_Trumble_SR-74 PM
 Site Code : 00322221
 Start Date : 3/10/2022
 Page No : 2



Peak Hour Analysis From 04:15 PM to 05:00 PM - Peak 1 of 1
 Peak Hour for Each Approach Begins at:

	04:15 PM			04:15 PM			04:15 PM		
+0 mins.	0	1	1	3	0	3	1	1	2
+15 mins.	0	0	0	1	0	1	2	1	3
+30 mins.	0	0	0	2	0	2	1	0	1
+45 mins.	0	0	0	0	0	0	1	0	1
Total Volume	0	1	1	6	0	6	5	2	7
% App. Total	0	100		100	0		71.4	28.6	
PHF	.000	.250	.250	.500	.000	.500	.625	.500	.583

Location: SR 74 @ BONNIE DRIVE

Designed By:

System:

District:

Installed By: SAFWAN SAYED

Master At:

I/C:

Service Info:

Timing Change:
11/17/2020

Date Start:
5/19/2016

Date End:

Designed:

Installed:
5/19/2016

Intersection Layout

FLASH

- | | | |
|---|--------------------------|-------|
| | 1) W/B SR 74 - LEFT TURN | [R] |
| P | 2) E/B SR 74 | [R] |
| H | 3) | [] |
| A | 4) | [] |
| S | 5) | [] |
| E | 6) W/B SR 74 | [R] |
| | 7) W/B BONNIE DRIVE | [R] |
| | 8) E/B BONNIE DRIVE | [R] |
| | | |
| O | A) | [] |
| V | B) | [] |
| E | C) | [] |
| R | D) | [] |
| L | E) | [] |
| A | F) | [] |
| P | | [] |

Comments and Notes:

1. Phase 8 stop Bar is too far from the front Loop, the Red and Yellow lock is turned-on (5/4/17)
2. The Advanced and Mid Loops, 8J6U and 8J6L, are placed on Count+Extend only

RAM Checksum

Page 2: 4132	Page 8: 85AF
Page 3: 774B	Page 9: D2FD
Page 4: F29E	Page 10: 637B
Page 5: 191A	Page 11: C3CB
Page 6: 191A	Page 12: D68F
Page 7: 398C	Page 13: 86F7

CONFIGURATION PHASE FLAGS

Cabinet
332
Configuration
CALTRANS

Phases (2-1-1-1)	
Permitted	1 2 . . . 6 7 8
Restricted

Phase Features (2-1-1-4)	
Double Entry
Rest In Walk
Rest In Red
Walk 2
Max Green 2
Max Green 3

Startup (2-1-1-5)	
First Green Phases	. 2 . . . 6 . .
Yellow Start Phases
Vehicle Calls	1 2 . . . 6 7 8
Pedestrian Calls
Yellow Start Overlaps
Startup All-Red	6.0

Phase Recalls (2-1-1-2)	
Vehicle Min	. 2 . . . 6 . .
Vehicle Max
Pedestrian
Bicycle

Phase Locks (2-1-1-3)	
Red 8
Yellow 8
Force/Max

Call To Phase (2-1-2-1)		Omit On Green	
1	1
2	2
3	3
4	4
5	5
6	6
7	7
8	8

Flashing Colors (2-1-2-2)	
Yellow Flash Phases
Yellow Flash Overlap
Flash In Red Phases
Flash In Red Overlap

Special Operation (2-1-2-3)	
Single Exit Phase
Driveway Signal Phases
Driveway Signal Overlaps
Leading Ped Phases

Protected Permissive (2-1-2-4)	
Protected Permissive

Pedestrian (2-1-3)	
P1
P2
P3
P4
P5
P6
P7
P8

Overlap (2-1-4)				
Overlap	Parent	Omit	No Start	Not
A
B
C
D
E
F

P
H
A
S
E

T
I
M
I
N
G

Phase (2-2)	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
--- Walk 1 ---	0	0	0	0	0	0	0	0
Flash Don't Walk	0	0	0	0	0	0	0	0
Minimum Green	5	5	0	0	0	5	5	5
Det Limit	0	0	0	0	0	0	0	0
Max Initial	0	0	0	0	0	0	0	0
Max Green 1	25	35	0	0	0	35	15	25
Max Green 2	0	0	0	0	0	0	0	0
Max Green 3	0	0	0	0	0	0	0	0
Extension	2.0	2.0	0.0	0.0	0.0	2.0	2.0	2.0
Maximum Gap	2.0	2.0	0.0	0.0	0.0	2.0	2.0	2.0
Minimum Gap	2.0	2.0	0.0	0.0	0.0	2.0	2.0	2.0
Add Per Vehicle	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Reduce Gap By	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Reduce Every	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	4.0	5.0	3.0	3.0	3.0	5.0	3.0	5.0
All-Red	1.0	1.0	0.0	0.0	0.0	1.0	1.0	1.0
Ped/Bike (2-3)	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
--- Walk 2 ---	0	0	0	0	0	0	0	0
Delay/Early Walk	0	0	0	0	0	0	0	0
Solid Don't Walk	0	0	0	0	0	0	0	0
Bike Green	0	0	0	0	0	0	0	0
Bike All-Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

OVERLAP TIMING

Overlap (2-4)	A	B	C	D	E	F
Green	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	5.0	5.0	5.0	5.0	5.0	5.0
Red	0.0	0.0	0.0	0.0	0.0	0.0

Red Revert

Red Revert (2-5)	
Time	5.0
All-Red Sec/Min (2-6)	
All-Red Sec/Min:	OFF

Max 2 Extension

Max/Gap Out (2-7)	
Max Cnt	0
Gap Cnt	0

Local Plan 1...9 (7-1) TIMING DATA

COORDINATION

[Offsets] Green Factors or Press [F] to Select Force-Off

		Cycle	Multi	Lag Gap	A	B	C	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
Plan 1	Green Factor													
Plan 2	Green Factor													
Plan 3	Green Factor													
Plan 4	Green Factor													
Plan 5	Green Factor													
Plan 6	Green Factor													
Plan 7	Green Factor													
Plan 8	Green Factor													
Plan 9	Green Factor													

Master Timer Sync (7-A)	
Enable in Plans	
1-9
11-19
21-29

Master Sub Master	
Input	
Output	

FREE PLAN PHASE FLAGS

(7-E) Free	
Lag	Omit
. 2 . 4 . 6 . 8
Veh Min	Veh Max
. 2 ... 6
Ped	Bike
.....
Cond	Cond Grn
.....	10

Local Plan 1...9 (7-1) PHASE FLAGS

	Lag	Sync	Hold	Omit	Veh Min	Veh Max	Ped	Bike
Plan 1
Plan 2
Plan 3
Plan 4
Plan 5
Plan 6
Plan 7
Plan 8
Plan 9

MANUAL COMMANDS

Manual Plan (4-1)		<i>Plan: 1-9</i>
Plan	OffSet	<i>15 or 254 = Flash</i>
	A	<i>14 or 255 = Free</i>
		<i>Offset A, B, or C</i>

Special Function Override (4-2)			
#	Control	#	Control
1	NORMAL	3	NORMAL
2	NORMAL	4	NORMAL

Detector Reset	(4-3)
Local Manual (4-4)	OFF

Local Plan 11...19 (7-2) TIMING DATA

COORDINATION

[Offsets]

Green Factors or Press [F] to Select Force-Off

		Cycle	Multi	Lag Gap	A	B	C	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
Plan 11	Green Factor													
Plan 12	Green Factor													
Plan 13	Green Factor													
Plan 14	Green Factor													
Plan 15	Green Factor													
Plan 16	Green Factor													
Plan 17	Green Factor													
Plan 18	Green Factor													
Plan 19	Green Factor													

Local Plan 11...19 (7-2) PHASE FLAGS

	Lag	Sync	Hold	Omit	Veh Min	Veh Max	Ped	Bike
Plan 11
Plan 12
Plan 13
Plan 14
Plan 15
Plan 16
Plan 17
Plan 18
Plan 19

Local Plan 21...29 (7-3) TIMING DATA

COORDINATION

[Offsets]

Green Factors or Press [F] to Select Force-Off

		Cycle	Multi	Lag Gap	A	B	C	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
Plan 21	Green Factor													
Plan 22	Green Factor													
Plan 23	Green Factor													
Plan 24	Green Factor													
Plan 25	Green Factor													
Plan 26	Green Factor													
Plan 27	Green Factor													
Plan 28	Green Factor													
Plan 29	Green Factor													

Local Plan 21...29 (7-3) PHASE FLAGS

	Lag	Sync	Hold	Omit	Veh Min	Veh Max	Ped	Bike
Plan 21
Plan 22
Plan 23
Plan 24
Plan 25
Plan 26
Plan 27
Plan 28
Plan 29

DETECTORS

Detector Attributes (5-1)				Slot	Detector Configuration (5-2)				
Det	Type	Phases	Lock		Det	Delay	Extend	Recall	Port
1	COUNT+CALL+EXTEND	1.....	NO	I1U	1			10	3.2
2	COUNT+CALL+EXTEND	1.....	NO	I1L	2			10	7.2
3	COUNT+CALL+EXTEND	.2.....	NO	I2U	3			10	1.1
4	COUNT+CALL+EXTEND	.2.....	NO	I2L	4			10	1.5
5	COUNT+CALL+EXTEND	.2.....	NO	I3U	5			10	4.5
6	COUNT+CALL+EXTEND	.2.....	NO	I3L	6			10	6.2
7	COUNT+CALL+EXTEND	.2.....	NO	I4U	7			10	2.1
8	COUNT+CALL+EXTEND	.2.....	NO	I4L	8			10	7.4
9	COUNT+CALL+EXTEND	..3.....	NO	I5U	9			10	3.4
10	COUNT+CALL+EXTEND	..3.....	NO	I5L	10			10	7.6
11	COUNT+CALL+EXTEND	...4....	NO	I6U	11			10	1.3
12	COUNT+CALL+EXTEND	...4....	NO	I6L	12			10	1.7
13	COUNT+CALL+EXTEND	...4....	NO	I7U	13			10	4.7
14	COUNT+CALL+EXTEND	...4....	NO	I7L	14			10	6.4
15	COUNT+CALL+EXTEND	...4....	NO	I8U	15			10	2.3
16	COUNT+CALL+EXTEND	...4....	NO	I8L	16			10	7.8
17	COUNT+CALL+EXTEND	1.....	NO	I9U	17			10	3.6
18	COUNT+CALL+EXTEND	..3.....	NO	I9L	18			10	3.8
19	COUNT+CALL+EXTEND	.2.....	NO	I10U	19			10	4.1
20	COUNT+CALL+EXTEND	...4....	NO	I10L	20			10	4.2
21	COUNT+CALL+EXTEND	...5...	NO	J1U	21			10	3.1
22	COUNT+CALL+EXTEND	...5...	NO	J1L	22			10	7.1
23	COUNT+CALL+EXTEND6..	NO	J2U	23			10	1.2
24	COUNT+CALL+EXTEND6..	NO	J2L	24			10	1.6
25	COUNT+CALL+EXTEND6..	NO	J3U	25			10	4.6
26	COUNT+CALL+EXTEND6..	NO	J3L	26			10	6.3
27	COUNT+CALL+EXTEND6..	NO	J4U	27			10	2.2
28	COUNT+CALL+EXTEND6..	NO	J4L	28			10	7.3
29	COUNT+CALL+EXTEND7.	NO	J5U	29			10	3.3
30	COUNT+CALL+EXTEND7.	NO	J5L	30			10	7.5
31	COUNT+CALL+EXTEND8	NO	J6U	31			10	1.4
32	COUNT+CALL+EXTEND8	NO	J6L	32			10	1.8
33	COUNT+CALL+EXTEND8	NO	J7U	33			10	4.8
34	COUNT+CALL+EXTEND8	NO	J7L	34			10	6.5
35	COUNT+CALL+EXTEND8	NO	J8U	35			10	2.4
36	COUNT+CALL+EXTEND8	NO	J8L	36			10	7.7
37	COUNT+CALL+EXTEND	...5...	NO	J9U	37			10	3.5
38	COUNT+CALL+EXTEND	...7...	NO	J9L	38			10	3.7
39	COUNT+CALL+EXTEND6..	NO	J10U	39			10	4.3
40	COUNT+CALL+EXTEND8	NO	J10L	40			10	4.4
41	PEDESTRIAN	.2.....	NO	I12U	41			10	5.1
42	PEDESTRIAN	...4....	NO	I12L	42			10	5.3
43	PEDESTRIAN6..	NO	I13U	43			10	5.2
44	PEDESTRIAN8	NO	I13L	44			10	5.4

Failure Times(5-3)	Minutes
Maximum On Time	
Fail Reset Time	

Failure Override (5-4)	
Detectors 1-8
Detectors 9-16
Detectors 17-24
Detectors 25-32
Detectors 33-40
Detectors 41-44

System Detector Assignment (5-5)								
Sys Det	1	2	3	4	5	6	7	8
Det Nu								
Sys Det	9	10	11	12	13	14	15	16
Det Nu								

CIC Operation (5-6-1)	
Enable in Plans

CIC Values (5-6-2)	Volume	Occupancy	Demand
Smoothing	0.66	0.66	0.66
Multiplier	4.0	0.33	
Exponent	0.50	1.00	

Detector-to-Phase Assignment (5-6-3)								
Sys Det	1	2	3	4	5	6	7	8
Phase								
Sys Det	9	10	11	12	13	14	15	16
Phase								

Input File Port-Bit Assignments

332 Cabinet - For Reference Only

	1	2	3	4	5	6	7	8	9	10	11	12	13	14
I-	3.2	1.1	4.5	2.1	3.4	1.3	4.7	2.3	3.6	4.1	6.6	5.1	5.2	6.7
	7.2	1.5	6.2	7.4	7.6	1.7	6.4	7.8	3.8	4.2	2.7	5.3	5.4	6.8
J-	3.1	1.2	4.6	2.2	3.3	1.4	4.8	2.4	3.5	4.3	2.8	5.5	5.6	2.5
	7.1	1.6	6.3	7.3	7.5	1.8	6.5	7.7	3.7	4.4	6.1	5.7	5.8	2.6

TOD SCHEDULE

Table 1 (8-2-1)			Table 2 (8-2-2)			Table 3 (8-2-3)			Table 4 (8-2-4)			Table 5 (8-2-5)			Table 6 (8-2-6)		
Time	Plan	OS	Time	Plan	OS	Time	Plan	OS	Time	Plan	OS	Time	Plan	OS	Time	Plan	OS
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A

WEEKDAY ASSIGNMENT

Weekday Table Assignments (8-2-7)						
Mon	Tue	Wed	Thu	Fri	Sat	Sun
1	1	1	1	1	2	2

HOLIDAY TABLES

Floating Holiday Table (8-2-8)				
#	Mnth	Week	DOW	Table
1			
2			
3			
4			
5			
6			
7			
8			
9			
10			
11			
12			
13			
14			
15			
16			

Fixed Holiday Table (8-2-9)				
#	Mnth	Day	DOW	Table
1			
2			
3			
4			
5			
6			
7			
8			
9			
10			
11			
12			
13			
14			
15			
16			

Solar Clock Data (8-4)	
North Latitude	34
West Longitude	118
Local Time Zone	8

Sabbatical Clock (8-5)	
Hebrew	Ped Recall
Sabbath
Holiday

Daylight Saving (8-6)	
Enabled	YES

TOD FUNCTIONS

TOD Functions (8-3)					
#	Start	End	DOW	Action	Phases
1		
2		
3		
4		
5		
6		
7		
8		
9		
10		
11		
12		
13		
14		
15		
16		

- Action Codes:
- 0. None
 - 1. Permitted
 - 2. Restricted
 - 4. Veh Min Recall
 - 5. Veh Max Recall
 - 6. Ped Recall
 - 7. Bike Recall
 - 8. Red Lock
 - 9. Yellow Lock
 - 10. Force/Max Lock
 - 11. Double Entry
 - 12. Y-Coord C
 - 13. Y-Coord D
 - 14. Free
 - 15. Flashing
 - 16. Walk 2
 - 17. Max Green 2

- 18. Max Green 3
- 19. Rest in Walk
- 20. Rest in Red
- 21. Free Lag Phases
- 22. Special Functions
- 23. Truck Preempt
- 24. Conditional Service
- 25. Conditional Service
- 26. Leading Ped
- 27. Traffic Actuated Max 2
- 41. Protected Permissive
- 42. Protected Permissive

Action Code = Phases added to normal setting
 100+Action Code = Phases removed
 200+Action Code = Phases replaced

COMMUNICATIONS

C2 (6-1-1)	
Address	
Protocol	AB3418
Access Level	0
Baud	1200
Parity	NONE
Data Bits	8
Stop Bits	1
RTS On Time	20
RTS Off Time	20
Handshaking	NORMAL

C20 (6-1-2)	
Address	
Protocol	AB3418
Access Level	0
Baud	1200
Parity	NONE
Data Bits	8
Stop Bits	1
RTS On Time	20
RTS Off Time	20
Handshaking	NORMAL

C21 (6-1-3)	
Address	
Protocol	AB3418
Access Level	0
Baud	1200
Parity	NONE
Data Bits	8
Stop Bits	1
RTS On Time	20
RTS Off Time	20
Handshaking	NORMAL

Access Levels:

- 0-Full Access
- 1-Status Only
- 2-Status, Set Pattern, Time
- 3-Status, Set Pattern, Time, Manual Plan
- 4-Reserved
- 5-Full Access with No Set Pattern
- 6-Full Access with No Set Time
- 7-Full Access with No Set Pattern, Manual Plan
- 8-Full Access with No Set Time, Pattern, Manual Plan

SOFT LOGIC

Soft Logic (6-2)							
#	Data	OP	Data	OP	Data	OP	Data
1							
2							
3							
4							
5							
6							
7							
8							
9							
10							
11							
12							
13							
14							
15							
16							

*Refer to User's Manual for Data and OP Codes

CALLBACK NUMBERS

Callback Numbers (6-3...3)	
Line Out	
Local Toll	
Long Distance	
Delay	10
Area Code	
Phone Number	
Line Out	
Local Toll	
Long Distance	
Delay	10
Area Code	
Phone Number	
Line Out	
Local Toll	
Long Distance	
Delay	10
Area Code	
Phone Number	

NETWORK

Network (6-4)	
Address	1
Protocol	AB3418
Port	27001
Type	STATIC
Central Access	6
Field Access	7

IP Address	172	.	25	.	52	.	5
Netmask	255	.	255	.	255	.	192
Broadcast	172	.	25	.	52	.	255
Gateway	172	.	25	.	52	.	1

RAILROAD PREEMPTION

RR 1	(3-1-1)	Timing	Phase Flags (3-1-2)			Pedestrian Flags (3-1-3)			Overlap Flags (3-1-4)		
	Delay		Grn Hold	Yel Flash	Red Flash	Walk	Flash DW	Solid DW	Grn Hold	Yel Flash	Red Flash
	Clear 1	10	. 2 . . 5 2 . 4 . 6 . 8
	Clear 2	
	Clear 3	
	Hold		1 2 3 4 5 6 7 8	A B C D E F
	Exit		Exit Parameters (3-1-5)				Configuration (3-1-6)				
Min Grn		Phase Green	Overlap Green	Vehicle Call	Ped Call	Primary Port	Secondary Port	Latching	Power-Up		
Ped Clr		1 2 3 4 5 6 7 8	. 2 . 4 . 6 . 8	2.5	0.0	YES	FLASHING		

RR 2	(3-2-1)	Timing	Phase Flags (3-2-2)			Pedestrian Flags (3-2-3)			Overlap Flags (3-2-4)		
	Delay		Grn Hold	Yel Flash	Red Flash	Walk	Flash DW	Solid DW	Grn Hold	Yel Flash	Red Flash
	Clear 1	10	. . . 4 . . 7 2 . 4 . 6 . 8
	Clear 2	
	Clear 3	
	Hold		1 2 3 . . 6 2 . . . 6 4 . . . 8
	Exit		Exit Parameters (3-2-5)				Configuration (3-2-6)				
Min Grn		Phase Green	Overlap Green	Vehicle Call	Ped Call	Primary Port	Secondary Port	Latching	Power-up		
Ped Clr	 4 . . 7	2.6	0.0	YES	DARK		

EMERGENCY VEHICLE PREEMPTION

EVA (3-A)	Preempt Timers			Phase Green	Overlap Green
	Delay	Clear	Max		
		30	30	. 2 . . 5
	Port	Latching	Phase Termination		
	5.5	NO	ADVANCE		

EVB (3-B)	Preempt Timers			Phase Green	Overlap Green
	Delay	Clear	Max		
		30	30	. . . 4 . . 7
	Port	Latching	Phase Termination		
	5.6	NO	ADVANCE		

EVC (3-C)	Preempt Timers			Phase Green	Overlap Green
	Delay	Clear	Max		
		30	30	1 6
	Port	Latching	Phase Termination		
	5.7	NO	ADVANCE		

EVD (3-D)	Preempt Timers			Phase Green	Overlap Green
	Delay	Clear	Max		
		30	30	. . 3 8
	Port	Latching	Phase Termination		
	5.8	NO	ADVANCE		

INPUTS

7 Wire I/C (2-1-5-1)					
		Input	Port	Input	Port
Enable	NO	R1	3.8	Free	3.6
Max ON		R2	3.5	D2	2.8
Max OFF		R3	3.7	D3	6.1

Manual Control (2-1-5-2)	
Input	Port
Manual Advance	
Advance Enable	

Enable	NO	R1	3.8	Free	3.6
Max ON		R2	3.5	D2	2.8
Max OFF		R3	3.7	D3	6.1

Battery Backup (2-1-5-5)	
Port	Operation
2.7	FLASHING

Cabinet Status (2-1-5-3)	
Input	Port
Flash Bus	
Door Ajar	
Flash Sense	6.7
Stop Time	6.8

Special Function (2-1-5-4)	
Input	Port
1	
2	
3	
4	

Y-Coordination (2-1-5-6)	
Port C	Port D
6.1	2.8

OUTPUTS

Loadswitch Assignments (2-1-6)								+
A	1	2	22	3	4	24	9	
B	5	6	26	7	8	28	10	
X	13	14	0	11	12	0	0	

- Loadswitch Codes:
- 0 Unused (no output)
 - 1-8 Vehicle 1-8
 - 9-14 Overlap A-F
 - 21-28 Ped 1-8
 - 41-47 Special Functions
 - 41 Protected Permissive Flashing Phase 1
 - 43 Protected Permissive Flashing Phase 3
 - 45 Protected Permissive Flashing Phase 5
 - 47 Protected Permissive Flashing Phase 7

- 51-57 Special Functions
- 71-72 Seven Wire I/C

+ middle output of loadswitches 3 and 6 Channel 9 and 10

TRANSIT PRIORITY

Local Plans (3-E) 1...9 11...19		Early Green	Green Extend	Inhibit Cycles	Phase 1 Minimum	Phase 2 Minimum	Phase 3 Minimum	Phase 4 Minimum	Phase 5 Minimum	Phase 6 Minimum	Phase 7 Minimum	Phase 8 Minimum
Plan 1	Green Factor											
Plan 2	Green Factor											
Plan 3	Green Factor											
Plan 4	Green Factor											
Plan 5	Green Factor											
Plan 6	Green Factor											
Plan 7	Green Factor											
Plan 8	Green Factor											
Plan 9	Green Factor											

Plan 11	Green Factor											
Plan 12	Green Factor											
Plan 13	Green Factor											
Plan 14	Green Factor											
Plan 15	Green Factor											
Plan 16	Green Factor											
Plan 17	Green Factor											
Plan 18	Green Factor											
Plan 19	Green Factor											

Transit Priority Configuration (3-E-A)		Indicator Output			
Enable in Plans	Input	Type	Stop	Go	
Plan 1-9	0.0	OPT	0	0
Plan 11-19	0.0	OPT	0	0

Queue Jump (3-E-B)	
Grn Hold	Hold Phase

Free Plans (3-E-E)	
Max Grn Hold	Hold Phase

Access Utilities (9-5)	
Password	***
Timeout	30

YELLOW YIELD COORDINATION

Y-Coord Plans (7-C,D)	Long Grn	No Grn	Offset	Perm	Force-Offs								Coord	Lag	Min Recall	Restricted
					-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-				
Plan C													. 2 . . . 6 . .	. 2 . 4 . 6 . 8
Plan D													. 2 . . . 6 . .	. 2 . 4 . 6 . 8

TRUCK PRIORITY

Truck Priority (3-F)	Passage	CarryOver	Clearance	Next Priority	Phase Green	Det 2 Port	Det 3 Port	Det 4 Port	Sign Output	Slave Input	Slave Output
					0.0	0.0	0.0	0	0.0	0

Location:

Designed By:

System:

District:

Installed By: SAFWAN

Master At:

I/C:

Service Info:

Timing Change:

Date Start:

Date End:

Designed:

Installed:

11/17/2020

3/28/2013

3/28/2013

Intersection Layout

FLASH

- | | | | |
|------|------------------------------|---|-----|
| 1) | | [|] |
| P 2) | EASTBOUND RTE 74 | [| R] |
| H 3) | | [|] |
| A 4) | I-215 NORTHBOUND OFF RAMP | [| R] |
| S 5) | EASTBOUND RTE 74 --LEFT TURN | [| R] |
| E 6) | WESTBOUND RTE 74 | [| R] |
| 7) | | [|] |
| 8) | | [|] |
| | | | |
| O A) | | [|] |
| V B) | | [|] |
| E C) | | [|] |
| R D) | | [|] |
| L E) | | [|] |
| A F) | | [|] |
| P | | [|] |

Comments and Notes:

RAM Checksum

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CONFIGURATION PHASE FLAGS

Cabinet
332
Configuration
CALTRANS

Phases (2-1-1-1)	
Permitted	. 2 . 4 5 6 ..
Restricted

Phase Features (2-1-1-4)	
Double Entry
Rest In Walk
Rest In Red
Walk 2
Max Green 2
Max Green 3

Startup (2-1-1-5)	
First Green Phases	. 2 ... 6 ..
Yellow Start Phases
Vehicle Calls	. 2 . 4 5 6 ..
Pedestrian Calls
Yellow Start Overlaps
Startup All-Red	6.0

Phase Recalls (2-1-1-2)	
Vehicle Min	. 2 ... 6 ..
Vehicle Max
Pedestrian
Bicycle

Phase Locks (2-1-1-3)	
Red
Yellow
Force/Max

Call To Phase (2-1-2-1)		Omit On Green	
1	1
2	2
3	3
4	4
5	5
6	6
7	7
8	8

Flashing Colors (2-1-2-2)	
Yellow Flash Phases
Yellow Flash Overlap
Flash In Red Phases
Flash In Red Overlap

Special Operation (2-1-2-3)	
Single Exit Phase
Driveway Signal Phases
Driveway Signal Overlaps
Leading Ped Phases

Protected Permissive (2-1-2-4)	
Protected Permissive

Pedestrian (2-1-3)	
P1
P2
P3
P4
P5
P6
P7
P8

Overlap (2-1-4)				
Overlap	Parent	Omit	No Start	Not
A
B
C
D
E
F

P
H
A
S
E

T
I
M
I
N
G

Phase (2-2)	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
--- Walk 1 ---	0	0	0	0	0	0	0	0
Flash Don't Walk	0	0	0	0	0	0	0	0
Minimum Green	0	5	0	5	5	5	0	0
Det Limit	0	0	0	0	0	0	0	0
Max Initial	0	0	0	0	0	0	0	0
Max Green 1	0	35	0	35	25	35	0	0
Max Green 2	0	0	0	0	0	0	0	0
Max Green 3	0	0	0	0	0	0	0	0
Extension	0.0	2.0	0.0	2.0	2.0	2.0	0.0	0.0
Maximum Gap	0.0	2.0	0.0	2.0	2.0	2.0	0.0	0.0
Minimum Gap	0.0	2.0	0.0	2.0	2.0	2.0	0.0	0.0
Add Per Vehicle	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Reduce Gap By	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Reduce Every	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	3.0	4.3	3.0	4.3	4.3	4.3	3.0	3.0
All-Red	0.0	1.0	0.0	1.0	1.0	1.0	0.0	0.0
Ped/Bike (2-3)	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
--- Walk 2 ---	0	0	0	0	0	0	0	0
Delay/Early Walk	0	0	0	0	0	0	0	0
Solid Don't Walk	0	0	0	0	0	0	0	0
Bike Green	0	0	0	0	0	0	0	0
Bike All-Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

OVERLAP TIMING

Overlap (2-4)	A	B	C	D	E	F
Green	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	5.0	5.0	5.0	5.0	5.0	5.0
Red	0.0	0.0	0.0	0.0	0.0	0.0

Red Revert

Red Revert (2-5)	
Time	5.0
All-Red Sec/Min (2-6)	
All-Red Sec/Min:	OFF

Max 2 Extension

Max/Gap Out (2-7)	
Max Cnt	0
Gap Cnt	0

Local Plan 1...9 (7-1) TIMING DATA

COORDINATION

		[Offsets]			Green Factors or Press [F] to Select Force-Off											
		Cycle	Multi	Lag Gap	A	B	C	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-	
Plan 1	Green Factor														
Plan 2	Green Factor														
Plan 3	Green Factor														
Plan 4	Green Factor														
Plan 5	Green Factor														
Plan 6	Green Factor														
Plan 7	Green Factor														
Plan 8	Green Factor														
Plan 9	Green Factor														

Master Timer Sync (7-A)	
Enable in Plans	
1-9
11-19
21-29

Master Sub Master	
Input	
Output	

FREE PLAN PHASE FLAGS	
(7-E) Free	
Lag	Omit
. 2 . 4 . 6 . 8
Veh Min	Veh Max
. 2 ... 6
Ped	Bike
.....
Cond	Cond Grn
.....	10

Local Plan 1...9 (7-1) PHASE FLAGS

	Lag	Sync	Hold	Omit	Veh Min	Veh Max	Ped	Bike
Plan 1
Plan 2
Plan 3
Plan 4
Plan 5
Plan 6
Plan 7
Plan 8
Plan 9

MANUAL COMMANDS	
Manual Plan (4-1)	
Plan	OffSet
	A
Plan: 1-9 15 or 254 = Flash 14 or 255 = Free Offset A, B, or C	

Special Function Override (4-2)			
#	Control	#	Control
1	NORMAL	3	NORMAL
2	NORMAL	4	NORMAL

Detector Reset	(4-3)
Local Manual (4-4)	OFF

Local Plan 11...19 (7-2) TIMING DATA

COORDINATION

[Offsets]

Green Factors or Press [F] to Select Force-Off

		Cycle	Multi	Lag Gap	A	B	C	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
Plan 11	Green Factor													
Plan 12	Green Factor													
Plan 13	Green Factor													
Plan 14	Green Factor													
Plan 15	Green Factor													
Plan 16	Green Factor													
Plan 17	Green Factor													
Plan 18	Green Factor													
Plan 19	Green Factor													

Local Plan 11...19 (7-2) PHASE FLAGS

	Lag	Sync	Hold	Omit	Veh Min	Veh Max	Ped	Bike
Plan 11
Plan 12
Plan 13
Plan 14
Plan 15
Plan 16
Plan 17
Plan 18
Plan 19

Local Plan 21...29 (7-3) TIMING DATA

COORDINATION

[Offsets]

Green Factors or Press [F] to Select Force-Off

		Cycle	Multi	Lag Gap	A	B	C	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
Plan 21	Green Factor													
Plan 22	Green Factor													
Plan 23	Green Factor													
Plan 24	Green Factor													
Plan 25	Green Factor													
Plan 26	Green Factor													
Plan 27	Green Factor													
Plan 28	Green Factor													
Plan 29	Green Factor													

Local Plan 21...29 (7-3) PHASE FLAGS

	Lag	Sync	Hold	Omit	Veh Min	Veh Max	Ped	Bike
Plan 21
Plan 22
Plan 23
Plan 24
Plan 25
Plan 26
Plan 27
Plan 28
Plan 29

DETECTORS

Detector Attributes (5-1)				Slot	Detector Configuration (5-2)				
Det	Type	Phases	Lock		Det	Delay	Extend	Recall	Port
1	COUNT+CALL+EXTEND	1.....	NO	I1U	1			10	3.2
2	COUNT+CALL+EXTEND	1.....	NO	I1L	2			10	7.2
3	COUNT+CALL+EXTEND	.2.....	NO	I2U	3			10	1.1
4	COUNT+CALL+EXTEND	.2.....	NO	I2L	4			10	1.5
5	COUNT+CALL+EXTEND	.2.....	NO	I3U	5			10	4.5
6	COUNT+CALL+EXTEND	.2.....	NO	I3L	6			10	6.2
7	COUNT+CALL+EXTEND	.2.....	NO	I4U	7			10	2.1
8	COUNT+CALL+EXTEND	.2.....	NO	I4L	8			10	7.4
9	COUNT+CALL+EXTEND	..3.....	NO	I5U	9			10	3.4
10	COUNT+CALL+EXTEND	..3.....	NO	I5L	10			10	7.6
11	COUNT+CALL+EXTEND	...4....	NO	I6U	11			10	1.3
12	COUNT+CALL+EXTEND	...4....	NO	I6L	12			10	1.7
13	COUNT+CALL+EXTEND	...4....	NO	I7U	13			10	4.7
14	COUNT+CALL+EXTEND	...4....	NO	I7L	14			10	6.4
15	COUNT+CALL+EXTEND	...4....	NO	I8U	15			10	2.3
16	COUNT+CALL+EXTEND	...4....	NO	I8L	16			10	7.8
17	COUNT+CALL+EXTEND	1.....	NO	I9U	17			10	3.6
18	COUNT+CALL+EXTEND	..3.....	NO	I9L	18			10	3.8
19	COUNT+CALL+EXTEND	.2.....	NO	I10U	19			10	4.1
20	COUNT+CALL+EXTEND	...4....	NO	I10L	20			10	4.2
21	COUNT+CALL+EXTEND	...5...	NO	J1U	21			10	3.1
22	COUNT+CALL+EXTEND	...5...	NO	J1L	22			10	7.1
23	COUNT+CALL+EXTEND6..	NO	J2U	23			10	1.2
24	COUNT+CALL+EXTEND6..	NO	J2L	24			10	1.6
25	COUNT+CALL+EXTEND6..	NO	J3U	25			10	4.6
26	COUNT+CALL+EXTEND6..	NO	J3L	26			10	6.3
27	COUNT+CALL+EXTEND6..	NO	J4U	27			10	2.2
28	COUNT+CALL+EXTEND6..	NO	J4L	28			10	7.3
29	COUNT+CALL+EXTEND7.	NO	J5U	29			10	3.3
30	COUNT+CALL+EXTEND7.	NO	J5L	30			10	7.5
31	COUNT+CALL+EXTEND8	NO	J6U	31			10	1.4
32	COUNT+CALL+EXTEND8	NO	J6L	32			10	1.8
33	COUNT+CALL+EXTEND8	NO	J7U	33			10	4.8
34	COUNT+CALL+EXTEND8	NO	J7L	34			10	6.5
35	COUNT+CALL+EXTEND8	NO	J8U	35			10	2.4
36	COUNT+CALL+EXTEND8	NO	J8L	36			10	7.7
37	COUNT+CALL+EXTEND	...5...	NO	J9U	37			10	3.5
38	COUNT+CALL+EXTEND	...7...	NO	J9L	38			10	3.7
39	COUNT+CALL+EXTEND6..	NO	J10U	39			10	4.3
40	COUNT+CALL+EXTEND8	NO	J10L	40			10	4.4
41	PEDESTRIAN	.2.....	NO	I12U	41			10	5.1
42	PEDESTRIAN	...4....	NO	I12L	42			10	5.3
43	PEDESTRIAN6..	NO	I13U	43			10	5.2
44	PEDESTRIAN8	NO	I13L	44			10	5.4

Failure Times(5-3)	Minutes
Maximum On Time	
Fail Reset Time	

Failure Override (5-4)	
Detectors 1-8
Detectors 9-16
Detectors 17-24
Detectors 25-32
Detectors 33-40
Detectors 41-44

System Detector Assignment (5-5)								
Sys Det	1	2	3	4	5	6	7	8
Det Nu								
Sys Det	9	10	11	12	13	14	15	16
Det Nu								

CIC Operation (5-6-1)	
Enable in Plans

CIC Values (5-6-2)	Volume	Occupancy	Demand
Smoothing	0.66	0.66	0.66
Multiplier	4.0	0.33	
Exponent	0.50	1.00	

Detector-to-Phase Assignment (5-6-3)								
Sys Det	1	2	3	4	5	6	7	8
Phase								
Sys Det	9	10	11	12	13	14	15	16
Phase								

Input File Port-Bit Assignments

332 Cabinet - For Reference Only

	1	2	3	4	5	6	7	8	9	10	11	12	13	14
I-	3.2	1.1	4.5	2.1	3.4	1.3	4.7	2.3	3.6	4.1	6.6	5.1	5.2	6.7
	7.2	1.5	6.2	7.4	7.6	1.7	6.4	7.8	3.8	4.2	2.7	5.3	5.4	6.8
J-	3.1	1.2	4.6	2.2	3.3	1.4	4.8	2.4	3.5	4.3	2.8	5.5	5.6	2.5
	7.1	1.6	6.3	7.3	7.5	1.8	6.5	7.7	3.7	4.4	6.1	5.7	5.8	2.6

TOD SCHEDULE

Table 1 (8-2-1)			Table 2 (8-2-2)			Table 3 (8-2-3)			Table 4 (8-2-4)			Table 5 (8-2-5)			Table 6 (8-2-6)		
Time	Plan	OS	Time	Plan	OS	Time	Plan	OS	Time	Plan	OS	Time	Plan	OS	Time	Plan	OS
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A
		A			A			A			A			A			A

WEEKDAY ASSIGNMENT

Weekday Table Assignments (8-2-7)						
Mon	Tue	Wed	Thu	Fri	Sat	Sun
1	1	1	1	1	2	2

HOLIDAY TABLES

Floating Holiday Table (8-2-8)				
#	Mnth	Week	DOW	Table
1			
2			
3			
4			
5			
6			
7			
8			
9			
10			
11			
12			
13			
14			
15			
16			

Fixed Holiday Table (8-2-9)				
#	Mnth	Day	DOW	Table
1			
2			
3			
4			
5			
6			
7			
8			
9			
10			
11			
12			
13			
14			
15			
16			

Solar Clock Data (8-4)	
North Latitude	34
West Longitude	118
Local Time Zone	8

Sabbatical Clock (8-5)	
Hebrew	Ped Recall
Sabbath
Holiday

Daylight Saving (8-6)	
Enabled	YES

TOD FUNCTIONS

TOD Functions (8-3)					
#	Start	End	DOW	Action	Phases
1		
2		
3		
4		
5		
6		
7		
8		
9		
10		
11		
12		
13		
14		
15		
16		

- Action Codes:
- 0. None
 - 1. Permitted
 - 2. Restricted
 - 4. Veh Min Recall
 - 5. Veh Max Recall
 - 6. Ped Recall
 - 7. Bike Recall
 - 8. Red Lock
 - 9. Yellow Lock
 - 10. Force/Max Lock
 - 11. Double Entry
 - 12. Y-Coord C
 - 13. Y-Coord D
 - 14. Free
 - 15. Flashing
 - 16. Walk 2
 - 17. Max Green 2

- 18. Max Green 3
- 19. Rest in Walk
- 20. Rest in Red
- 21. Free Lag Phases
- 22. Special Functions
- 23. Truck Preempt
- 24. Conditional Service
- 25. Conditional Service
- 26. Leading Ped
- 27. Traffic Actuated Max 2
- 41. Protected Permissive
- 42. Protected Permissive

Action Code = Phases added to normal setting
 100+Action Code = Phases removed
 200+Action Code = Phases replaced

COMMUNICATIONS

C2 (6-1-1)	
Address	
Protocol	AB3418
Access Level	0
Baud	1200
Parity	NONE
Data Bits	8
Stop Bits	1
RTS On Time	20
RTS Off Time	20
Handshaking	NORMAL

C20 (6-1-2)	
Address	
Protocol	AB3418
Access Level	0
Baud	1200
Parity	NONE
Data Bits	8
Stop Bits	1
RTS On Time	20
RTS Off Time	20
Handshaking	NORMAL

C21 (6-1-3)	
Address	
Protocol	AB3418
Access Level	0
Baud	1200
Parity	NONE
Data Bits	8
Stop Bits	1
RTS On Time	20
RTS Off Time	20
Handshaking	NORMAL

Access Levels:

- 0-Full Access
- 1-Status Only
- 2-Status, Set Pattern, Time
- 3-Status, Set Pattern, Time, Manual Plan
- 4-Reserved
- 5-Full Access with No Set Pattern
- 6-Full Access with No Set Time
- 7-Full Access with No Set Pattern, Manual Plan
- 8-Full Access with No Set Time, Pattern, Manual Plan

SOFT LOGIC

Soft Logic (6-2)							
#	Data	OP	Data	OP	Data	OP	Data
1							
2							
3							
4							
5							
6							
7							
8							
9							
10							
11							
12							
13							
14							
15							
16							

*Refer to User's Manual for Data and OP Codes

CALLBACK NUMBERS

Callback Numbers (6-3...3)	
Line Out	
Local Toll	
Long Distance	
Delay	10
Area Code	
Phone Number	
Line Out	
Local Toll	
Long Distance	
Delay	10
Area Code	
Phone Number	
Line Out	
Local Toll	
Long Distance	
Delay	10
Area Code	
Phone Number	

NETWORK

Network (6-4)	
Address	1
Protocol	AB3418
Port	27001
Type	STATIC
Central Access	6
Field Access	7

IP Address	172	.	25	.	52	.	7
Netmask	255	.	255	.	255	.	192
Broadcast	172	.	25	.	52	.	255
Gateway	172	.	25	.	52	.	1

RAILROAD PREEMPTION

RR 1	(3-1-1)	Timing	Phase Flags (3-1-2)			Pedestrian Flags (3-1-3)			Overlap Flags (3-1-4)		
	Delay		Grn Hold	Yel Flash	Red Flash	Walk	Flash DW	Solid DW	Grn Hold	Yel Flash	Red Flash
	Clear 1	10	. 2 . . 5 2 . 4 . 6 . 8
	Clear 2	
	Clear 3	
	Hold		1 2 3 4 5 6 7 8	A B C D E F
	Exit		Exit Parameters (3-1-5)				Configuration (3-1-6)				
Min Grn		Phase Green	Overlap Green	Vehicle Call	Ped Call	Primary Port	Secondary Port	Latching	Power-Up		
Ped Clr		1 2 3 4 5 6 7 8	. 2 . 4 . 6 . 8	2.5	0.0	YES	FLASHING		

RR 2	(3-2-1)	Timing	Phase Flags (3-2-2)			Pedestrian Flags (3-2-3)			Overlap Flags (3-2-4)		
	Delay		Grn Hold	Yel Flash	Red Flash	Walk	Flash DW	Solid DW	Grn Hold	Yel Flash	Red Flash
	Clear 1	10	. . . 4 . . 7 2 . 4 . 6 . 8
	Clear 2	
	Clear 3	
	Hold		1 2 3 . . 6 2 . . . 6 4 . . . 8
	Exit		Exit Parameters (3-2-5)				Configuration (3-2-6)				
Min Grn		Phase Green	Overlap Green	Vehicle Call	Ped Call	Primary Port	Secondary Port	Latching	Power-up		
Ped Clr	 4 . . 7	2.6	0.0	YES	DARK		

EMERGENCY VEHICLE PREEMPTION

EVA (3-A)	Preempt Timers			Phase Green	Overlap Green
	Delay	Clear	Max		
		30	30	. 2 . . 5
	Port	Latching	Phase Termination		
	5.5	NO	ADVANCE		

EVB (3-B)	Preempt Timers			Phase Green	Overlap Green
	Delay	Clear	Max		
		30	30	. . . 4 . . 7
	Port	Latching	Phase Termination		
	5.6	NO	ADVANCE		

EVC (3-C)	Preempt Timers			Phase Green	Overlap Green
	Delay	Clear	Max		
		30	30	1 6
	Port	Latching	Phase Termination		
	5.7	NO	ADVANCE		

EVD (3-D)	Preempt Timers			Phase Green	Overlap Green
	Delay	Clear	Max		
		30	30	. . 3 8
	Port	Latching	Phase Termination		
	5.8	NO	ADVANCE		

INPUTS

7 Wire I/C (2-1-5-1)					
		Input	Port	Input	Port
Enable	NO	R1	3.8	Free	3.6
Max ON		R2	3.5	D2	2.8
Max OFF		R3	3.7	D3	6.1

Manual Control (2-1-5-2)	
Input	Port
Manual Advance	
Advance Enable	

Enable	NO	R1	3.8	Free	3.6
Max ON		R2	3.5	D2	2.8
Max OFF		R3	3.7	D3	6.1

Battery Backup (2-1-5-5)	
Port	Operation
2.7	FLASHING

Cabinet Status (2-1-5-3)	
Input	Port
Flash Bus	
Door Ajar	
Flash Sense	6.7
Stop Time	6.8

Special Function (2-1-5-4)	
Input	Port
1	
2	
3	
4	

Y-Coordination (2-1-5-6)	
Port C	Port D
6.1	2.8

OUTPUTS

Loadswitch Assignments (2-1-6)								+
A	1	2	22	3	4	24	9	
B	5	6	26	7	8	28	10	
X	13	14	0	11	12	0	0	

- Loadswitch Codes:
- 0 Unused (no output)
 - 1-8 Vehicle 1-8
 - 9-14 Overlap A-F
 - 21-28 Ped 1-8
 - 41-47 Special Functions
 - 41 Protected Permissive Flashing Phase 1
 - 43 Protected Permissive Flashing Phase 3
 - 45 Protected Permissive Flashing Phase 5
 - 47 Protected Permissive Flashing Phase 7

- 51-57 Special Functions
- 71-72 Seven Wire I/C

+ middle output of loadswitches 3 and 6 Channel 9 and 10

TRANSIT PRIORITY

Local Plans (3-E) 1...9 11...19		Early Green	Green Extend	Inhibit Cycles	Phase 1 Minimum	Phase 2 Minimum	Phase 3 Minimum	Phase 4 Minimum	Phase 5 Minimum	Phase 6 Minimum	Phase 7 Minimum	Phase 8 Minimum
Plan 1	Green Factor											
Plan 2	Green Factor											
Plan 3	Green Factor											
Plan 4	Green Factor											
Plan 5	Green Factor											
Plan 6	Green Factor											
Plan 7	Green Factor											
Plan 8	Green Factor											
Plan 9	Green Factor											
Plan 11	Green Factor											
Plan 12	Green Factor											
Plan 13	Green Factor											
Plan 14	Green Factor											
Plan 15	Green Factor											
Plan 16	Green Factor											
Plan 17	Green Factor											
Plan 18	Green Factor											
Plan 19	Green Factor											

Transit Priority Configuration (3-E-A)		Indicator Output			
Enable in Plans	Input	Type	Stop	Go	
Plan 1-9	0.0	OPT	0	0
Plan 11-19	0.0	OPT	0	0

Queue Jump (3-E-B)	
Grn Hold	Hold Phase

Free Plans (3-E-E)	
Max Grn Hold	Hold Phase

Access Utilities (9-5)	
Password	***
Timeout	30

YELLOW YIELD COORDINATION

Y-Coord Plans (7-C,D)	Long Grn	No Grn	Offset	Perm	Force-Offs								Coord	Lag	Min Recall	Restricted
					-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-				
Plan C													. 2 . . . 6 . .	. 2 . 4 . 6 . 8
Plan D													. 2 . . . 6 . .	. 2 . 4 . 6 . 8

TRUCK PRIORITY

Truck Priority (3-F)	Passage	CarryOver	Clearance	Next Priority	Phase Green	Det 2 Port	Det 3 Port	Det 4 Port	Sign Output	Slave Input	Slave Output
					0.0	0.0	0.0	0	0.0	0

Location: SR 74 @ TRUMBLE ROAD

Designed By:

System: ISOLATED

District: 08-RIVERSIDE

Installed By: SAFWAN SAYED

Master At:

I/C:

Service Info:

Timing Change:
3/23/2010

Date Start:
5/23/1995

Date End:

Designed:

Installed:
3/23/2010

Intersection Layout

FLASH

- 1) []
- P 2) E/B SR 74 []
- H 3) []
- A 4) S/B TRUMBLE ROAD []
- S 5) N/B SR 74 - LEFT TURN []
- E 6) W/B SR 74 []
- 7) []
- 8) []

- O A) []
- V B) []
- E C) []
- R D) []
- L E) []
- A F) []
- P

Comments and Notes:

RAM Checksum

Page 2: F8D8	Page 7: D2FD
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Page 6: 85AF	Page 11: C381

CONFIGURATION PHASE FLAGS

Phases (2-1-1-1) *	
Permitted	. 2 . 4 5 6 ..
Restricted

Phase Recalls (2-1-1-2)	
Vehicle Min	. 2 ... 6 ..
Vehicle Max
Pedestrian
Bicycle

Phase Locks (2-1-1-3) *	
Red
Yellow
Force/Max

Phase Features (2-1-1-4)	
Double Entry
Rest In Walk
Rest In Red
Walk 2
Max Green 2
Max Green 3

Startup (2-1-1-5) *	
First Green Phases	... 4 ...
Yellow Start Phases	. 2 ... 6 ..
Yellow Start Overlaps
Startup All-Red	5.0
Vehicle Calls	. 2 . 4 5 6 ..
Pedestrian Calls	... 4 . 6 ..

Call To Phase (2-1-2-1)		Omit On Green	
1	1
2	2
3	3
4	4
5	5
6	6
7	7
8	8

Flashing Colors (2-1-2-2)	
Yellow Flash Phases
Yellow Flash Overlap
Flash In Red Phases
Flash In Red Overlap

Special Operation (2-1-2-3)	
Single Exit Phase
Driveway Signal Phases
Driveway Signal Overlaps
Leading Ped Phases

Protected Permissive (2-1-2-4)	
Protected Permissive

Pedestrian (2-1-3) *	
P1
P2
P3
P4	... 4 ...
P5
P6 6 ..
P7
P8

Overlap (2-1-4)				
Overlap	Parent	Omit	No Start	Not
A
B
C
D
E
F

PHASE TIMING

Phase (2-2)	-1- *	-2- *	-3- *	-4- *	-5- *	-6- *	-7- *	-8- *
--- Walk 1 ---	0	0	0	7	0	7	0	0
Flash Don't Walk	0	0	0	19	0	22	0	0
Minimum Green	0	5	0	5	5	5	0	0
Det Limit	0	0	0	0	0	0	0	0
Max Initial	0	0	0	0	0	0	0	0
Max Green 1	0	35	0	20	20	35	0	0
Max Green 2	0	0	0	0	0	0	0	0
Max Green 3	0	0	0	0	0	0	0	0
Extension	0.0	3.0	0.0	2.0	2.0	3.0	0.0	0.0
Maximum Gap	0.0	3.0	0.0	2.0	2.0	3.0	0.0	0.0
Minimum Gap	0.0	3.0	0.0	2.0	2.0	3.0	0.0	0.0
Add Per Vehicle	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Reduce Gap By	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Reduce Every	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	3.0	5.0	3.0	3.5	3.0	5.0	3.0	3.0
All-Red	0.0	1.0	0.0	1.0	1.0	1.0	0.0	0.0
Ped/Bike (2-3)	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
--- Walk 2 ---	0	0	0	0	0	0	0	0
Delay/Early Walk	0	0	0	0	0	0	0	0
Solid Don't Walk	0	0	0	0	0	0	0	0
Bike Green	0	0	0	0	0	0	0	0
Bike All-Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

OVERLAP TIMING

Overlap (2-4)	A	B	C	D	E	F
Green	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	5.0	5.0	5.0	5.0	5.0	5.0
Red	0.0	0.0	0.0	0.0	0.0	0.0

Red Revert

Red Revert (2-5)	
Time	5.0
Red To Sec (2-6)	
Red To Sec	OFF

COORDINATION

Local Plan (7-1...9) TIMING DATA [Offsets] Green Factors or Press [F] to Select Force-Off

		Cycle	Multi	Perm	A	B	C	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
Plan 1	Green Factor														
Plan 2	Green Factor														
Plan 3	Green Factor														
Plan 4	Green Factor														
Plan 5	Green Factor														
Plan 6	Green Factor														
Plan 7	Green Factor														
Plan 8	Green Factor														
Plan 9	Green Factor														

Master Timer Sync (7-A)	
Enable in Plans	
.....	

Master Sub Master	
Input	
Output	

FREE PLAN PHASE FLAGS

(7-E) Free	
Lag	Omit
. 2 . 4 . 6 . 8
Veh Min	Veh Max
. 2 ... 6
Ped	Bike
.....
Cond	Cond Grn
.....	10

Local Plan (7-1...9) PHASE FLAGS

	Lag	Sync	Hold	Omit	Veh Min	Veh Max	Ped	Bike
Plan 1
Plan 2
Plan 3
Plan 4
Plan 5
Plan 6
Plan 7
Plan 8
Plan 9

MANUAL COMMANDS

Manual Plan (4-1)		<i>Plan: 1-9</i>
Plan	OffSet	<i>15 or 254 = Flash</i>
	A	<i>14 or 255 = Free</i>
		<i>Offset A, B, or C</i>

Special Function Override (4-2)			
#	Control	#	Control
1	NORMAL	3	NORMAL
2	NORMAL	4	NORMAL

Detector Reset	(4-3)
Local Manual (4-4)	OFF

DETECTORS

Detector Attributes (5-1) *				Slot	Detector Configuration (5-2)				
Det	Type	Phases	Lock		Det	Delay	Extend	Recall	Port
1	COUNT+CALL+EXTEND	.2	NO	I2U	1			10	1.1
2	COUNT+CALL+EXTEND 6 . .	NO	J2U	2			10	1.2
3	COUNT+CALL+EXTEND	. . . 4	NO	I6U	3			10	1.3
4	COUNT+CALL+EXTEND 8	NO	J6U	4			10	1.4
5	COUNT+CALL+EXTEND	. 2	NO	I2L	5			10	1.5
6	COUNT+CALL+EXTEND 6 . .	NO	J2L	6			10	1.6
7	COUNT+CALL+EXTEND	. . . 4	NO	I6L	7			10	1.7
8	COUNT+CALL+EXTEND 8	NO	J6L	8			10	1.8
9	COUNT+CALL+EXTEND	. 2	NO	I4	9			10	2.1
10	COUNT+CALL+EXTEND 6 . .	NO	J4	10			10	2.2
11	COUNT+CALL+EXTEND	. . . 4	NO	I8	11			10	2.3
12	COUNT+CALL+EXTEND 8	NO	J8	12			10	2.4
13	COUNT+CALL+EXTEND 5 . . .	NO	J1	13			10	3.1
14	COUNT+CALL+EXTEND	1	NO	I1	14			10	3.2
15	COUNT+CALL+EXTEND 7 . .	NO	J5	15			10	3.3
16	COUNT+CALL+EXTEND	. . 3	NO	I5	16			10	3.4
17	COUNT+CALL+EXTEND 5 . . .	NO	J9U	17			10	3.5
18	COUNT+CALL+EXTEND	1	NO	I9U	18			10	3.6
19	COUNT+CALL+EXTEND 7 . .	NO	J9L	19			10	3.7
20	COUNT+CALL+EXTEND	. . 3	NO	I9L	20			10	3.8
21	COUNT+CALL+EXTEND	. 2	NO	I3L	21			10	6.2
22	COUNT+CALL+EXTEND 6 . .	NO	J3L	22			10	6.3
23	COUNT+CALL+EXTEND	. . . 4	NO	I7L	23			10	6.4
24	COUNT+CALL+EXTEND 8	NO	J7L	24			10	6.5
25	COUNT+CALL+EXTEND	. 2	NO	I3U	25			10	4.5
26	COUNT+CALL+EXTEND 6 . .	NO	J3U	26			10	4.6
27	COUNT+CALL+EXTEND	. . . 4	NO	I7U	27			10	4.7
28	COUNT+CALL+EXTEND 8	NO	J7U	28			10	4.8
29	PEDESTRIAN	. 2	NO	I 12U	29			10	5.1
30	PEDESTRIAN 6 . .	NO	I 13U	30			10	5.2
31	PEDESTRIAN	. . . 4	NO	I 12L	31			10	5.3
32	PEDESTRIAN 8	NO	I 13L	32			10	5.4

Failure Times(5-3)	Minutes
Maximum On Time	
Fail Reset Time	

Failure Override (5-4)	
Detectors 1-8
Detectors 9-16
Detectors 17-24
Detectors 25-32

System Detector Assignment (5-5)								
Sys Det	1	2	3	4	5	6	7	8
Det Num								
Sys Det	9	10	11	12	13	14	15	16
Det Num								

CIC Operation (5-6-1)	
Enable in Plans

CIC Values (5-6-2)	Volume	Occupancy	Demand
Smoothing	0.66	0.66	0.66
Multiplier	4.0	0.33	
Exponent	0.50	1.00	

Detector-to-Phase Assignment (5-6-3)								
Sys Det	1	2	3	4	5	6	7	8
Phase								
Sys Det	9	10	11	12	13	14	15	16
Phase								

Input File Port-Bit Assignments

332 Cabinet - For Reference Only

	1	2	3	4	5	6	7	8	9	10	11	12	13	14
I-	3.2	1.1	4.5	2.1	3.4	1.3	4.7	2.3	3.6		6.6	5.1	5.2	6.7
		1.5	6.2			1.7	6.4		3.8		2.7	5.3	5.4	6.8
J-	3.1	1.2	4.6	2.2	3.3	1.4	4.8	2.4	3.5		2.8	5.5	5.6	2.5
		1.6	6.3			1.8	6.5		3.7		6.1	5.7	5.8	2.6

HOLIDAY TABLES

Floating Holiday Table (8-2-8)				
#	Mnth	Week	DOW	Table
1			
2			
3			
4			
5			
6			
7			
8			
9			
10			
11			
12			
13			
14			
15			
16			

Fixed Holiday Table (8-2-9)				
#	Mnth	Day	DOW	Table
1			
2			
3			
4			
5			
6			
7			
8			
9			
10			
11			
12			
13			
14			
15			
16			

Solar Clock Data (8-4)	
North Latitude	34
West Longitude	118
Local Time Zone	8

Sabbatical Clock (8-5)	
Hebrew	Ped Recall
Sabbath
Holiday

Daylight Saving (8-6)	
Enabled	YES

TOD FUNCTIONS

TOD Functions (8-3)					
#	Start	End	DOW	Action	Phases
1		
2		
3		
4		
5		
6		
7		
8		
9		
10		
11		
12		
13		
14		
15		
16		

- Action Codes:**
- 0. None
 - 1. Permitted
 - 2. Restricted
 - 4. Veh Min Recall
 - 5. Veh Max Recall
 - 6. Ped Recall
 - 7. Bike Recall
 - 8. Red Lock
 - 9. Yellow Lock
 - 10. Force/Max Lock
 - 11. Double Entry
 - 12. Y-Coord C
 - 13. Y-Coord D
 - 14. Free
 - 15. Flashing
 - 16. Walk 2
 - 17. Max Green 2

- 18. Max Green 3
- 19. Rest in Walk
- 20. Rest in Red
- 21. Free Lag Phases
- 22. Special Functions
- 23. Truck Preempt
- 24. Conditional Service
- 25. Conditional Service
- 26. Leading Ped
- 41. Protected Permissive
- 42. Protected Permissive

Action Code = Phases added to normal setting
 100+Action Code = Phases removed
 200+Action Code = Phases replaced

COMMUNICATIONS

C2 (6-1-1)	
Address	
Protocol	AB3418
Limit Access	
Baud	1200
Parity	NONE
Data Bits	8
Stop Bits	1
RTS On Time	20
RTS Off Time	20
Handshaking	NORMAL

C20 (6-1-2)	
Address	
Protocol	AB3418
Limit Access	
Baud	1200
Parity	NONE
Data Bits	8
Stop Bits	1
RTS On Time	20
RTS Off Time	20
Handshaking	NORMAL

C21 (6-1-3)	
Address	
Protocol	AB3418
Limit Access	
Baud	1200
Parity	NONE
Data Bits	8
Stop Bits	1
RTS On Time	20
RTS Off Time	20
Handshaking	NORMAL

Limit Access:

0-None

1-Status Only

2-Status, Set Pattern, Time

3-Status, Set Pattern, Time, Manual Plan

SOFT LOGIC

Soft Logic (6-2)							
#	Data	OP	Data	OP	Data	OP	Data
1							
2							
3							
4							
5							
6							
7							
8							
9							
10							
11							
12							
13							
14							
15							
16							

*Refer to User's Manual for Data and OP Codes

CALLBACK NUMBERS

Callback Numbers (6-3...3)	
Line Out	
Local Toll	
Long Distance	
Delay	10
Area Code	
Phone Number	

Line Out	
Local Toll	
Long Distance	
Delay	10
Area Code	
Phone Number	

Line Out	
Local Toll	
Long Distance	
Delay	10
Area Code	
Phone Number	

RAILROAD PREEMPTION

RR 1	(3-1-1)	Timing	Phase Flags (3-1-2)			Pedestrian Flags (3-1-3)			Overlap Flags (3-1-4)		
	Delay		Grn Hold	Yel Flash	Red Flash	Walk	Flash DW	Solid DW	Grn Hold	Yel Flash	Red Flash
	Clear1	10	. 2 . . 5 2 . 4 . 6 . 8
	Clear 2	
	Clear 3	
	Hold		1 2 3 4 5 6 7 8	A B C D E F
	Exit	5	Exit Parameters (3-1-5)				Configuration (3-1-6)				
	Min Grn		Phase Green	Overlap Green	Vehicle Recall	Ped Call	Port	Latching	Power-Up		
	Ped Clr		1 2 3 4 5 6 7 8	. 2 . 4 . 6 . 8	2.5	YES	FLASHING		

RR 2	(3-2-1)	Timing	Phase Flags (3-2-2)			Pedestrian Flags (3-2-3)			Overlap Flags (3-2-4)		
	Delay		Grn Hold	Yel Flash	Red Flash	Walk	Flash DW	Solid DW	Grn Hold	Yel Flash	Red Flash
	Clear1	10	. . . 4 . . 7 2 . 4 . 6 . 8
	Clear 2	
	Clear 3	
	Hold		1 2 3 . . 6 2 . . . 6 4 . . . 8
	Exit		Exit Parameters (3-2-5)				Configuration (3-2-6)				
	Min Grn		Phase Green	Overlap Green	Vehicle Recall	Ped Recall	Port	Latching	Power-up		
	Ped Clr	 4 . . 7	2.6	YES	DARK		

EMERGENCY VEHICLE PREEMPTION

EVA (3-A)	Preempt Timers			Phase Green	Overlap Green
	Delay	Clear	Max		
		30	30	. 2 . . 5
	Port	Latching	Phase Termination		
	5.5	NO	ADVANCE		

EVB (3-B)	Preempt Timers			Phase Green	Overlap Green
	Delay	Clear	Max		
		30	30	. . . 4 . . 7
	Port	Latching	Phase Termination		
	5.6	NO	ADVANCE		

EVC (3-C)	Preempt Timers			Phase Green	Overlap Green
	Delay	Clear	Max		
		30	30	1 6
	Port	Latching	Phase Termination		
	5.7	NO	ADVANCE		

EVD (3-D)	Preempt Timers			Phase Green	Overlap Green
	Delay	Clear	Max		
		30	30	. . 3 8
	Port	Latching	Phase Termination		
	5.8	NO	ADVANCE		

INPUTS

		7 Wire I/C (2-1-5-1)			
		Input	Port	Input	Port
Enable	NO	R1	3.8	Free	3.6
Max ON		R2	3.5	D2	2.8
Max OFF		R3	3.7	D3	6.1

Manual Control (2-1-5-2)	
Input	Port
Manual Advance	6.6
Advance Enable	6.6

Battery Backup (2-1-5-5) *	
Port	Operation
2.7	NORMAL

Y-Coordination (2-1-5-6)	
Port C	Port D
6.1	2.8

Cabinet Status (2-1-5-3)	
Input	Port
Flash Bus	
Door Ajar	
Flash Sense	6.7
Stop Time	6.8

Special Function (2-1-5-4)	
Input	Port
1	
2	
3	
4	

OUTPUTS

Loadswitch Assignments (2-1-6)								+
A	1	2	22	3	4	24	9	
B	5	6	26	7	8	28	10	
X	13	14	0	11	12	0	0	

Loadswitch Codes:

- 0 Unused (no output)
- 1-8 Vehicle 1-8
- 9-14 Overlap A-F
- 21-28 Ped 1-8
- 41-47 Special Functions
- 41 Protected Permissive Flashing Phase 1
- 43 Protected Permissive Flashing Phase 3
- 45 Protected Permissive Flashing Phase 5
- 47 Protected Permissive Flashing Phase 7

51-57 Special Functions

71-72 Seven Wire I/C

+ middle output of loadswitches 3 and 6 Channel 9 and 10

YELLOW YIELD COORDINATION

Y-Coord Plans (7-C,D)	Long Grn	No Grn	Offset	Perm	Force-Offs								Coord	Lag	Min Recall	Restricted
					-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-				
Plan C													. 2 . . . 6 . .	. 2 . 4 . 6 . 8
Plan D													. 2 . . . 6 . .	. 2 . 4 . 6 . 8

TRANSIT PRIORITY

Local Plans (3-E1...9)		Early Green	Green Extend	Inhibit Cycles	Phase 1 Minimum	Phase 2 Minimum	Phase 3 Minimum	Phase 4 Minimum	Phase 5 Minimum	Phase 6 Minimum	Phase 7 Minimum	Phase 8 Minimum
Plan 1	Green Factor											
Plan 2	Green Factor											
Plan 3	Green Factor											
Plan 4	Green Factor											
Plan 5	Green Factor											
Plan 6	Green Factor											
Plan 7	Green Factor											
Plan 8	Green Factor											
Plan 9	Green Factor											

Enable Priority (3-E-A)	
Enable in Plan

Free Plans (3-E-E)	
Max Green Hold	Hold Phase

Access Utilities (9-5)	
Password	***
Timeout	

TRUCK PREEMPTION

Truck Preemption (3-F)	Passage	CarryOver	Clearance	Next Preempt	Phase Green	Det 2 Port	Det 3 Port	Det 4 Port	Sign Output	Slave Input	Slave Output

APPENDIX C:

VOLUME DEVELOPMENT WORKSHEETS

Table C-1 - Existing Peak Hour PCE Volume Summary

	AM Peak Hour			PM Peak Hour		
	Existing Without Project	Project Trips	Existing With Project	Existing Without Project	Project Trips	Existing With Project
1 I-215 Southbound Ramps/Bonnie Drive						
NBL	233	1	234	250	2	252
NBT	493	12	505	297	22	319
NBR	0	0	0	0	0	0
SBL	0	0	0	0	0	0
SBT	599	22	621	774	16	790
SBR	25	0	25	23	0	23
EBL	28	0	28	32	0	32
EBT	0	0	0	0	0	0
EBR	159	2	161	269	1	270
WBL	0	0	0	0	0	0
WBT	0	0	0	0	0	0
WBR	0	0	0	0	0	0
North Leg						
Approach	624	22	646	797	16	813
Departure	521	12	533	329	22	351
Total	1,145	34	1,179	1,126	38	1,164
South Leg						
Approach	726	13	739	547	24	571
Departure	758	24	782	1,043	17	1,060
Total	1,484	37	1,521	1,590	41	1,631
East Leg						
Approach	0	0	0	0	0	0
Departure	0	0	0	0	0	0
Total	0	0	0	0	0	0
West Leg						
Approach	187	2	189	301	1	302
Departure	258	1	259	273	2	275
Total	445	3	448	574	3	577
Total Approaches						
Approach	1,537	37	1,574	1,645	41	1,686
Departure	1,537	37	1,574	1,645	41	1,686
Total	3,074	74	3,148	3,290	82	3,372

Table C-1 - Existing Peak Hour PCE Volume Summary

	AM Peak Hour			PM Peak Hour		
	Existing Without Project	Project Trips	Existing With Project	Existing Without Project	Project Trips	Existing With Project
2 I-215 Northbound Ramps/CA-74						
NBL	0	0	0	0	0	0
NBT	0	0	0	0	0	0
NBR	0	0	0	0	0	0
SBL	170	24	194	231	17	248
SBT	0	0	0	0	0	0
SBR	16	0	16	39	0	39
EBL	14	0	14	14	0	14
EBT	744	24	768	1,029	17	1,046
EBR	0	0	0	0	0	0
WBL	0	0	0	0	0	0
WBT	710	13	723	508	24	532
WBR	784	12	796	713	21	734
North Leg						
Approach	186	24	210	270	17	287
Departure	798	12	810	727	21	748
Total	984	36	1,020	997	38	1,035
South Leg						
Approach	0	0	0	0	0	0
Departure	0	0	0	0	0	0
Total	0	0	0	0	0	0
East Leg						
Approach	1,494	25	1,519	1,221	45	1,266
Departure	914	48	962	1,260	34	1,294
Total	2,408	73	2,481	2,481	79	2,560
West Leg						
Approach	758	24	782	1,043	17	1,060
Departure	726	13	739	547	24	571
Total	1,484	37	1,521	1,590	41	1,631
Total Approaches						
Approach	2,438	73	2,511	2,534	79	2,613
Departure	2,438	73	2,511	2,534	79	2,613
Total	4,876	146	5,022	5,068	158	5,226

Table C-1 - Existing Peak Hour PCE Volume Summary

	AM Peak Hour			PM Peak Hour		
	Existing Without Project	Project Trips	Existing With Project	Existing Without Project	Project Trips	Existing With Project
3 Trumble Road/Mapes Road						
NBL	55	8	63	1	4	5
NBT	31	0	31	117	0	117
NBR	79	0	79	219	0	219
SBL	1	0	1	27	0	27
SBT	48	0	48	132	0	132
SBR	2	2	4	0	1	1
EBL	3	0	3	2	1	3
EBT	18	1	19	1	1	2
EBR	55	2	57	5	7	12
WBL	214	0	214	131	0	131
WBT	17	2	19	1	1	2
WBR	3	0	3	28	0	28
North Leg						
Approach	51	2	53	159	1	160
Departure	37	0	37	147	1	148
Total	88	2	90	306	2	308
South Leg						
Approach	165	8	173	337	4	341
Departure	317	2	319	268	7	275
Total	482	10	492	605	11	616
East Leg						
Approach	234	2	236	160	1	161
Departure	98	1	99	247	1	248
Total	332	3	335	407	2	409
West Leg						
Approach	76	3	79	8	9	17
Departure	74	12	86	2	6	8
Total	150	15	165	10	15	25
Total Approaches						
Approach	526	15	541	664	15	679
Departure	526	15	541	664	15	679
Total	1,052	30	1,082	1,328	30	1,358

Table C-1 - Existing Peak Hour PCE Volume Summary

	AM Peak Hour			PM Peak Hour		
	Existing Without Project	Project Trips	Existing With Project	Existing Without Project	Project Trips	Existing With Project
4 Trumble Road/Exceed Road-Sturgeon Electric Dwy						
NBL	5	25	30	2	22	24
NBT	164	29	193	336	15	351
NBR	9	0	9	1	0	1
SBL	0	0	0	1	0	1
SBT	315	7	322	267	25	292
SBR	2	0	2	0	0	0
EBL	0	0	0	0	0	0
EBT	0	0	0	0	0	0
EBR	1	20	21	5	26	31
WBL	54	0	54	28	0	28
WBT	1	0	1	0	0	0
WBR	1	0	1	1	0	1
North Leg						
Approach	317	7	324	268	25	293
Departure	165	29	194	337	15	352
Total	482	36	518	605	40	645
South Leg						
Approach	178	54	232	339	37	376
Departure	370	27	397	300	51	351
Total	548	81	629	639	88	727
East Leg						
Approach	56	0	56	29	0	29
Departure	9	0	9	2	0	2
Total	65	0	65	31	0	31
West Leg						
Approach	1	20	21	5	26	31
Departure	8	25	33	2	22	24
Total	9	45	54	7	48	55
Total Approaches						
Approach	552	81	633	641	88	729
Departure	552	81	633	641	88	729
Total	1,104	162	1,266	1,282	176	1,458

Table C-1 - Existing Peak Hour PCE Volume Summary

	AM Peak Hour			PM Peak Hour		
	Existing Without Project	Project Trips	Existing With Project	Existing Without Project	Project Trips	Existing With Project
5 Trumble Road/CA-74						
NBL	0	0	0	0	0	0
NBT	0	0	0	0	0	0
NBR	0	0	0	0	0	0
SBL	43	2	45	50	6	56
SBT	0	0	0	0	0	0
SBR	457	25	482	382	45	427
EBL	177	48	225	304	34	338
EBT	737	0	737	956	0	956
EBR	0	0	0	0	0	0
WBL	0	0	0	0	0	0
WBT	1,037	0	1,037	839	0	839
WBR	30	6	36	22	3	25
North Leg						
Approach	500	27	527	432	51	483
Departure	207	54	261	326	37	363
Total	707	81	788	758	88	846
South Leg						
Approach	0	0	0	0	0	0
Departure	0	0	0	0	0	0
Total	0	0	0	0	0	0
East Leg						
Approach	1,067	6	1,073	861	3	864
Departure	780	2	782	1,006	6	1,012
Total	1,847	8	1,855	1,867	9	1,876
West Leg						
Approach	914	48	962	1,260	34	1,294
Departure	1,494	25	1,519	1,221	45	1,266
Total	2,408	73	2,481	2,481	79	2,560
Total Approaches						
Approach	2,481	81	2,562	2,553	88	2,641
Departure	2,481	81	2,562	2,553	88	2,641
Total	4,962	162	5,124	5,106	176	5,282

Table C-1 - Existing Peak Hour PCE Volume Summary

	AM Peak Hour			PM Peak Hour		
	Existing Without Project	Project Trips	Existing With Project	Existing Without Project	Project Trips	Existing With Project
6 Project Driveway 1/Mapes Road						
NBL	0	0	0	0	0	0
NBT	0	0	0	0	0	0
NBR	0	3	3	0	9	9
SBL	0	0	0	0	0	0
SBT	0	0	0	0	0	0
SBR	0	0	0	0	0	0
EBL	0	0	0	0	0	0
EBT	0	0	0	0	0	0
EBR	0	0	0	0	0	0
WBL	0	12	12	0	6	6
WBT	0	0	0	0	0	0
WBR	0	0	0	0	0	0
North Leg						
Approach	0	0	0	0	0	0
Departure	0	0	0	0	0	0
Total	0	0	0	0	0	0
South Leg						
Approach	0	3	3	0	9	9
Departure	0	12	12	0	6	6
Total	0	15	15	0	15	15
East Leg						
Approach	0	12	12	0	6	6
Departure	0	3	3	0	9	9
Total	0	15	15	0	15	15
West Leg						
Approach	0	0	0	0	0	0
Departure	0	0	0	0	0	0
Total	0	0	0	0	0	0
Total Approaches						
Approach	0	15	15	0	15	15
Departure	0	15	15	0	15	15
Total	0	30	30	0	30	30

Table C-1 - Existing Peak Hour PCE Volume Summary

	AM Peak Hour			PM Peak Hour		
	Existing Without Project	Project Trips	Existing With Project	Existing Without Project	Project Trips	Existing With Project
7 Trumble Road/Project Driveway 2						
NBL	0	21	21	0	11	11
NBT	165	8	173	337	4	341
NBR	0	0	0	0	0	0
SBL	0	0	0	0	0	0
SBT	317	2	319	268	7	275
SBR	0	0	0	0	0	0
EBL	0	0	0	0	0	0
EBT	0	0	0	0	0	0
EBR	0	5	5	0	18	18
WBL	0	0	0	0	0	0
WBT	0	0	0	0	0	0
WBR	0	0	0	0	0	0
North Leg						
Approach	317	2	319	268	7	275
Departure	165	8	173	337	4	341
Total	482	10	492	605	11	616
South Leg						
Approach	165	29	194	337	15	352
Departure	317	7	324	268	25	293
Total	482	36	518	605	40	645
East Leg						
Approach	0	0	0	0	0	0
Departure	0	0	0	0	0	0
Total	0	0	0	0	0	0
West Leg						
Approach	0	5	5	0	18	18
Departure	0	21	21	0	11	11
Total	0	26	26	0	29	29
Total Approaches						
Approach	482	36	518	605	40	645
Departure	482	36	518	605	40	645
Total	964	72	1,036	1,210	80	1,290

Table C-2 - Cumulative (2024) Peak Hour PCE Volume Summary

		AM Peak Hour						
		Existing (2022) PCE Volume	2022- 2024 Growth	2024 Volumes	Cumul Proj Trips	Cumul 2024 Volumes	Project Trips	Cumul 2024 With Project
1	I-215 Southbound Ramps/Bonnie Drive							
	NBL	233	14	247	89	336	1	337
	NBT	493	30	523	238	761	12	773
	NBR	0	0	0	0	0	0	0
	SBL	0	0	0	0	0	0	0
	SBT	599	36	635	126	761	22	783
	SBR	25	2	27	38	65	0	65
	EBL	28	2	30	39	69	0	69
	EBT	0	0	0	0	0	0	0
	EBR	159	10	169	94	263	2	265
	WBL	0	0	0	0	0	0	0
	WBT	0	0	0	0	0	0	0
	WBR	0	0	0	0	0	0	0
	North Leg							
	Approach	624	38	662	164	826	22	848
	Departure	521	32	553	277	830	12	842
	Total	1,145	70	1,215	441	1,656	34	1,690
	South Leg							
	Approach	726	44	770	327	1,097	13	1,110
	Departure	758	46	804	220	1,024	24	1,048
	Total	1,484	90	1,574	547	2,121	37	2,158
	East Leg							
	Approach	0	0	0	0	0	0	0
	Departure	0	0	0	0	0	0	0
	Total	0	0	0	0	0	0	0
	West Leg							
	Approach	187	12	199	133	332	2	334
	Departure	258	16	274	127	401	1	402
	Total	445	28	473	260	733	3	736
	Total Approaches							
	Approach	1,537	94	1,631	624	2,255	37	2,292
	Departure	1,537	94	1,631	624	2,255	37	2,292
	Total	3,074	188	3,262	1,248	4,510	74	4,584

Table C-2 - Cumulative (2024) Peak Hour PCE Volume Summary

		AM Peak Hour						
		Existing (2022) PCE Volume	2022- 2024 Growth	2024 Volumes	Cumul Proj Trips	Cumul 2024 Volumes	Project Trips	Cumul 2024 With Project
2	I-215 Northbound Ramps/CA-74							
	NBL	0	0	0	0	0	0	0
	NBT	0	0	0	0	0	0	0
	NBR	0	0	0	0	0	0	0
	SBL	170	10	180	129	309	24	333
	SBT	0	0	0	0	0	0	0
	SBR	16	1	17	43	60	0	60
	EBL	14	1	15	24	39	0	39
	EBT	744	45	789	196	985	24	1,009
	EBR	0	0	0	0	0	0	0
	WBL	0	0	0	0	0	0	0
	WBT	710	43	753	283	1,036	13	1,049
	WBR	784	47	831	237	1,068	12	1,080
	North Leg							
	Approach	186	11	197	172	369	24	393
	Departure	798	48	846	261	1,107	12	1,119
	Total	984	59	1,043	433	1,476	36	1,512
	South Leg							
	Approach	0	0	0	0	0	0	0
	Departure	0	0	0	0	0	0	0
	Total	0	0	0	0	0	0	0
	East Leg							
	Approach	1,494	90	1,584	520	2,104	25	2,129
	Departure	914	55	969	325	1,294	48	1,342
	Total	2,408	145	2,553	845	3,398	73	3,471
	West Leg							
	Approach	758	46	804	220	1,024	24	1,048
	Departure	726	44	770	326	1,096	13	1,109
	Total	1,484	90	1,574	546	2,120	37	2,157
	Total Approaches							
	Approach	2,438	147	2,585	912	3,497	73	3,570
	Departure	2,438	147	2,585	912	3,497	73	3,570
	Total	4,876	294	5,170	1,824	6,994	146	7,140

Table C-2 - Cumulative (2024) Peak Hour PCE Volume Summary

		AM Peak Hour						
		Existing (2022) PCE Volume:	2022- 2024 Growth	2024 Volumes	Cumul Proj Trips	Cumul 2024 Volumes	Project Trips	Cumul 2024 With Project
3	Trumble Road/Mapes Road							
	NBL	55	3	58	0	58	8	66
	NBT	31	2	33	25	58	0	58
	NBR	79	5	84	150	234	0	234
	SBL	1	0	1	0	1	0	1
	SBT	48	3	51	29	80	0	80
	SBR	2	0	2	0	2	2	4
	EBL	3	0	3	0	3	0	3
	EBT	18	1	19	0	19	1	20
	EBR	55	3	58	0	58	2	60
	WBL	214	13	227	200	427	0	427
	WBT	17	1	18	0	18	2	20
	WBR	3	0	3	0	3	0	3
	North Leg							
	Approach	51	3	54	29	83	2	85
	Departure	37	2	39	25	64	0	64
	Total	88	5	93	54	147	2	149
	South Leg							
	Approach	165	10	175	175	350	8	358
	Departure	317	19	336	229	565	2	567
	Total	482	29	511	404	915	10	925
	East Leg							
	Approach	234	14	248	200	448	2	450
	Departure	98	6	104	150	254	1	255
	Total	332	20	352	350	702	3	705
	West Leg							
	Approach	76	4	80	0	80	3	83
	Departure	74	4	78	0	78	12	90
	Total	150	8	158	0	158	15	173
	Total Approaches							
	Approach	526	31	557	404	961	15	976
	Departure	526	31	557	404	961	15	976
	Total	1,052	62	1,114	808	1,922	30	1,952

Table C-2 - Cumulative (2024) Peak Hour PCE Volume Summary

	AM Peak Hour						
	Existing (2022) PCE Volume:	2022- 2024 Growth	2024 Volumes	Cumul Proj Trips	Cumul 2024 Volumes	Project Trips	Cumul 2024 With Project
4 Trumble Road/Exceed Road-Sturgeon Electric Dwy							
NBL	5	0	5	0	5	25	30
NBT	164	10	174	175	349	29	378
NBR	9	1	10	0	10	0	10
SBL	0	0	0	0	0	0	0
SBT	315	19	334	226	560	7	567
SBR	2	0	2	0	2	0	2
EBL	0	0	0	0	0	0	0
EBT	0	0	0	0	0	0	0
EBR	1	0	1	0	1	20	21
WBL	54	3	57	0	57	0	57
WBT	1	0	1	0	1	0	1
WBR	1	0	1	0	1	0	1
North Leg							
Approach	317	19	336	226	562	7	569
Departure	165	10	175	175	350	29	379
Total	482	29	511	401	912	36	948
South Leg							
Approach	178	11	189	175	364	54	418
Departure	370	22	392	226	618	27	645
Total	548	33	581	401	982	81	1,063
East Leg							
Approach	56	3	59	0	59	0	59
Departure	9	1	10	0	10	0	10
Total	65	4	69	0	69	0	69
West Leg							
Approach	1	0	1	0	1	20	21
Departure	8	0	8	0	8	25	33
Total	9	0	9	0	9	45	54
Total Approaches							
Approach	552	33	585	401	986	81	1,067
Departure	552	33	585	401	986	81	1,067
Total	1,104	66	1,170	802	1,972	162	2,134

Table C-2 - Cumulative (2024) Peak Hour PCE Volume Summary

		AM Peak Hour						
		Existing (2022) PCE Volume:	2022- 2024 Growth	2024 Volumes	Cumul Proj Trips	Cumul 2024 Volumes	Project Trips	Cumul 2024 With Project
5	Trumble Road/CA-74							
NBL		0	0	0	0	0	0	0
NBT		0	0	0	0	0	0	0
NBR		0	0	0	0	0	0	0
SBL		43	3	46	111	157	2	159
SBT		0	0	0	0	0	0	0
SBR		457	27	484	173	657	25	682
EBL		177	11	188	144	332	48	380
EBT		737	44	781	186	967	0	967
EBR		0	0	0	0	0	0	0
WBL		0	0	0	0	0	0	0
WBT		1,037	62	1,099	337	1,436	0	1,436
WBR		30	2	32	112	144	6	150
North Leg								
	Approach	500	30	530	284	814	27	841
	Departure	207	13	220	256	476	54	530
	Total	707	43	750	540	1,290	81	1,371
South Leg								
	Approach	0	0	0	0	0	0	0
	Departure	0	0	0	0	0	0	0
	Total	0	0	0	0	0	0	0
East Leg								
	Approach	1,067	64	1,131	449	1,580	6	1,586
	Departure	780	47	827	297	1,124	2	1,126
	Total	1,847	111	1,958	746	2,704	8	2,712
West Leg								
	Approach	914	55	969	330	1,299	48	1,347
	Departure	1,494	89	1,583	510	2,093	25	2,118
	Total	2,408	144	2,552	840	3,392	73	3,465
Total Approaches								
	Approach	2,481	149	2,630	1,063	3,693	81	3,774
	Departure	2,481	149	2,630	1,063	3,693	81	3,774
	Total	4,962	298	5,260	2,126	7,386	162	7,548

Table C-2 - Cumulative (2024) Peak Hour PCE Volume Summary

		AM Peak Hour						
		Existing (2022) PCE Volume:	2022- 2024 Growth	2024 Volumes	Cumul Proj Trips	Cumul 2024 Volumes	Project Trips	Cumul 2024 With Project
6	Project Driveway 1/Mapes Road							
	NBL	0	0	0	0	0	0	0
	NBT	0	0	0	0	0	0	0
	NBR	0	0	0	0	0	3	3
	SBL	0	0	0	0	0	0	0
	SBT	0	0	0	0	0	0	0
	SBR	0	0	0	0	0	0	0
	EBL	0	0	0	0	0	0	0
	EBT	0	0	0	0	0	0	0
	EBR	0	0	0	0	0	0	0
	WBL	0	0	0	0	0	12	12
	WBT	0	0	0	0	0	0	0
	WBR	0	0	0	0	0	0	0
	North Leg							
	Approach	0	0	0	0	0	0	0
	Departure	0	0	0	0	0	0	0
	Total	0	0	0	0	0	0	0
	South Leg							
	Approach	0	0	0	0	0	3	3
	Departure	0	0	0	0	0	12	12
	Total	0	0	0	0	0	15	15
	East Leg							
	Approach	0	0	0	0	0	12	12
	Departure	0	0	0	0	0	3	3
	Total	0	0	0	0	0	15	15
	West Leg							
	Approach	0	0	0	0	0	0	0
	Departure	0	0	0	0	0	0	0
	Total	0	0	0	0	0	0	0
	Total Approaches							
	Approach	0	0	0	0	0	15	15
	Departure	0	0	0	0	0	15	15
	Total	0	0	0	0	0	30	30

Table C-2 - Cumulative (2024) Peak Hour PCE Volume Summary

		AM Peak Hour						
		Existing (2022) PCE Volume:	2022- 2024 Growth	2024 Volumes	Cumul Proj Trips	Cumul 2024 Volumes	Project Trips	Cumul 2024 With Project
7	Trumble Road/Project Driveway 2							
NBL		0	0	0	0	0	21	21
NBT		165	10	175	175	350	8	358
NBR		0	0	0	0	0	0	0
SBL		0	0	0	0	0	0	0
SBT		317	19	336	226	562	2	564
SBR		0	0	0	0	0	0	0
EBL		0	0	0	0	0	0	0
EBT		0	0	0	0	0	0	0
EBR		0	0	0	0	0	5	5
WBL		0	0	0	0	0	0	0
WBT		0	0	0	0	0	0	0
WBR		0	0	0	0	0	0	0
North Leg								
	Approach	317	19	336	226	562	2	564
	Departure	165	10	175	175	350	8	358
	Total	482	29	511	401	912	10	922
South Leg								
	Approach	165	10	175	175	350	29	379
	Departure	317	19	336	226	562	7	569
	Total	482	29	511	401	912	36	948
East Leg								
	Approach	0	0	0	0	0	0	0
	Departure	0	0	0	0	0	0	0
	Total	0	0	0	0	0	0	0
West Leg								
	Approach	0	0	0	0	0	5	5
	Departure	0	0	0	0	0	21	21
	Total	0	0	0	0	0	26	26
Total Approaches								
	Approach	482	29	511	401	912	36	948
	Departure	482	29	511	401	912	36	948
	Total	964	58	1,022	802	1,824	72	1,896

Table C-2 - Cumulative (2024) Peak Hour PCE Volume Summary

		PM Peak Hour						
		Existing (2022) PCE Volume	2022- 2024 Growth	2024 Volumes	Cumul Proj Trips	Cumul 2024 Volumes	Project Trips	Cumul 2024 With Project
1	I-215 Southbound Ramps/Bonnie Drive							
	NBL	250	15	265	124	389	2	391
	NBT	297	18	315	197	512	22	534
	NBR	0	0	0	0	0	0	0
	SBL	0	0	0	0	0	0	0
	SBT	774	46	820	275	1,095	16	1,111
	SBR	23	1	24	30	54	0	54
	EBL	32	2	34	49	83	0	83
	EBT	0	0	0	0	0	0	0
	EBR	269	16	285	104	389	1	390
	WBL	0	0	0	0	0	0	0
	WBT	0	0	0	0	0	0	0
	WBR	0	0	0	0	0	0	0
	North Leg							
	Approach	797	47	844	305	1,149	16	1,165
	Departure	329	20	349	246	595	22	617
	Total	1,126	67	1,193	551	1,744	38	1,782
	South Leg							
	Approach	547	33	580	321	901	24	925
	Departure	1,043	62	1,105	379	1,484	17	1,501
	Total	1,590	95	1,685	700	2,385	41	2,426
	East Leg							
	Approach	0	0	0	0	0	0	0
	Departure	0	0	0	0	0	0	0
	Total	0	0	0	0	0	0	0
	West Leg							
	Approach	301	18	319	153	472	1	473
	Departure	273	16	289	154	443	2	445
	Total	574	34	608	307	915	3	918
	Total Approaches							
	Approach	1,645	98	1,743	779	2,522	41	2,563
	Departure	1,645	98	1,743	779	2,522	41	2,563
	Total	3,290	196	3,486	1,558	5,044	82	5,126

Table C-2 - Cumulative (2024) Peak Hour PCE Volume Summary

		PM Peak Hour						
		Existing (2022) PCE Volume	2022- 2024 Growth	2024 Volumes	Cumul Proj Trips	Cumul 2024 Volumes	Project Trips	Cumul 2024 With Project
2	I-215 Northbound Ramps/CA-74							
	NBL	0	0	0	0	0	0	0
	NBT	0	0	0	0	0	0	0
	NBR	0	0	0	0	0	0	0
	SBL	231	14	245	277	522	17	539
	SBT	0	0	0	0	0	0	0
	SBR	39	2	41	46	87	0	87
	EBL	14	1	15	39	54	0	54
	EBT	1,029	62	1,091	340	1,431	17	1,448
	EBR	0	0	0	0	0	0	0
	WBL	0	0	0	0	0	0	0
	WBT	508	30	538	275	813	24	837
	WBR	713	43	756	195	951	21	972
	North Leg							
	Approach	270	16	286	323	609	17	626
	Departure	727	44	771	234	1,005	21	1,026
	Total	997	60	1,057	557	1,614	38	1,652
	South Leg							
	Approach	0	0	0	0	0	0	0
	Departure	0	0	0	0	0	0	0
	Total	0	0	0	0	0	0	0
	East Leg							
	Approach	1,221	73	1,294	470	1,764	45	1,809
	Departure	1,260	76	1,336	617	1,953	34	1,987
	Total	2,481	149	2,630	1,087	3,717	79	3,796
	West Leg							
	Approach	1,043	63	1,106	379	1,485	17	1,502
	Departure	547	32	579	321	900	24	924
	Total	1,590	95	1,685	700	2,385	41	2,426
	Total Approaches							
	Approach	2,534	152	2,686	1,172	3,858	79	3,937
	Departure	2,534	152	2,686	1,172	3,858	79	3,937
	Total	5,068	304	5,372	2,344	7,716	158	7,874

Table C-2 - Cumulative (2024) Peak Hour PCE Volume Summary

	PM Peak Hour						
	Existing (2022) PCE Volume	2022- 2024 Growth	2024 Volumes	Cumul Proj Trips	Cumul 2024 Volumes	Project Trips	Cumul 2024 With Project
3 Trumble Road/Mapes Road							
NBL	1	0	1	0	1	4	5
NBT	117	7	124	31	155	0	155
NBR	219	13	232	223	455	0	455
SBL	27	2	29	0	29	0	29
SBT	132	8	140	30	170	0	170
SBR	0	0	0	0	0	1	1
EBL	2	0	2	0	2	1	3
EBT	1	0	1	0	1	1	2
EBR	5	0	5	0	5	7	12
WBL	131	8	139	195	334	0	334
WBT	1	0	1	0	1	1	2
WBR	28	2	30	0	30	0	30
North Leg							
Approach	159	10	169	30	199	1	200
Departure	147	9	156	31	187	1	188
Total	306	19	325	61	386	2	388
South Leg							
Approach	337	20	357	254	611	4	615
Departure	268	16	284	225	509	7	516
Total	605	36	641	479	1,120	11	1,131
East Leg							
Approach	160	10	170	195	365	1	366
Departure	247	15	262	223	485	1	486
Total	407	25	432	418	850	2	852
West Leg							
Approach	8	0	8	0	8	9	17
Departure	2	0	2	0	2	6	8
Total	10	0	10	0	10	15	25
Total Approaches							
Approach	664	40	704	479	1,183	15	1,198
Departure	664	40	704	479	1,183	15	1,198
Total	1,328	80	1,408	958	2,366	30	2,396

Table C-2 - Cumulative (2024) Peak Hour PCE Volume Summary

	PM Peak Hour						
	Existing (2022) PCE Volume	2022- 2024 Growth	2024 Volumes	Cumul Proj Trips	Cumul 2024 Volumes	Project Trips	Cumul 2024 With Project
4 Trumble Road/Exceed Road-Sturgeon Electric Dwy							
NBL	2	0	2	0	2	22	24
NBT	336	20	356	254	610	15	625
NBR	1	0	1	0	1	0	1
SBL	1	0	1	0	1	0	1
SBT	267	16	283	227	510	25	535
SBR	0	0	0	0	0	0	0
EBL	0	0	0	0	0	0	0
EBT	0	0	0	0	0	0	0
EBR	5	0	5	0	5	26	31
WBL	28	2	30	0	30	0	30
WBT	0	0	0	0	0	0	0
WBR	1	0	1	0	1	0	1
North Leg							
Approach	268	16	284	227	511	25	536
Departure	337	20	357	254	611	15	626
Total	605	36	641	481	1,122	40	1,162
South Leg							
Approach	339	20	359	254	613	37	650
Departure	300	18	318	227	545	51	596
Total	639	38	677	481	1,158	88	1,246
East Leg							
Approach	29	2	31	0	31	0	31
Departure	2	0	2	0	2	0	2
Total	31	2	33	0	33	0	33
West Leg							
Approach	5	0	5	0	5	26	31
Departure	2	0	2	0	2	22	24
Total	7	0	7	0	7	48	55
Total Approaches							
Approach	641	38	679	481	1,160	88	1,248
Departure	641	38	679	481	1,160	88	1,248
Total	1,282	76	1,358	962	2,320	176	2,496

Table C-2 - Cumulative (2024) Peak Hour PCE Volume Summary

		PM Peak Hour						
		Existing (2022) PCE Volume	2022- 2024 Growth	2024 Volumes	Cumul Proj Trips	Cumul 2024 Volumes	Project Trips	Cumul 2024 With Project
5	Trumble Road/CA-74							
	NBL	0	0	0	0	0	0	0
	NBT	0	0	0	0	0	0	0
	NBR	0	0	0	0	0	0	0
	SBL	50	3	53	119	172	6	178
	SBT	0	0	0	0	0	0	0
	SBR	382	23	405	174	579	45	624
	EBL	304	18	322	199	521	34	555
	EBT	956	57	1,013	409	1,422	0	1,422
	EBR	0	0	0	0	0	0	0
	WBL	0	0	0	0	0	0	0
	WBT	839	50	889	300	1,189	0	1,189
	WBR	22	1	23	126	149	3	152
	North Leg							
	Approach	432	26	458	293	751	51	802
	Departure	326	19	345	325	670	37	707
	Total	758	45	803	618	1,421	88	1,509
	South Leg							
	Approach	0	0	0	0	0	0	0
	Departure	0	0	0	0	0	0	0
	Total	0	0	0	0	0	0	0
	East Leg							
	Approach	861	51	912	426	1,338	3	1,341
	Departure	1,006	60	1,066	528	1,594	6	1,600
	Total	1,867	111	1,978	954	2,932	9	2,941
	West Leg							
	Approach	1,260	75	1,335	608	1,943	34	1,977
	Departure	1,221	73	1,294	474	1,768	45	1,813
	Total	2,481	148	2,629	1,082	3,711	79	3,790
	Total Approaches							
	Approach	2,553	152	2,705	1,327	4,032	88	4,120
	Departure	2,553	152	2,705	1,327	4,032	88	4,120
	Total	5,106	304	5,410	2,654	8,064	176	8,240

Table C-2 - Cumulative (2024) Peak Hour PCE Volume Summary

	PM Peak Hour						
	Existing (2022) PCE Volume	2022- 2024 Growth	2024 Volumes	Cumul Proj Trips	Cumul 2024 Volumes	Project Trips	Cumul 2024 With Project
6	Project Driveway 1/Mapes Road						
NBL	0	0	0	0	0	0	0
NBT	0	0	0	0	0	0	0
NBR	0	0	0	0	0	9	9
SBL	0	0	0	0	0	0	0
SBT	0	0	0	0	0	0	0
SBR	0	0	0	0	0	0	0
EBL	0	0	0	0	0	0	0
EBT	0	0	0	0	0	0	0
EBR	0	0	0	0	0	0	0
WBL	0	0	0	0	0	6	6
WBT	0	0	0	0	0	0	0
WBR	0	0	0	0	0	0	0
North Leg							
Approach	0	0	0	0	0	0	0
Departure	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0
South Leg							
Approach	0	0	0	0	0	9	9
Departure	0	0	0	0	0	6	6
Total	0	0	0	0	0	15	15
East Leg							
Approach	0	0	0	0	0	6	6
Departure	0	0	0	0	0	9	9
Total	0	0	0	0	0	15	15
West Leg							
Approach	0	0	0	0	0	0	0
Departure	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0
Total Approaches							
Approach	0	0	0	0	0	15	15
Departure	0	0	0	0	0	15	15
Total	0	0	0	0	0	30	30

Table C-2 - Cumulative (2024) Peak Hour PCE Volume Summary

	PM Peak Hour						
	Existing (2022) PCE Volume	2022- 2024 Growth	2024 Volumes	Cumul Proj Trips	Cumul 2024 Volumes	Project Trips	Cumul 2024 With Project
7 Trumble Road/Project Driveway 2							
NBL	0	0	0	0	0	11	11
NBT	337	20	357	254	611	4	615
NBR	0	0	0	0	0	0	0
SBL	0	0	0	0	0	0	0
SBT	268	16	284	227	511	7	518
SBR	0	0	0	0	0	0	0
EBL	0	0	0	0	0	0	0
EBT	0	0	0	0	0	0	0
EBR	0	0	0	0	0	18	18
WBL	0	0	0	0	0	0	0
WBT	0	0	0	0	0	0	0
WBR	0	0	0	0	0	0	0
North Leg							
Approach	268	16	284	227	511	7	518
Departure	337	20	357	254	611	4	615
Total	605	36	641	481	1,122	11	1,133
South Leg							
Approach	337	20	357	254	611	15	626
Departure	268	16	284	227	511	25	536
Total	605	36	641	481	1,122	40	1,162
East Leg							
Approach	0	0	0	0	0	0	0
Departure	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0
West Leg							
Approach	0	0	0	0	0	18	18
Departure	0	0	0	0	0	11	11
Total	0	0	0	0	0	29	29
Total Approaches							
Approach	605	36	641	481	1,122	40	1,162
Departure	605	36	641	481	1,122	40	1,162
Total	1,210	72	1,282	962	2,244	80	2,324

APPENDIX D:

LEVEL OF SERVICE WORKSHEETS

Timings
1: SR-74/I-215 SB Ramps & Bonnie Drive

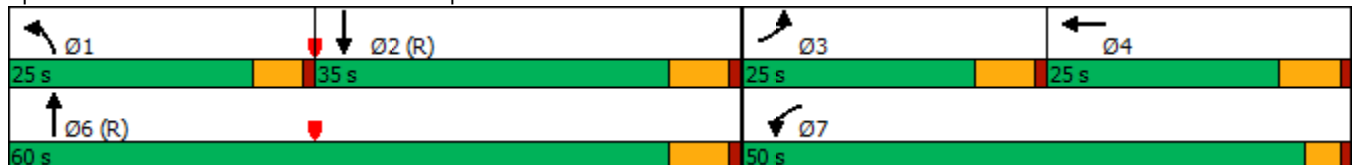


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø4	Ø7
Lane Configurations								
Traffic Volume (vph)	28	159	233	493	599	25		
Future Volume (vph)	28	159	233	493	599	25		
Turn Type	Prot	Free	Prot	NA	NA	Free		
Protected Phases	3		1	6	2		4	7
Permitted Phases		Free				Free		
Detector Phase	3		1	6	2			
Switch Phase								
Minimum Initial (s)	5.0		5.0	5.0	5.0		5.0	5.0
Minimum Split (s)	11.0		10.0	11.0	11.0		11.0	9.0
Total Split (s)	25.0		25.0	60.0	35.0		25.0	50.0
Total Split (%)	22.7%		22.7%	54.5%	31.8%		23%	45%
Yellow Time (s)	5.0		4.0	5.0	5.0		5.0	3.0
All-Red Time (s)	1.0		1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			
Total Lost Time (s)	6.0		5.0	6.0	6.0			
Lead/Lag	Lead		Lead		Lag		Lag	
Lead-Lag Optimize?	Yes		Yes		Yes		Yes	
Recall Mode	None		None	C-Min	C-Min		None	None
Act Effct Green (s)	6.5	110.0	20.0	98.4	70.9	110.0		
Actuated g/C Ratio	0.06	1.00	0.18	0.89	0.64	1.00		
v/c Ratio	0.28	0.11	0.77	0.32	0.53	0.02		
Control Delay	55.6	0.1	58.2	2.3	15.0	0.0		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	55.6	0.1	58.2	2.3	15.0	0.0		
LOS	E	A	E	A	B	A		
Approach Delay				20.2	14.4			
Approach LOS				C	B			




















Intersection Summary

Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 23 (21%), Referenced to phase 2:SBT and 6:NBT, Start of Green
 Natural Cycle: 75
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.77
 Intersection Signal Delay: 16.4
 Intersection LOS: B
 Intersection Capacity Utilization 60.3%
 ICU Level of Service B
 Analysis Period (min) 15

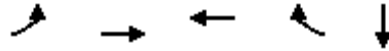
Splits and Phases: 1: SR-74/I-215 SB Ramps & Bonnie Drive



HCM 6th Signalized Intersection Summary
1: SR-74/I-215 SB Ramps & Bonnie Drive

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	28	0	159	0	0	0	233	493	0	0	599	25
Future Volume (veh/h)	28	0	159	0	0	0	233	493	0	0	599	25
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	0	1900	1900	1900	0	1900	1900	1900	0	1900	1900
Adj Flow Rate, veh/h	30	0	0	0	0	0	253	536	0	0	651	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	0	0	0	0	0	0	0	0	0	0
Cap, veh/h	49	0		0	2	0	283	1641	0	0	1258	
Arrive On Green	0.03	0.00	0.00	0.00	0.00	0.00	0.16	0.86	0.00	0.00	0.66	0.00
Sat Flow, veh/h	1810	30		0	1900	0	1810	1900	0	0	1900	1610
Grp Volume(v), veh/h	30	57.3		0	0	0	253	536	0	0	651	0
Grp Sat Flow(s),veh/h/ln	1810	E		0	1900	0	1810	1900	0	0	1900	1610
Q Serve(g_s), s	1.8			0.0	0.0	0.0	15.1	5.9	0.0	0.0	19.4	0.0
Cycle Q Clear(g_c), s	1.8			0.0	0.0	0.0	15.1	5.9	0.0	0.0	19.4	0.0
Prop In Lane	1.00			0.00		0.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h	49			0	2	0	283	1641	0	0	1258	
V/C Ratio(X)	0.61			0.00	0.00	0.00	0.89	0.33	0.00	0.00	0.52	
Avail Cap(c_a), veh/h	313			0	328	0	329	1641	0	0	1258	
HCM Platoon Ratio	1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00			0.00	0.00	0.00	1.00	1.00	0.00	0.00	1.00	0.00
Uniform Delay (d), s/veh	52.9			0.0	0.0	0.0	45.5	1.4	0.0	0.0	9.6	0.0
Incr Delay (d2), s/veh	4.4			0.0	0.0	0.0	21.5	0.5	0.0	0.0	1.5	0.0
Initial Q Delay(d3),s/veh	0.0			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.9			0.0	0.0	0.0	8.2	0.5	0.0	0.0	7.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	57.3			0.0	0.0	0.0	67.0	2.0	0.0	0.0	11.1	0.0
LnGrp LOS	E			A	A	A	E	A	A	A	B	
Approach Vol, veh/h					0			789			651	
Approach Delay, s/veh					0.0			22.8			11.1	
Approach LOS								C			B	
Timer - Assigned Phs	1	2	3	4		6	7					
Phs Duration (G+Y+Rc), s	22.2	78.8	9.0	0.0		101.0	9.0					
Change Period (Y+Rc), s	5.0	6.0	6.0	6.0		6.0	4.0					
Max Green Setting (Gmax), s	20.0	29.0	19.0	19.0		54.0	46.0					
Max Q Clear Time (g_c+I1), s	17.1	21.4	3.8	0.0		7.9	0.0					
Green Ext Time (p_c), s	0.1	1.5	0.0	0.0		1.9	0.0					
Intersection Summary												
HCM 6th Ctrl Delay			18.3									
HCM 6th LOS			B									
Notes												
Unsignalized Delay for [EBR, SBR] is excluded from calculations of the approach delay and intersection delay.												

Timings
2: SR-74 & I-215 NB Ramps



Lane Group	EBL	EBT	WBT	WBR	SBT
Lane Configurations					
Traffic Volume (vph)	14	744	710	784	0
Future Volume (vph)	14	744	710	784	0
Turn Type	Prot	NA	NA	Perm	NA
Protected Phases	5	2	6		4
Permitted Phases				6	
Detector Phase	5	2	6	6	4
Switch Phase					
Minimum Initial (s)	5.0	50.0	5.0	5.0	5.0
Minimum Split (s)	10.3	55.3	10.3	10.3	10.3
Total Split (s)	25.0	60.0	35.0	35.0	35.0
Total Split (%)	26.3%	63.2%	36.8%	36.8%	36.8%
Yellow Time (s)	4.3	4.3	4.3	4.3	4.3
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.3
Lead/Lag	Lead		Lag	Lag	
Lead-Lag Optimize?	Yes		Yes	Yes	
Recall Mode	None	C-Min	C-Min	C-Min	None
Act Effect Green (s)	5.6	73.7	69.0	69.0	10.7
Actuated g/C Ratio	0.06	0.78	0.73	0.73	0.11
v/c Ratio	0.14	0.28	0.29	0.60	0.71
Control Delay	45.1	3.8	6.2	2.7	35.8
Queue Delay	0.0	0.0	0.0	0.2	0.0
Total Delay	45.1	3.8	6.2	2.8	35.8
LOS	D	A	A	A	D
Approach Delay		4.5	4.5		35.8
Approach LOS		A	A		D


















Intersection Summary

Cycle Length: 95
 Actuated Cycle Length: 95
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 70
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.71
 Intersection Signal Delay: 6.9
 Intersection LOS: A
 Intersection Capacity Utilization 61.5%
 ICU Level of Service B
 Analysis Period (min) 15

Splits and Phases: 2: SR-74 & I-215 NB Ramps



HCM 6th Signalized Intersection Summary
 2: SR-74 & I-215 NB Ramps

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	14	744	0	0	710	784	0	0	0	170	0	16
Future Volume (veh/h)	14	744	0	0	710	784	0	0	0	170	0	16
Initial Q (Qb), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00				1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Work Zone On Approach		No			No						No	
Adj Sat Flow, veh/h/ln	1900	1900	0	0	1900	1900				1900	1900	1900
Adj Flow Rate, veh/h	15	791	0	0	755	0				181	0	17
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94				0.94	0.94	0.94
Percent Heavy Veh, %	0	0	0	0	0	0				0	0	0
Cap, veh/h	31	2724	0	0	2461					219	0	21
Arrive On Green	0.02	0.75	0.00	0.00	0.68	0.00				0.13	0.00	0.13
Sat Flow, veh/h	1810	3705	0	0	3705	1610				1637	0	154
Grp Volume(v), veh/h	15	791	0	0	755	0				198	0	0
Grp Sat Flow(s),veh/h/ln	1810	1805	0	0	1805	1610				1790	0	0
Q Serve(g_s), s	0.8	6.5	0.0	0.0	8.0	0.0				10.2	0.0	0.0
Cycle Q Clear(g_c), s	0.8	6.5	0.0	0.0	8.0	0.0				10.2	0.0	0.0
Prop In Lane	1.00		0.00	0.00		1.00				0.91		0.09
Lane Grp Cap(c), veh/h	31	2724	0	0	2461					240	0	0
V/C Ratio(X)	0.48	0.29	0.00	0.00	0.31					0.83	0.00	0.00
Avail Cap(c_a), veh/h	375	2724	0	0	2461					560	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	0.00	0.69	0.00				1.00	0.00	0.00
Uniform Delay (d), s/veh	46.3	3.7	0.0	0.0	6.1	0.0				40.1	0.0	0.0
Incr Delay (d2), s/veh	4.2	0.3	0.0	0.0	0.2	0.0				2.8	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	1.5	0.0	0.0	2.8	0.0				4.5	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	50.5	3.9	0.0	0.0	6.3	0.0				42.8	0.0	0.0
LnGrp LOS	D	A	A	A	A					D	A	A
Approach Vol, veh/h		806			755						198	
Approach Delay, s/veh		4.8			6.3						42.8	
Approach LOS		A			A						D	
Timer - Assigned Phs		2		4	5	6						
Phs Duration (G+Y+Rc), s		77.0		18.0	6.9	70.1						
Change Period (Y+Rc), s		5.3		5.3	5.3	5.3						
Max Green Setting (Gmax), s		54.7		29.7	19.7	29.7						
Max Q Clear Time (g_c+I1), s		8.5		12.2	2.8	10.0						
Green Ext Time (p_c), s		3.3		0.6	0.0	3.8						
Intersection Summary												
HCM 6th Ctrl Delay				9.7								
HCM 6th LOS				A								
Notes												
Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.												

Intersection												
Intersection Delay, s/veh	9.1											
Intersection LOS	A											

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔		↔	↔		↔	↔		↔	↔	
Traffic Vol, veh/h	3	18	55	214	17	3	55	31	79	1	48	2
Future Vol, veh/h	3	18	55	214	17	3	55	31	79	1	48	2
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	3	19	59	230	18	3	59	33	85	1	52	2
Number of Lanes	0	2	0	2	1	0	1	2	0	1	2	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	3	2	3	3
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	3	3	2	3
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	3	3	3	2
HCM Control Delay	8.4	9.7	8.8	8.8
HCM LOS	A	A	A	A

Lane	NBLn1	NBLn2	NBLn3	EBLn1	EBLn2	WBLn1	WBLn2	WBLn3	SBLn1	SBLn2	SBLn3
Vol Left, %	100%	0%	0%	25%	0%	100%	100%	0%	100%	0%	0%
Vol Thru, %	0%	100%	12%	75%	14%	0%	0%	85%	0%	100%	89%
Vol Right, %	0%	0%	88%	0%	86%	0%	0%	15%	0%	0%	11%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	55	21	89	12	64	107	107	20	1	32	18
LT Vol	55	0	0	3	0	107	107	0	1	0	0
Through Vol	0	21	10	9	9	0	0	17	0	32	16
RT Vol	0	0	79	0	55	0	0	3	0	0	2
Lane Flow Rate	59	22	96	13	69	115	115	22	1	34	19
Geometry Grp	8	8	8	8	8	8	8	8	8	8	8
Degree of Util (X)	0.1	0.034	0.132	0.021	0.096	0.185	0.185	0.031	0.002	0.055	0.031
Departure Headway (Hd)	6.067	5.565	4.943	5.775	5.048	5.783	5.783	5.178	6.301	5.799	5.721
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Cap	588	640	720	616	704	618	618	687	564	613	621
Service Time	3.831	3.328	2.707	3.548	2.822	3.546	3.546	2.942	4.083	3.58	3.502
HCM Lane V/C Ratio	0.1	0.034	0.133	0.021	0.098	0.186	0.186	0.032	0.002	0.055	0.031
HCM Control Delay	9.5	8.5	8.5	8.7	8.4	9.9	9.9	8.1	9.1	8.9	8.7
HCM Lane LOS	A	A	A	A	A	A	A	A	A	A	A
HCM 95th-tile Q	0.3	0.1	0.5	0.1	0.3	0.7	0.7	0.1	0	0.2	0.1

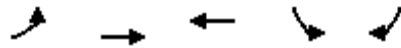
Intersection												
Int Delay, s/veh	1.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	0	0	1	54	1	1	5	164	9	0	315	2
Future Vol, veh/h	0	0	1	54	1	1	5	164	9	0	315	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91	91	91	91	91
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	0	0	1	59	1	1	5	180	10	0	346	2

Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	448	547	174	368	543	95	348	0	0	190	0	0
Stage 1	347	347	-	195	195	-	-	-	-	-	-	-
Stage 2	101	200	-	173	348	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	6.9	7.5	6.5	6.9	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	499	447	846	568	450	949	1222	-	-	1396	-	-
Stage 1	648	638	-	794	743	-	-	-	-	-	-	-
Stage 2	900	739	-	818	638	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	496	445	846	566	448	949	1222	-	-	1396	-	-
Mov Cap-2 Maneuver	496	445	-	566	448	-	-	-	-	-	-	-
Stage 1	645	638	-	791	740	-	-	-	-	-	-	-
Stage 2	894	736	-	817	638	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	9.3	12.1	0.2	0
HCM LOS	A	B		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1222	-	-	846	567	1396	-	-
HCM Lane V/C Ratio	0.004	-	-	0.001	0.109	-	-	-
HCM Control Delay (s)	8	-	-	9.3	12.1	0	-	-
HCM Lane LOS	A	-	-	A	B	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0	0.4	0	-	-

Timings
5: SR-74 & Trumble Road

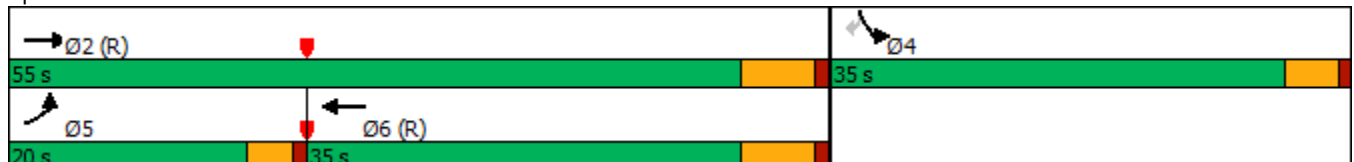


Lane Group	EBL	EBT	WBT	SBL	SBR
Lane Configurations	↘	↑↑	↑↑	↘	↗
Traffic Volume (vph)	177	737	1037	43	457
Future Volume (vph)	177	737	1037	43	457
Turn Type	Prot	NA	NA	Prot	Perm
Protected Phases	5	2	6	4	
Permitted Phases					4
Detector Phase	5	2	6	4	4
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.0	11.0	35.0	30.5	30.5
Total Split (s)	20.0	55.0	35.0	35.0	35.0
Total Split (%)	22.2%	61.1%	38.9%	38.9%	38.9%
Yellow Time (s)	3.0	5.0	5.0	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	6.0	6.0	4.5	4.5
Lead/Lag	Lead		Lag		
Lead-Lag Optimize?	Yes		Yes		
Recall Mode	None	C-Min	C-Min	None	None
Act Effct Green (s)	13.7	66.5	48.8	13.0	13.0
Actuated g/C Ratio	0.15	0.74	0.54	0.14	0.14
v/c Ratio	0.69	0.30	0.59	0.18	0.86
Control Delay	48.8	5.3	18.6	31.3	23.4
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	48.8	5.3	18.6	31.3	23.4
LOS	D	A	B	C	C
Approach Delay		13.7	18.6	24.1	
Approach LOS		B	B	C	

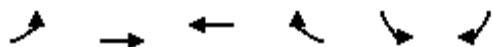
Intersection Summary

Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 32.5 (36%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 75
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.86
 Intersection Signal Delay: 17.9
 Intersection LOS: B
 Intersection Capacity Utilization 66.7%
 ICU Level of Service C
 Analysis Period (min) 15

Splits and Phases: 5: SR-74 & Trumble Road



HCM 6th Signalized Intersection Summary
5: SR-74 & Trumble Road



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↖	↑↑	↗		↖	↗
Traffic Volume (veh/h)	177	737	1037	30	43	457
Future Volume (veh/h)	177	737	1037	30	43	457
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1900
Adj Flow Rate, veh/h	190	792	1115	32	46	491
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	0	0	0	0	0	0
Cap, veh/h	226	2026	1404	40	583	519
Arrive On Green	0.12	0.56	0.39	0.39	0.32	0.32
Sat Flow, veh/h	1810	3705	3679	103	1810	1610
Grp Volume(v), veh/h	190	792	562	585	46	491
Grp Sat Flow(s),veh/h/ln	1810	1805	1805	1881	1810	1610
Q Serve(g_s), s	9.2	11.1	24.7	24.7	1.6	26.8
Cycle Q Clear(g_c), s	9.2	11.1	24.7	24.7	1.6	26.8
Prop In Lane	1.00			0.05	1.00	1.00
Lane Grp Cap(c), veh/h	226	2026	707	737	583	519
V/C Ratio(X)	0.84	0.39	0.79	0.79	0.08	0.95
Avail Cap(c_a), veh/h	322	2026	707	737	613	546
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.97	0.97	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	38.5	11.1	24.2	24.2	21.2	29.7
Incr Delay (d2), s/veh	8.9	0.6	9.0	8.6	0.0	24.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.4	3.8	11.1	11.5	0.6	24.1
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	47.4	11.7	33.1	32.8	21.2	54.4
LnGrp LOS	D	B	C	C	C	D
Approach Vol, veh/h		982	1147		537	
Approach Delay, s/veh		18.6	33.0		51.6	
Approach LOS		B	C		D	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		56.5		33.5	15.2	41.3
Change Period (Y+Rc), s		6.0		4.5	4.0	6.0
Max Green Setting (Gmax), s		49.0		30.5	16.0	29.0
Max Q Clear Time (g_c+I1), s		13.1		28.8	11.2	26.7
Green Ext Time (p_c), s		5.4		0.2	0.1	1.4
Intersection Summary						
HCM 6th Ctrl Delay			31.4			
HCM 6th LOS			C			

Timings
1: SR-74/I-215 SB Ramps & Bonnie Drive

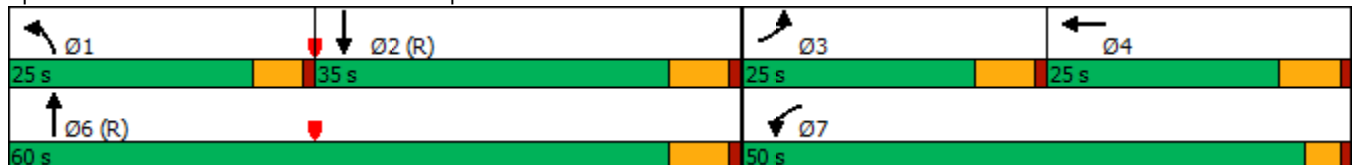


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø4	Ø7
Lane Configurations	↙	↗	↙	↗	↑	↗		
Traffic Volume (vph)	32	269	250	297	774	23		
Future Volume (vph)	32	269	250	297	774	23		
Turn Type	Prot	Free	Prot	NA	NA	Free		
Protected Phases	3		1	6	2		4	7
Permitted Phases		Free				Free		
Detector Phase	3		1	6	2			
Switch Phase								
Minimum Initial (s)	5.0		5.0	5.0	5.0		5.0	5.0
Minimum Split (s)	11.0		10.0	11.0	11.0		11.0	9.0
Total Split (s)	25.0		25.0	60.0	35.0		25.0	50.0
Total Split (%)	22.7%		22.7%	54.5%	31.8%		23%	45%
Yellow Time (s)	5.0		4.0	5.0	5.0		5.0	3.0
All-Red Time (s)	1.0		1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			
Total Lost Time (s)	6.0		5.0	6.0	6.0			
Lead/Lag	Lead		Lead		Lag		Lag	
Lead-Lag Optimize?	Yes		Yes		Yes		Yes	
Recall Mode	None		None	C-Min	C-Min		None	None
Act Effct Green (s)	6.8	110.0	22.0	98.1	68.7	110.0		
Actuated g/C Ratio	0.06	1.00	0.20	0.89	0.62	1.00		
v/c Ratio	0.32	0.19	0.77	0.19	0.73	0.02		
Control Delay	56.4	0.3	55.8	1.9	21.9	0.0		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	56.4	0.3	55.8	1.9	21.9	0.0		
LOS	E	A	E	A	C	A		
Approach Delay				26.5	21.3			
Approach LOS				C	C			

Intersection Summary




















Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 23 (21%), Referenced to phase 2:SBT and 6:NBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.77
 Intersection Signal Delay: 20.3
 Intersection LOS: C
 Intersection Capacity Utilization 70.4%
 ICU Level of Service C
 Analysis Period (min) 15

Splits and Phases: 1: SR-74/I-215 SB Ramps & Bonnie Drive

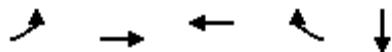


HCM 6th Signalized Intersection Summary
1: SR-74/I-215 SB Ramps & Bonnie Drive

Trumble and Mapes Warehouse Project
Exist_NP_PM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	32	0	269	0	0	0	250	297	0	0	774	23
Future Volume (veh/h)	32	0	269	0	0	0	250	297	0	0	774	23
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	0	1900	1900	1900	0	1900	1900	1900	0	1900	1900
Adj Flow Rate, veh/h	36	0	0	0	0	0	278	330	0	0	860	0
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	0	0	0	0	0	0	0	0	0	0	0	0
Cap, veh/h	55	0		0	2	0	307	1635	0	0	1227	
Arrive On Green	0.03	0.00	0.00	0.00	0.00	0.00	0.17	0.86	0.00	0.00	0.65	0.00
Sat Flow, veh/h	1810	36		0	1900	0	1810	1900	0	0	1900	1610
Grp Volume(v), veh/h	36	57.6		0	0	0	278	330	0	0	860	0
Grp Sat Flow(s),veh/h/ln	1810	E		0	1900	0	1810	1900	0	0	1900	1610
Q Serve(g_s), s	2.2			0.0	0.0	0.0	16.6	3.2	0.0	0.0	32.2	0.0
Cycle Q Clear(g_c), s	2.2			0.0	0.0	0.0	16.6	3.2	0.0	0.0	32.2	0.0
Prop In Lane	1.00			0.00		0.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h	55			0	2	0	307	1635	0	0	1227	
V/C Ratio(X)	0.66			0.00	0.00	0.00	0.91	0.20	0.00	0.00	0.70	
Avail Cap(c_a), veh/h	313			0	328	0	329	1635	0	0	1227	
HCM Platoon Ratio	1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00			0.00	0.00	0.00	1.00	1.00	0.00	0.00	1.00	0.00
Uniform Delay (d), s/veh	52.8			0.0	0.0	0.0	44.8	1.3	0.0	0.0	12.6	0.0
Incr Delay (d2), s/veh	4.9			0.0	0.0	0.0	25.4	0.3	0.0	0.0	3.4	0.0
Initial Q Delay(d3),s/veh	0.0			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.0			0.0	0.0	0.0	9.3	0.3	0.0	0.0	12.2	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	57.6			0.0	0.0	0.0	70.2	1.6	0.0	0.0	16.0	0.0
LnGrp LOS	E			A	A	A	E	A	A	A	B	
Approach Vol, veh/h					0			608			860	
Approach Delay, s/veh					0.0			32.9			16.0	
Approach LOS								C			B	
Timer - Assigned Phs	1	2	3	4		6	7					
Phs Duration (G+Y+Rc), s	23.6	77.0	9.3	0.0		100.7	9.3					
Change Period (Y+Rc), s	5.0	6.0	6.0	6.0		6.0	4.0					
Max Green Setting (Gmax), s	20.0	29.0	19.0	19.0		54.0	46.0					
Max Q Clear Time (g_c+I1), s	18.6	34.2	4.2	0.0		5.2	0.0					
Green Ext Time (p_c), s	0.1	0.0	0.0	0.0		1.0	0.0					
Intersection Summary												
HCM 6th Ctrl Delay			23.8									
HCM 6th LOS			C									
Notes												
Unsignalized Delay for [EBR, SBR] is excluded from calculations of the approach delay and intersection delay.												

Timings
2: SR-74 & I-215 NB Ramps

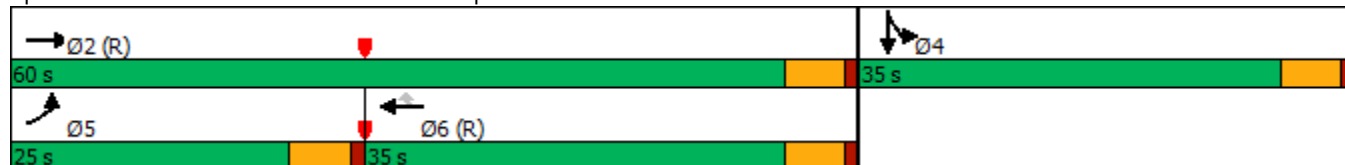


Lane Group	EBL	EBT	WBT	WBR	SBT
Lane Configurations	↖	↑↑	↑↑	↗	↔
Traffic Volume (vph)	14	1029	508	713	0
Future Volume (vph)	14	1029	508	713	0
Turn Type	Prot	NA	NA	Perm	NA
Protected Phases	5	2	6		4
Permitted Phases				6	
Detector Phase	5	2	6	6	4
Switch Phase					
Minimum Initial (s)	5.0	50.0	5.0	5.0	5.0
Minimum Split (s)	10.3	55.3	10.3	10.3	10.3
Total Split (s)	25.0	60.0	35.0	35.0	35.0
Total Split (%)	26.3%	63.2%	36.8%	36.8%	36.8%
Yellow Time (s)	4.3	4.3	4.3	4.3	4.3
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.3
Lead/Lag	Lead		Lag	Lag	
Lead-Lag Optimize?	Yes		Yes	Yes	
Recall Mode	None	C-Min	C-Min	C-Min	None
Act Effct Green (s)	5.6	68.6	63.9	63.9	15.8
Actuated g/C Ratio	0.06	0.72	0.67	0.67	0.17
v/c Ratio	0.14	0.42	0.22	0.57	0.79
Control Delay	45.1	6.6	8.0	2.9	40.8
Queue Delay	0.0	0.0	0.0	0.1	0.0
Total Delay	45.1	6.6	8.0	3.0	40.8
LOS	D	A	A	A	D
Approach Delay		7.1	5.1		40.8
Approach LOS		A	A		D

Intersection Summary


















Cycle Length: 95
 Actuated Cycle Length: 95
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 70
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.79
 Intersection Signal Delay: 9.7
 Intersection LOS: A
 Intersection Capacity Utilization 65.7%
 ICU Level of Service C
 Analysis Period (min) 15

Splits and Phases: 2: SR-74 & I-215 NB Ramps



HCM 6th Signalized Intersection Summary
2: SR-74 & I-215 NB Ramps

Trumble and Mapes Warehouse Project
Exist_NP_PM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	14	1029	0	0	508	713	0	0	0	231	0	39
Future Volume (veh/h)	14	1029	0	0	508	713	0	0	0	231	0	39
Initial Q (Qb), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00				1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Work Zone On Approach		No			No						No	
Adj Sat Flow, veh/h/ln	1900	1900	0	0	1900	1900				1900	1900	1900
Adj Flow Rate, veh/h	15	1106	0	0	546	0				248	0	42
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93				0.93	0.93	0.93
Percent Heavy Veh, %	0	0	0	0	0	0				0	0	0
Cap, veh/h	31	2530	0	0	2267					285	0	48
Arrive On Green	0.02	0.70	0.00	0.00	0.63	0.00				0.19	0.00	0.19
Sat Flow, veh/h	1810	3705	0	0	3705	1610				1520	0	257
Grp Volume(v), veh/h	15	1106	0	0	546	0				290	0	0
Grp Sat Flow(s),veh/h/ln	1810	1805	0	0	1805	1610				1778	0	0
Q Serve(g_s), s	0.8	12.6	0.0	0.0	6.3	0.0				15.0	0.0	0.0
Cycle Q Clear(g_c), s	0.8	12.6	0.0	0.0	6.3	0.0				15.0	0.0	0.0
Prop In Lane	1.00		0.00	0.00		1.00				0.86		0.14
Lane Grp Cap(c), veh/h	31	2530	0	0	2267					333	0	0
V/C Ratio(X)	0.48	0.44	0.00	0.00	0.24					0.87	0.00	0.00
Avail Cap(c_a), veh/h	375	2530	0	0	2267					556	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	0.00	0.76	0.00				1.00	0.00	0.00
Uniform Delay (d), s/veh	46.3	6.1	0.0	0.0	7.8	0.0				37.5	0.0	0.0
Incr Delay (d2), s/veh	4.2	0.6	0.0	0.0	0.2	0.0				4.1	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	3.5	0.0	0.0	2.4	0.0				6.6	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	50.5	6.7	0.0	0.0	7.9	0.0				41.5	0.0	0.0
LnGrp LOS	D	A	A	A	A					D	A	A
Approach Vol, veh/h		1121			546						290	
Approach Delay, s/veh		7.3			7.9						41.5	
Approach LOS		A			A						D	
Timer - Assigned Phs		2		4	5	6						
Phs Duration (G+Y+Rc), s		71.9		23.1	6.9	64.9						
Change Period (Y+Rc), s		5.3		5.3	5.3	5.3						
Max Green Setting (Gmax), s		54.7		29.7	19.7	29.7						
Max Q Clear Time (g_c+I1), s		14.6		17.0	2.8	8.3						
Green Ext Time (p_c), s		5.1		0.8	0.0	2.6						
Intersection Summary												
HCM 6th Ctrl Delay				12.5								
HCM 6th LOS				B								
Notes												
Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.												

Intersection												
Intersection Delay, s/veh	10.6											
Intersection LOS	B											

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔↔		↔↔	↔		↔	↔↔		↔	↔↔	
Traffic Vol, veh/h	2	1	5	131	1	28	1	117	219	27	132	0
Future Vol, veh/h	2	1	5	131	1	28	1	117	219	27	132	0
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	3	1	6	170	1	36	1	152	284	35	171	0
Number of Lanes	0	2	0	2	1	0	1	2	0	1	2	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	3	2	3	3
Conflicting Approach Left SB		NB	EB	WB
Conflicting Lanes Left	3	3	2	3
Conflicting Approach Right NB		SB	WB	EB
Conflicting Lanes Right	3	3	3	2
HCM Control Delay	9.1	10.3	11.6	9
HCM LOS	A	B	B	A

Lane	NBLn1	NBLn2	NBLn3	EBLn1	EBLn2	WBLn1	WBLn2	WBLn3	SBLn1	SBLn2	SBLn3
Vol Left, %	100%	0%	0%	80%	0%	100%	100%	0%	100%	0%	0%
Vol Thru, %	0%	100%	15%	20%	9%	0%	0%	3%	0%	100%	100%
Vol Right, %	0%	0%	85%	0%	91%	0%	0%	97%	0%	0%	0%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	1	78	258	3	6	66	66	29	27	66	66
LT Vol	1	0	0	2	0	66	66	0	27	0	0
Through Vol	0	78	39	1	1	0	0	1	0	66	66
RT Vol	0	0	219	0	5	0	0	28	0	0	0
Lane Flow Rate	1	101	335	3	7	85	85	38	35	86	86
Geometry Grp	8	8	8	8	8	8	8	8	8	8	8
Degree of Util (X)	0.002	0.161	0.475	0.006	0.012	0.158	0.158	0.058	0.064	0.146	0.105
Departure Headway (Hd)	6.207	5.705	5.108	7.046	6.01	6.684	6.684	5.509	6.618	6.115	4.407
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Cap	580	632	710	508	595	538	538	650	542	587	813
Service Time	3.907	3.405	2.808	4.789	3.752	4.417	4.417	3.242	4.35	3.848	2.139
HCM Lane V/C Ratio	0.002	0.16	0.472	0.006	0.012	0.158	0.158	0.058	0.065	0.147	0.106
HCM Control Delay	8.9	9.5	12.3	9.8	8.8	10.7	10.7	8.6	9.8	9.9	7.7
HCM Lane LOS	A	A	B	A	A	B	B	A	A	A	A
HCM 95th-tile Q	0	0.6	2.6	0	0	0.6	0.6	0.2	0.2	0.5	0.4

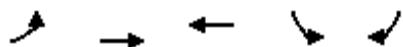
Intersection												
Int Delay, s/veh	0.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	0	0	5	28	0	1	2	336	1	1	267	0
Future Vol, veh/h	0	0	5	28	0	1	2	336	1	1	267	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	79	79	79	79	79	79	79	79	79	79	79	79
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	0	0	6	35	0	1	3	425	1	1	338	0

Major/Minor	Minor2		Minor1			Major1			Major2			
Conflicting Flow All	559	772	169	603	772	213	338	0	0	426	0	0
Stage 1	340	340	-	432	432	-	-	-	-	-	-	-
Stage 2	219	432	-	171	340	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	6.9	7.5	6.5	6.9	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	416	333	852	387	333	798	1232	-	-	1144	-	-
Stage 1	654	643	-	577	586	-	-	-	-	-	-	-
Stage 2	769	586	-	820	643	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	414	332	852	383	332	798	1232	-	-	1144	-	-
Mov Cap-2 Maneuver	414	332	-	383	332	-	-	-	-	-	-	-
Stage 1	653	642	-	576	585	-	-	-	-	-	-	-
Stage 2	766	585	-	813	642	-	-	-	-	-	-	-

Approach	EB		WB			NB			SB		
HCM Control Delay, s	9.3		15.2			0			0		
HCM LOS	A		C								

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1232	-	-	852	390	1144	-	-
HCM Lane V/C Ratio	0.002	-	-	0.007	0.094	0.001	-	-
HCM Control Delay (s)	7.9	-	-	9.3	15.2	8.2	-	-
HCM Lane LOS	A	-	-	A	C	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0	0.3	0	-	-

Timings
5: SR-74 & Trumble Road

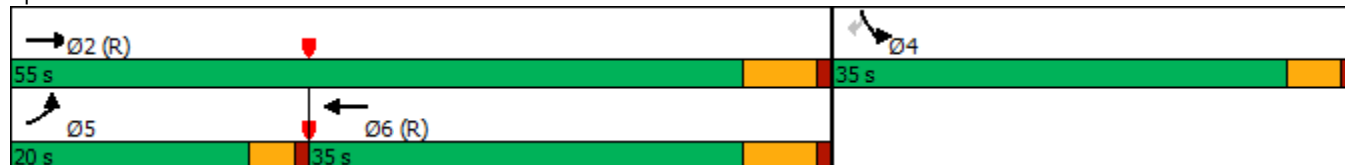


Lane Group	EBL	EBT	WBT	SBL	SBR
Lane Configurations	↖	↗	↗↖	↖	↗
Traffic Volume (vph)	304	956	839	50	382
Future Volume (vph)	304	956	839	50	382
Turn Type	Prot	NA	NA	Prot	Perm
Protected Phases	5	2	6	4	
Permitted Phases					4
Detector Phase	5	2	6	4	4
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.0	11.0	35.0	30.5	30.5
Total Split (s)	20.0	55.0	35.0	35.0	35.0
Total Split (%)	22.2%	61.1%	38.9%	38.9%	38.9%
Yellow Time (s)	3.0	5.0	5.0	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	6.0	6.0	4.5	4.5
Lead/Lag	Lead		Lag		
Lead-Lag Optimize?	Yes		Yes		
Recall Mode	None	C-Min	C-Min	None	None
Act Effct Green (s)	23.4	70.6	43.3	8.9	8.9
Actuated g/C Ratio	0.26	0.78	0.48	0.10	0.10
v/c Ratio	0.69	0.36	0.53	0.30	0.78
Control Delay	38.3	3.8	18.9	40.0	14.9
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	38.3	3.8	18.9	40.0	14.9
LOS	D	A	B	D	B
Approach Delay		12.1	18.9	17.8	
Approach LOS		B	B	B	

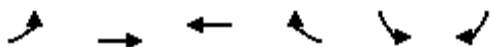
Intersection Summary

Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 32.5 (36%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 80
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.78
 Intersection Signal Delay: 15.4
 Intersection LOS: B
 Intersection Capacity Utilization 57.0%
 ICU Level of Service B
 Analysis Period (min) 15

Splits and Phases: 5: SR-74 & Trumble Road



HCM 6th Signalized Intersection Summary
5: SR-74 & Trumble Road



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↖	↕	↕		↖	↗
Traffic Volume (veh/h)	304	956	839	22	50	382
Future Volume (veh/h)	304	956	839	22	50	382
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1900
Adj Flow Rate, veh/h	323	1017	893	23	53	406
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	0	0	0	0	0	0
Cap, veh/h	322	2204	1396	36	494	439
Arrive On Green	0.18	0.61	0.39	0.39	0.27	0.27
Sat Flow, veh/h	1810	3705	3691	93	1810	1610
Grp Volume(v), veh/h	323	1017	448	468	53	406
Grp Sat Flow(s),veh/h/ln	1810	1805	1805	1883	1810	1610
Q Serve(g_s), s	16.0	13.8	18.2	18.2	2.0	22.1
Cycle Q Clear(g_c), s	16.0	13.8	18.2	18.2	2.0	22.1
Prop In Lane	1.00			0.05	1.00	1.00
Lane Grp Cap(c), veh/h	322	2204	701	731	494	439
V/C Ratio(X)	1.00	0.46	0.64	0.64	0.11	0.92
Avail Cap(c_a), veh/h	322	2204	701	731	613	546
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.91	0.91	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	37.0	9.5	22.4	22.4	24.5	31.8
Incr Delay (d2), s/veh	48.9	0.6	4.4	4.3	0.0	17.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	10.9	4.4	7.7	8.0	0.8	19.6
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	85.9	10.1	26.9	26.7	24.5	49.2
LnGrp LOS	F	B	C	C	C	D
Approach Vol, veh/h		1340	916		459	
Approach Delay, s/veh		28.4	26.8		46.4	
Approach LOS		C	C		D	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		60.9		29.1	20.0	40.9
Change Period (Y+Rc), s		6.0		4.5	4.0	6.0
Max Green Setting (Gmax), s		49.0		30.5	16.0	29.0
Max Q Clear Time (g_c+I1), s		15.8		24.1	18.0	20.2
Green Ext Time (p_c), s		7.5		0.5	0.0	3.4
Intersection Summary						
HCM 6th Ctrl Delay			30.9			
HCM 6th LOS			C			

Timings
1: SR-74/I-215 SB Ramps & Bonnie Drive

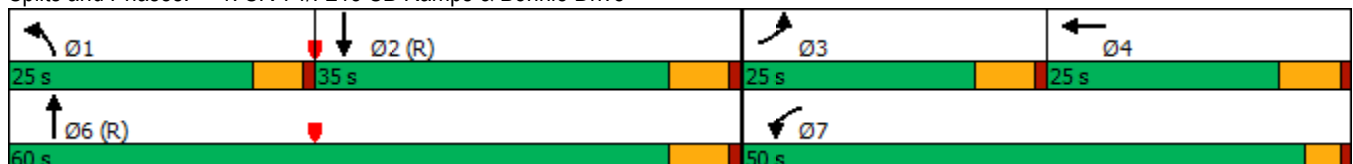


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø4	Ø7
Lane Configurations								
Traffic Volume (vph)	28	161	234	505	621	25		
Future Volume (vph)	28	161	234	505	621	25		
Turn Type	Prot	Free	Prot	NA	NA	Free		
Protected Phases	3		1	6	2		4	7
Permitted Phases		Free				Free		
Detector Phase	3		1	6	2			
Switch Phase								
Minimum Initial (s)	5.0		5.0	5.0	5.0		5.0	5.0
Minimum Split (s)	11.0		10.0	11.0	11.0		11.0	9.0
Total Split (s)	25.0		25.0	60.0	35.0		25.0	50.0
Total Split (%)	22.7%		22.7%	54.5%	31.8%		23%	45%
Yellow Time (s)	5.0		4.0	5.0	5.0		5.0	3.0
All-Red Time (s)	1.0		1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			
Total Lost Time (s)	6.0		5.0	6.0	6.0			
Lead/Lag	Lead		Lead		Lag		Lag	
Lead-Lag Optimize?	Yes		Yes		Yes		Yes	
Recall Mode	None		None	C-Min	C-Min		None	None
Act Effct Green (s)	6.5	110.0	20.1	98.4	70.9	110.0		
Actuated g/C Ratio	0.06	1.00	0.18	0.89	0.64	1.00		
v/c Ratio	0.28	0.11	0.77	0.32	0.55	0.02		
Control Delay	55.6	0.1	58.1	2.3	15.5	0.0		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	55.6	0.1	58.1	2.3	15.5	0.0		
LOS	E	A	E	A	B	A		
Approach Delay				20.0	14.9			
Approach LOS				B	B			

Intersection Summary

Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 23 (21%), Referenced to phase 2:SBT and 6:NBT, Start of Green
 Natural Cycle: 75
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.77
 Intersection Signal Delay: 16.5
 Intersection LOS: B
 Intersection Capacity Utilization 61.5%
 ICU Level of Service B
 Analysis Period (min) 15

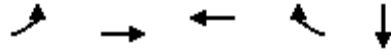
Splits and Phases: 1: SR-74/I-215 SB Ramps & Bonnie Drive



HCM 6th Signalized Intersection Summary
1: SR-74/I-215 SB Ramps & Bonnie Drive

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	28	0	161	0	0	0	234	505	0	0	621	25
Future Volume (veh/h)	28	0	161	0	0	0	234	505	0	0	621	25
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	0	1900	1900	1900	0	1900	1900	1900	0	1900	1900
Adj Flow Rate, veh/h	30	0	0	0	0	0	254	549	0	0	675	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	0	0	0	0	0	0	0	0	0	0
Cap, veh/h	49	0		0	2	0	284	1641	0	0	1257	
Arrive On Green	0.03	0.00	0.00	0.00	0.00	0.00	0.16	0.86	0.00	0.00	0.66	0.00
Sat Flow, veh/h	1810	30		0	1900	0	1810	1900	0	0	1900	1610
Grp Volume(v), veh/h	30	57.3		0	0	0	254	549	0	0	675	0
Grp Sat Flow(s),veh/h/ln	1810	E		0	1900	0	1810	1900	0	0	1900	1610
Q Serve(g_s), s	1.8			0.0	0.0	0.0	15.1	6.1	0.0	0.0	20.5	0.0
Cycle Q Clear(g_c), s	1.8			0.0	0.0	0.0	15.1	6.1	0.0	0.0	20.5	0.0
Prop In Lane	1.00			0.00		0.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h	49			0	2	0	284	1641	0	0	1257	
V/C Ratio(X)	0.61			0.00	0.00	0.00	0.90	0.33	0.00	0.00	0.54	
Avail Cap(c_a), veh/h	313			0	328	0	329	1641	0	0	1257	
HCM Platoon Ratio	1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00			0.00	0.00	0.00	1.00	1.00	0.00	0.00	1.00	0.00
Uniform Delay (d), s/veh	52.9			0.0	0.0	0.0	45.5	1.4	0.0	0.0	9.8	0.0
Incr Delay (d2), s/veh	4.4			0.0	0.0	0.0	21.6	0.6	0.0	0.0	1.6	0.0
Initial Q Delay(d3),s/veh	0.0			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.9			0.0	0.0	0.0	8.2	0.6	0.0	0.0	7.4	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	57.3			0.0	0.0	0.0	67.1	2.0	0.0	0.0	11.4	0.0
LnGrp LOS	E			A	A	A	E	A	A	A	B	
Approach Vol, veh/h					0			803			675	
Approach Delay, s/veh					0.0			22.6			11.4	
Approach LOS								C			B	
Timer - Assigned Phs	1	2	3	4		6	7					
Phs Duration (G+Y+Rc), s	22.2	78.8	9.0	0.0		101.0	9.0					
Change Period (Y+Rc), s	5.0	6.0	6.0	6.0		6.0	4.0					
Max Green Setting (Gmax), s	20.0	29.0	19.0	19.0		54.0	46.0					
Max Q Clear Time (g_c+I1), s	17.1	22.5	3.8	0.0		8.1	0.0					
Green Ext Time (p_c), s	0.1	1.5	0.0	0.0		1.9	0.0					
Intersection Summary												
HCM 6th Ctrl Delay			18.3									
HCM 6th LOS			B									
Notes												
Unsignalized Delay for [EBR, SBR] is excluded from calculations of the approach delay and intersection delay.												

Timings
2: SR-74 & I-215 NB Ramps



Lane Group	EBL	EBT	WBT	WBR	SBT
Lane Configurations	↖	↑↑	↑↑	↗	↔
Traffic Volume (vph)	14	768	723	796	0
Future Volume (vph)	14	768	723	796	0
Turn Type	Prot	NA	NA	Perm	NA
Protected Phases	5	2	6		4
Permitted Phases				6	
Detector Phase	5	2	6	6	4
Switch Phase					
Minimum Initial (s)	5.0	50.0	5.0	5.0	5.0
Minimum Split (s)	10.3	55.3	10.3	10.3	10.3
Total Split (s)	25.0	60.0	35.0	35.0	35.0
Total Split (%)	26.3%	63.2%	36.8%	36.8%	36.8%
Yellow Time (s)	4.3	4.3	4.3	4.3	4.3
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.3
Lead/Lag	Lead		Lag	Lag	
Lead-Lag Optimize?	Yes		Yes	Yes	
Recall Mode	None	C-Min	C-Min	C-Min	None
Act Effct Green (s)	5.6	72.4	67.7	67.7	12.0
Actuated g/C Ratio	0.06	0.76	0.71	0.71	0.13
v/c Ratio	0.14	0.30	0.30	0.61	0.74
Control Delay	45.1	4.3	6.9	2.8	38.0
Queue Delay	0.0	0.0	0.0	0.2	0.0
Total Delay	45.1	4.3	6.9	3.0	38.0
LOS	D	A	A	A	D
Approach Delay		5.0	4.9		38.0
Approach LOS		A	A		D

Intersection Summary


















Cycle Length: 95
 Actuated Cycle Length: 95
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 70
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.74
 Intersection Signal Delay: 7.7
 Intersection LOS: A
 Intersection Capacity Utilization 62.3%
 ICU Level of Service B
 Analysis Period (min) 15

Splits and Phases: 2: SR-74 & I-215 NB Ramps



HCM 6th Signalized Intersection Summary
2: SR-74 & I-215 NB Ramps

Trumble and Mapes Warehouse Project
Exist_WP_AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	14	768	0	0	723	796	0	0	0	194	0	16
Future Volume (veh/h)	14	768	0	0	723	796	0	0	0	194	0	16
Initial Q (Qb), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00				1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Work Zone On Approach		No			No						No	
Adj Sat Flow, veh/h/ln	1900	1900	0	0	1900	1900				1900	1900	1900
Adj Flow Rate, veh/h	15	817	0	0	769	0				206	0	17
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94				0.94	0.94	0.94
Percent Heavy Veh, %	0	0	0	0	0	0				0	0	0
Cap, veh/h	31	2672	0	0	2409					245	0	20
Arrive On Green	0.02	0.74	0.00	0.00	0.67	0.00				0.15	0.00	0.15
Sat Flow, veh/h	1810	3705	0	0	3705	1610				1656	0	137
Grp Volume(v), veh/h	15	817	0	0	769	0				223	0	0
Grp Sat Flow(s),veh/h/ln	1810	1805	0	0	1805	1610				1793	0	0
Q Serve(g_s), s	0.8	7.2	0.0	0.0	8.6	0.0				11.5	0.0	0.0
Cycle Q Clear(g_c), s	0.8	7.2	0.0	0.0	8.6	0.0				11.5	0.0	0.0
Prop In Lane	1.00		0.00	0.00		1.00				0.92		0.08
Lane Grp Cap(c), veh/h	31	2672	0	0	2409					266	0	0
V/C Ratio(X)	0.48	0.31	0.00	0.00	0.32					0.84	0.00	0.00
Avail Cap(c_a), veh/h	375	2672	0	0	2409					560	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	0.00	0.63	0.00				1.00	0.00	0.00
Uniform Delay (d), s/veh	46.3	4.1	0.0	0.0	6.7	0.0				39.4	0.0	0.0
Incr Delay (d2), s/veh	4.2	0.3	0.0	0.0	0.2	0.0				2.7	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	1.7	0.0	0.0	3.1	0.0				5.0	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	50.5	4.4	0.0	0.0	6.9	0.0				42.1	0.0	0.0
LnGrp LOS	D	A	A	A	A					D	A	A
Approach Vol, veh/h		832			769						223	
Approach Delay, s/veh		5.3			6.9						42.1	
Approach LOS		A			A						D	
Timer - Assigned Phs		2		4	5	6						
Phs Duration (G+Y+Rc), s		75.6		19.4	6.9	68.7						
Change Period (Y+Rc), s		5.3		5.3	5.3	5.3						
Max Green Setting (Gmax), s		54.7		29.7	19.7	29.7						
Max Q Clear Time (g_c+I1), s		9.2		13.5	2.8	10.6						
Green Ext Time (p_c), s		3.4		0.6	0.0	3.8						
Intersection Summary												
HCM 6th Ctrl Delay				10.5								
HCM 6th LOS				B								
Notes												
Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.												

Intersection												
Intersection Delay, s/veh	9.2											
Intersection LOS	A											

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔↔		↔↔	↔		↔	↔↔		↔	↔↔	
Traffic Vol, veh/h	3	19	57	214	19	3	63	31	79	1	48	4
Future Vol, veh/h	3	19	57	214	19	3	63	31	79	1	48	4
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	3	20	61	230	20	3	68	33	85	1	52	4
Number of Lanes	0	2	0	2	1	0	1	2	0	1	2	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	3	2	3	3
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	3	3	2	3
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	3	3	3	2
HCM Control Delay	8.4	9.7	8.9	8.9
HCM LOS	A	A	A	A

Lane	NBLn1	NBLn2	NBLn3	EBLn1	EBLn2	WBLn1	WBLn2	WBLn3	SBLn1	SBLn2	SBLn3
Vol Left, %	100%	0%	0%	24%	0%	100%	100%	0%	100%	0%	0%
Vol Thru, %	0%	100%	12%	76%	14%	0%	0%	86%	0%	100%	80%
Vol Right, %	0%	0%	88%	0%	86%	0%	0%	14%	0%	0%	20%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	63	21	89	13	67	107	107	22	1	32	20
LT Vol	63	0	0	3	0	107	107	0	1	0	0
Through Vol	0	21	10	10	10	0	0	19	0	32	16
RT Vol	0	0	79	0	57	0	0	3	0	0	4
Lane Flow Rate	68	22	96	13	72	115	115	24	1	34	22
Geometry Grp	8	8	8	8	8	8	8	8	8	8	8
Degree of Util (X)	0.115	0.034	0.132	0.022	0.101	0.186	0.186	0.034	0.002	0.056	0.034
Departure Headway (Hd)	6.089	5.587	4.965	5.808	5.088	5.822	5.822	5.227	6.34	5.837	5.697
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Cap	586	637	717	612	699	613	613	680	561	609	623
Service Time	3.856	3.354	2.732	3.583	2.863	3.589	3.589	2.994	4.123	3.621	3.48
HCM Lane V/C Ratio	0.116	0.035	0.134	0.021	0.103	0.188	0.188	0.035	0.002	0.056	0.035
HCM Control Delay	9.7	8.6	8.5	8.7	8.4	9.9	9.9	8.2	9.1	9	8.7
HCM Lane LOS	A	A	A	A	A	A	A	A	A	A	A
HCM 95th-tile Q	0.4	0.1	0.5	0.1	0.3	0.7	0.7	0.1	0	0.2	0.1

Intersection												
Int Delay, s/veh	1.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	0	0	21	54	1	1	30	193	9	0	322	2
Future Vol, veh/h	0	0	21	54	1	1	30	193	9	0	322	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91	91	91	91	91
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	0	0	23	59	1	1	33	212	10	0	354	2

Major/Minor	Minor2		Minor1			Major1			Major2			
Conflicting Flow All	528	643	178	460	639	111	356	0	0	222	0	0
Stage 1	355	355	-	283	283	-	-	-	-	-	-	-
Stage 2	173	288	-	177	356	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	6.9	7.5	6.5	6.9	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	438	394	841	489	397	927	1214	-	-	1359	-	-
Stage 1	641	633	-	706	681	-	-	-	-	-	-	-
Stage 2	818	677	-	813	633	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	427	383	841	466	386	927	1214	-	-	1359	-	-
Mov Cap-2 Maneuver	427	383	-	466	386	-	-	-	-	-	-	-
Stage 1	624	633	-	687	663	-	-	-	-	-	-	-
Stage 2	794	659	-	791	633	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	9.4	13.9	1	0
HCM LOS	A	B		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1214	-	-	841	468	1359	-	-
HCM Lane V/C Ratio	0.027	-	-	0.027	0.131	-	-	-
HCM Control Delay (s)	8	-	-	9.4	13.9	0	-	-
HCM Lane LOS	A	-	-	A	B	A	-	-
HCM 95th %tile Q(veh)	0.1	-	-	0.1	0.5	0	-	-

Timings
5: SR-74 & Trumble Road

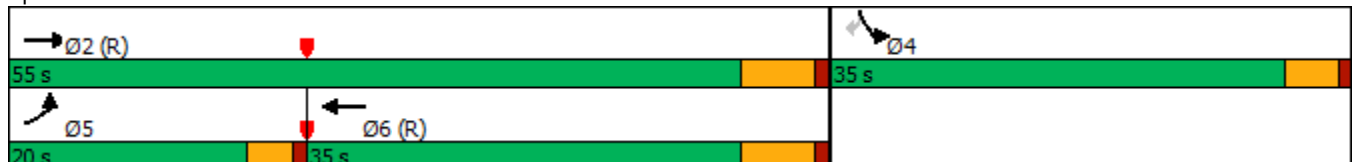


Lane Group	EBL	EBT	WBT	SBL	SBR
Lane Configurations	↗	↑↑	↑↑↔	↖	↗
Traffic Volume (vph)	225	737	1037	45	482
Future Volume (vph)	225	737	1037	45	482
Turn Type	Prot	NA	NA	Prot	Perm
Protected Phases	5	2	6	4	
Permitted Phases					4
Detector Phase	5	2	6	4	4
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.0	11.0	35.0	30.5	30.5
Total Split (s)	20.0	55.0	35.0	35.0	35.0
Total Split (%)	22.2%	61.1%	38.9%	38.9%	38.9%
Yellow Time (s)	3.0	5.0	5.0	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	6.0	6.0	4.5	4.5
Lead/Lag	Lead		Lag		
Lead-Lag Optimize?	Yes		Yes		
Recall Mode	None	C-Min	C-Min	None	None
Act Effct Green (s)	16.1	64.9	44.8	14.6	14.6
Actuated g/C Ratio	0.18	0.72	0.50	0.16	0.16
v/c Ratio	0.75	0.30	0.64	0.16	0.88
Control Delay	49.6	6.1	22.2	29.5	25.0
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	49.6	6.1	22.2	29.5	25.0
LOS	D	A	C	C	C
Approach Delay		16.2	22.2	25.4	
Approach LOS		B	C	C	

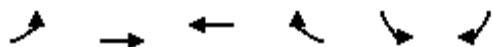
Intersection Summary

Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 32.5 (36%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 80
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.88
 Intersection Signal Delay: 20.6
 Intersection LOS: C
 Intersection Capacity Utilization 68.4%
 ICU Level of Service C
 Analysis Period (min) 15

Splits and Phases: 5: SR-74 & Trumble Road



HCM 6th Signalized Intersection Summary
5: SR-74 & Trumble Road



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	225	737	1037	36	45	482
Future Volume (veh/h)	225	737	1037	36	45	482
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1900
Adj Flow Rate, veh/h	242	792	1115	39	48	518
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	0	0	0	0	0	0
Cap, veh/h	278	1972	1240	43	610	543
Arrive On Green	0.15	0.55	0.35	0.35	0.34	0.34
Sat Flow, veh/h	1810	3705	3653	124	1810	1610
Grp Volume(v), veh/h	242	792	566	588	48	518
Grp Sat Flow(s),veh/h/ln	1810	1805	1805	1878	1810	1610
Q Serve(g_s), s	11.8	11.5	26.8	26.8	1.6	28.3
Cycle Q Clear(g_c), s	11.8	11.5	26.8	26.8	1.6	28.3
Prop In Lane	1.00			0.07	1.00	1.00
Lane Grp Cap(c), veh/h	278	1972	629	654	610	543
V/C Ratio(X)	0.87	0.40	0.90	0.90	0.08	0.95
Avail Cap(c_a), veh/h	322	1972	629	654	613	546
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.96	0.96	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	37.2	11.9	27.8	27.8	20.3	29.2
Incr Delay (d2), s/veh	17.6	0.6	18.2	17.7	0.0	27.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.2	4.0	13.5	13.9	0.6	25.6
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	54.9	12.5	46.0	45.5	20.3	56.3
LnGrp LOS	D	B	D	D	C	E
Approach Vol, veh/h		1034	1154		566	
Approach Delay, s/veh		22.4	45.8		53.2	
Approach LOS		C	D		D	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		55.2		34.8	17.8	37.4
Change Period (Y+Rc), s		6.0		4.5	4.0	6.0
Max Green Setting (Gmax), s		49.0		30.5	16.0	29.0
Max Q Clear Time (g_c+I1), s		13.5		30.3	13.8	28.8
Green Ext Time (p_c), s		5.4		0.0	0.1	0.2
Intersection Summary						
HCM 6th Ctrl Delay			38.5			
HCM 6th LOS			D			

Intersection						
Int Delay, s/veh	7					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑			↑		↑
Traffic Vol, veh/h	0	0	12	0	0	3
Future Vol, veh/h	0	0	12	0	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	13	0	0	3

Major/Minor	Major1	Major2	Minor1	Minor2	Minor3
Conflicting Flow All	0	-	1	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	-	4.12	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	-	2.218	-	-
Pot Cap-1 Maneuver	-	0	1622	-	0
Stage 1	-	0	-	-	0
Stage 2	-	0	-	-	0
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1622	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	WB	NB
HCM Control Delay, s	0	7.2	8.3
HCM LOS			A

Minor Lane/Major Mvmt	NBLn1	EBT	WBL	WBT
Capacity (veh/h)	1084	-	1622	-
HCM Lane V/C Ratio	0.003	-	0.008	-
HCM Control Delay (s)	8.3	-	7.2	0
HCM Lane LOS	A	-	A	A
HCM 95th %tile Q(veh)	0	-	0	-

Intersection						
Int Delay, s/veh	0.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔		↔	↑↑	↑↑	
Traffic Vol, veh/h	0	5	21	173	319	0
Future Vol, veh/h	0	5	21	173	319	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	50	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	5	23	188	347	0

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	487	174	347	0	-	0
Stage 1	347	-	-	-	-	-
Stage 2	140	-	-	-	-	-
Critical Hdwy	6.84	6.94	4.14	-	-	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	2.22	-	-	-
Pot Cap-1 Maneuver	510	839	1209	-	-	-
Stage 1	687	-	-	-	-	-
Stage 2	872	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	500	839	1209	-	-	-
Mov Cap-2 Maneuver	565	-	-	-	-	-
Stage 1	674	-	-	-	-	-
Stage 2	872	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	9.3	0.9	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1209	-	839	-	-
HCM Lane V/C Ratio	0.019	-	0.006	-	-
HCM Control Delay (s)	8	-	9.3	-	-
HCM Lane LOS	A	-	A	-	-
HCM 95th %tile Q(veh)	0.1	-	0	-	-

Timings
1: SR-74/I-215 SB Ramps & Bonnie Drive

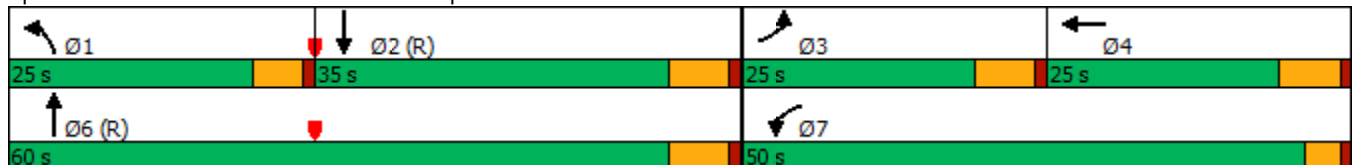


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø4	Ø7
Lane Configurations								
Traffic Volume (vph)	32	270	252	319	790	23		
Future Volume (vph)	32	270	252	319	790	23		
Turn Type	Prot	Free	Prot	NA	NA	Free		
Protected Phases	3		1	6	2		4	7
Permitted Phases		Free				Free		
Detector Phase	3		1	6	2			
Switch Phase								
Minimum Initial (s)	5.0		5.0	5.0	5.0		5.0	5.0
Minimum Split (s)	11.0		10.0	11.0	11.0		11.0	9.0
Total Split (s)	25.0		25.0	60.0	35.0		25.0	50.0
Total Split (%)	22.7%		22.7%	54.5%	31.8%		23%	45%
Yellow Time (s)	5.0		4.0	5.0	5.0		5.0	3.0
All-Red Time (s)	1.0		1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			
Total Lost Time (s)	6.0		5.0	6.0	6.0			
Lead/Lag	Lead		Lead		Lag		Lag	
Lead-Lag Optimize?	Yes		Yes		Yes		Yes	
Recall Mode	None		None	C-Min	C-Min		None	None
Act Effct Green (s)	6.8	110.0	22.2	98.1	68.5	110.0		
Actuated g/C Ratio	0.06	1.00	0.20	0.89	0.62	1.00		
v/c Ratio	0.32	0.19	0.77	0.21	0.74	0.02		
Control Delay	56.4	0.3	55.5	2.0	22.7	0.0		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	56.4	0.3	55.5	2.0	22.7	0.0		
LOS	E	A	E	A	C	A		
Approach Delay				25.6	22.1			
Approach LOS				C	C			

Intersection Summary

Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 23 (21%), Referenced to phase 2:SBT and 6:NBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.77
 Intersection Signal Delay: 20.4
 Intersection LOS: C
 Intersection Capacity Utilization 71.4%
 ICU Level of Service C
 Analysis Period (min) 15

Splits and Phases: 1: SR-74/I-215 SB Ramps & Bonnie Drive

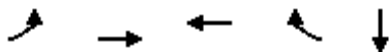


HCM 6th Signalized Intersection Summary
1: SR-74/I-215 SB Ramps & Bonnie Drive

Trumble and Mapes Warehouse Project
Exist_WP_PM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	32	0	270	0	0	0	252	319	0	0	790	23
Future Volume (veh/h)	32	0	270	0	0	0	252	319	0	0	790	23
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	0	1900	1900	1900	0	1900	1900	1900	0	1900	1900
Adj Flow Rate, veh/h	36	0	0	0	0	0	280	354	0	0	878	0
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	0	0	0	0	0	0	0	0	0	0	0	0
Cap, veh/h	55	0		0	2	0	309	1635	0	0	1225	
Arrive On Green	0.03	0.00	0.00	0.00	0.00	0.00	0.17	0.86	0.00	0.00	0.64	0.00
Sat Flow, veh/h	1810	36		0	1900	0	1810	1900	0	0	1900	1610
Grp Volume(v), veh/h	36	57.6		0	0	0	280	354	0	0	878	0
Grp Sat Flow(s),veh/h/ln	1810	E		0	1900	0	1810	1900	0	0	1900	1610
Q Serve(g_s), s	2.2			0.0	0.0	0.0	16.7	3.5	0.0	0.0	33.6	0.0
Cycle Q Clear(g_c), s	2.2			0.0	0.0	0.0	16.7	3.5	0.0	0.0	33.6	0.0
Prop In Lane	1.00			0.00		0.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h	55			0	2	0	309	1635	0	0	1225	
V/C Ratio(X)	0.66			0.00	0.00	0.00	0.91	0.22	0.00	0.00	0.72	
Avail Cap(c_a), veh/h	313			0	328	0	329	1635	0	0	1225	
HCM Platoon Ratio	1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00			0.00	0.00	0.00	1.00	1.00	0.00	0.00	1.00	0.00
Uniform Delay (d), s/veh	52.8			0.0	0.0	0.0	44.8	1.3	0.0	0.0	12.9	0.0
Incr Delay (d2), s/veh	4.9			0.0	0.0	0.0	25.7	0.3	0.0	0.0	3.6	0.0
Initial Q Delay(d3),s/veh	0.0			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.0			0.0	0.0	0.0	9.3	0.4	0.0	0.0	12.8	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	57.6			0.0	0.0	0.0	70.4	1.6	0.0	0.0	16.5	0.0
LnGrp LOS	E			A	A	A	E	A	A	A	B	
Approach Vol, veh/h					0			634			878	
Approach Delay, s/veh					0.0			32.0			16.5	
Approach LOS								C			B	
Timer - Assigned Phs	1	2	3	4		6	7					
Phs Duration (G+Y+Rc), s	23.8	76.9	9.3	0.0		100.7	9.3					
Change Period (Y+Rc), s	5.0	6.0	6.0	6.0		6.0	4.0					
Max Green Setting (Gmax), s	20.0	29.0	19.0	19.0		54.0	46.0					
Max Q Clear Time (g_c+I1), s	18.7	35.6	4.2	0.0		5.5	0.0					
Green Ext Time (p_c), s	0.1	0.0	0.0	0.0		1.1	0.0					
Intersection Summary												
HCM 6th Ctrl Delay				23.8								
HCM 6th LOS				C								
Notes												
Unsignalized Delay for [EBR, SBR] is excluded from calculations of the approach delay and intersection delay.												

Timings
2: SR-74 & I-215 NB Ramps



Lane Group	EBL	EBT	WBT	WBR	SBT
Lane Configurations	↖	↑↑	↑↑	↗	↔
Traffic Volume (vph)	14	1046	532	734	0
Future Volume (vph)	14	1046	532	734	0
Turn Type	Prot	NA	NA	Perm	NA
Protected Phases	5	2	6		4
Permitted Phases				6	
Detector Phase	5	2	6	6	4
Switch Phase					
Minimum Initial (s)	5.0	50.0	5.0	5.0	5.0
Minimum Split (s)	10.3	55.3	10.3	10.3	10.3
Total Split (s)	25.0	60.0	35.0	35.0	35.0
Total Split (%)	26.3%	63.2%	36.8%	36.8%	36.8%
Yellow Time (s)	4.3	4.3	4.3	4.3	4.3
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.3
Lead/Lag	Lead		Lag	Lag	
Lead-Lag Optimize?	Yes		Yes	Yes	
Recall Mode	None	C-Min	C-Min	C-Min	None
Act Effct Green (s)	5.6	67.5	62.9	62.9	16.9
Actuated g/C Ratio	0.06	0.71	0.66	0.66	0.18
v/c Ratio	0.14	0.44	0.24	0.59	0.79
Control Delay	45.1	7.2	8.6	3.1	41.1
Queue Delay	0.0	0.0	0.0	0.1	0.0
Total Delay	45.1	7.2	8.6	3.2	41.1
LOS	D	A	A	A	D
Approach Delay		7.7	5.5		41.1
Approach LOS		A	A		D

Intersection Summary

Cycle Length: 95
 Actuated Cycle Length: 95
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 70
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.79
 Intersection Signal Delay: 10.3
 Intersection LOS: B
 Intersection Capacity Utilization 66.6%
 ICU Level of Service C
 Analysis Period (min) 15

Splits and Phases: 2: SR-74 & I-215 NB Ramps



HCM 6th Signalized Intersection Summary
2: SR-74 & I-215 NB Ramps

Trumble and Mapes Warehouse Project
Exist_WP_PM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	14	1046	0	0	532	734	0	0	0	248	0	39
Future Volume (veh/h)	14	1046	0	0	532	734	0	0	0	248	0	39
Initial Q (Qb), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00				1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Work Zone On Approach		No			No						No	
Adj Sat Flow, veh/h/ln	1900	1900	0	0	1900	1900				1900	1900	1900
Adj Flow Rate, veh/h	15	1125	0	0	572	0				267	0	42
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93				0.93	0.93	0.93
Percent Heavy Veh, %	0	0	0	0	0	0				0	0	0
Cap, veh/h	31	2492	0	0	2229					305	0	48
Arrive On Green	0.02	0.69	0.00	0.00	0.62	0.00				0.20	0.00	0.20
Sat Flow, veh/h	1810	3705	0	0	3705	1610				1538	0	242
Grp Volume(v), veh/h	15	1125	0	0	572	0				309	0	0
Grp Sat Flow(s),veh/h/ln	1810	1805	0	0	1805	1610				1780	0	0
Q Serve(g_s), s	0.8	13.3	0.0	0.0	6.8	0.0				16.0	0.0	0.0
Cycle Q Clear(g_c), s	0.8	13.3	0.0	0.0	6.8	0.0				16.0	0.0	0.0
Prop In Lane	1.00		0.00	0.00		1.00				0.86		0.14
Lane Grp Cap(c), veh/h	31	2492	0	0	2229					353	0	0
V/C Ratio(X)	0.48	0.45	0.00	0.00	0.26					0.88	0.00	0.00
Avail Cap(c_a), veh/h	375	2492	0	0	2229					556	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	0.00	0.67	0.00				1.00	0.00	0.00
Uniform Delay (d), s/veh	46.3	6.6	0.0	0.0	8.3	0.0				37.0	0.0	0.0
Incr Delay (d2), s/veh	4.2	0.6	0.0	0.0	0.2	0.0				6.0	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	3.8	0.0	0.0	2.6	0.0				7.2	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	50.5	7.2	0.0	0.0	8.5	0.0				42.9	0.0	0.0
LnGrp LOS	D	A	A	A	A					D	A	A
Approach Vol, veh/h		1140			572						309	
Approach Delay, s/veh		7.8			8.5						42.9	
Approach LOS		A			A						D	
Timer - Assigned Phs		2		4	5	6						
Phs Duration (G+Y+Rc), s		70.9		24.1	6.9	63.9						
Change Period (Y+Rc), s		5.3		5.3	5.3	5.3						
Max Green Setting (Gmax), s		54.7		29.7	19.7	29.7						
Max Q Clear Time (g_c+I1), s		15.3		18.0	2.8	8.8						
Green Ext Time (p_c), s		5.2		0.8	0.0	2.7						
Intersection Summary												
HCM 6th Ctrl Delay				13.3								
HCM 6th LOS				B								
Notes												
Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.												

Intersection												
Intersection Delay, s/veh	11.1											
Intersection LOS	B											

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔		↔	↔		↔	↔		↔	↔	
Traffic Vol, veh/h	3	2	12	131	2	28	5	117	219	27	132	1
Future Vol, veh/h	3	2	12	131	2	28	5	117	219	27	132	1
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	4	3	16	170	3	36	6	152	284	35	171	1
Number of Lanes	0	2	0	2	1	0	1	2	0	1	2	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	3	2	3	3
Conflicting Approach Left SB		NB	EB	WB
Conflicting Lanes Left	3	3	2	3
Conflicting Approach Right NB		SB	WB	EB
Conflicting Lanes Right	3	3	3	2
HCM Control Delay	9.2	10.5	12	10.1
HCM LOS	A	B	B	B

Lane	NBLn1	NBLn2	NBLn3	EBLn1	EBLn2	WBLn1	WBLn2	WBLn3	SBLn1	SBLn2	SBLn3
Vol Left, %	100%	0%	0%	75%	0%	100%	100%	0%	100%	0%	0%
Vol Thru, %	0%	100%	15%	25%	8%	0%	0%	7%	0%	100%	98%
Vol Right, %	0%	0%	85%	0%	92%	0%	0%	93%	0%	0%	2%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	5	78	258	4	13	66	66	30	27	88	45
LT Vol	5	0	0	3	0	66	66	0	27	0	0
Through Vol	0	78	39	1	1	0	0	2	0	88	44
RT Vol	0	0	219	0	12	0	0	28	0	0	1
Lane Flow Rate	6	101	335	5	17	85	85	39	35	114	58
Geometry Grp	8	8	8	8	8	8	8	8	8	8	8
Degree of Util (X)	0.011	0.163	0.484	0.01	0.029	0.161	0.161	0.061	0.065	0.197	0.1
Departure Headway (Hd)	6.299	5.797	5.2	7.131	6.11	6.818	6.818	5.665	6.697	6.194	6.178
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Cap	569	619	694	502	585	527	527	632	535	579	580
Service Time	4.028	3.526	2.929	4.875	3.854	4.552	4.552	3.399	4.432	3.929	3.913
HCM Lane V/C Ratio	0.011	0.163	0.483	0.01	0.029	0.161	0.161	0.062	0.065	0.197	0.1
HCM Control Delay	9.1	9.7	12.7	9.9	9	10.9	10.9	8.8	9.9	10.5	9.6
HCM Lane LOS	A	A	B	A	A	B	B	A	A	B	A
HCM 95th-tile Q	0	0.6	2.7	0	0.1	0.6	0.6	0.2	0.2	0.7	0.3

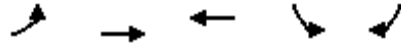
Intersection												
Int Delay, s/veh	1.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	0	0	31	28	0	1	24	351	1	1	292	0
Future Vol, veh/h	0	0	31	28	0	1	24	351	1	1	292	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	79	79	79	79	79	79	79	79	79	79	79	79
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	0	0	39	35	0	1	30	444	1	1	370	0

Major/Minor	Minor2		Minor1			Major1		Major2				
Conflicting Flow All	654	877	185	692	877	223	370	0	0	445	0	0
Stage 1	372	372	-	505	505	-	-	-	-	-	-	-
Stage 2	282	505	-	187	372	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	6.9	7.5	6.5	6.9	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	356	289	832	334	289	787	1200	-	-	1126	-	-
Stage 1	626	622	-	523	544	-	-	-	-	-	-	-
Stage 2	707	544	-	803	622	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	349	281	832	312	281	787	1200	-	-	1126	-	-
Mov Cap-2 Maneuver	349	281	-	312	281	-	-	-	-	-	-	-
Stage 1	610	621	-	510	530	-	-	-	-	-	-	-
Stage 2	688	530	-	764	621	-	-	-	-	-	-	-

Approach	EB		WB			NB		SB		
HCM Control Delay, s	9.5		17.7			0.5		0		
HCM LOS	A		C							

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1200	-	-	832	319	1126	-	-
HCM Lane V/C Ratio	0.025	-	-	0.047	0.115	0.001	-	-
HCM Control Delay (s)	8.1	-	-	9.5	17.7	8.2	-	-
HCM Lane LOS	A	-	-	A	C	A	-	-
HCM 95th %tile Q(veh)	0.1	-	-	0.1	0.4	0	-	-

Timings
5: SR-74 & Trumble Road

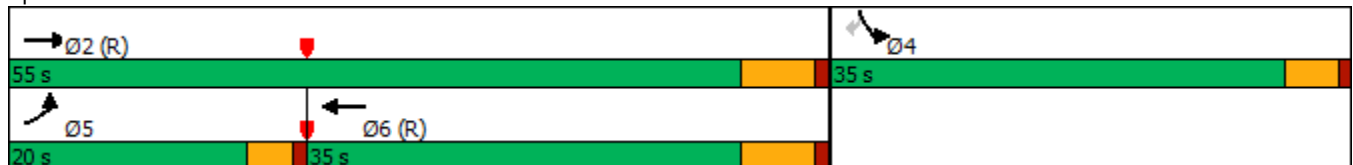


Lane Group	EBL	EBT	WBT	SBL	SBR
Lane Configurations	↗	↑↑	↑↑↔	↖	↗
Traffic Volume (vph)	338	956	839	56	427
Future Volume (vph)	338	956	839	56	427
Turn Type	Prot	NA	NA	Prot	Perm
Protected Phases	5	2	6	4	
Permitted Phases					4
Detector Phase	5	2	6	4	4
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.0	11.0	35.0	30.5	30.5
Total Split (s)	20.0	55.0	35.0	35.0	35.0
Total Split (%)	22.2%	61.1%	38.9%	38.9%	38.9%
Yellow Time (s)	3.0	5.0	5.0	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	6.0	6.0	4.5	4.5
Lead/Lag	Lead		Lag		
Lead-Lag Optimize?	Yes		Yes		
Recall Mode	None	C-Min	C-Min	None	None
Act Effct Green (s)	27.0	68.7	37.8	10.8	10.8
Actuated g/C Ratio	0.30	0.76	0.42	0.12	0.12
v/c Ratio	0.67	0.37	0.61	0.28	0.83
Control Delay	35.2	4.7	23.5	36.4	19.4
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	35.2	4.7	23.5	36.4	19.4
LOS	D	A	C	D	B
Approach Delay		12.7	23.5	21.4	
Approach LOS		B	C	C	

Intersection Summary

Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 32.5 (36%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.83
 Intersection Signal Delay: 17.8
 Intersection LOS: B
 Intersection Capacity Utilization 59.2%
 ICU Level of Service B
 Analysis Period (min) 15

Splits and Phases: 5: SR-74 & Trumble Road



HCM 6th Signalized Intersection Summary
5: SR-74 & Trumble Road



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↖	↑↑	↗		↖	↗
Traffic Volume (veh/h)	338	956	839	25	56	427
Future Volume (veh/h)	338	956	839	25	56	427
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1900
Adj Flow Rate, veh/h	360	1017	893	27	60	454
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	0	0	0	0	0	0
Cap, veh/h	322	2101	1287	39	545	485
Arrive On Green	0.18	0.58	0.36	0.36	0.30	0.30
Sat Flow, veh/h	1810	3705	3672	108	1810	1610
Grp Volume(v), veh/h	360	1017	451	469	60	454
Grp Sat Flow(s),veh/h/ln	1810	1805	1805	1881	1810	1610
Q Serve(g_s), s	16.0	14.8	19.2	19.2	2.2	24.7
Cycle Q Clear(g_c), s	16.0	14.8	19.2	19.2	2.2	24.7
Prop In Lane	1.00			0.06	1.00	1.00
Lane Grp Cap(c), veh/h	322	2101	650	677	545	485
V/C Ratio(X)	1.12	0.48	0.69	0.69	0.11	0.94
Avail Cap(c_a), veh/h	322	2101	650	677	613	546
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.90	0.90	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	37.0	10.9	24.6	24.6	22.7	30.6
Incr Delay (d2), s/veh	83.8	0.7	6.0	5.8	0.0	21.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	14.1	4.9	8.4	8.7	0.9	22.2
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	120.8	11.7	30.6	30.4	22.8	52.1
LnGrp LOS	F	B	C	C	C	D
Approach Vol, veh/h		1377	920		514	
Approach Delay, s/veh		40.2	30.5		48.7	
Approach LOS		D	C		D	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		58.4		31.6	20.0	38.4
Change Period (Y+Rc), s		6.0		4.5	4.0	6.0
Max Green Setting (Gmax), s		49.0		30.5	16.0	29.0
Max Q Clear Time (g_c+I1), s		16.8		26.7	18.0	21.2
Green Ext Time (p_c), s		7.4		0.4	0.0	3.1
Intersection Summary						
HCM 6th Ctrl Delay			38.6			
HCM 6th LOS			D			

Intersection						
Int Delay, s/veh	7.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑			↑		↑
Traffic Vol, veh/h	0	0	6	0	0	9
Future Vol, veh/h	0	0	6	0	0	9
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	7	0	0	10

Major/Minor	Major1	Major2	Minor1	Minor2	Minor3
Conflicting Flow All	0	-	1	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	-	4.12	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	-	2.218	-	-
Pot Cap-1 Maneuver	-	0	1622	-	0
Stage 1	-	0	-	-	0
Stage 2	-	0	-	-	0
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1622	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	WB	NB
HCM Control Delay, s	0	7.2	8.4
HCM LOS			A

Minor Lane/Major Mvmt	NBLn1	EBT	WBL	WBT
Capacity (veh/h)	1084	-	1622	-
HCM Lane V/C Ratio	0.009	-	0.004	-
HCM Control Delay (s)	8.4	-	7.2	0
HCM Lane LOS	A	-	A	A
HCM 95th %tile Q(veh)	0	-	0	-

Intersection						
Int Delay, s/veh	0.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔		↔	↑↑	↑↑	
Traffic Vol, veh/h	0	18	11	341	275	0
Future Vol, veh/h	0	18	11	341	275	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	50	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	20	12	371	299	0

Major/Minor	Minor2	Major1		Major2	
Conflicting Flow All	509	150	299	0	0
Stage 1	299	-	-	-	-
Stage 2	210	-	-	-	-
Critical Hdwy	6.84	6.94	4.14	-	-
Critical Hdwy Stg 1	5.84	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-
Follow-up Hdwy	3.52	3.32	2.22	-	-
Pot Cap-1 Maneuver	494	870	1259	-	-
Stage 1	726	-	-	-	-
Stage 2	805	-	-	-	-
Platoon blocked, %				-	-
Mov Cap-1 Maneuver	489	870	1259	-	-
Mov Cap-2 Maneuver	568	-	-	-	-
Stage 1	719	-	-	-	-
Stage 2	805	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	9.2	0.2	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1259	-	870	-	-
HCM Lane V/C Ratio	0.009	-	0.022	-	-
HCM Control Delay (s)	7.9	-	9.2	-	-
HCM Lane LOS	A	-	A	-	-
HCM 95th %tile Q(veh)	0	-	0.1	-	-

Timings
1: SR-74/I-215 SB Ramps & Bonnie Drive

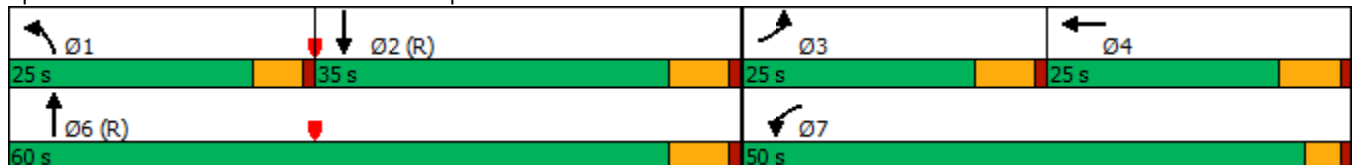


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø4	Ø7
Lane Configurations								
Traffic Volume (vph)	69	263	336	761	761	65		
Future Volume (vph)	69	263	336	761	761	65		
Turn Type	Prot	Free	Prot	NA	NA	Free		
Protected Phases	3		1	6	2		4	7
Permitted Phases		Free				Free		
Detector Phase	3		1	6	2			
Switch Phase								
Minimum Initial (s)	5.0		5.0	5.0	5.0		5.0	5.0
Minimum Split (s)	11.0		10.0	11.0	11.0		11.0	9.0
Total Split (s)	25.0		25.0	60.0	35.0		25.0	50.0
Total Split (%)	22.7%		22.7%	54.5%	31.8%		23%	45%
Yellow Time (s)	5.0		4.0	5.0	5.0		5.0	3.0
All-Red Time (s)	1.0		1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			
Total Lost Time (s)	6.0		5.0	6.0	6.0			
Lead/Lag	Lead		Lead		Lag		Lag	
Lead-Lag Optimize?	Yes		Yes		Yes		Yes	
Recall Mode	None		None	C-Min	C-Min		None	None
Act Effct Green (s)	9.0	110.0	32.4	92.5	53.9	110.0		
Actuated g/C Ratio	0.08	1.00	0.29	0.84	0.49	1.00		
v/c Ratio	0.51	0.18	0.69	0.52	0.89	0.04		
Control Delay	59.8	0.2	42.5	4.9	40.0	0.0		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	59.8	0.2	42.5	4.9	40.0	0.0		
LOS	E	A	D	A	D	A		
Approach Delay				16.4	36.8			
Approach LOS				B	D			

Intersection Summary

Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 23 (21%), Referenced to phase 2:SBT and 6:NBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.89
 Intersection Signal Delay: 23.3
 Intersection LOS: C
 Intersection Capacity Utilization 75.0%
 ICU Level of Service D
 Analysis Period (min) 15

Splits and Phases: 1: SR-74/I-215 SB Ramps & Bonnie Drive



HCM 6th Signalized Intersection Summary
 1: SR-74/I-215 SB Ramps & Bonnie Drive

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	69	0	263	0	0	0	336	761	0	0	761	65
Future Volume (veh/h)	69	0	263	0	0	0	336	761	0	0	761	65
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	0	1900	1900	1900	0	1900	1900	1900	0	1900	1900
Adj Flow Rate, veh/h	75	0	0	0	0	0	365	827	0	0	827	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	0	0	0	0	0	0	0	0	0	0
Cap, veh/h	97	0		0	2	0	329	1591	0	0	1159	
Arrive On Green	0.05	0.00	0.00	0.00	0.00	0.00	0.18	0.84	0.00	0.00	0.61	0.00
Sat Flow, veh/h	1810	75		0	1900	0	1810	1900	0	0	1900	1610
Grp Volume(v), veh/h	75	56.3		0	0	0	365	827	0	0	827	0
Grp Sat Flow(s),veh/h/ln	1810	E		0	1900	0	1810	1900	0	0	1900	1610
Q Serve(g_s), s	4.5			0.0	0.0	0.0	20.0	13.8	0.0	0.0	33.1	0.0
Cycle Q Clear(g_c), s	4.5			0.0	0.0	0.0	20.0	13.8	0.0	0.0	33.1	0.0
Prop In Lane	1.00			0.00		0.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h	97			0	2	0	329	1591	0	0	1159	
V/C Ratio(X)	0.77			0.00	0.00	0.00	1.11	0.52	0.00	0.00	0.71	
Avail Cap(c_a), veh/h	313			0	328	0	329	1591	0	0	1159	
HCM Platoon Ratio	1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00			0.00	0.00	0.00	1.00	1.00	0.00	0.00	1.00	0.00
Uniform Delay (d), s/veh	51.4			0.0	0.0	0.0	45.0	2.6	0.0	0.0	14.8	0.0
Incr Delay (d2), s/veh	4.9			0.0	0.0	0.0	82.4	1.2	0.0	0.0	3.8	0.0
Initial Q Delay(d3),s/veh	0.0			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.1			0.0	0.0	0.0	16.1	2.3	0.0	0.0	13.1	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	56.3			0.0	0.0	0.0	127.4	3.8	0.0	0.0	18.6	0.0
LnGrp LOS	E			A	A	A	F	A	A	A	B	
Approach Vol, veh/h					0			1192			827	
Approach Delay, s/veh					0.0			41.6			18.6	
Approach LOS								D			B	
Timer - Assigned Phs	1	2	3	4		6	7					
Phs Duration (G+Y+Rc), s	25.0	73.1	11.9	0.0		98.1	11.9					
Change Period (Y+Rc), s	5.0	6.0	6.0	6.0		6.0	4.0					
Max Green Setting (Gmax), s	20.0	29.0	19.0	19.0		54.0	46.0					
Max Q Clear Time (g_c+I1), s	22.0	35.1	6.5	0.0		15.8	0.0					
Green Ext Time (p_c), s	0.0	0.0	0.1	0.0		3.4	0.0					

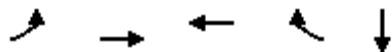
Intersection Summary

HCM 6th Ctrl Delay	33.0
HCM 6th LOS	C

Notes

Unsignalized Delay for [EBR, SBR] is excluded from calculations of the approach delay and intersection delay.

Timings
2: SR-74 & I-215 NB Ramps

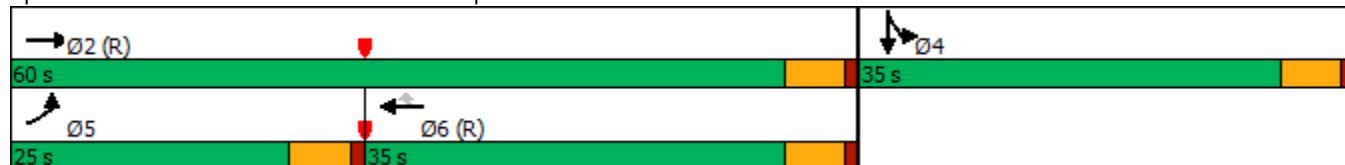


Lane Group	EBL	EBT	WBT	WBR	SBT
Lane Configurations	↖	↗↗	↗↗	↖	↔
Traffic Volume (vph)	39	985	1036	1068	0
Future Volume (vph)	39	985	1036	1068	0
Turn Type	Prot	NA	NA	Perm	NA
Protected Phases	5	2	6		4
Permitted Phases				6	
Detector Phase	5	2	6	6	4
Switch Phase					
Minimum Initial (s)	5.0	50.0	5.0	5.0	5.0
Minimum Split (s)	10.3	55.3	10.3	10.3	10.3
Total Split (s)	25.0	60.0	35.0	35.0	35.0
Total Split (%)	26.3%	63.2%	36.8%	36.8%	36.8%
Yellow Time (s)	4.3	4.3	4.3	4.3	4.3
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.3
Lead/Lag	Lead		Lag	Lag	
Lead-Lag Optimize?	Yes		Yes	Yes	
Recall Mode	None	C-Min	C-Min	C-Min	None
Act Effct Green (s)	6.8	62.9	55.1	55.1	21.5
Actuated g/C Ratio	0.07	0.66	0.58	0.58	0.23
v/c Ratio	0.32	0.44	0.53	0.84	0.84
Control Delay	47.8	9.2	15.9	10.5	42.3
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	47.8	9.2	15.9	10.5	42.3
LOS	D	A	B	B	D
Approach Delay		10.7	13.2		42.3
Approach LOS		B	B		D

Intersection Summary

Cycle Length: 95
 Actuated Cycle Length: 95
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 80
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.84
 Intersection Signal Delay: 15.5
 Intersection LOS: B
 Intersection Capacity Utilization 79.1%
 ICU Level of Service D
 Analysis Period (min) 15

Splits and Phases: 2: SR-74 & I-215 NB Ramps



HCM 6th Signalized Intersection Summary
2: SR-74 & I-215 NB Ramps

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	39	985	0	0	1036	1068	0	0	0	309	0	60
Future Volume (veh/h)	39	985	0	0	1036	1068	0	0	0	309	0	60
Initial Q (Qb), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00				1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Work Zone On Approach		No			No						No	
Adj Sat Flow, veh/h/ln	1900	1900	0	0	1900	1900				1900	1900	1900
Adj Flow Rate, veh/h	41	1048	0	0	1102	0				329	0	64
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94				0.94	0.94	0.94
Percent Heavy Veh, %	0	0	0	0	0	0				0	0	0
Cap, veh/h	63	2323	0	0	1996					364	0	71
Arrive On Green	0.03	0.64	0.00	0.00	0.55	0.00				0.25	0.00	0.25
Sat Flow, veh/h	1810	3705	0	0	3705	1610				1485	0	289
Grp Volume(v), veh/h	41	1048	0	0	1102	0				393	0	0
Grp Sat Flow(s),veh/h/ln	1810	1805	0	0	1805	1610				1774	0	0
Q Serve(g_s), s	2.1	13.9	0.0	0.0	18.7	0.0				20.4	0.0	0.0
Cycle Q Clear(g_c), s	2.1	13.9	0.0	0.0	18.7	0.0				20.4	0.0	0.0
Prop In Lane	1.00		0.00	0.00		1.00				0.84		0.16
Lane Grp Cap(c), veh/h	63	2323	0	0	1996					435	0	0
V/C Ratio(X)	0.65	0.45	0.00	0.00	0.55					0.90	0.00	0.00
Avail Cap(c_a), veh/h	375	2323	0	0	1996					555	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	0.00	0.09	0.00				1.00	0.00	0.00
Uniform Delay (d), s/veh	45.3	8.5	0.0	0.0	13.7	0.0				34.8	0.0	0.0
Incr Delay (d2), s/veh	4.2	0.6	0.0	0.0	0.1	0.0				13.8	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.0	4.4	0.0	0.0	7.3	0.0				9.9	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	49.5	9.1	0.0	0.0	13.8	0.0				48.5	0.0	0.0
LnGrp LOS	D	A	A	A	B					D	A	A
Approach Vol, veh/h		1089			1102						393	
Approach Delay, s/veh		10.7			13.8						48.5	
Approach LOS		B			B						D	
Timer - Assigned Phs		2		4	5	6						
Phs Duration (G+Y+Rc), s		66.4		28.6	8.6	57.8						
Change Period (Y+Rc), s		5.3		5.3	5.3	5.3						
Max Green Setting (Gmax), s		54.7		29.7	19.7	29.7						
Max Q Clear Time (g_c+I1), s		15.9		22.4	4.1	20.7						
Green Ext Time (p_c), s		4.7		0.9	0.0	4.0						
Intersection Summary												
HCM 6th Ctrl Delay				17.8								
HCM 6th LOS				B								
Notes												
Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.												

Intersection												
Intersection Delay, s/veh	12.9											
Intersection LOS	B											

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔		↔	↔		↔	↔		↔	↔	
Traffic Vol, veh/h	3	19	58	427	18	3	58	58	234	1	80	2
Future Vol, veh/h	3	19	58	427	18	3	58	58	234	1	80	2
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	3	20	62	459	19	3	62	62	252	1	86	2
Number of Lanes	0	2	0	2	1	0	1	2	0	1	2	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	3	2	3	3
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	3	3	2	3
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	3	3	3	2
HCM Control Delay	10.1	14.1	12.5	10.7
HCM LOS	B	B	B	B

Lane	NBLn1	NBLn2	NBLn3	EBLn1	EBLn2	WBLn1	WBLn2	WBLn3	SBLn1	SBLn2	SBLn3
Vol Left, %	100%	0%	0%	24%	0%	100%	100%	0%	100%	0%	0%
Vol Thru, %	0%	100%	8%	76%	14%	0%	0%	86%	0%	100%	93%
Vol Right, %	0%	0%	92%	0%	86%	0%	0%	14%	0%	0%	7%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	58	39	253	13	68	214	214	21	1	53	29
LT Vol	58	0	0	3	0	214	214	0	1	0	0
Through Vol	0	39	19	10	10	0	0	18	0	53	27
RT Vol	0	0	234	0	58	0	0	3	0	0	2
Lane Flow Rate	62	42	272	13	73	230	230	23	1	57	31
Geometry Grp	8	8	8	8	8	8	8	8	8	8	8
Degree of Util (X)	0.122	0.076	0.446	0.027	0.129	0.424	0.424	0.038	0.002	0.114	0.061
Departure Headway (Hd)	7.045	6.54	5.888	7.136	6.415	6.656	6.656	6.056	7.674	7.167	7.118
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Cap	508	546	610	500	557	539	539	590	465	498	501
Service Time	4.799	4.295	3.642	4.905	4.183	4.407	4.407	3.807	5.447	4.941	4.891
HCM Lane V/C Ratio	0.122	0.077	0.446	0.026	0.131	0.427	0.427	0.039	0.002	0.114	0.062
HCM Control Delay	10.8	9.8	13.3	10.1	10.1	14.3	14.3	9	10.5	10.9	10.4
HCM Lane LOS	B	A	B	B	B	B	B	A	B	B	B
HCM 95th-tile Q	0.4	0.2	2.3	0.1	0.4	2.1	2.1	0.1	0	0.4	0.2

Intersection												
Int Delay, s/veh	1.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	0	0	1	57	1	1	5	349	10	0	560	2
Future Vol, veh/h	0	0	1	57	1	1	5	349	10	0	560	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91	91	91	91	91
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	0	0	1	63	1	1	5	384	11	0	615	2

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	819	1021	309	708	1017	198	617	0	0	395	0	0
Stage 1	616	616	-	400	400	-	-	-	-	-	-	-
Stage 2	203	405	-	308	617	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	6.9	7.5	6.5	6.9	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	271	238	693	326	239	816	973	-	-	1175	-	-
Stage 1	450	485	-	603	605	-	-	-	-	-	-	-
Stage 2	786	602	-	683	484	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	269	237	693	324	238	816	973	-	-	1175	-	-
Mov Cap-2 Maneuver	269	237	-	324	238	-	-	-	-	-	-	-
Stage 1	448	485	-	600	602	-	-	-	-	-	-	-
Stage 2	779	599	-	682	484	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	10.2		18.8		0.1		0	
HCM LOS	B		C					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	973	-	-	693	325	1175	-	-
HCM Lane V/C Ratio	0.006	-	-	0.002	0.199	-	-	-
HCM Control Delay (s)	8.7	-	-	10.2	18.8	0	-	-
HCM Lane LOS	A	-	-	B	C	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0	0.7	0	-	-

Timings
5: SR-74 & Trumble Road

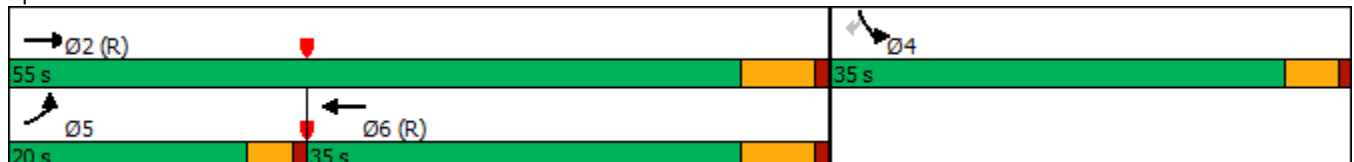


Lane Group	EBL	EBT	WBT	SBL	SBR
Lane Configurations	↖	↗↗	↗↖	↖	↗
Traffic Volume (vph)	332	967	1436	157	657
Future Volume (vph)	332	967	1436	157	657
Turn Type	Prot	NA	NA	Prot	Perm
Protected Phases	5	2	6	4	
Permitted Phases					4
Detector Phase	5	2	6	4	4
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.0	11.0	35.0	30.5	30.5
Total Split (s)	20.0	55.0	35.0	35.0	35.0
Total Split (%)	22.2%	61.1%	38.9%	38.9%	38.9%
Yellow Time (s)	3.0	5.0	5.0	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	6.0	6.0	4.5	4.5
Lead/Lag	Lead		Lag		
Lead-Lag Optimize?	Yes		Yes		
Recall Mode	None	C-Min	C-Min	None	None
Act Effct Green (s)	20.1	53.6	29.6	25.9	25.9
Actuated g/C Ratio	0.22	0.60	0.33	0.29	0.29
v/c Ratio	0.89	0.48	1.44	0.33	0.95
Control Delay	62.3	12.2	232.0	25.6	37.6
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	62.3	12.2	232.0	25.6	37.6
LOS	E	B	F	C	D
Approach Delay		25.0	232.0	35.3	
Approach LOS		C	F	D	

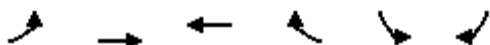
Intersection Summary

Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 32.5 (36%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 130
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.44
 Intersection Signal Delay: 115.8
 Intersection LOS: F
 Intersection Capacity Utilization 93.7%
 ICU Level of Service F
 Analysis Period (min) 15

Splits and Phases: 5: SR-74 & Trumble Road



HCM 6th Signalized Intersection Summary
5: SR-74 & Trumble Road



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↖	↑↑	↑↗		↘	↙
Traffic Volume (veh/h)	332	967	1436	144	157	657
Future Volume (veh/h)	332	967	1436	144	157	657
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1900
Adj Flow Rate, veh/h	357	1040	1544	155	169	706
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	0	0	0	0	0	0
Cap, veh/h	322	1965	1068	106	613	546
Arrive On Green	0.18	0.54	0.32	0.32	0.34	0.34
Sat Flow, veh/h	1810	3705	3411	330	1810	1610
Grp Volume(v), veh/h	357	1040	834	865	169	706
Grp Sat Flow(s),veh/h/ln	1810	1805	1805	1841	1810	1610
Q Serve(g_s), s	16.0	16.6	29.0	29.0	6.1	30.5
Cycle Q Clear(g_c), s	16.0	16.6	29.0	29.0	6.1	30.5
Prop In Lane	1.00			0.18	1.00	1.00
Lane Grp Cap(c), veh/h	322	1965	582	593	613	546
V/C Ratio(X)	1.11	0.53	1.43	1.46	0.28	1.29
Avail Cap(c_a), veh/h	322	1965	582	593	613	546
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.90	0.90	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	37.0	13.1	30.5	30.5	21.7	29.8
Incr Delay (d2), s/veh	80.6	0.9	204.8	215.7	0.1	145.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	13.8	5.8	44.4	47.0	2.4	44.7
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	117.6	14.0	235.3	246.2	21.8	175.2
LnGrp LOS	F	B	F	F	C	F
Approach Vol, veh/h		1397	1699		875	
Approach Delay, s/veh		40.5	240.9		145.6	
Approach LOS		D	F		F	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		55.0		35.0	20.0	35.0
Change Period (Y+Rc), s		6.0		4.5	4.0	6.0
Max Green Setting (Gmax), s		49.0		30.5	16.0	29.0
Max Q Clear Time (g_c+I1), s		18.6		32.5	18.0	31.0
Green Ext Time (p_c), s		7.6		0.0	0.0	0.0
Intersection Summary						
HCM 6th Ctrl Delay			149.4			
HCM 6th LOS			F			

Timings
1: SR-74/I-215 SB Ramps & Bonnie Drive

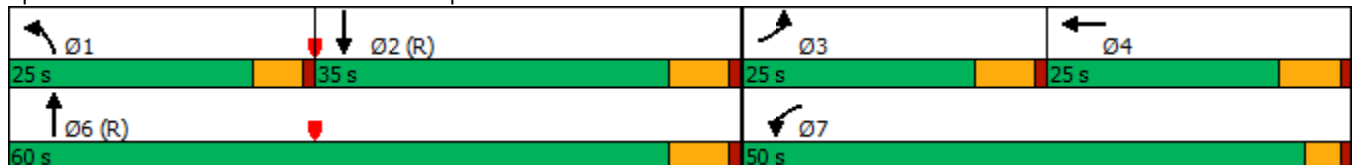


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø4	Ø7
Lane Configurations								
Traffic Volume (vph)	83	389	389	512	1095	54		
Future Volume (vph)	83	389	389	512	1095	54		
Turn Type	Prot	Free	Prot	NA	NA	Free		
Protected Phases	3		1	6	2		4	7
Permitted Phases		Free				Free		
Detector Phase	3		1	6	2			
Switch Phase								
Minimum Initial (s)	5.0		5.0	5.0	5.0		5.0	5.0
Minimum Split (s)	11.0		10.0	11.0	11.0		11.0	9.0
Total Split (s)	25.0		25.0	60.0	35.0		25.0	50.0
Total Split (%)	22.7%		22.7%	54.5%	31.8%		23%	45%
Yellow Time (s)	5.0		4.0	5.0	5.0		5.0	3.0
All-Red Time (s)	1.0		1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			
Total Lost Time (s)	6.0		5.0	6.0	6.0			
Lead/Lag	Lead		Lead		Lag		Lag	
Lead-Lag Optimize?	Yes		Yes		Yes		Yes	
Recall Mode	None		None	C-Min	C-Min		None	None
Act Effct Green (s)	10.0	110.0	40.6	91.6	44.8	110.0		
Actuated g/C Ratio	0.09	1.00	0.37	0.83	0.41	1.00		
v/c Ratio	0.56	0.27	0.65	0.36	1.58	0.04		
Control Delay	60.4	0.4	36.2	3.9	292.3	0.0		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	60.4	0.4	36.2	3.9	292.3	0.0		
LOS	E	A	D	A	F	A		
Approach Delay				17.8	278.6			
Approach LOS				B	F			

Intersection Summary

Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 23 (21%), Referenced to phase 2:SBT and 6:NBT, Start of Green
 Natural Cycle: 150
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.58
 Intersection Signal Delay: 135.4
 Intersection LOS: F
 Intersection Capacity Utilization 96.3%
 ICU Level of Service F
 Analysis Period (min) 15

Splits and Phases: 1: SR-74/I-215 SB Ramps & Bonnie Drive



HCM 6th Signalized Intersection Summary
1: SR-74/I-215 SB Ramps & Bonnie Drive



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	83	0	389	0	0	0	389	512	0	0	1095	54
Future Volume (veh/h)	83	0	389	0	0	0	389	512	0	0	1095	54
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	0	1900	1900	1900	0	1900	1900	1900	0	1900	1900
Adj Flow Rate, veh/h	92	0	0	0	0	0	432	569	0	0	1217	0
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	0	0	0	0	0	0	0	0	0	0	0	0
Cap, veh/h	117	0		0	2	0	329	1570	0	0	1138	
Arrive On Green	0.06	0.00	0.00	0.00	0.00	0.00	0.18	0.83	0.00	0.00	0.60	0.00
Sat Flow, veh/h	1810	92		0	1900	0	1810	1900	0	0	1900	1610
Grp Volume(v), veh/h	92	55.0		0	0	0	432	569	0	0	1217	0
Grp Sat Flow(s),veh/h/ln	1810	E		0	1900	0	1810	1900	0	0	1900	1610
Q Serve(g_s), s	5.5			0.0	0.0	0.0	20.0	8.2	0.0	0.0	65.9	0.0
Cycle Q Clear(g_c), s	5.5			0.0	0.0	0.0	20.0	8.2	0.0	0.0	65.9	0.0
Prop In Lane	1.00			0.00		0.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h	117			0	2	0	329	1570	0	0	1138	
V/C Ratio(X)	0.79			0.00	0.00	0.00	1.31	0.36	0.00	0.00	1.07	
Avail Cap(c_a), veh/h	313			0	328	0	329	1570	0	0	1138	
HCM Platoon Ratio	1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00			0.00	0.00	0.00	1.00	1.00	0.00	0.00	1.00	0.00
Uniform Delay (d), s/veh	50.7			0.0	0.0	0.0	45.0	2.4	0.0	0.0	22.1	0.0
Incr Delay (d2), s/veh	4.3			0.0	0.0	0.0	161.0	0.7	0.0	0.0	47.3	0.0
Initial Q Delay(d3),s/veh	0.0			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.5			0.0	0.0	0.0	23.2	1.5	0.0	0.0	39.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	55.0			0.0	0.0	0.0	206.0	3.0	0.0	0.0	69.4	0.0
LnGrp LOS	E			A	A	A	F	A	A	A	F	
Approach Vol, veh/h					0			1001			1217	
Approach Delay, s/veh					0.0			90.6			69.4	
Approach LOS								F			E	
Timer - Assigned Phs	1	2	3	4		6	7					
Phs Duration (G+Y+Rc), s	25.0	71.9	13.1	0.0		96.9	13.1					
Change Period (Y+Rc), s	5.0	6.0	6.0	6.0		6.0	4.0					
Max Green Setting (Gmax), s	20.0	29.0	19.0	19.0		54.0	46.0					
Max Q Clear Time (g_c+I1), s	22.0	67.9	7.5	0.0		10.2	0.0					
Green Ext Time (p_c), s	0.0	0.0	0.1	0.0		2.0	0.0					

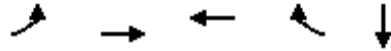
Intersection Summary

HCM 6th Ctrl Delay	78.0
HCM 6th LOS	E

Notes

Unsignalized Delay for [EBR, SBR] is excluded from calculations of the approach delay and intersection delay.

Timings
2: SR-74 & I-215 NB Ramps

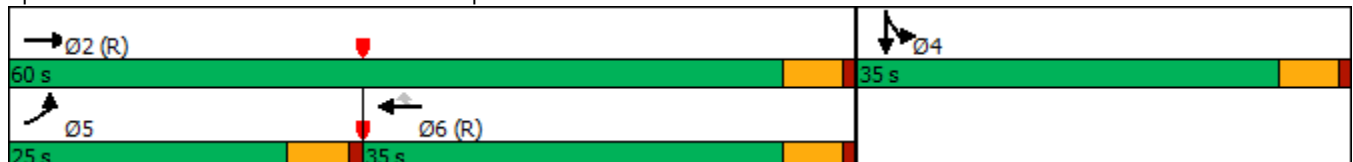


Lane Group	EBL	EBT	WBT	WBR	SBT
Lane Configurations					
Traffic Volume (vph)	54	1431	813	951	0
Future Volume (vph)	54	1431	813	951	0
Turn Type	Prot	NA	NA	Perm	NA
Protected Phases	5	2	6		4
Permitted Phases				6	
Detector Phase	5	2	6	6	4
Switch Phase					
Minimum Initial (s)	5.0	50.0	5.0	5.0	5.0
Minimum Split (s)	10.3	55.3	10.3	10.3	10.3
Total Split (s)	25.0	60.0	35.0	35.0	35.0
Total Split (%)	26.3%	63.2%	36.8%	36.8%	36.8%
Yellow Time (s)	4.3	4.3	4.3	4.3	4.3
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.3
Lead/Lag	Lead		Lag	Lag	
Lead-Lag Optimize?	Yes		Yes	Yes	
Recall Mode	None	C-Min	C-Min	C-Min	None
Act Effct Green (s)	7.6	53.8	42.9	42.9	30.6
Actuated g/C Ratio	0.08	0.57	0.45	0.45	0.32
v/c Ratio	0.40	0.75	0.54	0.82	1.03
Control Delay	49.0	18.4	21.1	8.8	73.7
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	49.0	18.4	21.1	8.8	73.7
LOS	D	B	C	A	E
Approach Delay		19.5	14.5		73.7
Approach LOS		B	B		E


















Intersection Summary

Cycle Length: 95
 Actuated Cycle Length: 95
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.03
 Intersection Signal Delay: 25.8
 Intersection LOS: C
 Intersection Capacity Utilization 84.7%
 ICU Level of Service E
 Analysis Period (min) 15

Splits and Phases: 2: SR-74 & I-215 NB Ramps



HCM 6th Signalized Intersection Summary
2: SR-74 & I-215 NB Ramps

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	54	1431	0	0	813	951	0	0	0	522	0	87
Future Volume (veh/h)	54	1431	0	0	813	951	0	0	0	522	0	87
Initial Q (Qb), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00				1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Work Zone On Approach		No			No						No	
Adj Sat Flow, veh/h/ln	1900	1900	0	0	1900	1900				1900	1900	1900
Adj Flow Rate, veh/h	58	1539	0	0	874	0				561	0	94
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93				0.93	0.93	0.93
Percent Heavy Veh, %	0	0	0	0	0	0				0	0	0
Cap, veh/h	75	2079	0	0	1727					476	0	80
Arrive On Green	0.04	0.58	0.00	0.00	0.48	0.00				0.31	0.00	0.31
Sat Flow, veh/h	1810	3705	0	0	3705	1610				1523	0	255
Grp Volume(v), veh/h	58	1539	0	0	874	0				655	0	0
Grp Sat Flow(s),veh/h/ln	1810	1805	0	0	1805	1610				1778	0	0
Q Serve(g_s), s	3.0	29.9	0.0	0.0	15.8	0.0				29.7	0.0	0.0
Cycle Q Clear(g_c), s	3.0	29.9	0.0	0.0	15.8	0.0				29.7	0.0	0.0
Prop In Lane	1.00		0.00	0.00		1.00				0.86		0.14
Lane Grp Cap(c), veh/h	75	2079	0	0	1727					556	0	0
V/C Ratio(X)	0.77	0.74	0.00	0.00	0.51					1.18	0.00	0.00
Avail Cap(c_a), veh/h	375	2079	0	0	1727					556	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	0.00	0.09	0.00				1.00	0.00	0.00
Uniform Delay (d), s/veh	45.1	14.9	0.0	0.0	17.1	0.0				32.7	0.0	0.0
Incr Delay (d2), s/veh	6.0	2.4	0.0	0.0	0.1	0.0				97.8	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.4	10.6	0.0	0.0	6.4	0.0				26.9	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	51.1	17.3	0.0	0.0	17.2	0.0				130.5	0.0	0.0
LnGrp LOS	D	B	A	A	B					F	A	A
Approach Vol, veh/h		1597			874						655	
Approach Delay, s/veh		18.5			17.2						130.5	
Approach LOS		B			B						F	
Timer - Assigned Phs		2		4	5	6						
Phs Duration (G+Y+Rc), s		60.0		35.0	9.3	50.7						
Change Period (Y+Rc), s		5.3		5.3	5.3	5.3						
Max Green Setting (Gmax), s		54.7		29.7	19.7	29.7						
Max Q Clear Time (g_c+I1), s		31.9		31.7	5.0	17.8						
Green Ext Time (p_c), s		7.5		0.0	0.0	3.6						
Intersection Summary												
HCM 6th Ctrl Delay				41.6								
HCM 6th LOS				D								
Notes												
Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.												

Intersection												
Intersection Delay, s/veh55.5												
Intersection LOS F												

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔		↔	↔		↔	↔		↔	↔	
Traffic Vol, veh/h	2	1	5	334	1	30	1	155	455	29	170	0
Future Vol, veh/h	2	1	5	334	1	30	1	155	455	29	170	0
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	3	1	6	434	1	39	1	201	591	38	221	0
Number of Lanes	0	2	0	2	1	0	1	2	0	1	2	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	3	2	3	3
Conflicting Approach Left SB		NB	EB	WB
Conflicting Lanes Left	3	3	2	3
Conflicting Approach Right NB		SB	WB	EB
Conflicting Lanes Right	3	3	3	2
HCM Control Delay	11.4	17.3	93.1	12
HCM LOS	B	C	F	B

Lane	NBLn1	NBLn2	NBLn3	EBLn1	EBLn2	WBLn1	WBLn2	WBLn3	SBLn1	SBLn2	SBLn3
Vol Left, %	100%	0%	0%	80%	0%	100%	100%	0%	100%	0%	0%
Vol Thru, %	0%	100%	10%	20%	9%	0%	0%	3%	0%	100%	100%
Vol Right, %	0%	0%	90%	0%	91%	0%	0%	97%	0%	0%	0%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	1	103	507	3	6	167	167	31	29	85	85
LT Vol	1	0	0	2	0	167	167	0	29	0	0
Through Vol	0	103	52	1	1	0	0	1	0	85	85
RT Vol	0	0	455	0	5	0	0	30	0	0	0
Lane Flow Rate	1	134	658	3	7	217	217	40	38	110	110
Geometry Grp	8	8	8	8	8	8	8	8	8	8	8
Degree of Util (X)	0.003	0.258	1.151	0.008	0.016	0.475	0.475	0.075	0.087	0.241	0.189
Departure Headway (Hd)	7.435	6.93	6.295	9.304	8.268	8.166	8.166	6.985	8.666	8.157	6.429
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Cap	479	517	572	387	435	445	445	516	416	443	562
Service Time	5.209	4.703	4.068	7.004	5.968	5.866	5.866	4.685	6.366	5.857	4.129
HCM Lane V/C Ratio	0.002	0.259	1.15	0.008	0.016	0.488	0.488	0.078	0.091	0.248	0.196
HCM Control Delay	10.2	12.1	109.8	12.1	11.1	18	18	10.3	12.2	13.4	10.6
HCM Lane LOS	B	B	F	B	B	C	C	B	B	B	B
HCM 95th-tile Q	0	1	21.8	0	0	2.5	2.5	0.2	0.3	0.9	0.7

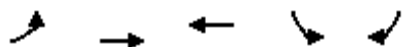
Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	0	0	5	30	0	1	2	610	1	1	510	0
Future Vol, veh/h	0	0	5	30	0	1	2	610	1	1	510	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	79	79	79	79	79	79	79	79	79	79	79	79
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	0	0	6	38	0	1	3	772	1	1	646	0

Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	1040	1427	323	1104	1427	387	646	0	0	773	0	0
Stage 1	648	648	-	779	779	-	-	-	-	-	-	-
Stage 2	392	779	-	325	648	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	6.9	7.5	6.5	6.9	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	187	136	679	168	136	617	949	-	-	851	-	-
Stage 1	430	469	-	359	409	-	-	-	-	-	-	-
Stage 2	610	409	-	667	469	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	186	135	679	166	135	617	949	-	-	851	-	-
Mov Cap-2 Maneuver	186	135	-	166	135	-	-	-	-	-	-	-
Stage 1	429	469	-	358	408	-	-	-	-	-	-	-
Stage 2	607	408	-	660	469	-	-	-	-	-	-	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	10.4			32.4			0			0		
HCM LOS	B			D								

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	949	-	-	679	170	851	-	-
HCM Lane V/C Ratio	0.003	-	-	0.009	0.231	0.001	-	-
HCM Control Delay (s)	8.8	-	-	10.4	32.4	9.2	-	-
HCM Lane LOS	A	-	-	B	D	A	-	-
HCM 95th %tile Q(veh)	0	-	-	0	0.9	0	-	-

Timings
5: SR-74 & Trumble Road

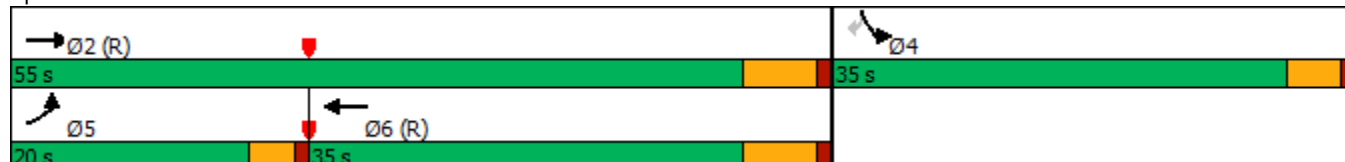


Lane Group	EBL	EBT	WBT	SBL	SBR
Lane Configurations	↗	↑↑	↑↑↔	↖	↖
Traffic Volume (vph)	521	1422	1189	172	579
Future Volume (vph)	521	1422	1189	172	579
Turn Type	Prot	NA	NA	Prot	Perm
Protected Phases	5	2	6	4	
Permitted Phases					4
Detector Phase	5	2	6	4	4
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.0	11.0	35.0	30.5	30.5
Total Split (s)	20.0	55.0	35.0	35.0	35.0
Total Split (%)	22.2%	61.1%	38.9%	38.9%	38.9%
Yellow Time (s)	3.0	5.0	5.0	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	6.0	6.0	4.5	4.5
Lead/Lag	Lead		Lag		
Lead-Lag Optimize?	Yes		Yes		
Recall Mode	None	C-Min	C-Min	None	None
Act Effct Green (s)	26.1	59.1	29.0	20.4	20.4
Actuated g/C Ratio	0.29	0.66	0.32	0.23	0.23
v/c Ratio	1.06	0.64	1.23	0.45	0.92
Control Delay	91.9	12.5	142.0	31.3	31.6
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	91.9	12.5	142.0	31.3	31.6
LOS	F	B	F	C	C
Approach Delay		33.8	142.0	31.5	
Approach LOS		C	F	C	

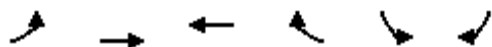
Intersection Summary

Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 32.5 (36%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 140
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.23
 Intersection Signal Delay: 69.3
 Intersection LOS: E
 Intersection Capacity Utilization 88.1%
 ICU Level of Service E
 Analysis Period (min) 15

Splits and Phases: 5: SR-74 & Trumble Road



HCM 6th Signalized Intersection Summary
5: SR-74 & Trumble Road



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↗	↑↑	↑↑		↖	↖
Traffic Volume (veh/h)	521	1422	1189	149	172	579
Future Volume (veh/h)	521	1422	1189	149	172	579
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1900
Adj Flow Rate, veh/h	554	1513	1265	159	183	616
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	0	0	0	0	0	0
Cap, veh/h	322	1965	1040	130	613	546
Arrive On Green	0.18	0.54	0.32	0.32	0.34	0.34
Sat Flow, veh/h	1810	3705	3323	404	1810	1610
Grp Volume(v), veh/h	554	1513	705	719	183	616
Grp Sat Flow(s),veh/h/ln	1810	1805	1805	1827	1810	1610
Q Serve(g_s), s	16.0	29.6	29.0	29.0	6.7	30.5
Cycle Q Clear(g_c), s	16.0	29.6	29.0	29.0	6.7	30.5
Prop In Lane	1.00			0.22	1.00	1.00
Lane Grp Cap(c), veh/h	322	1965	582	589	613	546
V/C Ratio(X)	1.72	0.77	1.21	1.22	0.30	1.13
Avail Cap(c_a), veh/h	322	1965	582	589	613	546
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.57	0.57	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	37.0	16.1	30.5	30.5	21.9	29.8
Incr Delay (d2), s/veh	332.4	1.7	110.5	114.4	0.1	79.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	36.3	10.4	29.1	30.1	2.7	34.6
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	369.4	17.8	141.0	144.9	22.0	108.9
LnGrp LOS	F	B	F	F	C	F
Approach Vol, veh/h		2067	1424		799	
Approach Delay, s/veh		112.0	143.0		89.0	
Approach LOS		F	F		F	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		55.0		35.0	20.0	35.0
Change Period (Y+Rc), s		6.0		4.5	4.0	6.0
Max Green Setting (Gmax), s		49.0		30.5	16.0	29.0
Max Q Clear Time (g_c+I1), s		31.6		32.5	18.0	31.0
Green Ext Time (p_c), s		9.6		0.0	0.0	0.0
Intersection Summary						
HCM 6th Ctrl Delay			118.0			
HCM 6th LOS			F			

Timings
1: SR-74/I-215 SB Ramps & Bonnie Drive

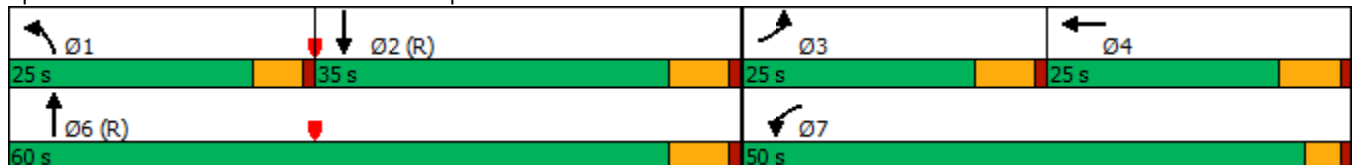


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø4	Ø7
Lane Configurations								
Traffic Volume (vph)	69	265	337	773	783	65		
Future Volume (vph)	69	265	337	773	783	65		
Turn Type	Prot	Free	Prot	NA	NA	Free		
Protected Phases	3		1	6	2		4	7
Permitted Phases		Free				Free		
Detector Phase	3		1	6	2			
Switch Phase								
Minimum Initial (s)	5.0		5.0	5.0	5.0		5.0	5.0
Minimum Split (s)	11.0		10.0	11.0	11.0		11.0	9.0
Total Split (s)	25.0		25.0	60.0	35.0		25.0	50.0
Total Split (%)	22.7%		22.7%	54.5%	31.8%		23%	45%
Yellow Time (s)	5.0		4.0	5.0	5.0		5.0	3.0
All-Red Time (s)	1.0		1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			
Total Lost Time (s)	6.0		5.0	6.0	6.0			
Lead/Lag	Lead		Lead		Lag		Lag	
Lead-Lag Optimize?	Yes		Yes		Yes		Yes	
Recall Mode	None		None	C-Min	C-Min		None	None
Act Effct Green (s)	9.0	110.0	32.5	92.5	53.8	110.0		
Actuated g/C Ratio	0.08	1.00	0.30	0.84	0.49	1.00		
v/c Ratio	0.51	0.18	0.69	0.53	0.92	0.04		
Control Delay	59.8	0.2	42.4	5.0	43.4	0.0		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	59.8	0.2	42.4	5.0	43.4	0.0		
LOS	E	A	D	A	D	A		
Approach Delay				16.4	40.0			
Approach LOS				B	D			




















Intersection Summary

Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 23 (21%), Referenced to phase 2:SBT and 6:NBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.92
 Intersection Signal Delay: 24.6
 Intersection LOS: C
 Intersection Capacity Utilization 76.2%
 ICU Level of Service D
 Analysis Period (min) 15

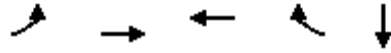
Splits and Phases: 1: SR-74/I-215 SB Ramps & Bonnie Drive



HCM 6th Signalized Intersection Summary
1: SR-74/I-215 SB Ramps & Bonnie Drive

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	69	0	265	0	0	0	337	773	0	0	783	65
Future Volume (veh/h)	69	0	265	0	0	0	337	773	0	0	783	65
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	0	1900	1900	1900	0	1900	1900	1900	0	1900	1900
Adj Flow Rate, veh/h	75	0	0	0	0	0	366	840	0	0	851	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	0	0	0	0	0	0	0	0	0	0
Cap, veh/h	97	0		0	2	0	329	1591	0	0	1159	
Arrive On Green	0.05	0.00	0.00	0.00	0.00	0.00	0.18	0.84	0.00	0.00	0.61	0.00
Sat Flow, veh/h	1810	75		0	1900	0	1810	1900	0	0	1900	1610
Grp Volume(v), veh/h	75	56.3		0	0	0	366	840	0	0	851	0
Grp Sat Flow(s),veh/h/ln	1810	E		0	1900	0	1810	1900	0	0	1900	1610
Q Serve(g_s), s	4.5			0.0	0.0	0.0	20.0	14.2	0.0	0.0	34.8	0.0
Cycle Q Clear(g_c), s	4.5			0.0	0.0	0.0	20.0	14.2	0.0	0.0	34.8	0.0
Prop In Lane	1.00			0.00		0.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h	97			0	2	0	329	1591	0	0	1159	
V/C Ratio(X)	0.77			0.00	0.00	0.00	1.11	0.53	0.00	0.00	0.73	
Avail Cap(c_a), veh/h	313			0	328	0	329	1591	0	0	1159	
HCM Platoon Ratio	1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00			0.00	0.00	0.00	1.00	1.00	0.00	0.00	1.00	0.00
Uniform Delay (d), s/veh	51.4			0.0	0.0	0.0	45.0	2.6	0.0	0.0	15.1	0.0
Incr Delay (d2), s/veh	4.9			0.0	0.0	0.0	83.4	1.3	0.0	0.0	4.1	0.0
Initial Q Delay(d3),s/veh	0.0			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.1			0.0	0.0	0.0	16.2	2.3	0.0	0.0	13.9	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	56.3			0.0	0.0	0.0	128.4	3.9	0.0	0.0	19.3	0.0
LnGrp LOS	E			A	A	A	F	A	A	A	B	
Approach Vol, veh/h					0			1206			851	
Approach Delay, s/veh					0.0			41.7			19.3	
Approach LOS								D			B	
Timer - Assigned Phs	1	2	3	4		6	7					
Phs Duration (G+Y+Rc), s	25.0	73.1	11.9	0.0		98.1	11.9					
Change Period (Y+Rc), s	5.0	6.0	6.0	6.0		6.0	4.0					
Max Green Setting (Gmax), s	20.0	29.0	19.0	19.0		54.0	46.0					
Max Q Clear Time (g_c+I1), s	22.0	36.8	6.5	0.0		16.2	0.0					
Green Ext Time (p_c), s	0.0	0.0	0.1	0.0		3.5	0.0					
Intersection Summary												
HCM 6th Ctrl Delay				33.2								
HCM 6th LOS				C								
Notes												
Unsignalized Delay for [EBR, SBR] is excluded from calculations of the approach delay and intersection delay.												

Timings
2: SR-74 & I-215 NB Ramps

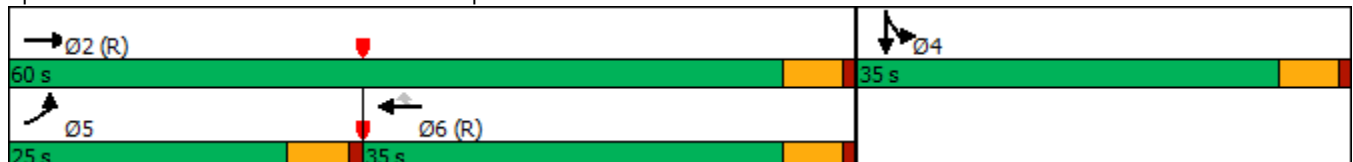


Lane Group	EBL	EBT	WBT	WBR	SBT
Lane Configurations	↖	↑↑	↑↑	↗	↔
Traffic Volume (vph)	39	1009	1049	1080	0
Future Volume (vph)	39	1009	1049	1080	0
Turn Type	Prot	NA	NA	Perm	NA
Protected Phases	5	2	6		4
Permitted Phases				6	
Detector Phase	5	2	6	6	4
Switch Phase					
Minimum Initial (s)	5.0	50.0	5.0	5.0	5.0
Minimum Split (s)	10.3	55.3	10.3	10.3	10.3
Total Split (s)	25.0	60.0	35.0	35.0	35.0
Total Split (%)	26.3%	63.2%	36.8%	36.8%	36.8%
Yellow Time (s)	4.3	4.3	4.3	4.3	4.3
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.3
Lead/Lag	Lead		Lag	Lag	
Lead-Lag Optimize?	Yes		Yes	Yes	
Recall Mode	None	C-Min	C-Min	C-Min	None
Act Effct Green (s)	6.8	61.9	54.0	54.0	22.5
Actuated g/C Ratio	0.07	0.65	0.57	0.57	0.24
v/c Ratio	0.32	0.46	0.54	0.85	0.86
Control Delay	47.8	9.8	16.7	11.4	43.9
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	47.8	9.8	16.7	11.4	43.9
LOS	D	A	B	B	D
Approach Delay		11.2	14.0		43.9
Approach LOS		B	B		D

Intersection Summary


















Cycle Length: 95
 Actuated Cycle Length: 95
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 80
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.86
 Intersection Signal Delay: 16.5
 Intersection LOS: B
 Intersection Capacity Utilization 79.9%
 ICU Level of Service D
 Analysis Period (min) 15

Splits and Phases: 2: SR-74 & I-215 NB Ramps



HCM 6th Signalized Intersection Summary
2: SR-74 & I-215 NB Ramps

Trumble and Mapes Warehouse Project
OY_WP_AM

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	39	1009	0	0	1049	1080	0	0	0	333	0	60
Future Volume (veh/h)	39	1009	0	0	1049	1080	0	0	0	333	0	60
Initial Q (Qb), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00				1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Work Zone On Approach		No			No						No	
Adj Sat Flow, veh/h/ln	1900	1900	0	0	1900	1900				1900	1900	1900
Adj Flow Rate, veh/h	41	1073	0	0	1116	0				354	0	64
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94				0.94	0.94	0.94
Percent Heavy Veh, %	0	0	0	0	0	0				0	0	0
Cap, veh/h	63	2275	0	0	1948					388	0	70
Arrive On Green	0.03	0.63	0.00	0.00	0.54	0.00				0.26	0.00	0.26
Sat Flow, veh/h	1810	3705	0	0	3705	1610				1504	0	272
Grp Volume(v), veh/h	41	1073	0	0	1116	0				418	0	0
Grp Sat Flow(s),veh/h/ln	1810	1805	0	0	1805	1610				1776	0	0
Q Serve(g_s), s	2.1	14.9	0.0	0.0	19.6	0.0				21.7	0.0	0.0
Cycle Q Clear(g_c), s	2.1	14.9	0.0	0.0	19.6	0.0				21.7	0.0	0.0
Prop In Lane	1.00		0.00	0.00		1.00				0.85		0.15
Lane Grp Cap(c), veh/h	63	2275	0	0	1948					458	0	0
V/C Ratio(X)	0.65	0.47	0.00	0.00	0.57					0.91	0.00	0.00
Avail Cap(c_a), veh/h	375	2275	0	0	1948					555	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	0.00	0.09	0.00				1.00	0.00	0.00
Uniform Delay (d), s/veh	45.3	9.2	0.0	0.0	14.6	0.0				34.2	0.0	0.0
Incr Delay (d2), s/veh	4.2	0.7	0.0	0.0	0.1	0.0				15.8	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.0	4.8	0.0	0.0	7.7	0.0				10.8	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	49.5	9.9	0.0	0.0	14.7	0.0				50.0	0.0	0.0
LnGrp LOS	D	A	A	A	B					D	A	A
Approach Vol, veh/h		1114			1116						418	
Approach Delay, s/veh		11.4			14.7						50.0	
Approach LOS		B			B						D	
Timer - Assigned Phs		2		4	5	6						
Phs Duration (G+Y+Rc), s		65.2		29.8	8.6	56.6						
Change Period (Y+Rc), s		5.3		5.3	5.3	5.3						
Max Green Setting (Gmax), s		54.7		29.7	19.7	29.7						
Max Q Clear Time (g_c+I1), s		16.9		23.7	4.1	21.6						
Green Ext Time (p_c), s		4.9		0.8	0.0	3.8						
Intersection Summary												
HCM 6th Ctrl Delay				18.9								
HCM 6th LOS				B								
Notes												
Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.												

Intersection												
Intersection Delay, s/veh	12.9											
Intersection LOS	B											

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔↔		↔↔	↔		↔	↔↔		↔	↔↔	
Traffic Vol, veh/h	3	20	60	427	20	3	66	58	234	1	80	4
Future Vol, veh/h	3	20	60	427	20	3	66	58	234	1	80	4
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	3	22	65	459	22	3	71	62	252	1	86	4
Number of Lanes	0	2	0	2	1	0	1	2	0	1	2	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	3	2	3	3
Conflicting Approach Left SB		NB	EB	WB
Conflicting Lanes Left	3	3	2	3
Conflicting Approach Right NB		SB	WB	EB
Conflicting Lanes Right	3	3	3	2
HCM Control Delay	10.2	14.1	12.6	10.7
HCM LOS	B	B	B	B

Lane	NBLn1	NBLn2	NBLn3	EBLn1	EBLn2	WBLn1	WBLn2	WBLn3	SBLn1	SBLn2	SBLn3
Vol Left, %	100%	0%	0%	23%	0%	100%	100%	0%	100%	0%	0%
Vol Thru, %	0%	100%	8%	77%	14%	0%	0%	87%	0%	100%	87%
Vol Right, %	0%	0%	92%	0%	86%	0%	0%	13%	0%	0%	13%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	66	39	253	13	70	214	214	23	1	53	31
LT Vol	66	0	0	3	0	214	214	0	1	0	0
Through Vol	0	39	19	10	10	0	0	20	0	53	27
RT Vol	0	0	234	0	60	0	0	3	0	0	4
Lane Flow Rate	71	42	272	14	75	230	230	25	1	57	33
Geometry Grp	8	8	8	8	8	8	8	8	8	8	8
Degree of Util (X)	0.139	0.076	0.448	0.028	0.135	0.427	0.427	0.042	0.002	0.115	0.065
Departure Headway (Hd)	7.074	6.57	5.917	7.174	6.458	6.7	6.7	6.109	7.72	7.213	7.121
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Cap	506	544	607	497	552	537	537	585	462	495	501
Service Time	4.831	4.327	3.674	4.944	4.229	4.451	4.451	3.86	5.496	4.989	4.897
HCM Lane V/C Ratio	0.14	0.077	0.448	0.028	0.136	0.428	0.428	0.043	0.002	0.115	0.066
HCM Control Delay	11	9.9	13.4	10.2	10.2	14.4	14.4	9.1	10.5	10.9	10.4
HCM Lane LOS	B	A	B	B	B	B	B	A	B	B	B
HCM 95th-tile Q	0.5	0.2	2.3	0.1	0.5	2.1	2.1	0.1	0	0.4	0.2

Intersection

Int Delay, s/veh 1.7

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	0	0	21	57	1	1	30	378	10	0	567	2
Future Vol, veh/h	0	0	21	57	1	1	30	378	10	0	567	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91	91	91	91	91
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	0	0	23	63	1	1	33	415	11	0	623	2

Major/Minor	Minor2		Minor1			Major1			Major2			
Conflicting Flow All	898	1116	313	799	1112	213	625	0	0	426	0	0
Stage 1	624	624	-	487	487	-	-	-	-	-	-	-
Stage 2	274	492	-	312	625	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	6.9	7.5	6.5	6.9	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	237	209	689	280	211	798	966	-	-	1144	-	-
Stage 1	445	481	-	536	554	-	-	-	-	-	-	-
Stage 2	714	551	-	679	480	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	230	202	689	263	204	798	966	-	-	1144	-	-
Mov Cap-2 Maneuver	230	202	-	263	204	-	-	-	-	-	-	-
Stage 1	430	481	-	518	535	-	-	-	-	-	-	-
Stage 2	687	532	-	656	480	-	-	-	-	-	-	-

Approach	EB		WB			NB			SB		
HCM Control Delay, s	10.4		22.9			0.6			0		
HCM LOS	B		C								

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	966	-	-	689	265	1144	-	-
HCM Lane V/C Ratio	0.034	-	-	0.033	0.245	-	-	-
HCM Control Delay (s)	8.9	-	-	10.4	22.9	0	-	-
HCM Lane LOS	A	-	-	B	C	A	-	-
HCM 95th %tile Q(veh)	0.1	-	-	0.1	0.9	0	-	-

Timings
5: SR-74 & Trumble Road

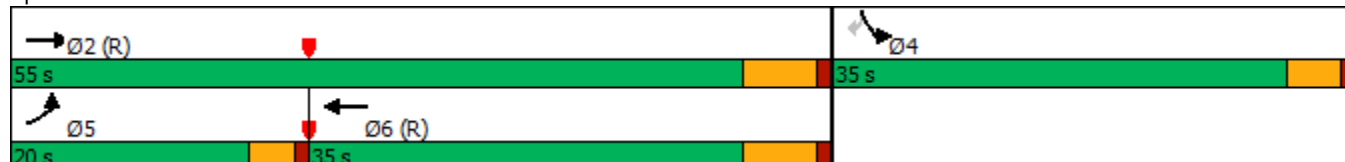


Lane Group	EBL	EBT	WBT	SBL	SBR
Lane Configurations	↘	↑↑	↑↑↔	↘	↗
Traffic Volume (vph)	380	967	1436	159	682
Future Volume (vph)	380	967	1436	159	682
Turn Type	Prot	NA	NA	Prot	Perm
Protected Phases	5	2	6	4	
Permitted Phases					4
Detector Phase	5	2	6	4	4
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.0	11.0	35.0	30.5	30.5
Total Split (s)	20.0	55.0	35.0	35.0	35.0
Total Split (%)	22.2%	61.1%	38.9%	38.9%	38.9%
Yellow Time (s)	3.0	5.0	5.0	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	6.0	6.0	4.5	4.5
Lead/Lag	Lead		Lag		
Lead-Lag Optimize?	Yes		Yes		
Recall Mode	None	C-Min	C-Min	None	None
Act Effct Green (s)	19.3	52.3	29.0	27.2	27.2
Actuated g/C Ratio	0.21	0.58	0.32	0.30	0.30
v/c Ratio	1.06	0.50	1.48	0.31	0.96
Control Delay	101.0	12.9	245.9	24.8	40.2
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	101.0	12.9	245.9	24.8	40.2
LOS	F	B	F	C	D
Approach Delay		37.7	245.9	37.3	
Approach LOS		D	F	D	

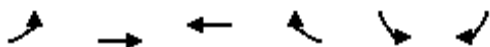
Intersection Summary

Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 32.5 (36%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 140
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.48
 Intersection Signal Delay: 125.1
 Intersection LOS: F
 Intersection Capacity Utilization 95.5%
 ICU Level of Service F
 Analysis Period (min) 15

Splits and Phases: 5: SR-74 & Trumble Road



HCM 6th Signalized Intersection Summary
5: SR-74 & Trumble Road



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↖	↑↑	↑↗		↘	↘
Traffic Volume (veh/h)	380	967	1436	150	159	682
Future Volume (veh/h)	380	967	1436	150	159	682
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1900
Adj Flow Rate, veh/h	409	1040	1544	161	171	733
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	0	0	0	0	0	0
Cap, veh/h	322	1965	1064	110	613	546
Arrive On Green	0.18	0.54	0.32	0.32	0.34	0.34
Sat Flow, veh/h	1810	3705	3398	341	1810	1610
Grp Volume(v), veh/h	409	1040	837	868	171	733
Grp Sat Flow(s),veh/h/ln	1810	1805	1805	1839	1810	1610
Q Serve(g_s), s	16.0	16.6	29.0	29.0	6.2	30.5
Cycle Q Clear(g_c), s	16.0	16.6	29.0	29.0	6.2	30.5
Prop In Lane	1.00			0.19	1.00	1.00
Lane Grp Cap(c), veh/h	322	1965	582	592	613	546
V/C Ratio(X)	1.27	0.53	1.44	1.47	0.28	1.34
Avail Cap(c_a), veh/h	322	1965	582	592	613	546
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.89	0.89	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	37.0	13.1	30.5	30.5	21.7	29.8
Incr Delay (d2), s/veh	142.2	0.9	207.1	218.6	0.1	166.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	19.3	5.8	44.8	47.5	2.5	47.9
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	179.2	14.0	237.6	249.1	21.8	196.2
LnGrp LOS	F	B	F	F	C	F
Approach Vol, veh/h		1449	1705		904	
Approach Delay, s/veh		60.6	243.5		163.2	
Approach LOS		E	F		F	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		55.0		35.0	20.0	35.0
Change Period (Y+Rc), s		6.0		4.5	4.0	6.0
Max Green Setting (Gmax), s		49.0		30.5	16.0	29.0
Max Q Clear Time (g_c+I1), s		18.6		32.5	18.0	31.0
Green Ext Time (p_c), s		7.6		0.0	0.0	0.0
Intersection Summary						
HCM 6th Ctrl Delay			160.3			
HCM 6th LOS			F			

Intersection						
Int Delay, s/veh	7					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑			↑		↑
Traffic Vol, veh/h	0	0	12	0	0	3
Future Vol, veh/h	0	0	12	0	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	13	0	0	3

Major/Minor	Major1	Major2	Minor1	Minor2	Minor3
Conflicting Flow All	0	-	1	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	-	4.12	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	-	2.218	-	-
Pot Cap-1 Maneuver	-	0	1622	-	0
Stage 1	-	0	-	-	0
Stage 2	-	0	-	-	0
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1622	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	WB	NB
HCM Control Delay, s	0	7.2	8.3
HCM LOS			A

Minor Lane/Major Mvmt	NBLn1	EBT	WBL	WBT
Capacity (veh/h)	1084	-	1622	-
HCM Lane V/C Ratio	0.003	-	0.008	-
HCM Control Delay (s)	8.3	-	7.2	0
HCM Lane LOS	A	-	A	A
HCM 95th %tile Q(veh)	0	-	0	-

Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↘↙		↘	↑↑	↑↑	
Traffic Vol, veh/h	0	5	21	358	564	0
Future Vol, veh/h	0	5	21	358	564	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	50	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	5	23	389	613	0

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	854	307	613	0	-	0
Stage 1	613	-	-	-	-	-
Stage 2	241	-	-	-	-	-
Critical Hdwy	6.84	6.94	4.14	-	-	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	2.22	-	-	-
Pot Cap-1 Maneuver	298	689	962	-	-	-
Stage 1	503	-	-	-	-	-
Stage 2	776	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	291	689	962	-	-	-
Mov Cap-2 Maneuver	395	-	-	-	-	-
Stage 1	491	-	-	-	-	-
Stage 2	776	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	10.3	0.5	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	962	-	689	-	-
HCM Lane V/C Ratio	0.024	-	0.008	-	-
HCM Control Delay (s)	8.8	-	10.3	-	-
HCM Lane LOS	A	-	B	-	-
HCM 95th %tile Q(veh)	0.1	-	0	-	-

Timings
1: SR-74/I-215 SB Ramps & Bonnie Drive



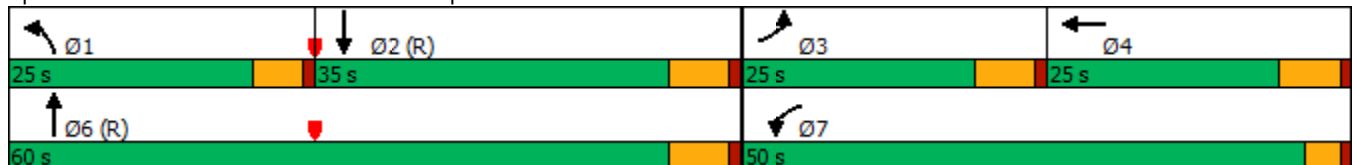
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø4	Ø7
Lane Configurations								
Traffic Volume (vph)	83	390	391	534	1111	54		
Future Volume (vph)	83	390	391	534	1111	54		
Turn Type	Prot	Free	Prot	NA	NA	Free		
Protected Phases	3		1	6	2		4	7
Permitted Phases		Free				Free		
Detector Phase	3		1	6	2			
Switch Phase								
Minimum Initial (s)	5.0		5.0	5.0	5.0		5.0	5.0
Minimum Split (s)	11.0		10.0	11.0	11.0		11.0	9.0
Total Split (s)	25.0		25.0	60.0	35.0		25.0	50.0
Total Split (%)	22.7%		22.7%	54.5%	31.8%		23%	45%
Yellow Time (s)	5.0		4.0	5.0	5.0		5.0	3.0
All-Red Time (s)	1.0		1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			
Total Lost Time (s)	6.0		5.0	6.0	6.0			
Lead/Lag	Lead		Lead		Lag		Lag	
Lead-Lag Optimize?	Yes		Yes		Yes		Yes	
Recall Mode	None		None	C-Min	C-Min		None	None
Act Effct Green (s)	10.0	110.0	40.9	91.6	44.5	110.0		
Actuated g/C Ratio	0.09	1.00	0.37	0.83	0.40	1.00		
v/c Ratio	0.56	0.27	0.65	0.37	1.61	0.04		
Control Delay	60.4	0.4	36.0	4.0	305.7	0.0		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	60.4	0.4	36.0	4.0	305.7	0.0		
LOS	E	A	D	A	F	A		
Approach Delay				17.5	291.5			
Approach LOS				B	F			

Intersection Summary




















Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 23 (21%), Referenced to phase 2:SBT and 6:NBT, Start of Green
 Natural Cycle: 150
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.61
 Intersection Signal Delay: 140.9
 Intersection Capacity Utilization 97.2%
 Analysis Period (min) 15

Intersection LOS: F
 ICU Level of Service F

Splits and Phases: 1: SR-74/I-215 SB Ramps & Bonnie Drive



HCM 6th Signalized Intersection Summary
 1: SR-74/I-215 SB Ramps & Bonnie Drive

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	83	0	390	0	0	0	391	534	0	0	1111	54
Future Volume (veh/h)	83	0	390	0	0	0	391	534	0	0	1111	54
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	0	1900	1900	1900	0	1900	1900	1900	0	1900	1900
Adj Flow Rate, veh/h	92	0	0	0	0	0	434	593	0	0	1234	0
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	0	0	0	0	0	0	0	0	0	0	0	0
Cap, veh/h	117	0		0	2	0	329	1570	0	0	1138	
Arrive On Green	0.06	0.00	0.00	0.00	0.00	0.00	0.18	0.83	0.00	0.00	0.60	0.00
Sat Flow, veh/h	1810	92		0	1900	0	1810	1900	0	0	1900	1610
Grp Volume(v), veh/h	92	55.0		0	0	0	434	593	0	0	1234	0
Grp Sat Flow(s),veh/h/ln	1810	E		0	1900	0	1810	1900	0	0	1900	1610
Q Serve(g_s), s	5.5			0.0	0.0	0.0	20.0	8.7	0.0	0.0	65.9	0.0
Cycle Q Clear(g_c), s	5.5			0.0	0.0	0.0	20.0	8.7	0.0	0.0	65.9	0.0
Prop In Lane	1.00			0.00		0.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h	117			0	2	0	329	1570	0	0	1138	
V/C Ratio(X)	0.79			0.00	0.00	0.00	1.32	0.38	0.00	0.00	1.08	
Avail Cap(c_a), veh/h	313			0	328	0	329	1570	0	0	1138	
HCM Platoon Ratio	1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00			0.00	0.00	0.00	1.00	1.00	0.00	0.00	1.00	0.00
Uniform Delay (d), s/veh	50.7			0.0	0.0	0.0	45.0	2.4	0.0	0.0	22.1	0.0
Incr Delay (d2), s/veh	4.3			0.0	0.0	0.0	163.5	0.7	0.0	0.0	52.7	0.0
Initial Q Delay(d3),s/veh	0.0			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.5			0.0	0.0	0.0	23.5	1.6	0.0	0.0	40.7	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	55.0			0.0	0.0	0.0	208.5	3.1	0.0	0.0	74.7	0.0
LnGrp LOS	E			A	A	A	F	A	A	A	F	
Approach Vol, veh/h					0			1027			1234	
Approach Delay, s/veh					0.0			89.9			74.7	
Approach LOS								F			E	
Timer - Assigned Phs	1	2	3	4		6	7					
Phs Duration (G+Y+Rc), s	25.0	71.9	13.1	0.0		96.9	13.1					
Change Period (Y+Rc), s	5.0	6.0	6.0	6.0		6.0	4.0					
Max Green Setting (Gmax), s	20.0	29.0	19.0	19.0		54.0	46.0					
Max Q Clear Time (g_c+I1), s	22.0	67.9	7.5	0.0		10.7	0.0					
Green Ext Time (p_c), s	0.0	0.0	0.1	0.0		2.1	0.0					

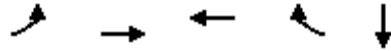
Intersection Summary

HCM 6th Ctrl Delay	80.6
HCM 6th LOS	F

Notes

Unsignalized Delay for [EBR, SBR] is excluded from calculations of the approach delay and intersection delay.

Timings
2: SR-74 & I-215 NB Ramps

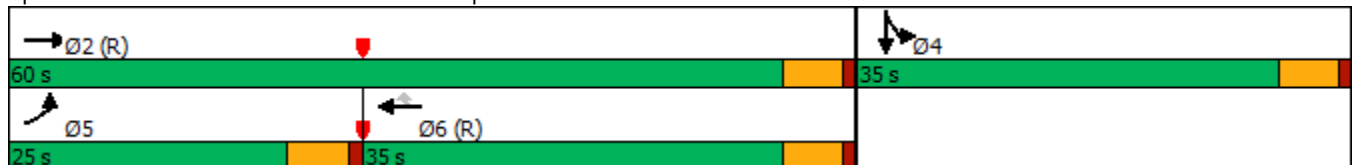


Lane Group	EBL	EBT	WBT	WBR	SBT
Lane Configurations	↖	↗↗	↗↗	↖	↔
Traffic Volume (vph)	54	1448	837	972	0
Future Volume (vph)	54	1448	837	972	0
Turn Type	Prot	NA	NA	Perm	NA
Protected Phases	5	2	6		4
Permitted Phases				6	
Detector Phase	5	2	6	6	4
Switch Phase					
Minimum Initial (s)	5.0	50.0	5.0	5.0	5.0
Minimum Split (s)	10.3	55.3	10.3	10.3	10.3
Total Split (s)	25.0	60.0	35.0	35.0	35.0
Total Split (%)	26.3%	63.2%	36.8%	36.8%	36.8%
Yellow Time (s)	4.3	4.3	4.3	4.3	4.3
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.3	5.3	5.3	5.3	5.3
Lead/Lag	Lead		Lag	Lag	
Lead-Lag Optimize?	Yes		Yes	Yes	
Recall Mode	None	C-Min	C-Min	C-Min	None
Act Effct Green (s)	7.6	53.8	42.9	42.9	30.6
Actuated g/C Ratio	0.08	0.57	0.45	0.45	0.32
v/c Ratio	0.40	0.76	0.55	0.83	1.06
Control Delay	49.0	18.7	21.4	9.9	82.6
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	49.0	18.7	21.4	9.9	82.6
LOS	D	B	C	A	F
Approach Delay		19.8	15.2		82.6
Approach LOS		B	B		F





















Intersection Summary

Cycle Length: 95
 Actuated Cycle Length: 95
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.06
 Intersection Signal Delay: 27.7
 Intersection LOS: C
 Intersection Capacity Utilization 85.7%
 ICU Level of Service E
 Analysis Period (min) 15

Splits and Phases: 2: SR-74 & I-215 NB Ramps



HCM 6th Signalized Intersection Summary
2: SR-74 & I-215 NB Ramps

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 			 						 	
Traffic Volume (veh/h)	54	1448	0	0	837	972	0	0	0	539	0	87
Future Volume (veh/h)	54	1448	0	0	837	972	0	0	0	539	0	87
Initial Q (Qb), veh	0	0	0	0	0	0				0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00				1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Work Zone On Approach		No			No						No	
Adj Sat Flow, veh/h/ln	1900	1900	0	0	1900	1900				1900	1900	1900
Adj Flow Rate, veh/h	58	1557	0	0	900	0				580	0	94
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93				0.93	0.93	0.93
Percent Heavy Veh, %	0	0	0	0	0	0				0	0	0
Cap, veh/h	75	2079	0	0	1727					479	0	78
Arrive On Green	0.04	0.58	0.00	0.00	0.48	0.00				0.31	0.00	0.31
Sat Flow, veh/h	1810	3705	0	0	3705	1610				1531	0	248
Grp Volume(v), veh/h	58	1557	0	0	900	0				674	0	0
Grp Sat Flow(s),veh/h/ln	1810	1805	0	0	1805	1610				1779	0	0
Q Serve(g_s), s	3.0	30.6	0.0	0.0	16.5	0.0				29.7	0.0	0.0
Cycle Q Clear(g_c), s	3.0	30.6	0.0	0.0	16.5	0.0				29.7	0.0	0.0
Prop In Lane	1.00		0.00	0.00		1.00				0.86		0.14
Lane Grp Cap(c), veh/h	75	2079	0	0	1727					556	0	0
V/C Ratio(X)	0.77	0.75	0.00	0.00	0.52					1.21	0.00	0.00
Avail Cap(c_a), veh/h	375	2079	0	0	1727					556	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00				1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	0.00	0.09	0.00				1.00	0.00	0.00
Uniform Delay (d), s/veh	45.1	15.0	0.0	0.0	17.2	0.0				32.7	0.0	0.0
Incr Delay (d2), s/veh	6.0	2.5	0.0	0.0	0.1	0.0				111.3	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0				0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.4	10.9	0.0	0.0	6.7	0.0				29.0	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	51.1	17.6	0.0	0.0	17.3	0.0				143.9	0.0	0.0
LnGrp LOS	D	B	A	A	B					F	A	A
Approach Vol, veh/h		1615			900						674	
Approach Delay, s/veh		18.8			17.3						143.9	
Approach LOS		B			B						F	
Timer - Assigned Phs		2		4	5	6						
Phs Duration (G+Y+Rc), s		60.0		35.0	9.3	50.7						
Change Period (Y+Rc), s		5.3		5.3	5.3	5.3						
Max Green Setting (Gmax), s		54.7		29.7	19.7	29.7						
Max Q Clear Time (g_c+I1), s		32.6		31.7	5.0	18.5						
Green Ext Time (p_c), s		7.6		0.0	0.0	3.6						
Intersection Summary												
HCM 6th Ctrl Delay				44.8								
HCM 6th LOS				D								
Notes												
Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.												

Intersection												
Intersection Delay, s/veh	60											
Intersection LOS	F											

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔↔		↔↔	↔		↔	↔↔		↔	↔↔	
Traffic Vol, veh/h	3	2	12	334	2	30	5	155	455	29	170	1
Future Vol, veh/h	3	2	12	334	2	30	5	155	455	29	170	1
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	4	3	16	434	3	39	6	201	591	38	221	1
Number of Lanes	0	2	0	2	1	0	1	2	0	1	2	0

Approach	EB	WB	NB	SB
Opposing Approach	WB	EB	SB	NB
Opposing Lanes	3	2	3	3
Conflicting Approach Left	SB	NB	EB	WB
Conflicting Lanes Left	3	3	2	3
Conflicting Approach Right	NB	SB	WB	EB
Conflicting Lanes Right	3	3	3	2
HCM Control Delay	11.7	17.9	101.4	13.9
HCM LOS	B	C	F	B

Lane	NBLn1	NBLn2	NBLn3	EBLn1	EBLn2	WBLn1	WBLn2	WBLn3	SBLn1	SBLn2	SBLn3
Vol Left, %	100%	0%	0%	75%	0%	100%	100%	0%	100%	0%	0%
Vol Thru, %	0%	100%	10%	25%	8%	0%	0%	6%	0%	100%	98%
Vol Right, %	0%	0%	90%	0%	92%	0%	0%	94%	0%	0%	2%
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop	Stop
Traffic Vol by Lane	5	103	507	4	13	167	167	32	29	113	58
LT Vol	5	0	0	3	0	167	167	0	29	0	0
Through Vol	0	103	52	1	1	0	0	2	0	113	57
RT Vol	0	0	455	0	12	0	0	30	0	0	1
Lane Flow Rate	6	134	658	5	17	217	217	42	38	147	75
Geometry Grp	8	8	8	8	8	8	8	8	8	8	8
Degree of Util (X)	0.014	0.264	1.178	0.013	0.038	0.483	0.483	0.079	0.088	0.325	0.165
Departure Headway (Hd)	7.588	7.083	6.447	9.441	8.42	8.344	8.344	7.183	8.799	8.29	8.278
Convergence, Y/N	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Cap	470	505	559	381	428	434	434	502	410	437	436
Service Time	5.366	4.86	4.224	7.141	6.12	6.044	6.044	4.883	6.499	5.99	5.978
HCM Lane V/C Ratio	0.013	0.265	1.177	0.013	0.04	0.5	0.5	0.084	0.093	0.336	0.172
HCM Control Delay	10.5	12.4	120.4	12.3	11.5	18.6	18.6	10.5	12.3	14.9	12.6
HCM Lane LOS	B	B	F	B	B	C	C	B	B	B	B
HCM 95th-tile Q	0	1.1	22.9	0	0.1	2.6	2.6	0.3	0.3	1.4	0.6

Intersection												
Int Delay, s/veh	1.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	0	0	31	30	0	1	24	625	1	1	535	0
Future Vol, veh/h	0	0	31	30	0	1	24	625	1	1	535	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	79	79	79	79	79	79	79	79	79	79	79	79
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	0	0	39	38	0	1	30	791	1	1	677	0

Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	1135	1531	339	1193	1531	396	677	0	0	792	0	0
Stage 1	679	679	-	852	852	-	-	-	-	-	-	-
Stage 2	456	852	-	341	679	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	6.9	7.5	6.5	6.9	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	160	118	663	145	118	609	924	-	-	838	-	-
Stage 1	412	454	-	325	379	-	-	-	-	-	-	-
Stage 2	559	379	-	653	454	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	156	114	663	133	114	609	924	-	-	838	-	-
Mov Cap-2 Maneuver	156	114	-	133	114	-	-	-	-	-	-	-
Stage 1	399	454	-	315	367	-	-	-	-	-	-	-
Stage 2	540	367	-	614	454	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	10.8		41.9		0.3		0	
HCM LOS	B		E					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	924	-	-	663	136	838	-	-
HCM Lane V/C Ratio	0.033	-	-	0.059	0.289	0.002	-	-
HCM Control Delay (s)	9	-	-	10.8	41.9	9.3	-	-
HCM Lane LOS	A	-	-	B	E	A	-	-
HCM 95th %tile Q(veh)	0.1	-	-	0.2	1.1	0	-	-

Timings
5: SR-74 & Trumble Road

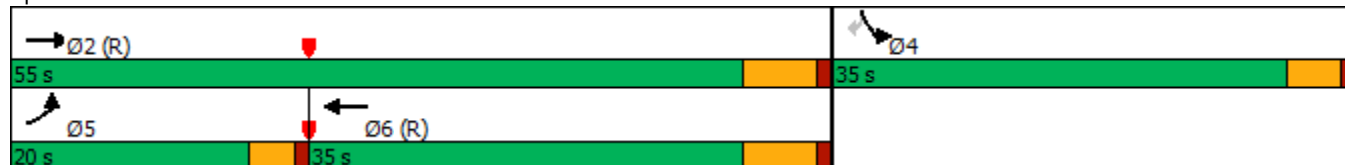


Lane Group	EBL	EBT	WBT	SBL	SBR
Lane Configurations	↖	↗	↗↖	↖	↗
Traffic Volume (vph)	555	1422	1189	178	624
Future Volume (vph)	555	1422	1189	178	624
Turn Type	Prot	NA	NA	Prot	Perm
Protected Phases	5	2	6	4	
Permitted Phases					4
Detector Phase	5	2	6	4	4
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.0	11.0	35.0	30.5	30.5
Total Split (s)	20.0	55.0	35.0	35.0	35.0
Total Split (%)	22.2%	61.1%	38.9%	38.9%	38.9%
Yellow Time (s)	3.0	5.0	5.0	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	6.0	6.0	4.5	4.5
Lead/Lag	Lead		Lag		
Lead-Lag Optimize?	Yes		Yes		
Recall Mode	None	C-Min	C-Min	None	None
Act Effct Green (s)	23.2	56.2	29.0	23.3	23.3
Actuated g/C Ratio	0.26	0.62	0.32	0.26	0.26
v/c Ratio	1.27	0.67	1.24	0.40	0.94
Control Delay	170.6	14.5	143.1	28.4	34.6
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	170.6	14.5	143.1	28.4	34.6
LOS	F	B	F	C	C
Approach Delay		58.3	143.1	33.2	
Approach LOS		E	F	C	

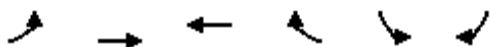
Intersection Summary

Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 32.5 (36%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 150
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.27
 Intersection Signal Delay: 81.0
 Intersection LOS: F
 Intersection Capacity Utilization 90.4%
 ICU Level of Service E
 Analysis Period (min) 15

Splits and Phases: 5: SR-74 & Trumble Road



HCM 6th Signalized Intersection Summary
5: SR-74 & Trumble Road



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↖	↑↑	↑↗	↑	↙	↘
Traffic Volume (veh/h)	555	1422	1189	152	178	624
Future Volume (veh/h)	555	1422	1189	152	178	624
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1900
Adj Flow Rate, veh/h	590	1513	1265	162	189	664
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	0	0	0	0	0	0
Cap, veh/h	322	1965	1038	132	613	546
Arrive On Green	0.18	0.54	0.32	0.32	0.34	0.34
Sat Flow, veh/h	1810	3705	3316	410	1810	1610
Grp Volume(v), veh/h	590	1513	706	721	189	664
Grp Sat Flow(s),veh/h/ln	1810	1805	1805	1826	1810	1610
Q Serve(g_s), s	16.0	29.6	29.0	29.0	6.9	30.5
Cycle Q Clear(g_c), s	16.0	29.6	29.0	29.0	6.9	30.5
Prop In Lane	1.00			0.22	1.00	1.00
Lane Grp Cap(c), veh/h	322	1965	582	588	613	546
V/C Ratio(X)	1.83	0.77	1.21	1.22	0.31	1.22
Avail Cap(c_a), veh/h	322	1965	582	588	613	546
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.56	0.56	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	37.0	16.1	30.5	30.5	22.0	29.8
Incr Delay (d2), s/veh	382.1	1.7	111.6	115.7	0.1	113.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	40.7	10.4	29.3	30.3	2.7	39.8
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	419.1	17.8	142.1	146.2	22.1	143.2
LnGrp LOS	F	B	F	F	C	F
Approach Vol, veh/h		2103	1427		853	
Approach Delay, s/veh		130.4	144.2		116.4	
Approach LOS		F	F		F	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		55.0		35.0	20.0	35.0
Change Period (Y+Rc), s		6.0		4.5	4.0	6.0
Max Green Setting (Gmax), s		49.0		30.5	16.0	29.0
Max Q Clear Time (g_c+1), s		31.6		32.5	18.0	31.0
Green Ext Time (p_c), s		9.6		0.0	0.0	0.0
Intersection Summary						
HCM 6th Ctrl Delay			132.1			
HCM 6th LOS			F			

Intersection						
Int Delay, s/veh	7.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑			↑		↑
Traffic Vol, veh/h	0	0	6	0	0	9
Future Vol, veh/h	0	0	6	0	0	9
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	7	0	0	10

Major/Minor	Major1	Major2	Minor1	Minor2	Minor3
Conflicting Flow All	0	-	1	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	-	4.12	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	-	2.218	-	-
Pot Cap-1 Maneuver	-	0	1622	-	0
Stage 1	-	0	-	-	0
Stage 2	-	0	-	-	0
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1622	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	WB	NB
HCM Control Delay, s	0	7.2	8.4
HCM LOS			A

Minor Lane/Major Mvmt	NBLn1	EBT	WBL	WBT
Capacity (veh/h)	1084	-	1622	-
HCM Lane V/C Ratio	0.009	-	0.004	-
HCM Control Delay (s)	8.4	-	7.2	0
HCM Lane LOS	A	-	A	A
HCM 95th %tile Q(veh)	0	-	0	-

Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↔		↔	↑↑	↑↑	
Traffic Vol, veh/h	0	18	11	615	518	0
Future Vol, veh/h	0	18	11	615	518	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	50	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	20	12	668	563	0

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	921	282	563	0	-	0
Stage 1	563	-	-	-	-	-
Stage 2	358	-	-	-	-	-
Critical Hdwy	6.84	6.94	4.14	-	-	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	2.22	-	-	-
Pot Cap-1 Maneuver	270	715	1005	-	-	-
Stage 1	534	-	-	-	-	-
Stage 2	678	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	267	715	1005	-	-	-
Mov Cap-2 Maneuver	389	-	-	-	-	-
Stage 1	528	-	-	-	-	-
Stage 2	678	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	10.2	0.2	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1005	-	715	-	-
HCM Lane V/C Ratio	0.012	-	0.027	-	-
HCM Control Delay (s)	8.6	-	10.2	-	-
HCM Lane LOS	A	-	B	-	-
HCM 95th %tile Q(veh)	0	-	0.1	-	-

Timings
1: SR-74/I-215 SB Ramps & Bonnie Drive

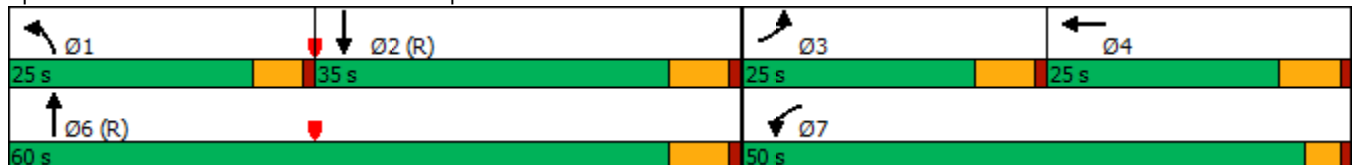


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø4	Ø7
Lane Configurations								
Traffic Volume (vph)	69	265	337	773	783	65		
Future Volume (vph)	69	265	337	773	783	65		
Turn Type	Prot	Free	Prot	NA	NA	Free		
Protected Phases	3		1	6	2		4	7
Permitted Phases		Free				Free		
Detector Phase	3		1	6	2			
Switch Phase								
Minimum Initial (s)	5.0		5.0	5.0	5.0		5.0	5.0
Minimum Split (s)	11.0		10.0	11.0	11.0		11.0	9.0
Total Split (s)	25.0		25.0	60.0	35.0		25.0	50.0
Total Split (%)	22.7%		22.7%	54.5%	31.8%		23%	45%
Yellow Time (s)	5.0		4.0	5.0	5.0		5.0	3.0
All-Red Time (s)	1.0		1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			
Total Lost Time (s)	6.0		5.0	6.0	6.0			
Lead/Lag	Lead		Lead		Lag		Lag	
Lead-Lag Optimize?	Yes		Yes		Yes		Yes	
Recall Mode	None		None	C-Min	C-Min		None	None
Act Effct Green (s)	9.0	110.0	32.5	92.5	53.8	110.0		
Actuated g/C Ratio	0.08	1.00	0.30	0.84	0.49	1.00		
v/c Ratio	0.51	0.18	0.69	0.53	0.48	0.04		
Control Delay	59.8	0.2	42.4	5.0	20.8	0.0		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	59.8	0.2	42.4	5.0	20.8	0.0		
LOS	E	A	D	A	C	A		
Approach Delay				16.4	19.2			
Approach LOS				B	B			

Intersection Summary

Cycle Length: 110
 Actuated Cycle Length: 110
 Offset: 23 (21%), Referenced to phase 2:SBT and 6:NBT, Start of Green
 Natural Cycle: 65
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.69
 Intersection Signal Delay: 16.9
 Intersection LOS: B
 Intersection Capacity Utilization 56.6%
 ICU Level of Service B
 Analysis Period (min) 15

Splits and Phases: 1: SR-74/I-215 SB Ramps & Bonnie Drive



HCM 6th Signalized Intersection Summary
1: SR-74/I-215 SB Ramps & Bonnie Drive



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖		↗		↖		↖	↗			↕	↗
Traffic Volume (veh/h)	69	0	265	0	0	0	337	773	0	0	783	65
Future Volume (veh/h)	69	0	265	0	0	0	337	773	0	0	783	65
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	0	1900	1900	1900	0	1900	1900	1900	0	1900	1900
Adj Flow Rate, veh/h	75	0	0	0	0	0	366	840	0	0	851	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	0	0	0	0	0	0	0	0	0	0	0
Cap, veh/h	97	0		0	2	0	329	1591	0	0	2202	
Arrive On Green	0.05	0.00	0.00	0.00	0.00	0.00	0.18	0.84	0.00	0.00	0.61	0.00
Sat Flow, veh/h	1810	75		0	1900	0	1810	1900	0	0	3705	1610
Grp Volume(v), veh/h	75	56.3		0	0	0	366	840	0	0	851	0
Grp Sat Flow(s),veh/h/ln	1810	E		0	1900	0	1810	1900	0	0	1805	1610
Q Serve(g_s), s	4.5			0.0	0.0	0.0	20.0	14.2	0.0	0.0	13.2	0.0
Cycle Q Clear(g_c), s	4.5			0.0	0.0	0.0	20.0	14.2	0.0	0.0	13.2	0.0
Prop In Lane	1.00			0.00		0.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h	97			0	2	0	329	1591	0	0	2202	
V/C Ratio(X)	0.77			0.00	0.00	0.00	1.11	0.53	0.00	0.00	0.39	
Avail Cap(c_a), veh/h	313			0	328	0	329	1591	0	0	2202	
HCM Platoon Ratio	1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00			0.00	0.00	0.00	1.00	1.00	0.00	0.00	1.00	0.00
Uniform Delay (d), s/veh	51.4			0.0	0.0	0.0	45.0	2.6	0.0	0.0	10.9	0.0
Incr Delay (d2), s/veh	4.9			0.0	0.0	0.0	83.4	1.3	0.0	0.0	0.5	0.0
Initial Q Delay(d3),s/veh	0.0			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.1			0.0	0.0	0.0	16.2	2.3	0.0	0.0	4.7	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	56.3			0.0	0.0	0.0	128.4	3.9	0.0	0.0	11.5	0.0
LnGrp LOS	E			A	A	A	F	A	A	A	B	
Approach Vol, veh/h					0			1206			851	
Approach Delay, s/veh					0.0			41.7			11.5	
Approach LOS								D			B	
Timer - Assigned Phs	1	2	3	4		6	7					
Phs Duration (G+Y+Rc), s	25.0	73.1	11.9	0.0		98.1	11.9					
Change Period (Y+Rc), s	5.0	6.0	6.0	6.0		6.0	4.0					
Max Green Setting (Gmax), s	20.0	29.0	19.0	19.0		54.0	46.0					
Max Q Clear Time (g_c+I1), s	22.0	15.2	6.5	0.0		16.2	0.0					
Green Ext Time (p_c), s	0.0	3.0	0.1	0.0		3.5	0.0					

Intersection Summary

HCM 6th Ctrl Delay	30.1
HCM 6th LOS	C

Notes

Unsignalized Delay for [EBR, SBR] is excluded from calculations of the approach delay and intersection delay.

Timings
3: Trumble Road & Mapes Road

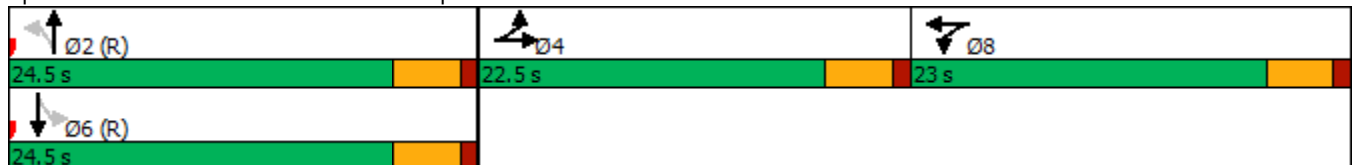


Lane Group	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	↔↔	↔↔	↔	↔	↔↔	↔	↔↔
Traffic Volume (vph)	20	427	20	66	58	1	80
Future Volume (vph)	20	427	20	66	58	1	80
Turn Type	NA	Split	NA	Perm	NA	Perm	NA
Protected Phases	4	8	8		2		6
Permitted Phases				2		6	
Detector Phase	4	8	8	2	2	6	6
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	22.5	22.5	22.5	22.5	22.5
Total Split (s)	22.5	23.0	23.0	24.5	24.5	24.5	24.5
Total Split (%)	32.1%	32.9%	32.9%	35.0%	35.0%	35.0%	35.0%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Lead/Lag							
Lead-Lag Optimize?							
Recall Mode	None	None	None	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	6.3	14.6	14.6	37.6	37.6	37.6	37.6
Actuated g/C Ratio	0.09	0.21	0.21	0.54	0.54	0.54	0.54
v/c Ratio	0.26	0.63	0.06	0.10	0.17	0.00	0.05
Control Delay	14.5	28.8	19.0	11.1	3.2	11.0	9.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	14.5	28.8	19.0	11.1	3.2	11.0	9.8
LOS	B	C	B	B	A	B	A
Approach Delay	14.5		28.3		4.6		9.8
Approach LOS	B		C		A		A




















Intersection Summary

Cycle Length: 70
 Actuated Cycle Length: 70
 Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 70
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.63
 Intersection Signal Delay: 16.8
 Intersection LOS: B
 Intersection Capacity Utilization 36.7%
 ICU Level of Service A
 Analysis Period (min) 15

Splits and Phases: 3: Trumble Road & Mapes Road



HCM 6th Signalized Intersection Summary
3: Trumble Road & Mapes Road

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	3	20	60	427	20	3	66	58	234	1	80	4
Future Volume (veh/h)	3	20	60	427	20	3	66	58	234	1	80	4
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adj Flow Rate, veh/h	3	22	65	459	22	3	71	62	252	1	86	4
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	0	0	0	0	0	0	0	0	0	0	0	0
Cap, veh/h	13	99	96	594	277	38	857	1044	931	644	2032	94
Arrive On Green	0.06	0.06	0.06	0.17	0.17	0.17	0.58	0.58	0.58	0.58	0.58	0.58
Sat Flow, veh/h	227	1662	1610	3510	1637	223	1327	1805	1610	1082	3513	162
Grp Volume(v), veh/h	25	0	65	459	0	25	71	62	252	1	44	46
Grp Sat Flow(s),veh/h/ln	1889	0	1610	1755	0	1860	1327	1805	1610	1082	1805	1871
Q Serve(g_s), s	0.9	0.0	2.8	8.7	0.0	0.8	1.7	1.0	5.5	0.0	0.7	0.7
Cycle Q Clear(g_c), s	0.9	0.0	2.8	8.7	0.0	0.8	2.5	1.0	5.5	5.5	0.7	0.7
Prop In Lane	0.12		1.00	1.00		0.12	1.00		1.00	1.00		0.09
Lane Grp Cap(c), veh/h	112	0	96	594	0	315	857	1044	931	644	1044	1082
V/C Ratio(X)	0.22	0.00	0.68	0.77	0.00	0.08	0.08	0.06	0.27	0.00	0.04	0.04
Avail Cap(c_a), veh/h	486	0	414	928	0	492	857	1044	931	644	1044	1082
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	31.4	0.0	32.3	27.8	0.0	24.5	6.9	6.4	7.4	8.8	6.4	6.4
Incr Delay (d2), s/veh	1.0	0.0	8.2	2.2	0.0	0.1	0.2	0.1	0.7	0.0	0.1	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	0.0	1.2	3.4	0.0	0.3	0.4	0.3	1.6	0.0	0.2	0.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	32.4	0.0	40.4	30.0	0.0	24.6	7.1	6.5	8.1	8.8	6.5	6.5
LnGrp LOS	C	A	D	C	A	C	A	A	A	A	A	A
Approach Vol, veh/h		90			484			385				91
Approach Delay, s/veh		38.2			29.7			7.7				6.5
Approach LOS		D			C			A				A
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		45.0		8.7		45.0		16.3				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		20.0		18.0		20.0		18.5				
Max Q Clear Time (g_c+I1), s		7.5		4.8		7.5		10.7				
Green Ext Time (p_c), s		1.6		0.3		0.2		1.1				
Intersection Summary												
HCM 6th Ctrl Delay				20.3								
HCM 6th LOS				C								

Intersection

Int Delay, s/veh 1.7

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕		↕	↕	
Traffic Vol, veh/h	0	0	21	57	1	1	30	378	10	0	567	2
Future Vol, veh/h	0	0	21	57	1	1	30	378	10	0	567	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91	91	91	91	91
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	0	0	23	63	1	1	33	415	11	0	623	2

Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	898	1116	313	799	1112	213	625	0	0	426	0	0
Stage 1	624	624	-	487	487	-	-	-	-	-	-	-
Stage 2	274	492	-	312	625	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	6.9	7.5	6.5	6.9	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	237	209	689	280	211	798	966	-	-	1144	-	-
Stage 1	445	481	-	536	554	-	-	-	-	-	-	-
Stage 2	714	551	-	679	480	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	230	202	689	263	204	798	966	-	-	1144	-	-
Mov Cap-2 Maneuver	230	202	-	263	204	-	-	-	-	-	-	-
Stage 1	430	481	-	518	535	-	-	-	-	-	-	-
Stage 2	687	532	-	656	480	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	10.4		22.9		0.6		0	
HCM LOS	B		C					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	966	-	-	689	265	1144	-	-
HCM Lane V/C Ratio	0.034	-	-	0.033	0.245	-	-	-
HCM Control Delay (s)	8.9	-	-	10.4	22.9	0	-	-
HCM Lane LOS	A	-	-	B	C	A	-	-
HCM 95th %tile Q(veh)	0.1	-	-	0.1	0.9	0	-	-

Timings
5: SR-74 & Trumble Road

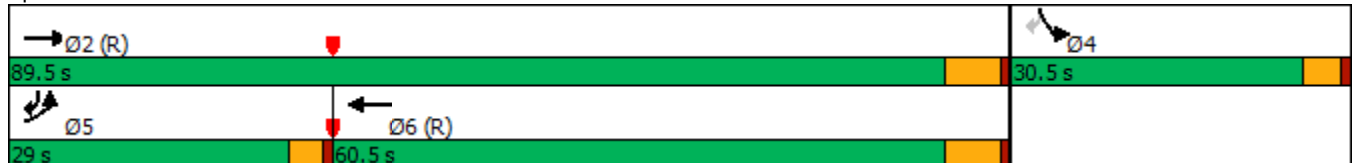


Lane Group	EBL	EBT	WBT	SBL	SBR
Lane Configurations	↖	↗	↗↖	↖	↗↖
Traffic Volume (vph)	380	967	1436	159	682
Future Volume (vph)	380	967	1436	159	682
Turn Type	Prot	NA	NA	Prot	pm+ov
Protected Phases	5	2	6	4	5
Permitted Phases					4
Detector Phase	5	2	6	4	5
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.0	11.0	35.0	30.5	9.0
Total Split (s)	29.0	89.5	60.5	30.5	29.0
Total Split (%)	24.2%	74.6%	50.4%	25.4%	24.2%
Yellow Time (s)	3.0	5.0	5.0	3.5	3.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	6.0	6.0	4.5	4.0
Lead/Lag	Lead		Lag		Lead
Lead-Lag Optimize?					
Recall Mode	None	C-Min	C-Min	None	None
Act Effct Green (s)	34.8	93.8	55.0	15.7	55.0
Actuated g/C Ratio	0.29	0.78	0.46	0.13	0.46
v/c Ratio	0.78	0.37	1.04	0.72	0.56
Control Delay	51.9	4.8	66.3	66.8	24.9
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	51.9	4.8	66.3	66.8	24.9
LOS	D	A	E	E	C
Approach Delay		18.1	66.3	32.8	
Approach LOS		B	E	C	

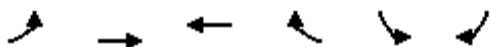
Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 130
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.04
 Intersection Signal Delay: 41.6
 Intersection LOS: D
 Intersection Capacity Utilization 86.4%
 ICU Level of Service E
 Analysis Period (min) 15

Splits and Phases: 5: SR-74 & Trumble Road



HCM 6th Signalized Intersection Summary
5: SR-74 & Trumble Road



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↖	↑↑	↑↗		↙	↘↘
Traffic Volume (veh/h)	380	967	1436	150	159	682
Future Volume (veh/h)	380	967	1436	150	159	682
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1900
Adj Flow Rate, veh/h	409	1040	1544	161	171	733
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	0	0	0	0	0	0
Cap, veh/h	377	2512	1500	155	392	1204
Arrive On Green	0.21	0.70	0.45	0.45	0.22	0.22
Sat Flow, veh/h	1810	3705	3398	341	1810	2834
Grp Volume(v), veh/h	409	1040	837	868	171	733
Grp Sat Flow(s),veh/h/ln	1810	1805	1805	1839	1810	1417
Q Serve(g_s), s	25.0	14.8	54.5	54.5	9.8	24.1
Cycle Q Clear(g_c), s	25.0	14.8	54.5	54.5	9.8	24.1
Prop In Lane	1.00			0.19	1.00	1.00
Lane Grp Cap(c), veh/h	377	2512	820	835	392	1204
V/C Ratio(X)	1.08	0.41	1.02	1.04	0.44	0.61
Avail Cap(c_a), veh/h	377	2512	820	835	392	1204
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.89	0.89	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	47.5	7.8	32.8	32.8	40.7	26.8
Incr Delay (d2), s/veh	68.5	0.4	36.8	41.9	0.3	0.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	17.9	4.8	30.0	31.7	4.3	19.3
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	116.0	8.2	69.5	74.7	40.9	27.4
LnGrp LOS	F	A	F	F	D	C
Approach Vol, veh/h		1449	1705		904	
Approach Delay, s/veh		38.7	72.1		30.0	
Approach LOS		D	E		C	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		89.5		30.5	29.0	60.5
Change Period (Y+Rc), s		6.0		4.5	4.0	6.0
Max Green Setting (Gmax), s		83.5		26.0	25.0	54.5
Max Q Clear Time (g_c+I1), s		16.8		26.1	27.0	56.5
Green Ext Time (p_c), s		8.3		0.0	0.0	0.0

Intersection Summary

HCM 6th Ctrl Delay	50.8
HCM 6th LOS	D

Notes

User approved pedestrian interval to be less than phase max green.

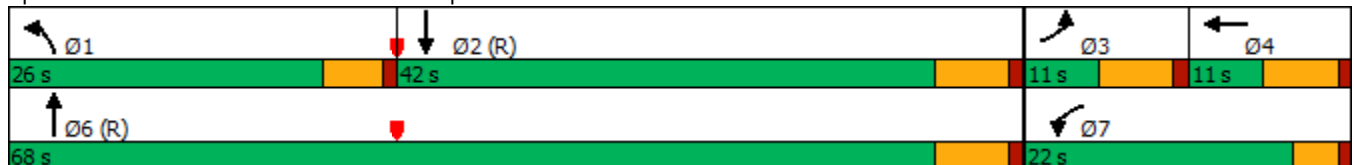
Timings
1: SR-74/I-215 SB Ramps & Bonnie Drive

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	Ø4	Ø7
Lane Configurations								
Traffic Volume (vph)	83	390	391	534	1111	54		
Future Volume (vph)	83	390	391	534	1111	54		
Turn Type	Prot	Free	Prot	NA	NA	Free		
Protected Phases	3		1	6	2		4	7
Permitted Phases		Free					Free	
Detector Phase	3		1	6	2			
Switch Phase								
Minimum Initial (s)	5.0		5.0	5.0	5.0		5.0	5.0
Minimum Split (s)	11.0		10.0	11.0	11.0		11.0	9.0
Total Split (s)	11.0		26.0	68.0	42.0		11.0	22.0
Total Split (%)	12.2%		28.9%	75.6%	46.7%		12%	24%
Yellow Time (s)	5.0		4.0	5.0	5.0		5.0	3.0
All-Red Time (s)	1.0		1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			
Total Lost Time (s)	6.0		5.0	6.0	6.0			
Lead/Lag	Lead		Lead		Lag		Lag	
Lead-Lag Optimize?	Yes		Yes		Yes		Yes	
Recall Mode	None		None		C-Min	C-Min	None	None
Act Effct Green (s)	5.9	90.0	29.8	72.1	37.2	90.0		
Actuated g/C Ratio	0.07	1.00	0.33	0.80	0.41	1.00		
v/c Ratio	0.77	0.27	0.73	0.39	0.83	0.04		
Control Delay	83.3	0.4	34.3	3.4	29.6	0.0		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	83.3	0.4	34.3	3.4	29.6	0.0		
LOS	F	A	C	A	C	A		
Approach Delay				16.4	28.2			
Approach LOS				B	C			

Intersection Summary

Cycle Length: 90
 Actuated Cycle Length: 90
 Offset: 0 (0%), Referenced to phase 2:SBT and 6:NBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.83
 Intersection Signal Delay: 21.5
 Intersection LOS: C
 Intersection Capacity Utilization 69.5%
 ICU Level of Service C
 Analysis Period (min) 15

Splits and Phases: 1: SR-74/I-215 SB Ramps & Bonnie Drive



HCM 6th Signalized Intersection Summary
 1: SR-74/I-215 SB Ramps & Bonnie Drive

Trumble and Mapes Warehouse Project
 OY_WP_PM_MIT

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	83	0	390	0	0	0	391	534	0	0	1111	54
Future Volume (veh/h)	83	0	390	0	0	0	391	534	0	0	1111	54
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	0	1900	1900	1900	0	1900	1900	1900	0	1900	1900
Adj Flow Rate, veh/h	92	0	0	0	0	0	434	593	0	0	1234	0
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	0	0	0	0	0	0	0	0	0	0	0	0
Cap, veh/h	101	0		0	2	0	422	1541	0	0	1885	
Arrive On Green	0.06	0.00	0.00	0.00	0.00	0.00	0.23	0.81	0.00	0.00	0.52	0.00
Sat Flow, veh/h	1810	92		0	1900	0	1810	1900	0	0	3705	1610
Grp Volume(v), veh/h	92	104.4		0	0	0	434	593	0	0	1234	0
Grp Sat Flow(s),veh/h/ln	1810	F		0	1900	0	1810	1900	0	0	1805	1610
Q Serve(g_s), s	4.6			0.0	0.0	0.0	21.0	7.7	0.0	0.0	22.3	0.0
Cycle Q Clear(g_c), s	4.6			0.0	0.0	0.0	21.0	7.7	0.0	0.0	22.3	0.0
Prop In Lane	1.00			0.00		0.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h	101			0	2	0	422	1541	0	0	1885	
V/C Ratio(X)	0.92			0.00	0.00	0.00	1.03	0.38	0.00	0.00	0.65	
Avail Cap(c_a), veh/h	101			0	106	0	422	1541	0	0	1885	
HCM Platoon Ratio	1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00			0.00	0.00	0.00	1.00	1.00	0.00	0.00	1.00	0.00
Uniform Delay (d), s/veh	42.3			0.0	0.0	0.0	34.5	2.3	0.0	0.0	15.6	0.0
Incr Delay (d2), s/veh	62.1			0.0	0.0	0.0	51.1	0.7	0.0	0.0	1.8	0.0
Initial Q Delay(d3),s/veh	0.0			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.7			0.0	0.0	0.0	14.5	1.1	0.0	0.0	8.1	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	104.4			0.0	0.0	0.0	85.6	3.1	0.0	0.0	17.4	0.0
LnGrp LOS	F			A	A	A	F	A	A	A	B	
Approach Vol, veh/h					0			1027			1234	
Approach Delay, s/veh					0.0			38.0			17.4	
Approach LOS								D			B	
Timer - Assigned Phs	1	2	3	4		6	7					
Phs Duration (G+Y+Rc), s	26.0	53.0	11.0	0.0		79.0	11.0					
Change Period (Y+Rc), s	5.0	6.0	6.0	6.0		6.0	4.0					
Max Green Setting (Gmax), s	21.0	36.0	5.0	5.0		62.0	18.0					
Max Q Clear Time (g_c+I1), s	23.0	24.3	6.6	0.0		9.7	0.0					
Green Ext Time (p_c), s	0.0	4.3	0.0	0.0		2.1	0.0					
Intersection Summary												
HCM 6th Ctrl Delay				29.8								
HCM 6th LOS				C								
Notes												
Unsignalized Delay for [EBR, SBR] is excluded from calculations of the approach delay and intersection delay.												

Timings
3: Trumble Road & Mapes Road



Lane Group	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations	↔↔	↔↔	↔	↔	↔↔	↔	↔↔
Traffic Volume (vph)	2	334	2	5	155	29	170
Future Volume (vph)	2	334	2	5	155	29	170
Turn Type	NA	Split	NA	Perm	NA	Perm	NA
Protected Phases	4	8	8		2		6
Permitted Phases				2		6	
Detector Phase	4	8	8	2	2	6	6
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	22.5	22.5	22.5	22.5	22.5	22.5	22.5
Total Split (s)	22.5	22.6	22.6	24.9	24.9	24.9	24.9
Total Split (%)	32.1%	32.3%	32.3%	35.6%	35.6%	35.6%	35.6%
Yellow Time (s)	3.5	3.5	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Lead/Lag							
Lead-Lag Optimize?							
Recall Mode	None	None	None	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	5.8	14.1	14.1	42.6	42.6	42.6	42.6
Actuated g/C Ratio	0.08	0.20	0.20	0.61	0.61	0.61	0.61
v/c Ratio	0.08	0.61	0.12	0.01	0.36	0.11	0.10
Control Delay	18.5	28.9	8.9	9.4	3.0	10.1	7.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	18.5	28.9	8.9	9.4	3.0	10.1	7.8
LOS	B	C	A	A	A	B	A
Approach Delay	18.5		27.2		3.1		8.1
Approach LOS	B		C		A		A

Intersection Summary




















Cycle Length: 70
 Actuated Cycle Length: 70
 Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 70
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.61
 Intersection Signal Delay: 11.5
 Intersection LOS: B
 Intersection Capacity Utilization 47.8%
 ICU Level of Service A
 Analysis Period (min) 15

Splits and Phases: 3: Trumble Road & Mapes Road



HCM 6th Signalized Intersection Summary
3: Trumble Road & Mapes Road

Trumble and Mapes Warehouse Project
OY_WP_PM_MIT

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	3	2	12	334	2	30	5	155	455	29	170	1
Future Volume (veh/h)	3	2	12	334	2	30	5	155	455	29	170	1
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adj Flow Rate, veh/h	4	3	16	434	3	39	6	201	591	38	221	1
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Percent Heavy Veh, %	0	0	0	0	0	0	0	0	0	0	0	0
Cap, veh/h	27	20	41	571	19	246	803	1117	996	380	2281	10
Arrive On Green	0.03	0.03	0.03	0.16	0.16	0.16	0.62	0.62	0.62	0.62	0.62	0.62
Sat Flow, veh/h	1056	792	1610	3510	116	1512	1177	1805	1610	696	3685	17
Grp Volume(v), veh/h	7	0	16	434	0	42	6	201	591	38	108	114
Grp Sat Flow(s),veh/h/ln	1847	0	1610	1755	0	1628	1177	1805	1610	696	1805	1897
Q Serve(g_s), s	0.3	0.0	0.7	8.3	0.0	1.6	0.1	3.3	15.5	2.4	1.7	1.7
Cycle Q Clear(g_c), s	0.3	0.0	0.7	8.3	0.0	1.6	1.8	3.3	15.5	17.9	1.7	1.7
Prop In Lane	0.57		1.00	1.00		0.93	1.00		1.00	1.00		0.01
Lane Grp Cap(c), veh/h	48	0	41	571	0	265	803	1117	996	380	1117	1174
V/C Ratio(X)	0.15	0.00	0.39	0.76	0.00	0.16	0.01	0.18	0.59	0.10	0.10	0.10
Avail Cap(c_a), veh/h	475	0	414	908	0	421	803	1117	996	380	1117	1174
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	33.3	0.0	33.6	28.0	0.0	25.2	5.8	5.7	8.0	13.4	5.4	5.4
Incr Delay (d2), s/veh	1.4	0.0	5.8	2.1	0.0	0.3	0.0	0.4	2.6	0.5	0.2	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.1	0.0	0.3	3.3	0.0	0.6	0.0	1.0	4.3	0.4	0.5	0.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	34.8	0.0	39.3	30.1	0.0	25.5	5.8	6.1	10.6	14.0	5.6	5.6
LnGrp LOS	C	A	D	C	A	C	A	A	B	B	A	A
Approach Vol, veh/h		23			476			798			260	
Approach Delay, s/veh		37.9			29.7			9.4			6.8	
Approach LOS		D			C			A			A	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		47.8		6.3		47.8		15.9				
Change Period (Y+Rc), s		4.5		4.5		4.5		4.5				
Max Green Setting (Gmax), s		20.4		18.0		20.4		18.1				
Max Q Clear Time (g_c+I1), s		17.5		2.7		19.9		10.3				
Green Ext Time (p_c), s		1.4		0.0		0.1		1.1				
Intersection Summary												
HCM 6th Ctrl Delay				15.6								
HCM 6th LOS				B								

HCM 6th TWSC
 4: Trumble Road & Exceed Road/Sturgeon Electric Driveway

Intersection												
Int Delay, s/veh	1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↑↑		↕	↑↑	
Traffic Vol, veh/h	0	0	31	30	0	1	24	625	1	1	535	0
Future Vol, veh/h	0	0	31	30	0	1	24	625	1	1	535	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	79	79	79	79	79	79	79	79	79	79	79	79
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	0	0	39	38	0	1	30	791	1	1	677	0

Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	1135	1531	339	1193	1531	396	677	0	0	792	0	0
Stage 1	679	679	-	852	852	-	-	-	-	-	-	-
Stage 2	456	852	-	341	679	-	-	-	-	-	-	-
Critical Hdwy	7.5	6.5	6.9	7.5	6.5	6.9	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.5	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	251	149	*904	*223	149	609	1147	-	-	838	-	0
Stage 1	651	611	-	*325	379	-	-	-	-	-	-	0
Stage 2	559	379	-	*853	611	-	-	-	-	-	-	0
Platoon blocked, %	1	1	1	1	1	1	1	-	-	-	-	-
Mov Cap-1 Maneuver	245	145	*904	*209	145	609	1147	-	-	838	-	-
Mov Cap-2 Maneuver	245	145	-	*209	145	-	-	-	-	-	-	-
Stage 1	634	611	-	*317	369	-	-	-	-	-	-	-
Stage 2	543	369	-	*815	611	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	9.2	25.6	0.3	0
HCM LOS	A	D		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT
Capacity (veh/h)	1147	-	-	904	214	838
HCM Lane V/C Ratio	0.026	-	-	0.043	0.183	0.002
HCM Control Delay (s)	8.2	-	-	9.2	25.6	9.3
HCM Lane LOS	A	-	-	A	D	A
HCM 95th %tile Q(veh)	0.1	-	-	0.1	0.7	0

Notes
 ~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

Timings
5: SR-74 & Trumble Road

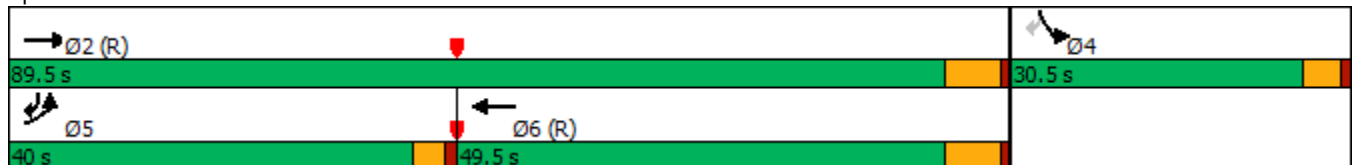


Lane Group	EBL	EBT	WBT	SBL	SBR
Lane Configurations	↗	↑↑	↑↑↔	↖	↖↖
Traffic Volume (vph)	555	1422	1189	178	624
Future Volume (vph)	555	1422	1189	178	624
Turn Type	Prot	NA	NA	Prot	pm+ov
Protected Phases	5	2	6	4	5
Permitted Phases					4
Detector Phase	5	2	6	4	5
Switch Phase					
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.0	11.0	35.0	30.5	9.0
Total Split (s)	40.0	89.5	49.5	30.5	40.0
Total Split (%)	33.3%	74.6%	41.3%	25.4%	33.3%
Yellow Time (s)	3.0	5.0	5.0	3.5	3.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	6.0	6.0	4.5	4.0
Lead/Lag	Lead		Lag		Lead
Lead-Lag Optimize?	Yes		Yes		Yes
Recall Mode	None	C-Min	C-Min	None	None
Act Effct Green (s)	45.1	92.6	43.5	16.9	66.5
Actuated g/C Ratio	0.38	0.77	0.36	0.14	0.55
v/c Ratio	0.87	0.54	1.10	0.74	0.42
Control Delay	50.8	6.8	94.4	66.5	16.1
Queue Delay	0.0	0.4	0.0	0.0	0.0
Total Delay	50.8	7.2	94.4	66.5	16.1
LOS	D	A	F	E	B
Approach Delay		19.4	94.4	27.3	
Approach LOS		B	F	C	

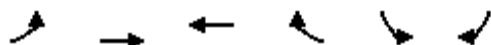
Intersection Summary

Cycle Length: 120
 Actuated Cycle Length: 120
 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 140
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.10
 Intersection Signal Delay: 45.4
 Intersection LOS: D
 Intersection Capacity Utilization 90.4%
 ICU Level of Service E
 Analysis Period (min) 15

Splits and Phases: 5: SR-74 & Trumble Road



HCM 6th Signalized Intersection Summary
5: SR-74 & Trumble Road



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	555	1422	1189	152	178	624
Future Volume (veh/h)	555	1422	1189	152	178	624
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1900	1900	1900	1900	1900	1900
Adj Flow Rate, veh/h	590	1513	1265	162	189	664
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	0	0	0	0	0	0
Cap, veh/h	543	2632	1275	162	332	1370
Arrive On Green	0.30	0.73	0.40	0.40	0.18	0.18
Sat Flow, veh/h	1810	3705	3316	410	1810	2834
Grp Volume(v), veh/h	590	1513	706	721	189	664
Grp Sat Flow(s),veh/h/ln	1810	1805	1805	1826	1810	1417
Q Serve(g_s), s	36.0	23.5	46.6	47.3	11.4	19.0
Cycle Q Clear(g_c), s	36.0	23.5	46.6	47.3	11.4	19.0
Prop In Lane	1.00			0.22	1.00	1.00
Lane Grp Cap(c), veh/h	543	2632	714	723	332	1370
V/C Ratio(X)	1.09	0.57	0.99	1.00	0.57	0.48
Avail Cap(c_a), veh/h	543	2632	714	723	392	1464
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.56	0.56	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	42.0	7.6	36.0	36.2	44.7	20.9
Incr Delay (d2), s/veh	55.5	0.5	31.0	32.8	0.6	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	23.5	7.0	25.2	26.1	5.0	16.8
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	97.5	8.1	67.0	69.0	45.3	21.0
LnGrp LOS	F	A	E	E	D	C
Approach Vol, veh/h		2103	1427		853	
Approach Delay, s/veh		33.2	68.0		26.4	
Approach LOS		C	E		C	
Timer - Assigned Phs		2		4	5	6
Phs Duration (G+Y+Rc), s		93.5		26.5	40.0	53.5
Change Period (Y+Rc), s		6.0		4.5	4.0	6.0
Max Green Setting (Gmax), s		83.5		26.0	36.0	43.5
Max Q Clear Time (g_c+1), s		25.5		21.0	38.0	49.3
Green Ext Time (p_c), s		15.5		1.0	0.0	0.0
Intersection Summary						
HCM 6th Ctrl Delay			43.2			
HCM 6th LOS			D			

APPENDIX E:

VMT ANALYSIS WORKSHEET



Appendix E
VMT Calculation Worksheet
Mapes and Trumble Warehouse - VMT Analysis

2018	Mapes and Trumble (Project) *	City of Perris (City) **
Total Population	-	72,886
Total Employment	485	17,465
Total service population	485	90,351
<hr/>		
Total OD VMT	12,719	2,931,359
<hr/>		
Total OD VMT per service population	26.2	32.4

*: Estimated using RIVCOM V3 model runs

**.: Estimage using LSA No Project RIVCOM V3 model runs

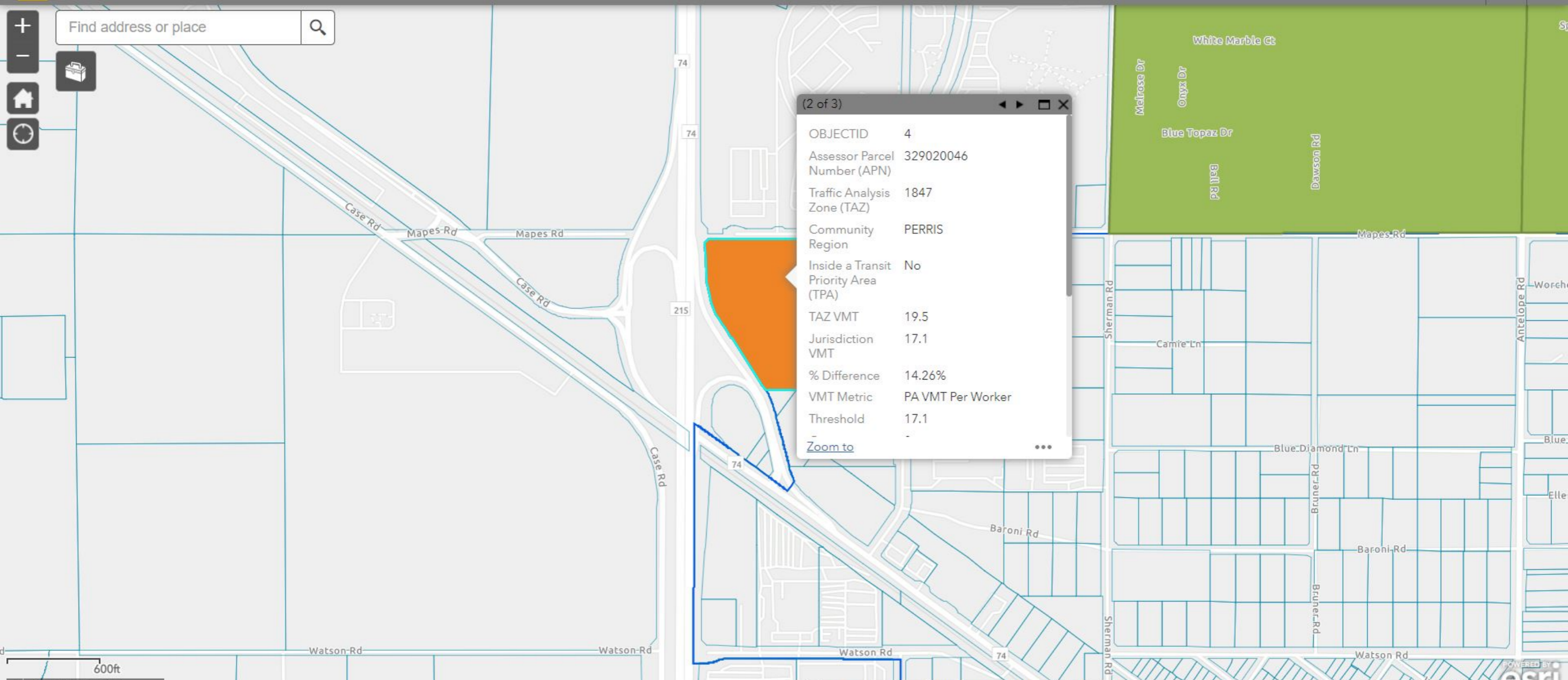
2045	Mapes and Trumble (Project) *	City of Perris (City) **
Total Population	-	130,959
Total Employment	485	34,275
Total service population	485	165,234
<hr/>		
Total OD VMT	11,374	5,228,215
<hr/>		
Total OD VMT per service population	23.5	31.6

*: Estimated using RIVCOM V3 model runs

**.: Estimage using LSA No Project RIVCOM V3 model runs



Find address or place



(2 of 3)

OBJECTID	4
Assessor Parcel Number (APN)	329020046
Traffic Analysis Zone (TAZ)	1847
Community Region	PERRIS
Inside a Transit Priority Area (TPA)	No
TAZ VMT	19.5
Jurisdiction VMT	17.1
% Difference	14.26%
VMT Metric	PA VMT Per Worker
Threshold	17.1
Zoom to	-

600ft

-117.171 33.753 Degrees

APPENDIX F:

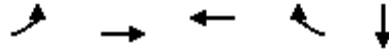
QUEUING ANALYSIS WORKSHEETS

Queues
1: SR-74/I-215 SB Ramps & Bonnie Drive



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	30	173	253	536	651	27
v/c Ratio	0.28	0.11	0.77	0.32	0.53	0.02
Control Delay	55.6	0.1	58.2	2.3	15.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	55.6	0.1	58.2	2.3	15.0	0.0
Queue Length 50th (ft)	21	0	171	64	258	0
Queue Length 95th (ft)	51	0	245	108	443	0
Internal Link Dist (ft)				949	251	
Turn Bay Length (ft)		100	110			120
Base Capacity (vph)	311	1615	359	1699	1225	1615
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.10	0.11	0.70	0.32	0.53	0.02
Intersection Summary						

Queues
2: SR-74 & I-215 NB Ramps



Lane Group	EBL	EBT	WBT	WBR	SBT
Lane Group Flow (vph)	15	791	755	834	198
v/c Ratio	0.14	0.28	0.29	0.60	0.71
Control Delay	45.1	3.8	6.2	2.7	35.8
Queue Delay	0.0	0.0	0.0	0.2	0.0
Total Delay	45.1	3.8	6.2	2.8	35.8
Queue Length 50th (ft)	9	56	53	0	64
Queue Length 95th (ft)	29	103	153	47	128
Internal Link Dist (ft)		414	649		228
Turn Bay Length (ft)	240			355	
Base Capacity (vph)	374	2799	2622	1401	620
Starvation Cap Reductn	0	0	0	98	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.04	0.28	0.29	0.64	0.32
Intersection Summary					

Queues
1: SR-74/I-215 SB Ramps & Bonnie Drive

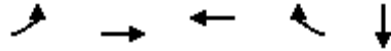


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	36	299	278	330	860	26
v/c Ratio	0.32	0.19	0.77	0.19	0.73	0.02
Control Delay	56.4	0.3	55.8	1.9	21.9	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	56.4	0.3	55.8	1.9	21.9	0.0
Queue Length 50th (ft)	25	0	186	35	438	0
Queue Length 95th (ft)	57	0	266	63	#793	0
Internal Link Dist (ft)				949	251	
Turn Bay Length (ft)		100	110			120
Base Capacity (vph)	311	1615	377	1694	1186	1615
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.12	0.19	0.74	0.19	0.73	0.02

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Queues
2: SR-74 & I-215 NB Ramps



Lane Group	EBL	EBT	WBT	WBR	SBT
Lane Group Flow (vph)	15	1106	546	767	290
v/c Ratio	0.14	0.42	0.22	0.57	0.79
Control Delay	45.1	6.6	8.0	2.9	40.8
Queue Delay	0.0	0.0	0.0	0.1	0.0
Total Delay	45.1	6.6	8.0	3.0	40.8
Queue Length 50th (ft)	9	118	47	0	119
Queue Length 95th (ft)	29	207	128	55	191
Internal Link Dist (ft)		414	649		228
Turn Bay Length (ft)	240			355	
Base Capacity (vph)	374	2606	2429	1337	618
Starvation Cap Reductn	0	0	0	76	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.04	0.42	0.22	0.61	0.47

Intersection Summary

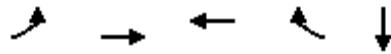
Queues
1: SR-74/I-215 SB Ramps & Bonnie Drive



Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	30	175	254	549	675	27
v/c Ratio	0.28	0.11	0.77	0.32	0.55	0.02
Control Delay	55.6	0.1	58.1	2.3	15.5	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	55.6	0.1	58.1	2.3	15.5	0.0
Queue Length 50th (ft)	21	0	172	66	274	0
Queue Length 95th (ft)	51	0	246	111	467	0
Internal Link Dist (ft)				949	251	
Turn Bay Length (ft)		100	110			120
Base Capacity (vph)	311	1615	359	1699	1224	1615
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.10	0.11	0.71	0.32	0.55	0.02

Intersection Summary

Queues
2: SR-74 & I-215 NB Ramps



Lane Group	EBL	EBT	WBT	WBR	SBT
Lane Group Flow (vph)	15	817	769	847	223
v/c Ratio	0.14	0.30	0.30	0.61	0.74
Control Delay	45.1	4.3	6.9	2.8	38.0
Queue Delay	0.0	0.0	0.0	0.2	0.0
Total Delay	45.1	4.3	6.9	3.0	38.0
Queue Length 50th (ft)	9	64	59	0	79
Queue Length 95th (ft)	29	116	165	50	146
Internal Link Dist (ft)		414	649		228
Turn Bay Length (ft)	240			355	
Base Capacity (vph)	374	2751	2574	1394	621
Starvation Cap Reductn	0	0	0	90	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.04	0.30	0.30	0.65	0.36

Intersection Summary

Queues
1: SR-74/I-215 SB Ramps & Bonnie Drive

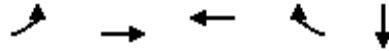


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	36	300	280	354	878	26
v/c Ratio	0.32	0.19	0.77	0.21	0.74	0.02
Control Delay	56.4	0.3	55.5	2.0	22.7	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	56.4	0.3	55.5	2.0	22.7	0.0
Queue Length 50th (ft)	25	0	187	38	457	0
Queue Length 95th (ft)	57	0	265	68	#824	0
Internal Link Dist (ft)				949	251	
Turn Bay Length (ft)		100	110			120
Base Capacity (vph)	311	1615	379	1694	1182	1615
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.12	0.19	0.74	0.21	0.74	0.02

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Queues
2: SR-74 & I-215 NB Ramps



Lane Group	EBL	EBT	WBT	WBR	SBT
Lane Group Flow (vph)	15	1125	572	789	309
v/c Ratio	0.14	0.44	0.24	0.59	0.79
Control Delay	45.1	7.2	8.6	3.1	41.1
Queue Delay	0.0	0.0	0.0	0.1	0.0
Total Delay	45.1	7.2	8.6	3.2	41.1
Queue Length 50th (ft)	9	128	53	0	130
Queue Length 95th (ft)	29	222	138	57	203
Internal Link Dist (ft)		414	649		228
Turn Bay Length (ft)	240			355	
Base Capacity (vph)	374	2566	2389	1335	619
Starvation Cap Reductn	0	0	0	68	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.04	0.44	0.24	0.62	0.50
Intersection Summary					

Queues
1: SR-74/I-215 SB Ramps & Bonnie Drive

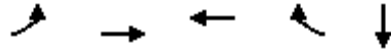


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	75	286	365	827	827	71
v/c Ratio	0.51	0.18	0.69	0.52	0.89	0.04
Control Delay	59.8	0.2	42.5	4.9	40.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	59.8	0.2	42.5	4.9	40.0	0.0
Queue Length 50th (ft)	52	0	232	153	519	0
Queue Length 95th (ft)	96	0	335	269	#848	0
Internal Link Dist (ft)				949	251	
Turn Bay Length (ft)		100	110			120
Base Capacity (vph)	311	1615	530	1597	931	1615
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.24	0.18	0.69	0.52	0.89	0.04

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Queues
2: SR-74 & I-215 NB Ramps



Lane Group	EBL	EBT	WBT	WBR	SBT
Lane Group Flow (vph)	41	1048	1102	1136	393
v/c Ratio	0.32	0.44	0.53	0.84	0.84
Control Delay	47.8	9.2	15.9	10.5	42.3
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	47.8	9.2	15.9	10.5	42.3
Queue Length 50th (ft)	24	141	221	41	179
Queue Length 95th (ft)	56	236	360	#510	259
Internal Link Dist (ft)		414	649		228
Turn Bay Length (ft)	240			355	
Base Capacity (vph)	374	2391	2092	1353	617
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.11	0.44	0.53	0.84	0.64

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Queues
1: SR-74/I-215 SB Ramps & Bonnie Drive

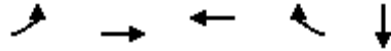


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	92	432	432	569	1217	60
v/c Ratio	0.56	0.27	0.65	0.36	1.58	0.04
Control Delay	60.4	0.4	36.2	3.9	292.3	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	60.4	0.4	36.2	3.9	292.3	0.0
Queue Length 50th (ft)	63	0	264	90	~1207	0
Queue Length 95th (ft)	112	0	394	160	#1492	0
Internal Link Dist (ft)				949	251	
Turn Bay Length (ft)		100	110			120
Base Capacity (vph)	311	1615	666	1582	772	1615
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.30	0.27	0.65	0.36	1.58	0.04

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Queues
2: SR-74 & I-215 NB Ramps



Lane Group	EBL	EBT	WBT	WBR	SBT
Lane Group Flow (vph)	58	1539	874	1023	655
v/c Ratio	0.40	0.75	0.54	0.82	1.03
Control Delay	49.0	18.4	21.1	8.8	73.7
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	49.0	18.4	21.1	8.8	73.7
Queue Length 50th (ft)	34	336	198	21	~399
Queue Length 95th (ft)	71	422	274	194	#616
Internal Link Dist (ft)		414	649		228
Turn Bay Length (ft)	240			355	
Base Capacity (vph)	374	2078	1630	1255	635
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.16	0.74	0.54	0.82	1.03

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Queues
1: SR-74/I-215 SB Ramps & Bonnie Drive

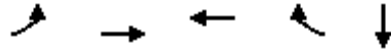


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	75	288	366	840	851	71
v/c Ratio	0.51	0.18	0.69	0.53	0.92	0.04
Control Delay	59.8	0.2	42.4	5.0	43.4	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	59.8	0.2	42.4	5.0	43.4	0.0
Queue Length 50th (ft)	52	0	232	157	548	0
Queue Length 95th (ft)	96	0	335	277	#885	0
Internal Link Dist (ft)				949	251	
Turn Bay Length (ft)		100	110			120
Base Capacity (vph)	311	1615	532	1597	929	1615
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.24	0.18	0.69	0.53	0.92	0.04

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Queues
2: SR-74 & I-215 NB Ramps



Lane Group	EBL	EBT	WBT	WBR	SBT
Lane Group Flow (vph)	41	1073	1116	1149	418
v/c Ratio	0.32	0.46	0.54	0.85	0.86
Control Delay	47.8	9.8	16.7	11.4	43.9
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	47.8	9.8	16.7	11.4	43.9
Queue Length 50th (ft)	24	154	233	47	194
Queue Length 95th (ft)	56	244	366	#528	281
Internal Link Dist (ft)		414	649		228
Turn Bay Length (ft)	240			355	
Base Capacity (vph)	374	2352	2052	1347	617
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.11	0.46	0.54	0.85	0.68

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Queues
1: SR-74/I-215 SB Ramps & Bonnie Drive

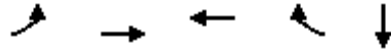


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	92	433	434	593	1234	60
v/c Ratio	0.56	0.27	0.65	0.37	1.61	0.04
Control Delay	60.4	0.4	36.0	4.0	305.7	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	60.4	0.4	36.0	4.0	305.7	0.0
Queue Length 50th (ft)	63	0	265	96	~1235	0
Queue Length 95th (ft)	112	0	396	170	#1520	0
Internal Link Dist (ft)				949	251	
Turn Bay Length (ft)		100	110			120
Base Capacity (vph)	311	1615	670	1582	768	1615
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.30	0.27	0.65	0.37	1.61	0.04

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Queues
2: SR-74 & I-215 NB Ramps



Lane Group	EBL	EBT	WBT	WBR	SBT
Lane Group Flow (vph)	58	1557	900	1045	674
v/c Ratio	0.40	0.76	0.55	0.83	1.06
Control Delay	49.0	18.7	21.4	9.9	82.6
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	49.0	18.7	21.4	9.9	82.6
Queue Length 50th (ft)	34	343	206	29	~423
Queue Length 95th (ft)	71	431	284	#260	#642
Internal Link Dist (ft)		414	649		228
Turn Bay Length (ft)	240			355	
Base Capacity (vph)	374	2078	1630	1255	635
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.16	0.75	0.55	0.83	1.06

Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Queues
3: Trumble Road & Mapes Road



Lane Group	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	90	459	25	71	314	1	90
v/c Ratio	0.26	0.63	0.06	0.10	0.17	0.00	0.05
Control Delay	14.5	28.8	19.0	11.1	3.2	11.0	9.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	14.5	28.8	19.0	11.1	3.2	11.0	9.8
Queue Length 50th (ft)	5	94	8	15	6	0	8
Queue Length 95th (ft)	24	126	24	41	29	3	23
Internal Link Dist (ft)	947		219		270		348
Turn Bay Length (ft)		125		135		125	
Base Capacity (vph)	874	928	497	709	1821	572	1925
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.10	0.49	0.05	0.10	0.17	0.00	0.05
Intersection Summary							

Intersection: 4: Trumble Road & Exceed Road/Sturgeon Electric Driveway

Movement	EB	WB	NB
Directions Served	LTR	LTR	L
Maximum Queue (ft)	31	75	26
Average Queue (ft)	13	38	6
95th Queue (ft)	38	70	24
Link Distance (ft)	535	314	
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			100
Storage Blk Time (%)			
Queuing Penalty (veh)			

Queues
3: Trumble Road & Mapes Road



Lane Group	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	23	434	42	6	792	38	222
v/c Ratio	0.08	0.61	0.12	0.01	0.36	0.11	0.10
Control Delay	18.5	28.9	8.9	9.4	3.0	10.1	7.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	18.5	28.9	8.9	9.4	3.0	10.1	7.8
Queue Length 50th (ft)	1	89	1	1	13	5	14
Queue Length 95th (ft)	10	101	17	6	35	22	38
Internal Link Dist (ft)	947		219		270		348
Turn Bay Length (ft)		125		135		125	
Base Capacity (vph)	836	906	452	708	2180	360	2192
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.03	0.48	0.09	0.01	0.36	0.11	0.10
Intersection Summary							

Intersection: 4: Trumble Road & Exceed Road/Sturgeon Electric Driveway

Movement	EB	WB	NB
Directions Served	LTR	LTR	L
Maximum Queue (ft)	76	70	26
Average Queue (ft)	32	25	8
95th Queue (ft)	60	60	27
Link Distance (ft)	535	314	
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)			100
Storage Blk Time (%)			
Queuing Penalty (veh)			