

Preliminary Utilities Study
For
Kortum Ranch Subdivision

500 Kortum Canyon Road
Calistoga, CA
APN 011-290-007, 038, 039, 011-310-023

JN 21105
July 10, 2023

Prepared For:
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Preliminary

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Preliminary Utilities Study Narrative

1 Project Description

The Kortum Ranch Subdivision is located at 500 Kortum Canyon Road, in Calistoga south-west of Highway 128. The project proposes to subdivide 3-lots into a 22-lot subdivision for residential use. The tentative map is predicated on the resultant parcels being approved through a lot line adjustment between APN 011-310-023 & 011-290-007.

The site previously had multiple residences, stockpiles, defunct car storage, construction material storage, and various other unpermitted activities. The Kortum Ranch, LLC has cleaned and cleared all previous structures, cars, and construction material in the past year. There are currently gravel access roads and building pads that will be utilized in the proposed improved.

Water and wastewater services for the subdivision will be provided through onsite private systems that will connect to the City's infrastructure. This subdivision proposes the use of water tanks to buffer the peak daily demands on the City of Calistoga's water system. This project does not currently have any water or sewer allocation from the City's utilities. Water and Sewer allocation shall be determined by City Staff with the help of the Engineer of Record.

2 Water & Wastewater Use Calculations

The water and wastewater use calculations are based on the Standardized Use Table for Resource Management System adopted by the City of Calistoga as Resolution No. 2017-083. The table outlines the water and wastewater use based on the type of use of the building. This project proposes 22 four-bedroom homes and 6 accessory dwelling unit for residential use. This project does not currently

The average and peak water and wastewater use for a 4-bedroom residence and an ADU is shown in Table 1. The peak use is 3x the Average Unit Rate. Table 1 shows the usage for each

Table 1: Water and Wastewater Use Table from the Standardized Use Table.

| Area Designation | Water Demand | | Wastewater Demand | |
|---------------------|---------------------------------|------------------------|---------------------------------|------------------------|
| | Average Unit Rate (gpd/unit) | Peak Use (gpd/unit) | Average Unit Rate (gpd/unit) | Peak Use (gpd/unit) |
| 4-Bedroom | 482 | 1,446 | 285 | 855 |
| ADU | 147 | 441 | 130 | 390 |

The total annual water and wastewater demand for the proposed subdivision is shown in Table 2 in acre-ft and gallons per year.

Table 2: Annual Water and Wastewater Use Table for the Project

| | Annual Water Demand | Annual Wastewater Demand |
|----------------------|---------------------|--------------------------|
| Acre-ft/ Year | 12.87 | 7.90 |
| Gallons/ Year | 3,870,000 | 2,573,250 |

3 Domestic Water Allocation

The domestic water use for the Kortum Ranch Subdivision will be supplied by the City of Calistoga and held onsite in a 20,000-gallon domestic water tank for distribution to the individual lots. In order to determine the minimum flow rate provided by the City, the Sequent Peak Method was used to maintain domestic water available for the project.

We have analyzed two alternatives using the sequent peak method. The first analysis uses the average daily use from the project (base demand) of 7.94-gallons per minute (gpm) with the typical municipal water usage per hour (diurnal curve). The second analysis uses the average daily use

from the project with the peak flow being three times the peak flow of the standard diurnal curve. The results are shown in

Table 3: Minimum Flow Rate from the City System

| | Flow Rate |
|--|------------------|
| Average Daily Use | 7.94-gpm |
| Average Daily Use with 3x Peak Flow | 9.00-gpm |

This analysis provides preliminary estimates of the domestic water needed for the project flowing at a constant rate. A more in-depth analysis shall be performed in order to optimize the tank volume and City's ability to provide domestic water to the site.

Appendix I

Water & Wastewater Use Calculations

Water Demand Calculations

Kortum Ranch Subdivision

July 10, 2023

22 Residences - 4 Bedroom Homes

| Area Designation | Use Type | Units (ea) | Water Demand | | | | Wastewater | | | |
|------------------|--------------------|------------|-----------------------|-------------------------|------------------------------------|-----------------------------|-----------------------|-------------------------|------------------------------------|-----------------------------|
| | | | Units Rate (gpd/unit) | Average Daily Use (gpd) | Annual Use (ac-ft/yr) ¹ | Peak Use (gpd) ⁴ | Units Rate (gpd/unit) | Average Daily Use (gpd) | Annual Use (ac-ft/yr) ¹ | Peak Use (gpd) ³ |
| 4 Bedroom | SFD ⁽²⁾ | 22 | 482 | 10,604 | 11.88 | 31,812 | 285 | 6,270 | 7.02 | 18,810 |
| ADU | Contingency | 6 | 147 | 882 | 0.99 | 2,646 | 130 | 780 | 0.87 | 2,340 |
| Total | | | | 11,486 | 12.87 | 34,458 | | 7,050 | 7.90 | 21,150 |

Notes:

1. 1 gallon per day = 0.00112 acre-feet per year
2. SFD = Single Family Dwelling
3. Peak Use is 3 times the Average Daily Use as obtained from City of Santa Rosa 'Sewage Peak Load Factor vs Population'

Standardized Use Table for Resource Management System¹

Amended 7.18.17

| Use Type | Water | | Wastewater | |
|---|--------------------|-----------------|--------------------|-----------------|
| | Acre feet per year | Gallons per day | Acre feet per year | Gallons per day |
| Residential | | | | |
| Single-family dwelling ² | | | | |
| 1 - 3 bedrooms | .428 | 382 | .224 | 200 |
| 4 bedrooms ³ | .540 | 482 | .319 | 285 |
| Apartment, condominium | | | | |
| 1 bedroom | .124 | ----- | .073 | ----- |
| 2 bedrooms | .249 | ----- | .146 | ----- |
| 3 bedrooms | .373 | ----- | .220 | ----- |
| Mobile home | .149 | 133 | .125 | 112 |
| Accessory dwelling unit | .165 | 147 | .146 | 130 |
| Transient lodging - Hotel, motel, resort, B&B – per living or rental unit | | | | |
| | .170 | 152 | .150 | 134 |
| Commercial - Retail, office, personal service – per 1000 sq. ft. of gross floor area | | | | |
| | .110 | 98 | .099 | 88 |
| Bar – per 1000 square feet of gross floor area | | | | |
| | .220 | 196 | .198 | 177 |
| Restaurant - per 1000 square feet of gross floor area | | | | |
| | .580 | 518 | .524 | 468 |

¹ The Department of Public Works may reduce the expected demand upon approval of an engineered water study demonstrating/ quantifying the site-specific water usage.

² The adoption of Resolution No. 2017-083 amending the Standardized Use Table is not intended to affect the number of bedrooms purchased through the now-defunct Leak Detection Program. All projects that purchased leak detection water shall only be vested for the original number of bedrooms purchased.

³ Each additional bedroom, add 100 gallons of water and 85 gallons of wastewater per day.

Source:

<https://www.ci.calistoga.ca.us/home/showpublisheddocument/33183/636994714554530000>

Appendix II

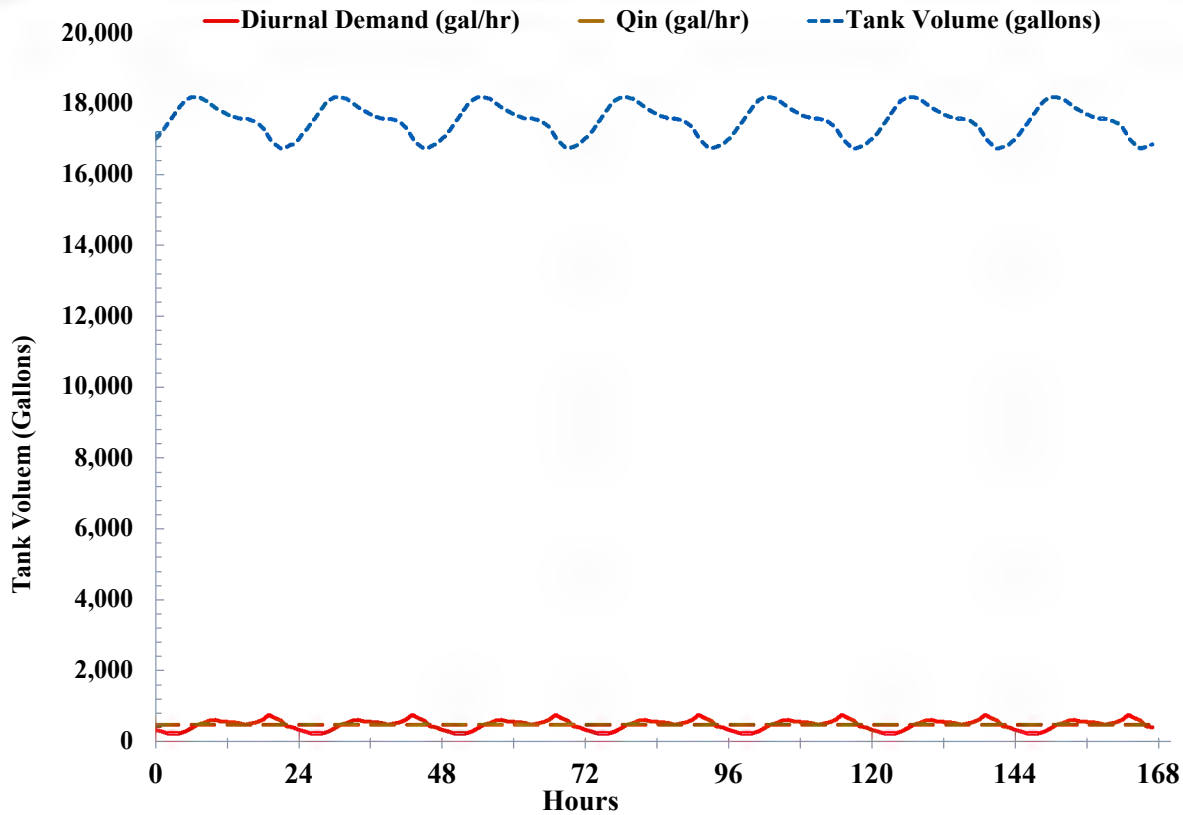
**Domestic Water Tank Sizing
& Diurnal Curve**

Sequent Peak Method Computations Domestic Tank Sizing - Kortum Ranch Subdivions Annual Average Daily Demand

Peak Demand = 12.7 gpm
Base Demand = 7.94 gpm
Tank Volume = 20,000 gallons

Supply = 7.94 gpm
Net 24-hr Volume = 11,434 gallons

| Hour | Base Demand (gal/hr) | Diurnal Factor | Dirunal Demand (gal/hr) | Inflow (gal/hr) | Mass Flow (In-Out) | Mass Flow < 0 (gpm) | Mass Flow > 0 (gpm) | Instantaneous Tank Volume (gallons) |
|---------|----------------------|----------------|-------------------------|-----------------|--------------------|---------------------|---------------------|-------------------------------------|
| 0 | 476 | 0.70 | 333 | 476 | 143 | 0 | 143 | 17000 |
| 1 | 476 | 0.60 | 286 | 476 | 191 | 0 | 191 | 17191 |
| 2 | 476 | 0.50 | 238 | 476 | 238 | 0 | 238 | 17429 |
| 3 | 476 | 0.50 | 238 | 476 | 238 | 0 | 238 | 17667 |
| 4 | 476 | 0.50 | 238 | 476 | 238 | 0 | 238 | 17905 |
| 5 | 476 | 0.60 | 286 | 476 | 191 | 0 | 191 | 18096 |
| 6 | 476 | 0.80 | 381 | 476 | 95 | 0 | 95 | 18191 |
| 7 | 476 | 1.00 | 476 | 476 | 0 | 0 | 0 | 18191 |
| 8 | 476 | 1.10 | 524 | 476 | -48 | 48 | 0 | 18143 |
| 9 | 476 | 1.25 | 596 | 476 | -119 | 119 | 0 | 18024 |
| 10 | 476 | 1.28 | 610 | 476 | -133 | 133 | 0 | 17891 |
| 11 | 476 | 1.20 | 572 | 476 | -95 | 95 | 0 | 17796 |
| 12 | 476 | 1.18 | 562 | 476 | -86 | 86 | 0 | 17710 |
| 13 (1) | 476 | 1.16 | 553 | 476 | -76 | 76 | 0 | 17634 |
| 14 (2) | 476 | 1.10 | 524 | 476 | -48 | 48 | 0 | 17586 |
| 15 (3) | 476 | 1.00 | 476 | 476 | 0 | 0 | 0 | 17586 |
| 16 (4) | 476 | 1.08 | 515 | 476 | -38 | 38 | 0 | 17548 |
| 17 (5) | 476 | 1.15 | 548 | 476 | -71 | 71 | 0 | 17476 |
| 18 (6) | 476 | 1.30 | 619 | 476 | -143 | 143 | 0 | 17333 |
| 19 (7) | 476 | 1.60 | 762 | 476 | -286 | 286 | 0 | 17048 |
| 20 (8) | 476 | 1.40 | 667 | 476 | -191 | 191 | 0 | 16857 |
| 21 (9) | 476 | 1.25 | 596 | 476 | -119 | 119 | 0 | 16738 |
| 22 (10) | 476 | 0.90 | 429 | 476 | 48 | 0 | 48 | 16786 |
| 23 (11) | 476 | 0.85 | 405 | 476 | 71 | 0 | 71 | 16857 |



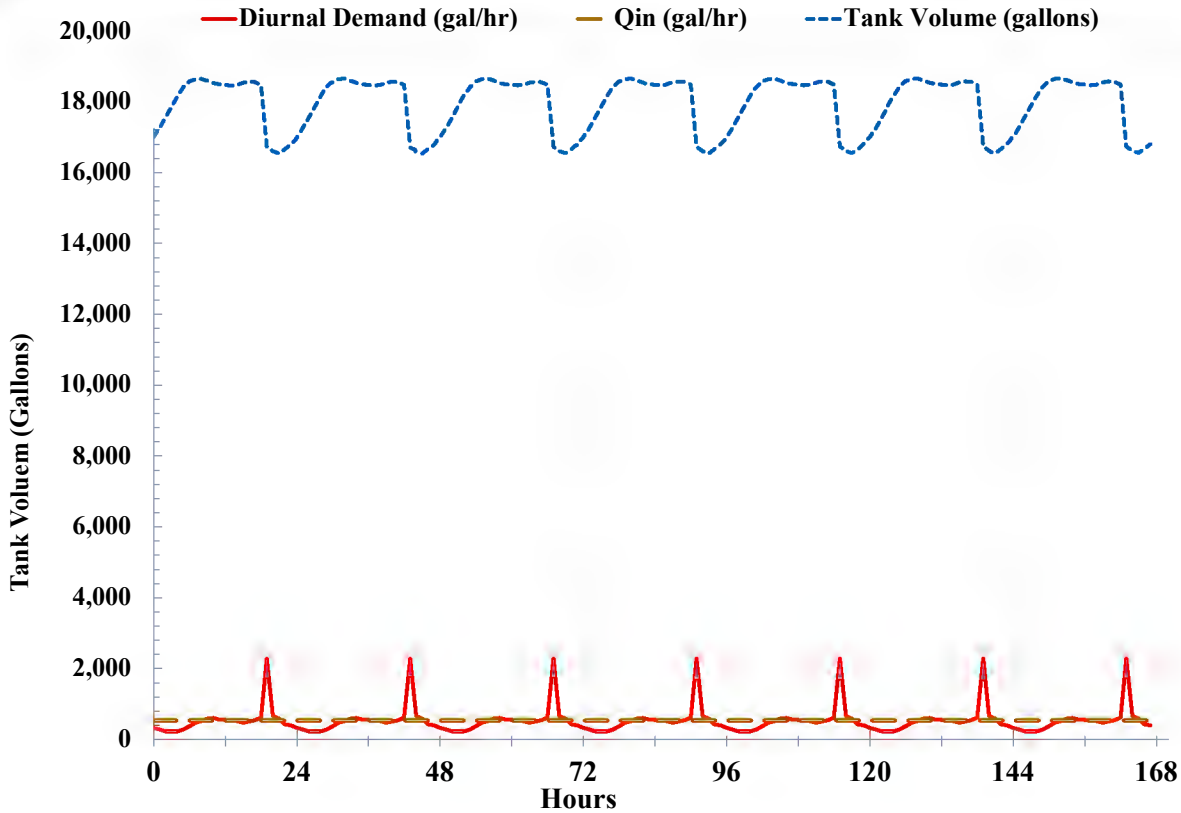
Sequent Peak Method Computations Domestic Tank Sizing - Kortum Ranch Subdivions

Annual Average Daily Demand - With High Peak Use

Peak Demand = 12.7 gpm
 Base Demand = 7.94 gpm
 Tank Volume = 20,000 gallons

Supply = 9.00 gpm
 Net 24-hr Volume = 12,958 gallons

| Hour | Base Demand (gal/hr) | Diurnal Factor | Dirunal Demand (gal/hr) | Inflow (gal/hr) | Mass Flow (In-Out) | Mass Flow < 0 (gpm) | Mass Flow > 0 (gpm) | Instantaneous Tank Volume (gallons) |
|---------|----------------------|----------------|-------------------------|-----------------|--------------------|---------------------|---------------------|-------------------------------------|
| 0 | 476 | 0.70 | 333 | 540 | 207 | 0 | 207 | 17006 |
| 1 | 476 | 0.60 | 286 | 540 | 254 | 0 | 254 | 17260 |
| 2 | 476 | 0.50 | 238 | 540 | 302 | 0 | 302 | 17562 |
| 3 | 476 | 0.50 | 238 | 540 | 302 | 0 | 302 | 17864 |
| 4 | 476 | 0.50 | 238 | 540 | 302 | 0 | 302 | 18165 |
| 5 | 476 | 0.60 | 286 | 540 | 254 | 0 | 254 | 18419 |
| 6 | 476 | 0.80 | 381 | 540 | 159 | 0 | 159 | 18578 |
| 7 | 476 | 1.00 | 476 | 540 | 64 | 0 | 64 | 18642 |
| 8 | 476 | 1.10 | 524 | 540 | 16 | 0 | 16 | 18658 |
| 9 | 476 | 1.25 | 596 | 540 | -56 | 56 | 0 | 18602 |
| 10 | 476 | 1.28 | 610 | 540 | -70 | 70 | 0 | 18533 |
| 11 | 476 | 1.20 | 572 | 540 | -32 | 32 | 0 | 18501 |
| 12 | 476 | 1.18 | 562 | 540 | -22 | 22 | 0 | 18479 |
| 13 (1) | 476 | 1.16 | 553 | 540 | -13 | 13 | 0 | 18466 |
| 14 (2) | 476 | 1.10 | 524 | 540 | 16 | 0 | 16 | 18482 |
| 15 (3) | 476 | 1.00 | 476 | 540 | 64 | 0 | 64 | 18546 |
| 16 (4) | 476 | 1.08 | 515 | 540 | 25 | 0 | 25 | 18571 |
| 17 (5) | 476 | 1.15 | 548 | 540 | -8 | 8 | 0 | 18563 |
| 18 (6) | 476 | 1.30 | 619 | 540 | -79 | 79 | 0 | 18484 |
| 19 (7) | 476 | 4.80 | 2287 | 540 | -1747 | 1747 | 0 | 16737 |
| 20 (8) | 476 | 1.40 | 667 | 540 | -127 | 127 | 0 | 16610 |
| 21 (9) | 476 | 1.25 | 596 | 540 | -56 | 56 | 0 | 16555 |
| 22 (10) | 476 | 0.90 | 429 | 540 | 111 | 0 | 111 | 16666 |
| 23 (11) | 476 | 0.85 | 405 | 540 | 135 | 0 | 135 | 16801 |



U.S. Fire Administration

Water Supply Systems and Evaluation Methods

Volume II: Water Supply Evaluation Methods

October 2008



FEMA



Table 4-1
Water Use and Storage Depletion of Maximum Day in a Typical Municipality

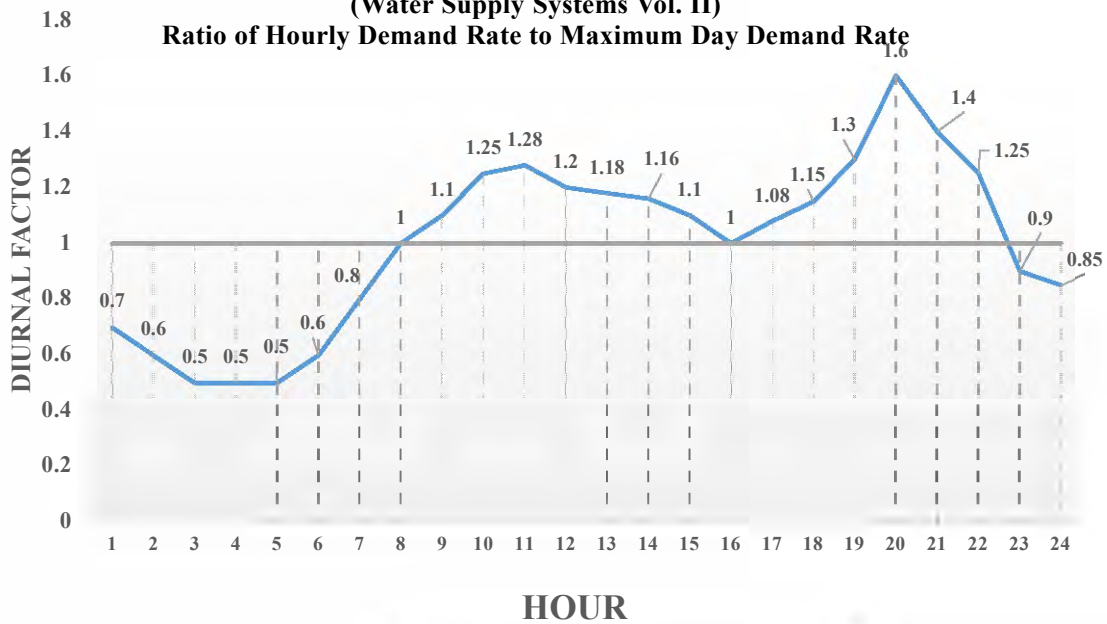
| Hour | Ratio of Hourly Demand Rate To Maximum Day Demand Rate | Hourly Variation in Distribution Storage Reserve <i>mil gal</i> | Cumulative Storage Depletion <i>mil gal</i> |
|-----------|---|--|---|
| 7-8 a.m. | 1.00 | -0.00 | 0.00 |
| 8-9 | 1.10 | -0.10 | 0.10 |
| 9-10 | 1.25 | -0.25 | 0.35 |
| 10-11 | 1.28 | -0.28 | 0.63 |
| 11-12 | 1.20 | -0.20 | 0.83 |
| 12-1 p.m. | 1.18 | -0.18 | 1.01 |
| 1-2 | 1.16 | -0.16 | 1.17 |
| 2-3 | 1.10 | -0.10 | 1.27 |
| 3-4 | 1.00 | -0.00 | 1.27 |
| 4-5 | 1.08 | -0.08 | 1.35 |
| 5-6 | 1.15 | -0.15 | 1.50 |
| 6-7 | 1.30 | -0.30 | 1.80 |
| 7-8 | 1.60 | -0.60 | 2.40 |
| 8-9 | 1.40 | -0.40 | 2.80 |
| 9-10 | 1.25 | -0.25 | 3.05** |
| 10-11 | 0.90 | +0.10 | 2.95 |
| 11-12 | 0.85 | +0.15 | 2.80 |
| 12-1 a.m. | 0.70 | +0.30 | 2.50 |
| 1-2 | 0.60 | +0.40 | 2.10 |
| 2-3 | 0.50 | +0.50 | 1.60 |
| 3-4 | 0.50 | +0.50 | 1.10 |
| 4-5 | 0.50 | +0.50 | 0.60 |
| 5-6 | 0.60 | +0.40 | 0.40 |
| 6-7 | 0.80 | +0.20 | 0.20 |

*Average day, 16 mil gal; maximum day, 25 mil gal; constant hourly supply rate (at maximum day demand rate), 24 mgd or 1 mil gal/h.

**Maximum storage depletion.

Rarely can distribution storage be justified economically in an amount greater than will take care of normal daily variations and provide the needed reserve for fire protection and **minor** emergencies. In systems of moderate size, the amount of water storage available for equalizing water production is 30 to

Diurnal Curve
(Water Supply Systems Vol. II)
Ratio of Hourly Demand Rate to Maximum Day Demand Rate



| Hour | Diurnal Factor | Base Factor |
|------|----------------|-------------|
| 0 | 0.70 | 1 |
| 1 | 0.60 | 1 |
| 2 | 0.50 | 1 |
| 3 | 0.50 | 1 |
| 4 | 0.50 | 1 |
| 5 | 0.60 | 1 |
| 6 | 0.80 | 1 |
| 7 | 1.00 | 1 |
| 8 | 1.10 | 1 |
| 9 | 1.25 | 1 |
| 10 | 1.28 | 1 |
| 11 | 1.20 | 1 |
| 12 | 1.18 | 1 |
| 13 | 1.16 | 1 |
| 14 | 1.10 | 1 |
| 15 | 1.00 | 1 |
| 16 | 1.08 | 1 |
| 17 | 1.15 | 1 |
| 18 | 1.30 | 1 |
| 19 | 1.60 | 1 |
| 20 | 1.40 | 1 |
| 21 | 1.25 | 1 |
| 22 | 0.90 | 1 |
| 23 | 0.85 | 1 |