

Preliminary Hydrology & LID Study

4112-4136 DEL REY AVENUE

**4112-4136 DEL REY AVENUE
Marina Del Rey, CA 90292**



TUSTIN

17542 E. 17th Street
Suite 150
Tustin, CA 92780-1947
714.665.4500
Fax: 714.665.4501

***LOS ANGELES**

201 S. Figueroa Street
Suite 240
Los Angeles, CA 90012-3601
213.337.3680

SANTA CLARITA

25152 Springfield Court
Suite 350
Santa Clarita, CA 91355-1096
661.284.7400
Fax 661.284.7401

ONTARIO

4141 E. Inland Empire Blvd.
Suite 250
Ontario, CA 91764
909.481.5750
Fax 909.481.5757

TEMECULA

41951 Remington Avenue
Suite 220
Temecula, CA 92590-2553
951.294.9300
Fax: 951.294-9301

APPLE VALLEY

18484 Outer Hwy 18 N
Suite 225
Apple Valley, CA 92307
760.524.9100
Fax 760.524.9101

www.deainc.com

Prepared For:

**MDR INVESTORS, LLC
1880 CENTURY PARK EAST, SUITE 1017
LOS ANGELES, CA 90067**

Prepared By:



Jodi Mensen, P.E.

**August 2022
LATDMDRI-0001**





TABLE OF CONTENTS

	<u>Page</u>
Introduction (Project Location, Description, Scope)	1
Existing Site Description.....	2
Proposed Development.....	3
Hydrology & LID Methodology.....	4-5
Conclusion	5

APPENDICES

Appendix A: Vicinity Map & Hydrologic Map & Information

Appendix B: Hydrology Maps

- Pre-development Hydrology Exhibit

- Post-development Hydrology & LID Exhibit

Appendix C: Pre-development Hydrology Calculations (Q₂₅)

Appendix D: Post-development Hydrology Calculations (Q₂₅)

Appendix E: Required Treatment Peak Flow Q_{pm} & LID-Stormwater Design Volume
(SWQDv) Calculations

Appendix F: Permavoid Device Details

Appendix G: Soil Percolation Test Information

Appendix H: Existing Storm Drain Information

Appendix J: BMP's Operate & Maintenance Agreement



INTRODUCTION

1. PROJECT LOCATION, DESCRIPTION & SCOPE

The Project Site is located at 4112-4136 Del Rey Avenue in Marina Del Rey, California. It consists of 3 commercially developed parcels and currently occupied by several one-to two-story commercial buildings with asphalt parking. The proposed redevelopment will consist of building a multi-story residential building with above ground parking.

This report has been prepared to assess at the planning level for the Preliminary Drainage & LID Study of the Del Rey Apartments and redevelopment site.

The scope of the study includes:

1. Analysis of the pre-development and post-development hydrologic conditions on-site 25-year storm runoff for each sub-area.
2. Perform the stormwater quality analysis of the designed and management of capture stormwater runoff, LID Volume & BMP's practices of this re-development site.
3. Provided the on-site & off-site existing drainage system information.
4. Provided the Soil information & Percolation test Investigation information of the project site.



2. EXISTING SITE DESCRIPTION AND CONDITIONS

As previously stated, the project site consists of several one- to two-story commercial buildings with asphalt parking. There are no landscaped areas on the project site.

The existing site flow direction is from north to south. Per the research of the project drainage system, there are no existing storm drains in Del Rey avenue. The site runoff discharges directly onto the street.

According to the soil percolation test by the Twining Consulting on April 21, 2022, the Percolation rate is 0.12 inch/hr. It is less than 0.3 in/hr, per the City of Los Angeles minimum required design infiltration rate. There is also a potential risk of liquefaction-induced settlement and loss of soil bearing strength due to the presence of relatively shallow liquefiable soils and therefore infiltration is infeasible.



3. PROPOSED DEVELOPMENT

The proposed project consists of a 6-story residential building with on-grade and above grade parking.

The site drainage will mimic the existing drainage conditions with overflow discharging onto Del Rey Avenue through curb drains. Per the current storm water quality requirement of the City, the proposed redeveloped area will be required to have the stormwater treatment before draining to the existing storm drain system downstream.

To meet the City of Los Angeles LID & storm water quality requirements, the Permavoid devices and trench drains/area drains are proposed for mitigating the low flow and treatment for the required LID volumes. The proposed Permavoid devices are considered as the capture & reuse device which will be designed following the standards and requirements of the City of Los Angeles the Development Best Management Practices Book & LID manual. All on-site drains, storm drain pipes, permeable pavers, LID devices and BMP's will be privately maintained. The Operation and Maintenance Agreement will be done in the final engineering stage. For the Hydrology and the proposed renovation area location see the Preliminary Post-development Hydrology/LID/ SUSMP & BMP's Exhibit in Appendix B of this report.



4. HYDROLOGY & METHODOLOGY

The hydrologic data and methodology used in this report is based on the Los Angeles County Department of Public Works, Hydrology Manual dated by 2006. The site doesn't fall within the Capital Flood Protection requirements and therefore the Urban Flood Protection only requires a 25-year frequency storm analysis. The site soil is class is # 016, the 50-year precipitation isohyetal is 5.4", and the 85th percentile precipitation is 1.1" (greater than 0.75"), is used for the Low Impact Design Volume calculations.

The Los County hydrologic calculator- HydroCalc was used to generate the 25-year peak discharge, time of concentration, LID Stormwater Quality Design Volume (Vd) calculations of this study. As the previously stated infiltration rate results show a percolation rate of 0.12 in/hr, which does not allow for efficient infiltration. The infiltration rate is below the required minimum design infiltration rate for design of stormwater infiltration improvements. Therefore, Permavoid capture & reuse treatment devices are the preferred method of stormwater mitigation per the City of Los Angeles LID Handbook.

Hydrology & LID rainfall values used in the study:

Storm Event & Duration	Rainfall (inches)
50-Year Storm, 24-Hour	5.4
85 th Percentile 24-Hours	1.1

On-site hydrology calculation summary tables see below:

Pre-Development Hydrology Summary Table:

Drainage Area ID	Tributary Area (acre)	Impervious Ratio	Tc (min.)	Q ₂₅ (cfs)
1A	2.83	1.0	7	6.15
TOTAL	2.83			

Post-Development Hydrology Summary Table:

Drainage Area ID	Tributary Area (acre)	Impervious Ratio	Tc (min.)	Q ₂₅ (cfs)
1A	2.8	0.86	10	5.04
TOTAL	2.8			



Post-Development LID/SUSMP Summary Table:

Drainage Area ID	Tributary Area (acre)	Impervious Ratio	Tc (min.)	Required Treatment Peak Flow Qpm (cfs)	Required Treatment Volume SWQDv (ft ³)	Provided Treatment Volume:
1	1.86	1.0	22	0.55	6,630	6,630
2	0.94	0.60	37	0.14	2,159	2,159
TOTAL	2.8				8,788	8,788

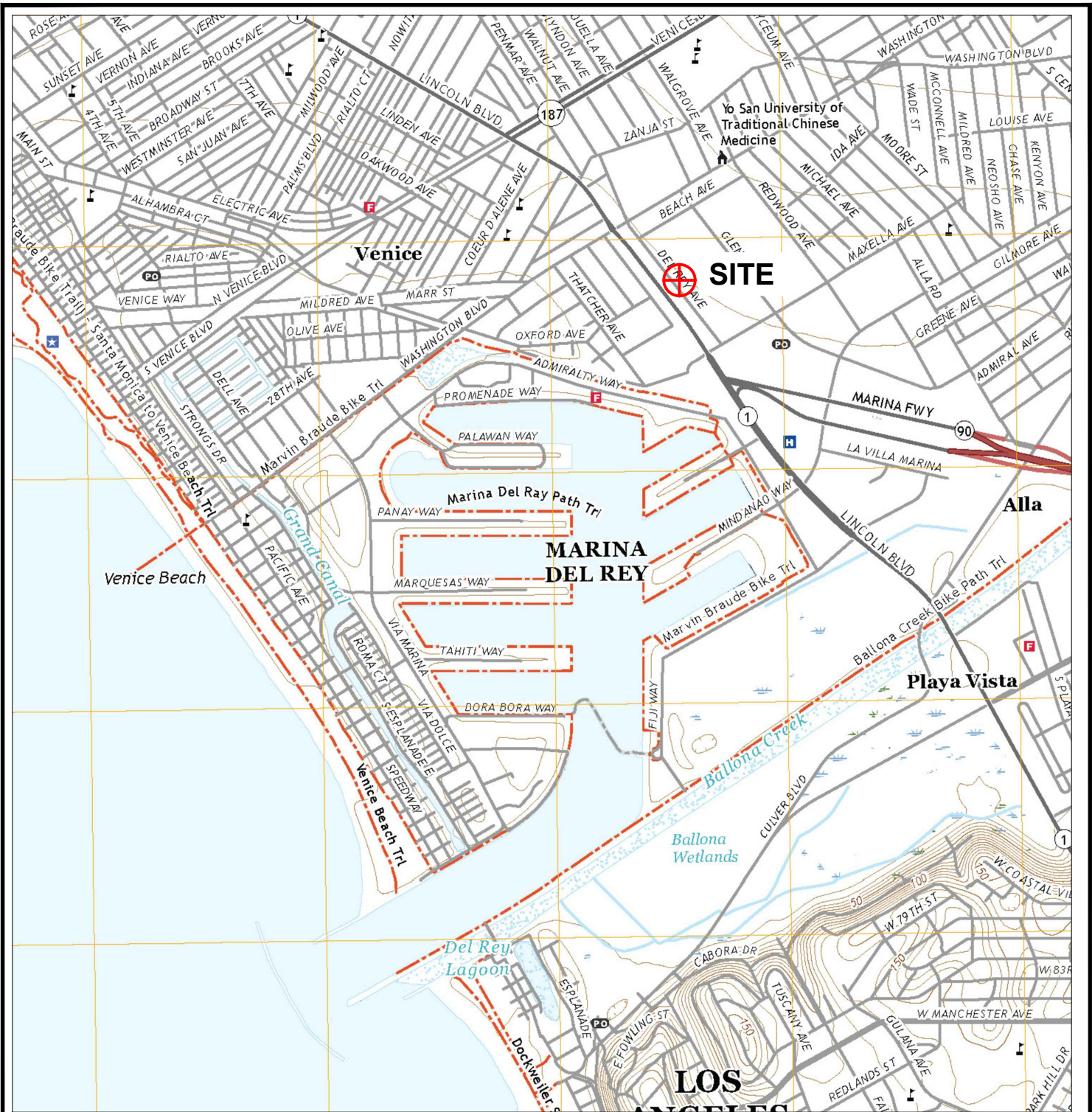
5. CONCLUSION

In conclusion, this report demonstrates that the proposed redevelopment is designed to follow the City of Los Angeles drainage and LID requirements. The development will not increase runoff and will not cause flood risk to neighboring properties, or the public storm drain system. The owner will privately maintain the on-site drainage system, consisting of trench drains, permeable pavers, landscaping, and BMP's. The proposed Permavoid devices and drainage devices will be designed and comply with the flood protection and storm water quality requirements of the City of Los Angeles.

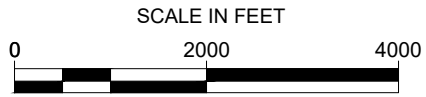


Appendix A:

Vicinity Map & Hydrologic Map & Information

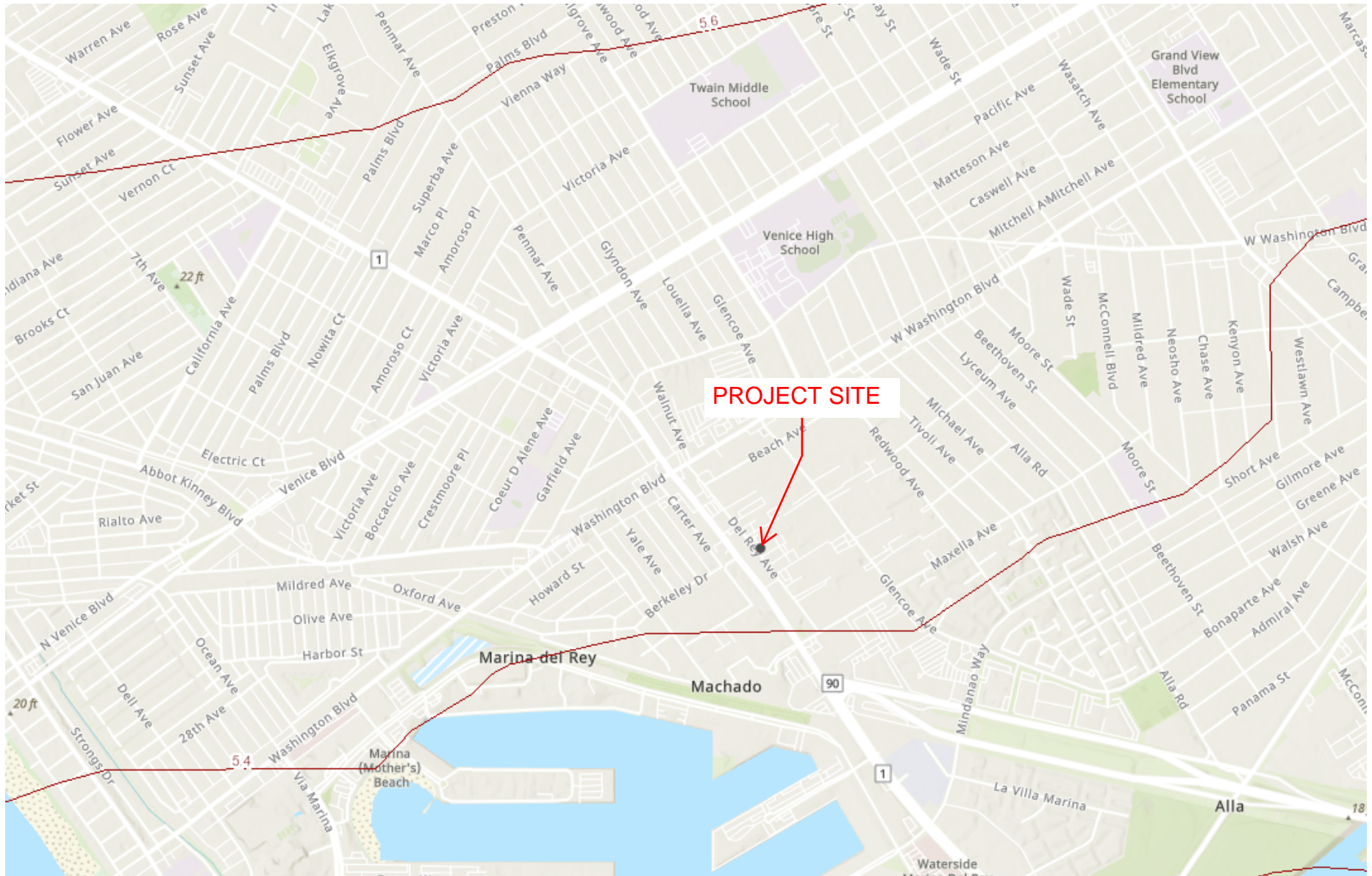


 APPROXIMATE LOCATION OF PROJECT



REFERENCE: USGS (2021)

SITE LOCATION MAP		
LATERRA MARINA DEL REY 412 DEL REY AVENUE MARINA DEL REY, CA		
PROJECT NO.	REPORT DATE	FIGURE 1








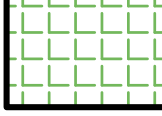


Appendix B: Hydrology Maps

- Pre-development Hydrology Exhibit
- Post-development Hydrology & LID Exhibit

LEGEND

-  SUBAREA DESIGNATION
-  SUBAREA'S AREA VALUE IN SQUARE FOOTAGE
-  PROPERTY LINE

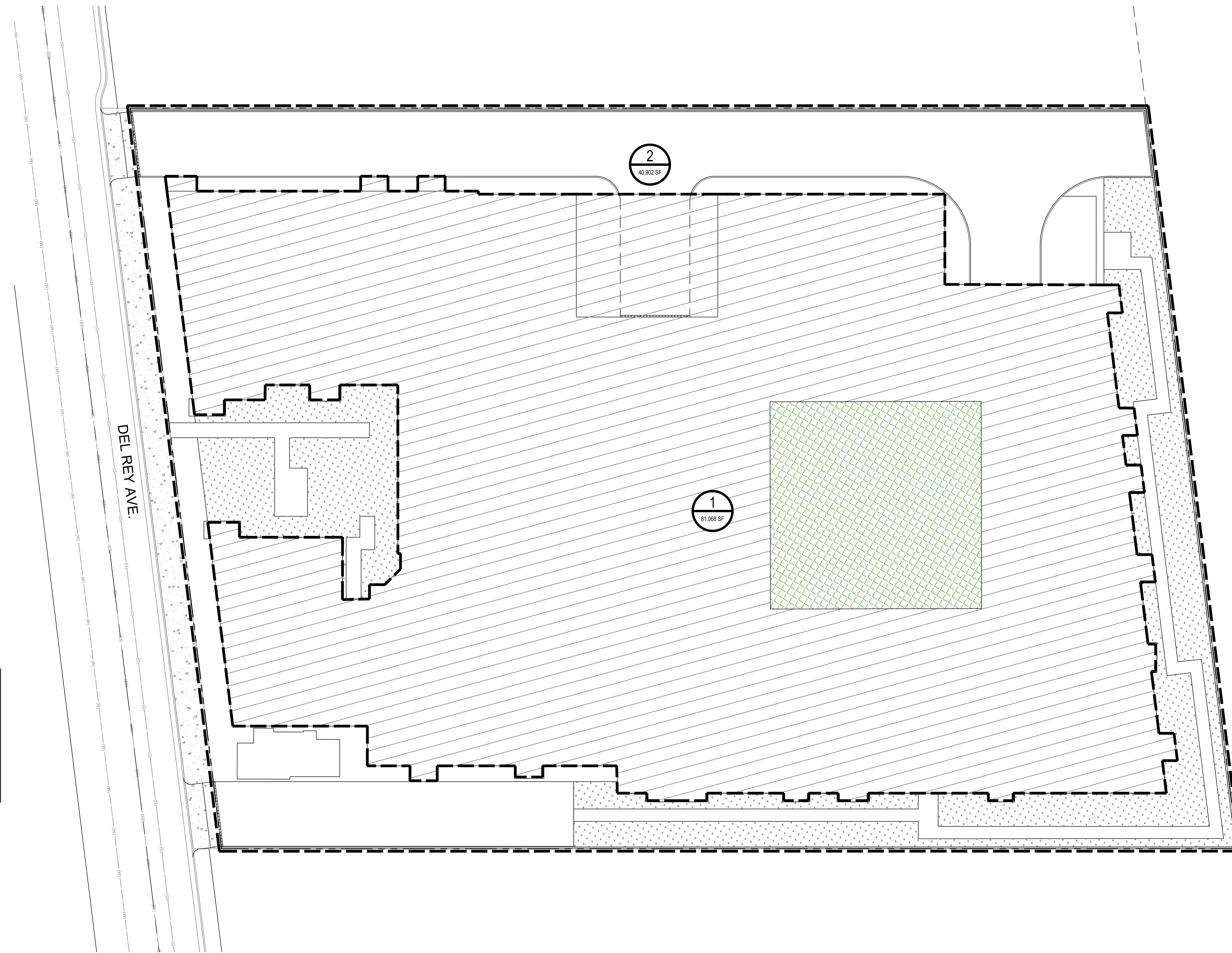
-  ROOF AREA
-  LANDSCAPE AREA (NON-LID)
-  PERMAVOID PLANTER
7,091 FT²

Area = 2.8 acres
Area Impervious = 2.4 acres

Vm = 8,788 CF

NOTE:

ALL TRASH BINS SHALL BE COVERED.
 ANY CHANGES (TYPE, SIZE, LOCATION) TO APPROVED STORMWATER BEST MANAGEMENT PRACTICE(S) (BMPs) MUST OBTAIN WRITTEN APPROVAL FROM LOS ANGELES, DEPARTMENT OF PUBLIC WORKS, BUREAU OF SANITATION PRIOR TO CONSTRUCTION OF BMP(S).
 DIRECT OVERFLOW DISCHARGE PER BUREAU OF ENGINEERING AND BUILDING AND SAFETY REQUIREMENTS.

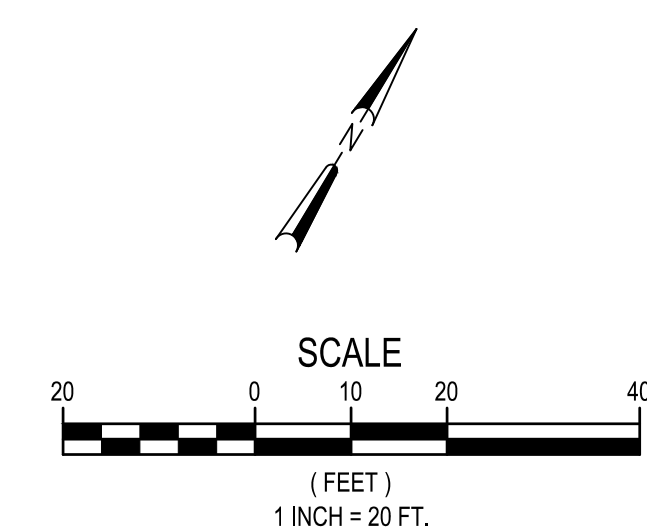


AREA FLOW AND MITIGATED FLOW INFORMATION:

DRAINAGE SUBAREA	AREA (SQ. FT.)	AREA (ACRE)	ROOF AREA (SQ. FT.)	GROUND AREA (SQ. FT.)	IMPERVIOUS AREA (SQ. FT.)	PERVIOUS AREA (SQ. FT.)	BMP TYPE PROVIDED	MITIGATED VOLUME V _{DES}	STORED VOLUME V _S	REQUIRED MITIGATED VOLUME V _S
1	81,068	1.86	81,068	0	81,068	0	PERMAVOID	-	-	-
2	40,902	0.94	0	40,902	24,176	16,726	FILTER INSERTS, LANDSCAPING, PERMEABLE PAVERS	-	-	-
TOTAL:	121,970	2.80	81,068	40,902	105,244	16,726		-	-	-



STENCIL DETAIL
 N.T.S.



By: [Signature] Date: 8/29/2022 4:28 PM
 Checked: [Signature] Date: 8/29/2022 4:28 PM
 Scale: 1/8" = 10'-0"

REVIEWED BY: [Signature] DATE: [] BY: CK
 NO. DATE REVISION



CHECKED BY: JOME
 DESIGNED BY: RAK
 DRAWN BY: RAK

FIRST SUBMITTAL DATE:
 PROJECT NO.
LATDMR10001

SHEET NO.
 OF 5



Appendix C:

Pre-development Hydrology Calculations (Q₂₅)

Peak Flow Hydrologic Analysis

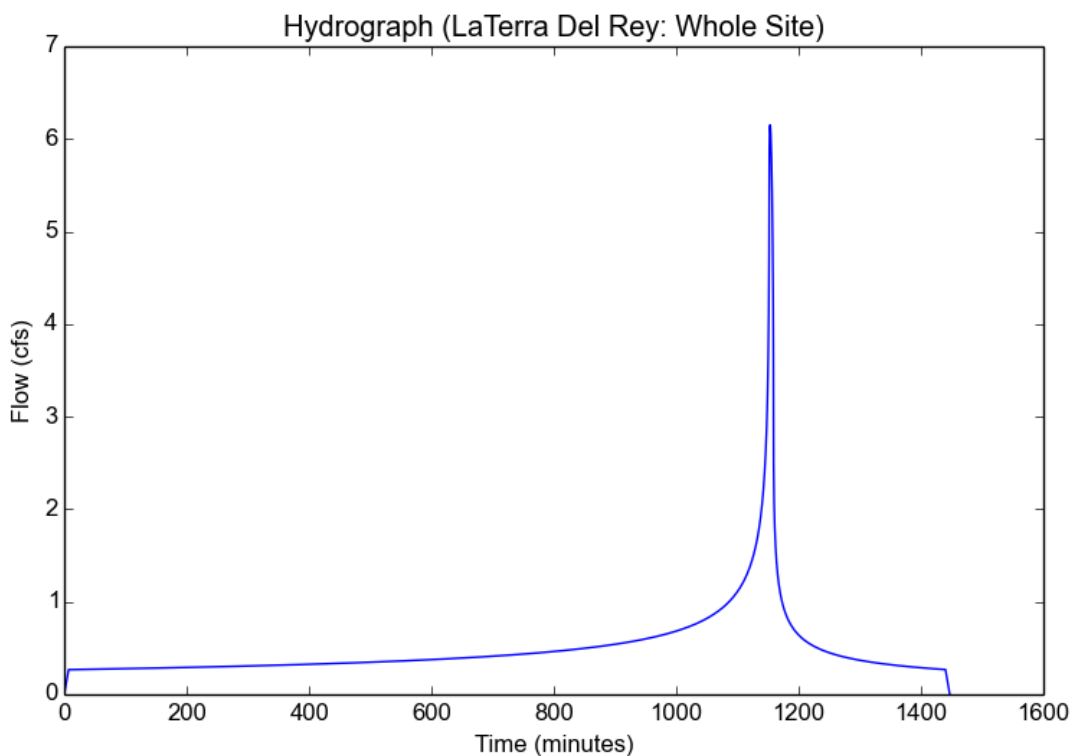
File location: C:/Users/Rxka/Desktop/LaTerra Del Rey - Existing Whole Site 25 yr.pdf
Version: HydroCalc 1.0.3

Input Parameters

Project Name	LaTerra Del Rey
Subarea ID	Whole Site
Area (ac)	2.83
Flow Path Length (ft)	400.0
Flow Path Slope (vft/hft)	0.012
50-yr Rainfall Depth (in)	5.4
Percent Impervious	1.0
Soil Type	16
Design Storm Frequency	25-yr
Fire Factor	0
LID	False

Output Results

Modeled (25-yr) Rainfall Depth (in)	4.7412
Peak Intensity (in/hr)	2.415
Undeveloped Runoff Coefficient (Cu)	0.8017
Developed Runoff Coefficient (Cd)	0.9
Time of Concentration (min)	7.0
Clear Peak Flow Rate (cfs)	6.1509
Burned Peak Flow Rate (cfs)	6.1509
24-Hr Clear Runoff Volume (ac-ft)	0.998
24-Hr Clear Runoff Volume (cu-ft)	43473.0378





Appendix D:

Post-development Hydrology Calculations (Q₂₅)

Peak Flow Hydrologic Analysis

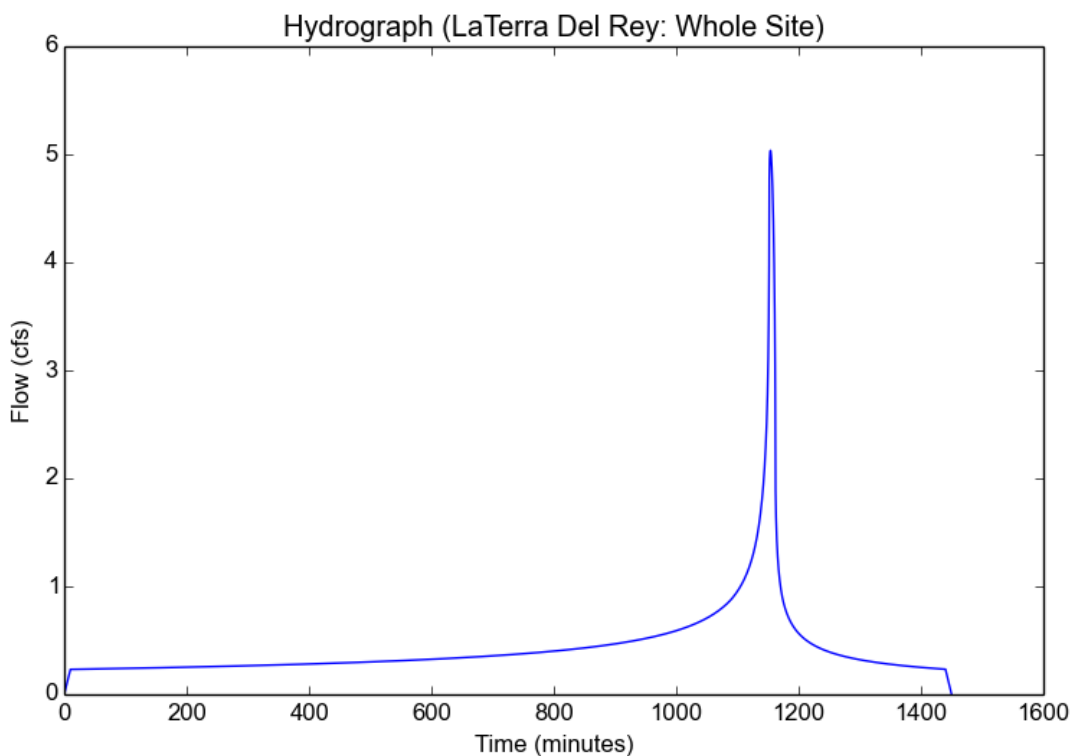
File location: C:/Users/Rxka/Desktop/LaTerra Del Rey - Whole Site.pdf
Version: HydroCalc 1.0.3

Input Parameters

Project Name	LaTerra Del Rey
Subarea ID	Whole Site
Area (ac)	2.8
Flow Path Length (ft)	690.0
Flow Path Slope (vft/hft)	0.0085
50-yr Rainfall Depth (in)	5.4
Percent Impervious	0.86
Soil Type	16
Design Storm Frequency	25-yr
Fire Factor	0
LID	False

Output Results

Modeled (25-yr) Rainfall Depth (in)	4.7412
Peak Intensity (in/hr)	2.0422
Undeveloped Runoff Coefficient (Cu)	0.7619
Developed Runoff Coefficient (Cd)	0.8807
Time of Concentration (min)	10.0
Clear Peak Flow Rate (cfs)	5.0359
Burned Peak Flow Rate (cfs)	5.0359
24-Hr Clear Runoff Volume (ac-ft)	0.876
24-Hr Clear Runoff Volume (cu-ft)	38159.379





Appendix E:

Required Treatment Peak Flow Qpm &

LID-Stormwater Design Volume (SWQDv) Calculations

Peak Flow Hydrologic Analysis

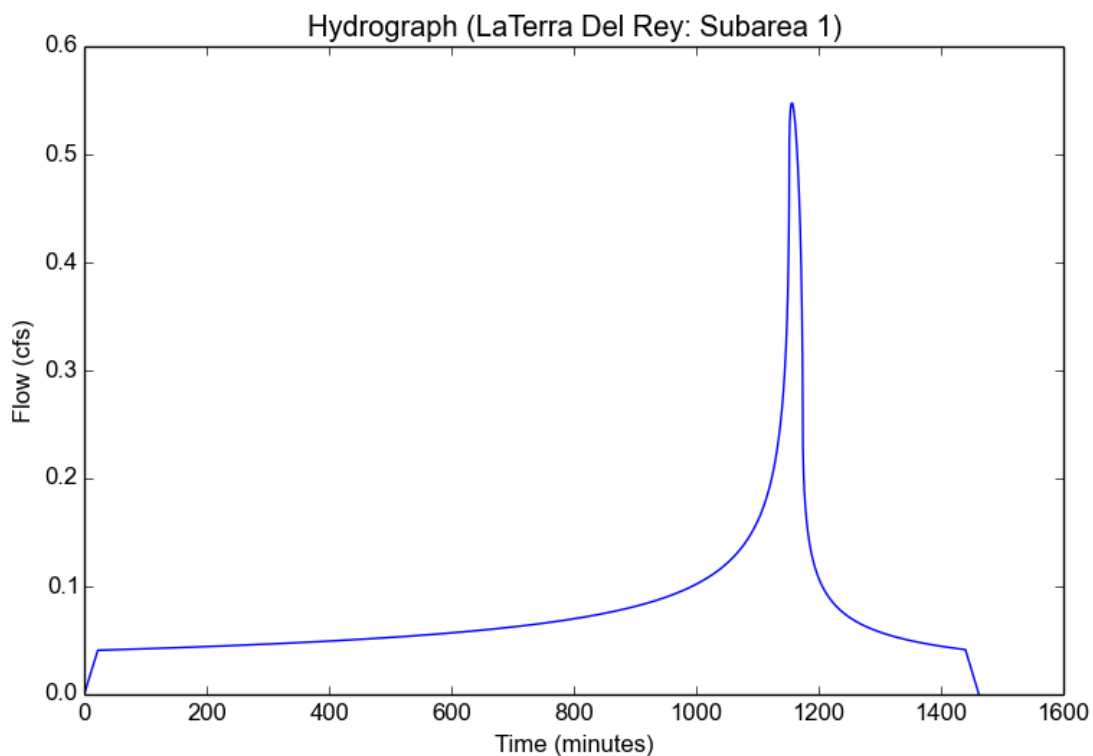
File location: C:/Users/Rxka/Desktop/LaTerra Del Rey - Subarea 1.pdf
Version: HydroCalc 1.0.3

Input Parameters

Project Name	LaTerra Del Rey
Subarea ID	Subarea 1
Area (ac)	1.86
Flow Path Length (ft)	500.0
Flow Path Slope (vft/hft)	0.01
85th Percentile Rainfall Depth (in)	1.1
Percent Impervious	1.0
Soil Type	16
Design Storm Frequency	85th percentile storm
Fire Factor	0
LID	True

Output Results

Modeled (85th percentile storm) Rainfall Depth (in)	1.1
Peak Intensity (in/hr)	0.3271
Undeveloped Runoff Coefficient (Cu)	0.1
Developed Runoff Coefficient (Cd)	0.9
Time of Concentration (min)	22.0
Clear Peak Flow Rate (cfs)	0.5476
Burned Peak Flow Rate (cfs)	0.5476
24-Hr Clear Runoff Volume (ac-ft)	0.1522
24-Hr Clear Runoff Volume (cu-ft)	6629.0807



Peak Flow Hydrologic Analysis

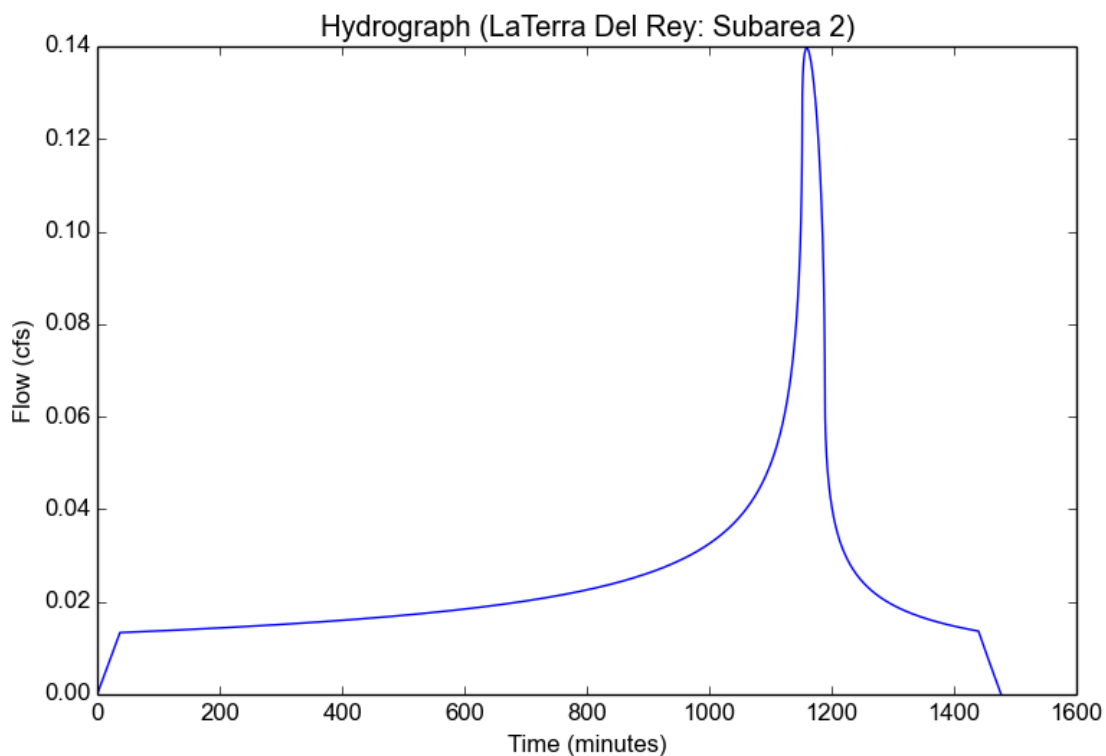
File location: C:/Users/Rxka/Desktop/LaTerra Del Rey - Subarea 2.pdf
Version: HydroCalc 1.0.3

Input Parameters

Project Name	LaTerra Del Rey
Subarea ID	Subarea 2
Area (ac)	0.94
Flow Path Length (ft)	690.0
Flow Path Slope (vft/hft)	0.0085
85th Percentile Rainfall Depth (in)	1.1
Percent Impervious	0.6
Soil Type	16
Design Storm Frequency	85th percentile storm
Fire Factor	0
LID	True

Output Results

Modeled (85th percentile storm) Rainfall Depth (in)	1.1
Peak Intensity (in/hr)	0.2562
Undeveloped Runoff Coefficient (Cu)	0.1
Developed Runoff Coefficient (Cd)	0.58
Time of Concentration (min)	37.0
Clear Peak Flow Rate (cfs)	0.1397
Burned Peak Flow Rate (cfs)	0.1397
24-Hr Clear Runoff Volume (ac-ft)	0.0496
24-Hr Clear Runoff Volume (cu-ft)	2159.0299





Appendix F:

Permavoid Device Details



Project Name: Dey Rey

Date: 8/9/2022

Project Number: PV323133 - "NOT FOR CONSTRUCTION - FOR YOUR REFERENCE ONLY"

Planter ID Number:	CU	2	
Catchment Area:	Acat	72,302	ft ²
85th Percentile Storm Depth:	D85	0.0917	ft
Soil Depth:	Dsoil	18.0	in.
Soil Available Water:	Wa	0%	%
Ponding Depth:	Dpond	0.0	Allowed
Soil Saturated Infiltration Rate:	Ksat	N/A	
Safety Factor:	SF	N/A	
Time To Fill:	Tf	N/A	
Permavoid System Depth:	150x2	11.8	in.
Permavoid Voids:	Vpv	95	%
7-Month Reference Evapotranspiration:	ETo7	21.7	in.
Permavoid Irrigation Efficiency:	IE	0.85	

Design Summary		
Planter Area:	Ap	7091.4 ft ²
Capture & Use Volume:	Vcu	6630.1 ft ³
Biofiltration Volume:	Vbf	N/A
Plant Factor (min.):	PF	0.44
Internal Planter Depth:	Dip	30 in.

Material Requirements		
Permavoid Units:	PV-150	5228 Units
Capillary Cones:	23/160 T	5228 Pairs
Wicking Geotextile Length:	Cap. Tex.	811 - 1183 ft
Planting Media:	By Others	433.4 CY

*NOTES: Quantities are for estimation only. Fluctuations in material quantities will occur based on the precise configuration. Contact ABT-Permavoid for planting media recommendations based on soil depth.

Calculate the Design Capture Volume (Vcap):

$$Vcap = D85 \cdot Acat$$

$$Vcap = 0.0917ft \cdot 72302.4ft^2$$

$$Vcap = 6630.1 ft^3$$

Calculate the Permavoid Capture & Use Depth (Dcu):

$$Dcu = (150x2 \cdot Vpv) + (Dsoil \cdot Wa)$$

$$Dcu = (11.8in. \cdot 95\%) + (18in. \cdot 0\%)$$

$$Dcu = 11.22 in. = 0.935 ft$$

Calculate the Planter Area (Ap):

$$Ap = 7091.4 ft^2$$

Calculate the Capture & Use Volume (Vcu):

$$Vcu = Ap \cdot Dcu$$

$$Vcu = 7091.4 ft^2 \cdot 0.935ft$$

$$Vcu = 6630.1 ft^3$$

Calculate the Minimum Required Plant Factor (PF):

$$PF = (Vcu \cdot IE) / (ETo7 \cdot Ap)$$

$$PF = (6630.1 ft^3 \cdot 0.85) / [(21.7in. / 12ipf) \cdot 7091.4 ft^2]$$

$$PF = 0.44$$

Calculate the 7-Month Estimated Total Water Use (ETWU-7):

$$ETWU-7 = (ETo7 \cdot PF \cdot Ap) / IE$$

$$ETWU-7 = [(21.7in. / 12ipf) \cdot 0.44 \cdot 7091.4 ft^2] / 0.85$$

$$ETWU-7 = 6630.1 ft^3$$

Check the Capture & Use Volume (Vcu) vs. the 7-Month Estimated Total Water Usage (ETWU-7):

Vcu vs. ETWU-7

$$6630.1 ft^3 \text{ vs. } 6630.1 ft^3$$

$$6630.1 ft^3 = 6630.1 ft^3$$

$$Vcu = ETWU-7$$

✓ - CHECKED

STRUCTURAL SUBBASE



PERMEABLE SUBBASE



TREE SOLUTIONS



GREEN STREETS



BLUE / GREEN ROOFS





Appendix G:

Soil Percolation Test Information



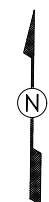
SCALE IN FEET



NOTE: ALL DIMENSIONS AND LOCATIONS ARE APPROXIMATE

LEGEND

- B-1**
TD=26.5'
 APPROXIMATE LOCATION OF BORING BY TWINING
TOTAL DEPTH IN FEET
- P-1**
TD=5'
 APPROXIMATE LOCATION OF PERCOLATION TEST BY TWINING
TOTAL DEPTH IN FEET
- CPT-1**
TD=27.8'
 APPROXIMATE LOCATION OF CPT BY TWINING
TOTAL DEPTH IN FEET
- APPROXIMATE PROJECT LIMITS



REFERENCE: GOOGLE EARTH (2022)

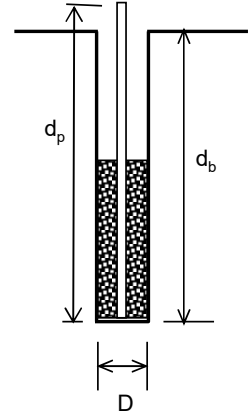


SITE PLAN AND BORING LOCATION MAP		
LATERRA MARINA DEL REY 4112 DEL REY AVENUE MARINA DEL REY, CA		
PROJECT No. 220205.1	REPORT DATE May 2022	FIGURE 2

BORING PERCOLATION FIELD Log

Project No.: 220205.1
 Project Name: 4112 Del Rey Ave

Boring No.: P-1
 Diameter of Boring (D): 8.0 inches
 Depth of Boring (d_b): 5.0 feet = 60 inches
 Diameter of Perc. Pipe : 3.0 inches
 Length of Pipe (d_p) : 5.0 feet = 60 inches



PRE-SOAK	
Date:	<u>4/7/2022</u>
Start Time:	<u>8:05 AM</u>
Elapsed Time:	<u>60.00</u> minutes

REDUCTION FACTORS	
Boring method:	$RF_t = \frac{2}{1}$
Site variability:	$RF_v = \frac{1}{1}$ (1 ~ 3)
Long-term siltation:	$RF_s = \frac{1}{1}$ (1 ~ 3)
Total Reduction Factor:	$RF = RF_t \times RF_v \times RF_s = 2$

PERCOLATION TEST Test Date: 4/7/2022 Test Performer: CD Calculated by: CD

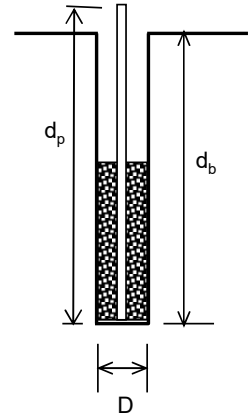
Reading Number	Initial Time T _i	Final Time T _f	Elapsed Time ΔT (min)	Initial depth to water surface dw _i (inches)	Final depth to water surface dw _f (inches)	Initial height of water column d _i (inches)	Drop of water column Δd (inches)	Water height drop rate k _i = Δd / ΔT (inch/hr)	Surface area factor S _f	Raw Percolation Rate k = k _i / S _f (inch/hr)
1	9:05 AM	9:35 AM	30	0.0	5.25	60.0	5.3	10.50	29.7	0.35
2	9:40 AM	10:10 AM	30	0.0	4.88	60.0	4.9	9.75	29.8	0.33
3	10:12 AM	10:42 AM	30	0.0	4.08	60.0	4.1	8.16	30.0	0.27
4	10:48 AM	11:18 AM	30	0.0	3.75	60.0	3.8	7.50	30.1	0.25
5	11:20 AM	11:50 AM	30	0.0	3.75	60.0	3.8	7.50	30.1	0.25
6	11:51 AM	12:21 PM	30	0.0	3.75	60.0	3.8	7.50	30.1	0.25

Measured Percolation Rate k_{measured} (inch/hr) = **0.25**
 Design Infiltration rate (inch/hr) = k_{measured}/RF = **0.12**

BORING PERCOLATION FIELD Log

Project No.: 220205.1
 Project Name: 4112 Del Rey Ave

Boring No.: P-2
 Diameter of Boring (D): 8.0 inches
 Depth of Boring (d_b): 5.0 feet = 60 inches
 Diameter of Perc. Pipe : 3.0 inches
 Length of Pipe (d_p) : 5.0 feet = 60 inches



PRE-SOAK	
Date:	<u>4/7/2022</u>
Start Time:	<u>8:35 AM</u>
Elapsed Time:	<u>60.00</u> minutes

REDUCTION FACTORS	
Boring method:	$RF_t = \frac{2}{1}$
Site variability:	$RF_v = \frac{1}{1}$ (1 ~ 3)
Long-term siltation:	$RF_s = \frac{1}{1}$ (1 ~ 3)
Total Reduction Factor:	$RF = RF_t \times RF_v \times RF_s = 2$

PERCOLATION TEST Test Date: 4/7/2022 Test Performer: CD Calculated by: CD

Reading Number	Initial Time T _i	Final Time T _f	Elapsed Time ΔT (min)	Initial depth to water surface dw _i (inches)	Final depth to water surface dw _f (inches)	Initial height of water column d _i (inches)	Drop of water column Δd (inches)	Water height drop rate k _i = Δd / ΔT (inch/hr)	Surface area factor S _f	Raw Percolation Rate k = k _i / S _f (inch/hr)
1	9:45 AM	10:15 AM	30	6.0	13.9	54.0	7.9	15.84	26.0	0.61
2	10:18 AM	10:48 AM	30	0.0	11.4	60.0	11.4	22.80	28.2	0.81
3	10:52 AM	11:22 AM	30	6.0	10.2	54.0	4.2	8.40	27.0	0.31
4	11:24 AM	11:54 AM	30	6.0	9.6	54.0	3.6	7.20	27.1	0.27
5	11:55 AM	12:25 PM	30	6.0	9.6	54.0	3.6	7.20	27.1	0.27
6	12:28 PM	12:58 PM	30	6.0	9.6	54.0	3.6	7.20	27.1	0.27

Measured Percolation Rate k_{measured} (inch/hr) = **0.27**
 Design Infiltration rate (inch/hr) = k_{measured} / RF = **0.13**



Appendix H:

Existing Storm Drain Information





Appendix J:

BMP's Operate & Maintenance Agreement
(Will be done in final engineering)