

PRELIMINARY HYDROLOGY AND HYDRAULICS REPORT FOR THE RIDGE – IDYLLWILD GUEST RANCH

**COUNTY OF RIVERSIDE
CALIFORNIA**

PREPARED FOR:

THE RIDGE
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MAY 26, 2023

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This report has been prepared by or under the direction of the following registered civil engineer who attests to the technical information contained herein. The registered civil engineer has also judged the qualifications of any technical specialists providing engineering data upon which recommendations, conclusions, and decisions are based.



5/26/2023



Joseph L. Castaneda RCE 59835
Registered Civil Engineer

Date

Seal

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I. PURPOSE AND SCOPE

The purpose of this report is to evaluate the drainage patterns and potential runoff that emanates from the project referred to as The Ridge – Idyllwild Guest Ranch. Surface runoff flows from the northern edge of the project site to the southern and western edges of the project site, discharging into Hurkey Creek.

The scope of the study includes the following:

1. Determination of points of flow concentration and watershed subareas for the project site.
2. Determination of the 100-year peak storm flows based on the post-project condition utilizing the Rational Method for the purpose of storm drain sizing.
3. Determination of the 2-year and 10-year peak storm flows based on the pre-project and post-project conditions utilizing the Unit Hydrograph method for the purpose of basin sizing.
4. Assessment of the onsite hydraulics.
5. Assessment of the offsite runoff to the north of Apple Canyon Rd comparing flow rates from the Rational Method and the hydraulic cross sectional capacity calculations.
6. Preparation of a hydrology report, which consist of hydrological and analytical results and exhibits.

II. PROJECT SITE AND DRAINAGE AREA OVERVIEW

The Ridge – Idyllwild Guest Ranch is a proposed wellness center. The project site is bounded by Apple Canyon Road to the north and Highway 74 to the south. Hurkey Creek runs along the western edge of the project site. The project site is located in the County of Riverside, to the northeast of Lake Hemet, see Figure 1 Vicinity Map.

The existing project site area is currently undeveloped with open brush land cover. Hurkey Creek runs adjacent to the western edge of the project site. Runoff from the project site flows from the northern edge to the south and west, discharging into Hurkey Creek. A preliminary study by Albert A. Webb Associates (WEBB) studied the 100-year flood plain of Hurkey Creek, see Excerpt A. The limits of the flood plain are shown on Figure 2 and the project site is proposed such as to minimize the impacts to this existing flood plain.

Apple Canyon Road borders the project site to the north. The existing roadway currently crosses Hurkey Creek. The existing roadway experiences flooding during the 100-year storm event, see Figure 2. WEBB and Associates prepared a hydrology and flood plain assessment for Hurkey Creek which has been included in Excerpt A. The project will not propose improvements that would resolve the Hurkey Creek flooding experienced by Apple Canyon Road. Moreover, the portion of Apple Canyon Road along the project frontage as approximately 5.9 acres of tributary watershed area. The 5.9 acre watershed will be collected by Apple Canyon Road and it will be directed to the east, see Figure 3.

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Hurkey Creek discharges into Lake Hemet. Any overflows from Lake Hemet will discharge into the San Jacinto River which is part of the Santa Ana watershed. As a result, the project will be required to implement the Santa Ana WQMP Guidance Document.

The proposed project collects runoff from the project site using the proposed 24 feet drive, the natural terrain, proposed grade shallow swale areas and proposed storm drain facilities. The runoff emanating from the project will be discharged into water quality self-retaining or proposed bioretention basins. The project has provided one bio-retention basin located on the southern portion of the project site which will be used as a multi-use basin to mitigate water quality and hydromodification impacts. The watershed study area for the project site is approximately 16.5 acres.

III. HYDROLOGY

The Riverside County Hydrology Manual (Reference 1), was used to develop the hydrological parameters for the hydrology analyses. The rational method and unit hydrograph method was used for the analyses and the computations were performed using the computer program developed by Civil Cadd/Civil Design.

Rainfall depths were obtained from NOAA Atlas 14 based on the values used in the WEBB Study. The rainfall depths used in the hydrology calculations for that rational method and unit hydrographs are as follows:

Storm Event & Duration	Rainfall (inches)
2-Year, 1-Hour	0.72
2-Year, 24-Hour	2.79
100-Year, 1-Hour	2.05
100-Year, 24-Hour	7.72
Slope of Intensity	0.55

The existing soil classification for the on-site area consists of Hydrologic Soil Group “A”, as shown in Excerpt B. Excerpt B is a Soils Map obtained from the National Resource Conservation Service Websoil Survey. An Antecedent Moisture Condition of II was used for the 10-year and 100-year calculations, and Antecedent Moisture Condition of I was used for the 2-year calculations.

The rational method outlined in the RCFC&WCD Hydrology Manual and implemented by the CivilDesign Hydrology Program restrict land use parameters to be entered into the program for initial areas due to the empirical equation used to establish the initial time of concentration. As part of the rational method hydrology study, the impervious ratios were calculated for each sub-area identified in the hydrology map for the project site plan. The impervious ratios for the subareas are documented in Table 1. The impervious ratio for the

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subareas are approximately 30%. Due to land use constraints used by for the with initial areas in the rational method program, JLC elected to use Residential ½ Acre Lots to represent the land use characteristics for Subarea A1 through A4. Residential ½ Acre lots has a higher impervious ratio than the actual pervious ratios within the site. Area A5 which has been designated as an open space land use. Table 2 provides the Runoff Index and Impervious Percent for these land uses.

Table 1 – Impervious Ratios for Subareas

Subarea	Actual Impervious Ratio	Land Cover Used in Rational Method	Impervious Ratio Used
A1	0.26	Residential, ½ Ac	0.4
A2	0.33	Residential, ½ Ac	0.4
A3	0.32	(User Input)	0.32
A4	0.32	Residential, ½ Ac	0.4
A5	0.37	Residential, ½ Ac	0.4
A6	0	(User Input)	0
A7	0	(User Input)	0

Table 2 – Runoff Index No. & Impervious Ratio Based on Land Use Cover

Land Use Cover	Runoff Index Number (Soil “A”)	Impervious Ratio
Residential, ½ Acre Lots	32	0.4
Open Brush (Fair)	46	0

Site specific topography was provided by the client and used for elevation values for the rational method analysis and unit hydrograph analyses. Additional topographic mapping was obtained for the regional area using the USGS 1 meter DEM data.

A post-project condition rational method analysis was performed to determine the peak flows for storm drain and inlet sizing purposes. It should be noted, a pre-project condition rational method analysis was not performed for the project. The rational method pre-project condition was not developed for the project site since pre-project peak flow rate were not required to assess any storm drain facilities. However, a pre-project and post-project unit hydrograph analysis were performed to determine mitigation requirements for hydromodifications which will be required to demonstrate that the project conforms with the WQMP Guidance Document.

The rational method analysis analyzed Area A, which consisted of 7 sub-boundary areas in the post-project condition. The analysis resulted in a time of concentration of 19.4 minutes and peak flow rate of 39.9 ft³/s for a 100-year storm.

The unit hydrograph analyses for the pre-project and post-project conditions utilized the 13.79 acre watershed boundary from the rational method analysis minus the pervious basin

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area that is held constant for both calculations. An overall impervious fraction of 0.27 was calculated for the post-project condition based on the site plans. However, an impervious ratio of 0.35 was used as calculated from the rational method analysis. And impervious fraction of 0 was used for the pre-project condition.

Table 3 provides the impervious percentage used in the unit hydrograph analysis and the true impervious fraction of the project site. The impervious fraction used for the unit hydrograph analyses was calculated from the rational method analysis. This number is more conservative in quantifying the impervious fraction of the project site for reasons discussed earlier such as the land cover estimation shown in Table 1. Since the impervious ratio used for the analyses is larger than the actual ratio, the mitigation volume tabulated from the unit hydrograph analyses will be larger and result in a more conservative estimation. Table 4 provides the Runoff Index.

Table 3 – Impervious Ratio: Actual vs Used in the Unit Hydrograph Analysis

Area A	Actual Impervious Ratio	Impervious Ratio Used
Pre-Project	0	0
Post-Project	0.27	0.35

Table 4 – Runoff Index No. & Impervious Ratio for Unit Hydrograph Analysis

Area A	Runoff Index Number (Soil “A”)	Impervious Ratio
Pre-Project	46.0	0
Post-Project	33.6	0.35

Unit hydrographs were run for the 2-year 24-hour storm and 10-year 24-hour storm, the results of which are shown below:

Area A	Area (ac)	2-Year, 24-Hour, Q (CFS)	10-Year, 24-Hour Q (CFS)
Pre-Project	12.23	0.5	1.4
Post-Project	12.23	1.8	3.0
Delta	0	+1.3	+1.6

The results show an increase in peak flow rate for both storm events. This increase is due to the increase in impervious surfaces within the project site. To mitigate this increase, flows are routed to a basin which is sized based on the following values taken out of the unit hydrograph analysis.

From the 2-year, 24-hour post-project unit hydrograph, the following values were taken:

Time (hours+minutes)	Volume (acre-feet)	Flow rate (ft ³ /s)
6+40	0.1	0.5
16+10	1.0	0.5

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Before time 6+40 and after time 16+10 the flow rate is below 0.5, which is the peak flow rate from the pre-project condition. The volume difference between these two times is 0.9 acre-feet ($1.0-0.1=0.9$).

From the 10-year, 24-hour post-project unit hydrograph, the following values were taken:

Time (hours+minutes)	Volume (acre-feet)	Flow rate (ft ³ /s)
8+40	0.4	1.4
16+05	1.7	1.4

Before time 8+40 and after time 16+05 the flow rate is below 1.4, which is the peak flow rate from the pre-project condition. The volume difference between these two times is 1.3 acre-feet ($1.7-0.4=1.3$).

Basin 1 has been designed to store 3.1 acre-ft of mitigation runoff which exceeds the 1.3 acre-ft needed to mitigate the flow rate for the 10-year, 24-hour storm. This ensures that the flow rates will not exceed the pre-project flow rate of 1.4 CFS for the 10-year, 24-hour storm.

The post-project rational method hydrology calculations have been included in Appendix A, and the post-project rational method hydrology map has been included as Exhibit A. The pre-project unit hydrograph calculations haven been included in Appendix B, and the pre-project unit hydrograph map has been included as Exhibit B. The post-project unit hydrograph calculations have been included in Appendix C, and the post-project unit hydrograph map has been included as Exhibit C. The basin volume calculations are included in Appendix D.

IV. HYDRAULIC ASSESSMENT

The project site is impacted by the Hurkey Creek flood plain, see Figure 2, which was studied by WEBB and Associates. The flood plain assessment performed by WEBB has been included in Excerpt A. The project site created a development that would be located outside the 100-year flood plain limits as defined by WEBB and shown in Figure 2. The study performed by WEBB indicates that the Hurkey Creek watershed area is approximately 7,000 acres and has a peak flow rate of 6,074 ft³/s. As part of this assessment, JLC reviewed the county of Riverside increased runoff criteria and ordinance number 458. Based upon previous projects processed through the county of Riverside, RCFC&WCD has exempted the increased runoff criteria when projects discharge into an adequate outlet. It is JLC's opinion that Hurkey Creek is an adequate outlet due to the magnitude of the watershed area and peak flow rate. Therefore, the county runoff criteria were not applied.

An offsite flow rate analysis was done to ensure runoff from the north of Apple Canyon Road did not overflow into the project site. The flow rate from node 108 to 109 of the Offsite

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Hydrology Map (Exhibit E), is 5.9-12.8 CFS. This flow runs parallel to the project site on the opposing side of Apple Canyon Road. To ensure that this flow did not cross the road into the project site, hydraulic cross section calculations were done. The hydraulic flow rate calculations of the cross sections detail the maximum capacity flow rate that the given terrain of the cross section can contain. This flow rate calculation is independent of the amount of rainfall because it strictly details the capacity of flow that the terrain can contain without the runoff from overflowing. Separately, the rational method flow rate is calculated for the 100 year peak rain fall storm. Based off this analysis, the offsite flow will not flow into the project site when the hydraulic capacity is larger than the rational method flow rate. Sections A, B, C, and D shown on Exhibit F detail the rational method flow rate as $Q_{HYD} = 5.9-12.8$ CFS. The hydraulic capacity flow rate for each cross section is defined as Q_{CAP} . Along sections A through D the hydraulic capacity flow rate is larger than the rational method flow rate. From this analysis the terrain on the northern side of Apple Canyon Road can sufficiently contain offsite flows, preventing runoff from crossing into the project site. For flows traveling from node 106 to 109, cross section J provides a $Q_{CAP} = 235.3$ CFS. The $Q_{HYD} = 178.5-221.4$ CFS for this flow path and does not exceed the hydraulic capacity flow rate for this cross section as shown on Exhibit F. The flow path can be seen to travel along the contours to node 109, to the northeast of the project site. It is JLC's opinion that according to the hydraulic cross section analysis, offsite flow will not cross into the project site and will be tributary to the stream A creek, west along Apple Canyon Road.

The offsite rational method hydrology map has been included as Exhibit E. The offsite rational method hydrology calculations have been included in Appendix E. An exhibit of the offsite flow rate analysis with cross sections has been included as Exhibit F. The cross section calculations have been included in Appendix F.

The project site proposed a drive aisle with an AC dike along the interior edge to collect runoff from the project site. Additionally, local storm drain facilities are proposed to collect and route runoff within the project site. A drainage facilities map has been included as Exhibit D. The drainage facilities have been proposed in order to flood protect the project and meet Riverside county ordinate number 458.

Bioretention Basin 1 is proposed on the project site for the purposes of water quality. Additionally, Basin 1 is sized as a detention basin to mitigate hydromodification and the 10-year, 24-hour storm event from increase in flows emanating from the project site. Basin 1, located in the southern portion of the project site is designed to store 3.1 acre-feet of mitigation runoff, which exceeds the required volume of 1.3 acre-feet to provide mitigation based on the 10-year, 24-hour unit hydrograph analysis. The basin volume calculations are included in Appendix D.

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V. FINDINGS

The hydrology analysis and hydraulic assessment evaluated the proposed development to determine the potential impact from the runoff emanating from the project site. It has been concluded that:

1. The proposed project created a grading and development strategy that avoids the Hurkey Creek flood plain which has a drainage area of approximately 7,000 acres and a flow rate of 6,000 ft³/s for the 100-year, 6-hour storm, see Figure 2 and Excerpt A. It is JLC's opinion that Hurkey Creek should be considered an adequate outlet due to the magnitude of the Hurkey Creek watershed in comparison to the 13.79 acres of the developed condition watershed.
2. The existing land cover is undeveloped open brush and is completely pervious. In comparison, the proposed project site will have a post-project impervious ratio of 27% and landscaped areas. This resulted in an increase in flow rate and shall be mitigated to meet the hydromodification criteria as part of the Santa Ana WQMP Guidance Document.
3. The hydromodification criteria requires project to mitigate runoff for the 10-year, 24-hour storm event. The peak flow rate increased from 1.4 to 3.0 ft³/s for the 10-year, 24-hour storm event. The required storage volume to mitigate the increased runoff is 1.3 acre-feet. The project proposes Basin 1 which provides 3.1 acre-feet of mitigation storage to mitigate the increased runoff.
4. The proposed mitigation strategy will sufficiently mitigate the increase in runoff as required per the hydromodification criteria.
5. A separate document defined as a Water Quality Management Plan was prepared to determine the water quality features and BMPs required for the project. The proposed basins identified as part of this project are sufficient to mitigate water quality runoff from the project site.
6. A drainage facilities map, Exhibit D, has been prepared to identify the storm drain systems required to collect runoff from the project site. The storm drain facilities identified in Exhibit D will provide the required flood protection, which will meet county ordinance 458.
7. Offsite flows to the north of Apple Canyon Rd will not impact the project site and are tributary to Stream A to the east of the project site.

VI. REFERENCES

1. Riverside County Flood Control and Water Conservation District Hydrology Manual, April 1978.
2. County of Riverside Ordinance 458
3. Santa Ana WQMP Guidance Document for Riverside County
4. Technical Memorandum – “Preliminary Hydrology & Floodplain Analysis for the Ridge, August 2020

FIGURES

FIGURE 1: VICINITY MAP

Drawing Name: O:\302.01.21\Engineering\Hydrology_Plan\Exhibits\FIGURE-1-VICINITY-MAP.dwg
Last Opened: Jul 16, 2021 - 11:16am by stanner



VICINITY MAP

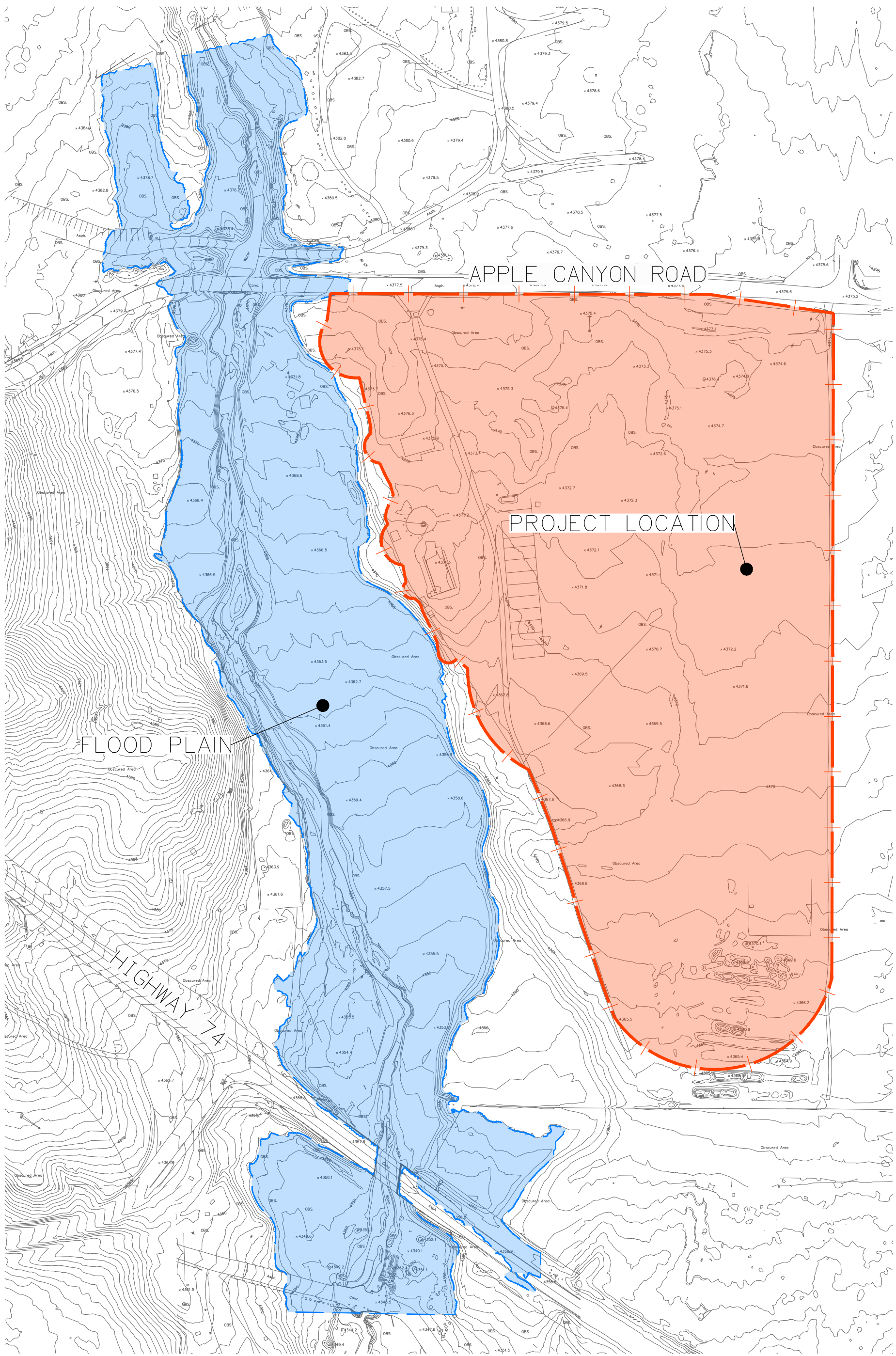


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FIGURE 1

FIGURE 2: FLOOD PLAIN MAP

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Last Opened: Jul 16, 2021 - 2:40pm by stanner



FLOOD PLAIN MAP

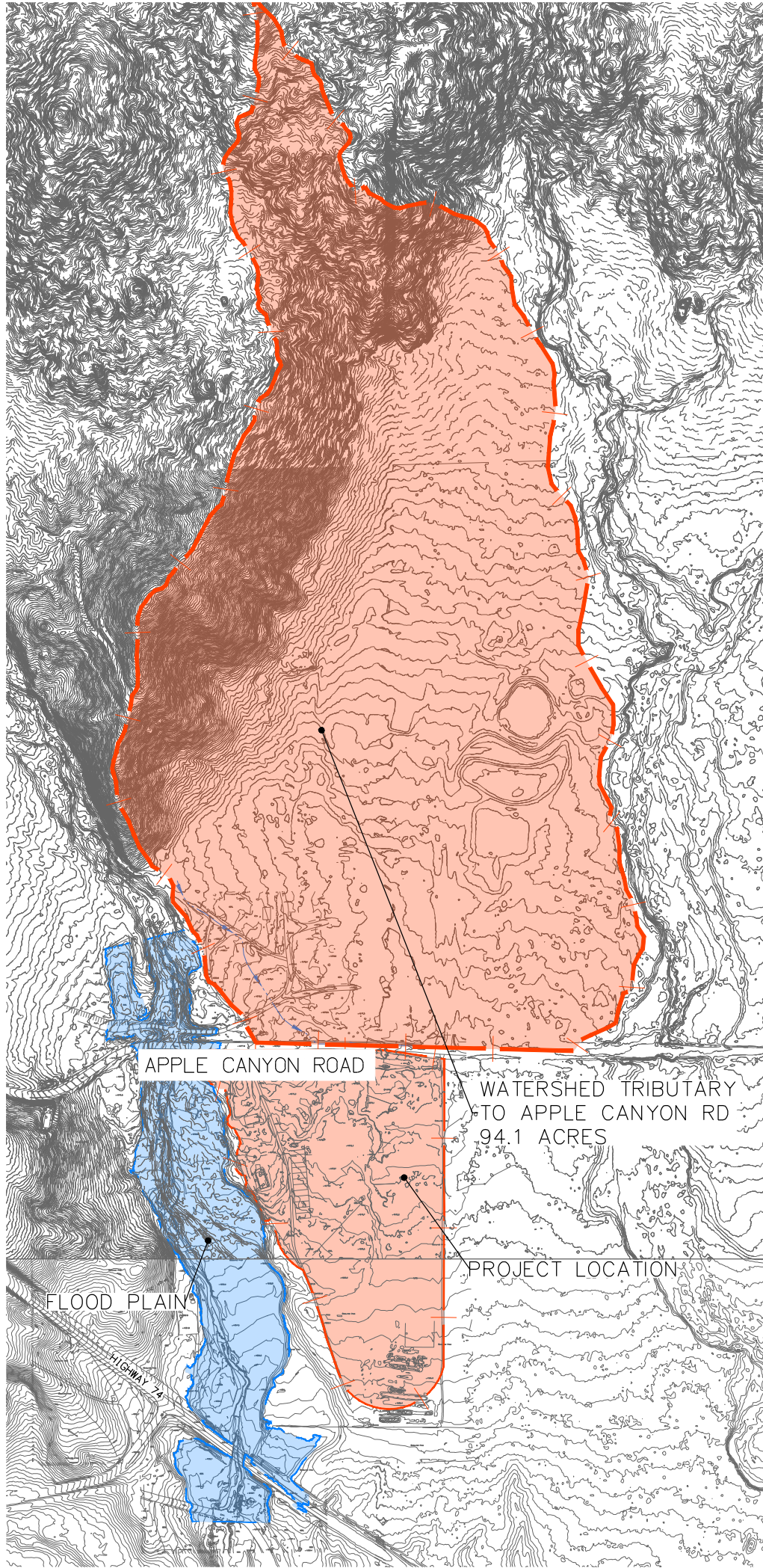


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FIGURE 2

FIGURE 3: WATERSHED TRIBUTARY TO APPLE CANYON ROAD

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Last Opened: Nov 18, 2022 - 3:16pm by mhunt



WATERSHED TRIBUTARY TO APPLE CANYON RD



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FIGURE 3

APPENDICES

APPENDIX A: POST-PROJECT CONDITION RATIONAL METHOD HYDROLOGY

APPENDIX A.1: RATIONAL METHOD 100 YR ANALYSIS, AREAS “A”

Riverside County Rational Hydrology Program

CIVILCADD/CIVILDESIGN Engineering Software, (c) 1989 - 2014 Version 9.0
Rational Hydrology Study Date: 05/23/23 File:ARA100.out

302.01.21
AREA A
100 YR STORM
POST PROJECT

***** Hydrology Study Control Information *****

English (in-lb) Units used in input data file

Program License Serial Number 6279

Rational Method Hydrology Program based on
Riverside County Flood Control & Water Conservation District
1978 hydrology manual

Storm event (year) = 100.00 Antecedent Moisture Condition = 2

2 year, 1 hour precipitation = 0.720(In.)
100 year, 1 hour precipitation = 2.100(In.)

Storm event year = 100.0
Calculated rainfall intensity data:
1 hour intensity = 2.100(In/Hr)
Slope of intensity duration curve = 0.5500

+++++
Process from Point/Station 101.000 to Point/Station 102.000
**** INITIAL AREA EVALUATION ****

Initial area flow distance = 695.000(Ft.)
Top (of initial area) elevation = 78.000(Ft.)
Bottom (of initial area) elevation = 70.300(Ft.)
Difference in elevation = 7.700(Ft.)
Slope = 0.01108 s(percent)= 1.11
TC = $k(0.420)*[(\text{length}^3)/(\text{elevation change})]^{0.2}$
Initial area time of concentration = 14.163 min.
Rainfall intensity = 4.646(In/Hr) for a 100.0 year storm
SINGLE FAMILY (1/2 Acre Lot)
Runoff Coefficient = 0.695
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil(AMC 2) = 32.00
Pervious area fraction = 0.600; Impervious fraction = 0.400
Initial subarea runoff = 14.475(CFS)

Total initial stream area = 4.480 (Ac.)
Pervious area fraction = 0.600

++++
Process from Point/Station 102.000 to Point/Station 103.000
**** IMPROVED CHANNEL TRAVEL TIME ****

Upstream point elevation = 70.300 (Ft.)
Downstream point elevation = 68.300 (Ft.)
Channel length thru subarea = 229.000 (Ft.)
Channel base width = 1.000 (Ft.)
Slope or 'Z' of left channel bank = 2.000
Slope or 'Z' of right channel bank = 2.000
Estimated mean flow rate at midpoint of channel = 15.427 (CFS)
Manning's 'N' = 0.015
Maximum depth of channel = 1.000 (Ft.)
Flow (q) thru subarea = 15.427 (CFS)
Depth of flow = 0.919 (Ft.), Average velocity = 5.913 (Ft/s)
Channel flow top width = 4.677 (Ft.)
Flow Velocity = 5.91 (Ft/s)
Travel time = 0.65 min.
Time of concentration = 14.81 min.

Sub-Channel No. 1 Critical depth = 1.063 (Ft.)
' ' ' Critical flow top width = 5.000 (Ft.)
' ' ' Critical flow velocity = 4.657 (Ft/s)
' ' ' Critical flow area = 3.313 (Sq.Ft)

Adding area flow to channel
USER INPUT of soil data for subarea
Runoff Coefficient = 0.665
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil (AMC 2) = 32.00
Pervious area fraction = 0.680; Impervious fraction = 0.320
Rainfall intensity = 4.533 (In/Hr) for a 100.0 year storm
Subarea runoff = 1.838 (CFS) for 0.610 (Ac.)
Total runoff = 16.313 (CFS) Total area = 5.090 (Ac.)
Depth of flow = 0.943 (Ft.), Average velocity = 5.998 (Ft/s)

Sub-Channel No. 1 Critical depth = 1.094 (Ft.)
' ' ' Critical flow top width = 5.000 (Ft.)
' ' ' Critical flow velocity = 4.703 (Ft/s)
' ' ' Critical flow area = 3.469 (Sq.Ft)

++++
Process from Point/Station 103.000 to Point/Station 104.000
**** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 67.200 (Ft.)
Downstream point/station elevation = 67.000 (Ft.)
Pipe length = 130.00 (Ft.) Manning's N = 0.013
No. of pipes = 1 Required pipe flow = 16.313 (CFS)
Nearest computed pipe diameter = 30.00 (In.)
Calculated individual pipe flow = 16.313 (CFS)

Normal flow depth in pipe = 24.98(In.)
Flow top width inside pipe = 22.39(In.)
Critical Depth = 16.38(In.)
Pipe flow velocity = 3.73(Ft/s)
Travel time through pipe = 0.58 min.
Time of concentration (TC) = 15.39 min.

++++
Process from Point/Station 104.000 to Point/Station 104.000
**** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 1
Stream flow area = 5.090(Ac.)
Runoff from this stream = 16.313(CFS)
Time of concentration = 15.39 min.
Rainfall intensity = 4.439(In/Hr)
Program is now starting with Main Stream No. 2

++++
Process from Point/Station 105.000 to Point/Station 106.000
**** INITIAL AREA EVALUATION ****

Initial area flow distance = 859.000(Ft.)
Top (of initial area) elevation = 78.000(Ft.)
Bottom (of initial area) elevation = 70.300(Ft.)
Difference in elevation = 7.700(Ft.)
Slope = 0.00896 s(percent)= 0.90
TC = $k(0.420)*[(length^3)/(elevation\ change)]^{0.2}$
Initial area time of concentration = 16.082 min.
Rainfall intensity = 4.332(In/Hr) for a 100.0 year storm
SINGLE FAMILY (1/2 Acre Lot)
Runoff Coefficient = 0.686
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil(AMC 2) = 32.00
Pervious area fraction = 0.600; Impervious fraction = 0.400
Initial subarea runoff = 14.216(CFS)
Total initial stream area = 4.780(Ac.)
Pervious area fraction = 0.600

++++
Process from Point/Station 106.000 to Point/Station 109.000
**** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 69.000(Ft.)
Downstream point/station elevation = 67.600(Ft.)
Pipe length = 460.00(Ft.) Manning's N = 0.013
No. of pipes = 1 Required pipe flow = 14.216(CFS)
Nearest computed pipe diameter = 27.00(In.)
Calculated individual pipe flow = 14.216(CFS)
Normal flow depth in pipe = 18.80(In.)
Flow top width inside pipe = 24.83(In.)
Critical Depth = 15.74(In.)
Pipe flow velocity = 4.81(Ft/s)

Travel time through pipe = 1.60 min.
Time of concentration (TC) = 17.68 min.

++++
Process from Point/Station 109.000 to Point/Station 109.000
**** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 1
Stream flow area = 4.780 (Ac.)
Runoff from this stream = 14.216 (CFS)
Time of concentration = 17.68 min.
Rainfall intensity = 4.113 (In/Hr)

++++
Process from Point/Station 107.000 to Point/Station 108.000
**** INITIAL AREA EVALUATION ****

Initial area flow distance = 573.000 (Ft.)
Top (of initial area) elevation = 75.700 (Ft.)
Bottom (of initial area) elevation = 69.500 (Ft.)
Difference in elevation = 6.200 (Ft.)
Slope = 0.01082 s(percent) = 1.08
TC = $k(0.420) * [(length^3) / (elevation\ change)]^{0.2}$
Initial area time of concentration = 13.172 min.
Rainfall intensity = 4.835 (In/Hr) for a 100.0 year storm
SINGLE FAMILY (1/2 Acre Lot)
Runoff Coefficient = 0.700
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil (AMC 2) = 32.00
Pervious area fraction = 0.600; Impervious fraction = 0.400
Initial subarea runoff = 5.453 (CFS)
Total initial stream area = 1.610 (Ac.)
Pervious area fraction = 0.600

++++
Process from Point/Station 108.000 to Point/Station 109.000
**** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 68.500 (Ft.)
Downstream point/station elevation = 67.600 (Ft.)
Pipe length = 252.00 (Ft.) Manning's N = 0.013
No. of pipes = 1 Required pipe flow = 5.453 (CFS)
Nearest computed pipe diameter = 18.00 (In.)
Calculated individual pipe flow = 5.453 (CFS)
Normal flow depth in pipe = 12.96 (In.)
Flow top width inside pipe = 16.16 (In.)
Critical Depth = 10.80 (In.)
Pipe flow velocity = 4.00 (Ft/s)
Travel time through pipe = 1.05 min.
Time of concentration (TC) = 14.22 min.

++++
Process from Point/Station 108.000 to Point/Station 109.000

**** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 2
 Stream flow area = 1.610 (Ac.)
 Runoff from this stream = 5.453 (CFS)
 Time of concentration = 14.22 min.
 Rainfall intensity = 4.635 (In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
------------	-----------------	----------	----------------------------

1	14.216	17.68	4.113
2	5.453	14.22	4.635

Largest stream flow has longer time of concentration

Qp = 14.216 + sum of
 Qb Ia/Ib
 5.453 * 0.887 = 4.838
 Qp = 19.054

Total of 2 streams to confluence:
 Flow rates before confluence point:
 14.216 5.453
 Area of streams before confluence:
 4.780 1.610

Results of confluence:
 Total flow rate = 19.054 (CFS)
 Time of concentration = 17.677 min.
 Effective stream area after confluence = 6.390 (Ac.)

++++
 Process from Point/Station 109.000 to Point/Station 111.000
 **** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 67.600 (Ft.)
 Downstream point/station elevation = 67.200 (Ft.)
 Pipe length = 87.00 (Ft.) Manning's N = 0.013
 No. of pipes = 1 Required pipe flow = 19.054 (CFS)
 Nearest computed pipe diameter = 27.00 (In.)
 Calculated individual pipe flow = 19.054 (CFS)
 Normal flow depth in pipe = 20.16 (In.)
 Flow top width inside pipe = 23.49 (In.)
 Critical Depth = 18.33 (In.)
 Pipe flow velocity = 5.98 (Ft/s)
 Travel time through pipe = 0.24 min.
 Time of concentration (TC) = 17.92 min.

++++
 Process from Point/Station 111.000 to Point/Station 111.000
 **** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 1
 Stream flow area = 6.390 (Ac.)
 Runoff from this stream = 19.054 (CFS)
 Time of concentration = 17.92 min.
 Rainfall intensity = 4.082 (In/Hr)

+++++
 Process from Point/Station 110.000 to Point/Station 111.000
 ***** INITIAL AREA EVALUATION *****

Initial area flow distance = 338.000 (Ft.)
 Top (of initial area) elevation = 70.200 (Ft.)
 Bottom (of initial area) elevation = 68.300 (Ft.)
 Difference in elevation = 1.900 (Ft.)
 Slope = 0.00562 s(percent) = 0.56
 $TC = k(0.420) * [(length^3) / (elevation\ change)]^{0.2}$
 Initial area time of concentration = 12.158 min.
 Rainfall intensity = 5.053 (In/Hr) for a 100.0 year storm
 SINGLE FAMILY (1/2 Acre Lot)
 Runoff Coefficient = 0.706
 Decimal fraction soil group A = 1.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 0.000
 Decimal fraction soil group D = 0.000
 RI index for soil (AMC 2) = 32.00
 Pervious area fraction = 0.600; Impervious fraction = 0.400
 Initial subarea runoff = 2.640 (CFS)
 Total initial stream area = 0.740 (Ac.)
 Pervious area fraction = 0.600

+++++
 Process from Point/Station 110.000 to Point/Station 111.000
 ***** CONFLUENCE OF MINOR STREAMS *****

Along Main Stream number: 2 in normal stream number 2
 Stream flow area = 0.740 (Ac.)
 Runoff from this stream = 2.640 (CFS)
 Time of concentration = 12.16 min.
 Rainfall intensity = 5.053 (In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
------------	-----------------	----------	----------------------------

1	19.054	17.92	4.082
2	2.640	12.16	5.053

Largest stream flow has longer time of concentration

Qp = 19.054 + sum of
 $Qb \cdot \frac{Ia}{Ib}$
 $2.640 * 0.808 = 2.133$
 Qp = 21.186

Total of 2 streams to confluence:
 Flow rates before confluence point:
 19.054 2.640

Area of streams before confluence:
 6.390 0.740

Results of confluence:
 Total flow rate = 21.186 (CFS)
 Time of concentration = 17.920 min.
 Effective stream area after confluence = 7.130 (Ac.)

+++++
 Process from Point/Station 111.000 to Point/Station 104.000
 **** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 67.200 (Ft.)
 Downstream point/station elevation = 67.000 (Ft.)
 Pipe length = 130.00 (Ft.) Manning's N = 0.013
 No. of pipes = 1 Required pipe flow = 21.186 (CFS)
 Nearest computed pipe diameter = 33.00 (In.)
 Calculated individual pipe flow = 21.186 (CFS)
 Normal flow depth in pipe = 27.75 (In.)
 Flow top width inside pipe = 24.14 (In.)
 Critical Depth = 18.23 (In.)
 Pipe flow velocity = 3.98 (Ft/s)
 Travel time through pipe = 0.54 min.
 Time of concentration (TC) = 18.46 min.

+++++
 Process from Point/Station 111.000 to Point/Station 104.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 2
 Stream flow area = 7.130 (Ac.)
 Runoff from this stream = 21.186 (CFS)
 Time of concentration = 18.46 min.
 Rainfall intensity = 4.015 (In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
------------	-----------------	----------	----------------------------

1	16.313	15.39	4.439
2	21.186	18.46	4.015

Largest stream flow has longer time of concentration

Qp = 21.186 + sum of
 Qb Ia/Ib
 16.313 * 0.905 = 14.757
 Qp = 35.943

Total of 2 main streams to confluence:

Flow rates before confluence point:

16.313 21.186

Area of streams before confluence:

5.090 7.130

Results of confluence:

Total flow rate = 35.943 (CFS)

Time of concentration = 18.465 min.

Effective stream area after confluence = 12.220 (Ac.)

+++++
 Process from Point/Station 104.000 to Point/Station 112.000
 **** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 67.000 (Ft.)

Downstream point/station elevation = 61.000 (Ft.)
Pipe length = 262.00 (Ft.) Manning's N = 0.013
No. of pipes = 1 Required pipe flow = 35.943 (CFS)
Nearest computed pipe diameter = 24.00 (In.)
Calculated individual pipe flow = 35.943 (CFS)
Normal flow depth in pipe = 21.00 (In.)
Flow top width inside pipe = 15.87 (In.)
Critical depth could not be calculated.
Pipe flow velocity = 12.34 (Ft/s)
Travel time through pipe = 0.35 min.
Time of concentration (TC) = 18.82 min.

++++
Process from Point/Station 112.000 to Point/Station 112.000
**** SUBAREA FLOW ADDITION ****

USER INPUT of soil data for subarea
Runoff Coefficient = 0.646
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil (AMC 2) = 46.00
Pervious area fraction = 1.000; Impervious fraction = 0.000
Time of concentration = 18.82 min.
Rainfall intensity = 3.974 (In/Hr) for a 100.0 year storm
Subarea runoff = 1.745 (CFS) for 0.680 (Ac.)
Total runoff = 37.687 (CFS) Total area = 12.900 (Ac.)

++++
Process from Point/Station 112.000 to Point/Station 113.000
**** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 61.000 (Ft.)
Downstream point/station elevation = 58.000 (Ft.)
Pipe length = 167.00 (Ft.) Manning's N = 0.013
No. of pipes = 1 Required pipe flow = 37.687 (CFS)
Nearest computed pipe diameter = 27.00 (In.)
Calculated individual pipe flow = 37.687 (CFS)
Normal flow depth in pipe = 20.18 (In.)
Flow top width inside pipe = 23.46 (In.)
Critical Depth = 24.70 (In.)
Pipe flow velocity = 11.83 (Ft/s)
Travel time through pipe = 0.24 min.
Time of concentration (TC) = 19.05 min.
End of computations, total study area = 12.90 (Ac.)
The following figures may
be used for a unit hydrograph study of the same area.

Area averaged pervious area fraction (Ap) = 0.625
Area averaged RI index number = 32.7

APPENDIX A.2: RATIONAL METHOD 10 YR ANALYSIS, AREAS “A”

Riverside County Rational Hydrology Program

CIVILCADD/CIVILDESIGN Engineering Software, (c) 1989 - 2014 Version 9.0
Rational Hydrology Study Date: 05/23/23 File:ARA10.out

302.01.21
AREA A
10 YR STORM
POST PROJECT

***** Hydrology Study Control Information *****

English (in-lb) Units used in input data file

Program License Serial Number 6279

Rational Method Hydrology Program based on
Riverside County Flood Control & Water Conservation District
1978 hydrology manual

Storm event (year) = 10.00 Antecedent Moisture Condition = 2

2 year, 1 hour precipitation = 0.720(In.)
100 year, 1 hour precipitation = 2.100(In.)

Storm event year = 10.0
Calculated rainfall intensity data:
1 hour intensity = 1.288(In/Hr)
Slope of intensity duration curve = 0.5500

+++++
Process from Point/Station 101.000 to Point/Station 102.000
**** INITIAL AREA EVALUATION ****

Initial area flow distance = 695.000(Ft.)
Top (of initial area) elevation = 78.000(Ft.)
Bottom (of initial area) elevation = 70.300(Ft.)
Difference in elevation = 7.700(Ft.)
Slope = 0.01108 s(percent)= 1.11
TC = $k(0.420)*[(\text{length}^3)/(\text{elevation change})]^{0.2}$
Initial area time of concentration = 14.163 min.
Rainfall intensity = 2.849(In/Hr) for a 10.0 year storm
SINGLE FAMILY (1/2 Acre Lot)
Runoff Coefficient = 0.631
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil(AMC 2) = 32.00
Pervious area fraction = 0.600; Impervious fraction = 0.400
Initial subarea runoff = 8.050(CFS)

Total initial stream area = 4.480 (Ac.)
Pervious area fraction = 0.600

++++
Process from Point/Station 102.000 to Point/Station 103.000
**** IMPROVED CHANNEL TRAVEL TIME ****

Upstream point elevation = 70.300 (Ft.)
Downstream point elevation = 68.300 (Ft.)
Channel length thru subarea = 229.000 (Ft.)
Channel base width = 1.000 (Ft.)
Slope or 'Z' of left channel bank = 2.000
Slope or 'Z' of right channel bank = 2.000
Estimated mean flow rate at midpoint of channel = 8.598 (CFS)
Manning's 'N' = 0.015
Maximum depth of channel = 1.000 (Ft.)
Flow(q) thru subarea = 8.598 (CFS)
Depth of flow = 0.702 (Ft.), Average velocity = 5.092 (Ft/s)
Channel flow top width = 3.809 (Ft.)
Flow Velocity = 5.09 (Ft/s)
Travel time = 0.75 min.
Time of concentration = 14.91 min.

Sub-Channel No. 1 Critical depth = 0.813 (Ft.)
' ' ' Critical flow top width = 4.250 (Ft.)
' ' ' Critical flow velocity = 4.031 (Ft/s)
' ' ' Critical flow area = 2.133 (Sq.Ft)

Adding area flow to channel
USER INPUT of soil data for subarea
Runoff Coefficient = 0.591
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil (AMC 2) = 32.00
Pervious area fraction = 0.680; Impervious fraction = 0.320
Rainfall intensity = 2.769 (In/Hr) for a 10.0 year storm
Subarea runoff = 0.998 (CFS) for 0.610 (Ac.)
Total runoff = 9.048 (CFS) Total area = 5.090 (Ac.)
Depth of flow = 0.719 (Ft.), Average velocity = 5.159 (Ft/s)

Sub-Channel No. 1 Critical depth = 0.836 (Ft.)
' ' ' Critical flow top width = 4.344 (Ft.)
' ' ' Critical flow velocity = 4.051 (Ft/s)
' ' ' Critical flow area = 2.234 (Sq.Ft)

++++
Process from Point/Station 103.000 to Point/Station 104.000
**** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 67.200 (Ft.)
Downstream point/station elevation = 67.000 (Ft.)
Pipe length = 130.00 (Ft.) Manning's N = 0.013
No. of pipes = 1 Required pipe flow = 9.048 (CFS)
Nearest computed pipe diameter = 24.00 (In.)
Calculated individual pipe flow = 9.048 (CFS)

Normal flow depth in pipe = 20.11(In.)
Flow top width inside pipe = 17.69(In.)
Critical Depth = 12.88(In.)
Pipe flow velocity = 3.22(Ft/s)
Travel time through pipe = 0.67 min.
Time of concentration (TC) = 15.59 min.

++++
Process from Point/Station 104.000 to Point/Station 104.000
**** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 1
Stream flow area = 5.090(Ac.)
Runoff from this stream = 9.048(CFS)
Time of concentration = 15.59 min.
Rainfall intensity = 2.703(In/Hr)
Program is now starting with Main Stream No. 2

++++
Process from Point/Station 105.000 to Point/Station 106.000
**** INITIAL AREA EVALUATION ****

Initial area flow distance = 859.000(Ft.)
Top (of initial area) elevation = 78.000(Ft.)
Bottom (of initial area) elevation = 70.300(Ft.)
Difference in elevation = 7.700(Ft.)
Slope = 0.00896 s(percent)= 0.90
TC = $k(0.420)*[(length^3)/(elevation\ change)]^{0.2}$
Initial area time of concentration = 16.082 min.
Rainfall intensity = 2.657(In/Hr) for a 10.0 year storm
SINGLE FAMILY (1/2 Acre Lot)
Runoff Coefficient = 0.621
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil(AMC 2) = 32.00
Pervious area fraction = 0.600; Impervious fraction = 0.400
Initial subarea runoff = 7.890(CFS)
Total initial stream area = 4.780(Ac.)
Pervious area fraction = 0.600

++++
Process from Point/Station 106.000 to Point/Station 109.000
**** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 69.000(Ft.)
Downstream point/station elevation = 67.600(Ft.)
Pipe length = 460.00(Ft.) Manning's N = 0.013
No. of pipes = 1 Required pipe flow = 7.890(CFS)
Nearest computed pipe diameter = 21.00(In.)
Calculated individual pipe flow = 7.890(CFS)
Normal flow depth in pipe = 15.61(In.)
Flow top width inside pipe = 18.35(In.)
Critical Depth = 12.49(In.)
Pipe flow velocity = 4.11(Ft/s)

Travel time through pipe = 1.86 min.
Time of concentration (TC) = 17.95 min.

++++
Process from Point/Station 109.000 to Point/Station 109.000
**** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 1
Stream flow area = 4.780 (Ac.)
Runoff from this stream = 7.890 (CFS)
Time of concentration = 17.95 min.
Rainfall intensity = 2.501 (In/Hr)

++++
Process from Point/Station 107.000 to Point/Station 108.000
**** INITIAL AREA EVALUATION ****

Initial area flow distance = 573.000 (Ft.)
Top (of initial area) elevation = 75.700 (Ft.)
Bottom (of initial area) elevation = 69.500 (Ft.)
Difference in elevation = 6.200 (Ft.)
Slope = 0.01082 s(percent) = 1.08
TC = $k(0.420) * [(length^3) / (elevation\ change)]^{0.2}$
Initial area time of concentration = 13.172 min.
Rainfall intensity = 2.965 (In/Hr) for a 10.0 year storm
SINGLE FAMILY (1/2 Acre Lot)
Runoff Coefficient = 0.636
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil (AMC 2) = 32.00
Pervious area fraction = 0.600; Impervious fraction = 0.400
Initial subarea runoff = 3.036 (CFS)
Total initial stream area = 1.610 (Ac.)
Pervious area fraction = 0.600

++++
Process from Point/Station 108.000 to Point/Station 109.000
**** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 68.500 (Ft.)
Downstream point/station elevation = 67.600 (Ft.)
Pipe length = 252.00 (Ft.) Manning's N = 0.013
No. of pipes = 1 Required pipe flow = 3.036 (CFS)
Nearest computed pipe diameter = 15.00 (In.)
Calculated individual pipe flow = 3.036 (CFS)
Normal flow depth in pipe = 10.03 (In.)
Flow top width inside pipe = 14.12 (In.)
Critical Depth = 8.40 (In.)
Pipe flow velocity = 3.48 (Ft/s)
Travel time through pipe = 1.21 min.
Time of concentration (TC) = 14.38 min.

++++
Process from Point/Station 108.000 to Point/Station 109.000

**** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 2

Stream flow area = 1.610 (Ac.)
 Runoff from this stream = 3.036 (CFS)
 Time of concentration = 14.38 min.
 Rainfall intensity = 2.825 (In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
------------	-----------------	----------	----------------------------

1	7.890	17.95	2.501
2	3.036	14.38	2.825

Largest stream flow has longer time of concentration

Qp = 7.890 + sum of
 $Q_b \cdot I_a/I_b$
 3.036 * 0.885 = 2.688
 Qp = 10.578

Total of 2 streams to confluence:
 Flow rates before confluence point:
 7.890 3.036
 Area of streams before confluence:
 4.780 1.610

Results of confluence:
 Total flow rate = 10.578 (CFS)
 Time of concentration = 17.946 min.
 Effective stream area after confluence = 6.390 (Ac.)

++++
 Process from Point/Station 109.000 to Point/Station 111.000
 **** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 67.600 (Ft.)
 Downstream point/station elevation = 67.200 (Ft.)
 Pipe length = 87.00 (Ft.) Manning's N = 0.013
 No. of pipes = 1 Required pipe flow = 10.578 (CFS)
 Nearest computed pipe diameter = 21.00 (In.)
 Calculated individual pipe flow = 10.578 (CFS)
 Normal flow depth in pipe = 16.92 (In.)
 Flow top width inside pipe = 16.61 (In.)
 Critical Depth = 14.55 (In.)
 Pipe flow velocity = 5.09 (Ft/s)
 Travel time through pipe = 0.28 min.
 Time of concentration (TC) = 18.23 min.

++++
 Process from Point/Station 111.000 to Point/Station 111.000
 **** CONFLUENCE OF MINOR STREAMS ****

Along Main Stream number: 2 in normal stream number 1
 Stream flow area = 6.390 (Ac.)
 Runoff from this stream = 10.578 (CFS)
 Time of concentration = 18.23 min.
 Rainfall intensity = 2.480 (In/Hr)

+++++
 Process from Point/Station 110.000 to Point/Station 111.000
 ***** INITIAL AREA EVALUATION *****

Initial area flow distance = 338.000(Ft.)
 Top (of initial area) elevation = 70.200(Ft.)
 Bottom (of initial area) elevation = 68.300(Ft.)
 Difference in elevation = 1.900(Ft.)
 Slope = 0.00562 s(percent)= 0.56
 $TC = k(0.420)*[(length^3)/(elevation\ change)]^{0.2}$
 Initial area time of concentration = 12.158 min.
 Rainfall intensity = 3.098(In/Hr) for a 10.0 year storm
 SINGLE FAMILY (1/2 Acre Lot)
 Runoff Coefficient = 0.642
 Decimal fraction soil group A = 1.000
 Decimal fraction soil group B = 0.000
 Decimal fraction soil group C = 0.000
 Decimal fraction soil group D = 0.000
 RI index for soil(AMC 2) = 32.00
 Pervious area fraction = 0.600; Impervious fraction = 0.400
 Initial subarea runoff = 1.472(CFS)
 Total initial stream area = 0.740(Ac.)
 Pervious area fraction = 0.600

+++++
 Process from Point/Station 110.000 to Point/Station 111.000
 ***** CONFLUENCE OF MINOR STREAMS *****

Along Main Stream number: 2 in normal stream number 2
 Stream flow area = 0.740(Ac.)
 Runoff from this stream = 1.472(CFS)
 Time of concentration = 12.16 min.
 Rainfall intensity = 3.098(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
------------	-----------------	----------	----------------------------

1	10.578	18.23	2.480
2	1.472	12.16	3.098

Largest stream flow has longer time of concentration

Qp = 10.578 + sum of
 $Qb \quad Ia/Ib$
 $1.472 * 0.800 = 1.178$
 Qp = 11.756

Total of 2 streams to confluence:
 Flow rates before confluence point:
 10.578 1.472

Area of streams before confluence:
 6.390 0.740

Results of confluence:
 Total flow rate = 11.756(CFS)
 Time of concentration = 18.231 min.
 Effective stream area after confluence = 7.130(Ac.)

+++++
 Process from Point/Station 111.000 to Point/Station 104.000
 **** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 67.200 (Ft.)
 Downstream point/station elevation = 67.000 (Ft.)
 Pipe length = 130.00 (Ft.) Manning's N = 0.013
 No. of pipes = 1 Required pipe flow = 11.756 (CFS)
 Nearest computed pipe diameter = 27.00 (In.)
 Calculated individual pipe flow = 11.756 (CFS)
 Normal flow depth in pipe = 21.38 (In.)
 Flow top width inside pipe = 21.93 (In.)
 Critical Depth = 14.24 (In.)
 Pipe flow velocity = 3.48 (Ft/s)
 Travel time through pipe = 0.62 min.
 Time of concentration (TC) = 18.85 min.

+++++
 Process from Point/Station 111.000 to Point/Station 104.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 2
 Stream flow area = 7.130 (Ac.)
 Runoff from this stream = 11.756 (CFS)
 Time of concentration = 18.85 min.
 Rainfall intensity = 2.434 (In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
------------	-----------------	----------	----------------------------

1	9.048	15.59	2.703
2	11.756	18.85	2.434

Largest stream flow has longer time of concentration

Qp = 11.756 + sum of

$$Q_b \quad I_a/I_b$$

$$9.048 * 0.901 = 8.149$$
 Qp = 19.904

Total of 2 main streams to confluence:

Flow rates before confluence point:
 9.048 11.756
 Area of streams before confluence:
 5.090 7.130

Results of confluence:

Total flow rate = 19.904 (CFS)
 Time of concentration = 18.853 min.
 Effective stream area after confluence = 12.220 (Ac.)

+++++
 Process from Point/Station 104.000 to Point/Station 112.000
 **** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 67.000 (Ft.)

Downstream point/station elevation = 61.000 (Ft.)
Pipe length = 262.00 (Ft.) Manning's N = 0.013
No. of pipes = 1 Required pipe flow = 19.904 (CFS)
Nearest computed pipe diameter = 21.00 (In.)
Calculated individual pipe flow = 19.904 (CFS)
Normal flow depth in pipe = 14.60 (In.)
Flow top width inside pipe = 19.33 (In.)
Critical Depth = 19.15 (In.)
Pipe flow velocity = 11.15 (Ft/s)
Travel time through pipe = 0.39 min.
Time of concentration (TC) = 19.24 min.

++++
Process from Point/Station 112.000 to Point/Station 112.000
**** SUBAREA FLOW ADDITION ****

USER INPUT of soil data for subarea
Runoff Coefficient = 0.545
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil (AMC 2) = 46.00
Pervious area fraction = 1.000; Impervious fraction = 0.000
Time of concentration = 19.24 min.
Rainfall intensity = 2.407 (In/Hr) for a 10.0 year storm
Subarea runoff = 0.893 (CFS) for 0.680 (Ac.)
Total runoff = 20.797 (CFS) Total area = 12.900 (Ac.)

++++
Process from Point/Station 112.000 to Point/Station 113.000
**** PIPEFLOW TRAVEL TIME (Program estimated size) ****

Upstream point/station elevation = 61.000 (Ft.)
Downstream point/station elevation = 58.000 (Ft.)
Pipe length = 167.00 (Ft.) Manning's N = 0.013
No. of pipes = 1 Required pipe flow = 20.797 (CFS)
Nearest computed pipe diameter = 21.00 (In.)
Calculated individual pipe flow = 20.797 (CFS)
Normal flow depth in pipe = 16.83 (In.)
Flow top width inside pipe = 16.76 (In.)
Critical Depth = 19.39 (In.)
Pipe flow velocity = 10.06 (Ft/s)
Travel time through pipe = 0.28 min.
Time of concentration (TC) = 19.52 min.
End of computations, total study area = 12.90 (Ac.)
The following figures may
be used for a unit hydrograph study of the same area.

Area averaged pervious area fraction (Ap) = 0.625
Area averaged RI index number = 32.7

APPENDIX B: PRE-PROJECT UNIT HYDROGRAPH METHOD HYDROLOGY

APPENDIX B.1: UNIT HYDROGRAPH ANALYSIS, AREAS “A”

2-YEAR, 24-HOUR STORM EVENT

Unit Hydrograph Analysis

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Study date 02/16/23 File: ara242.out

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Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978

Program License Serial Number 6279

English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used

English Units used in output format

302.01.21
AREA A
2 YR STORM
PRE PROJECT

Drainage Area = 12.23(Ac.) = 0.019 Sq. Mi.
Drainage Area for Depth-Area Areal Adjustment = 12.23(Ac.) = 0.019
Sq. Mi.
Length along longest watercourse = 1105.90(Ft.)
Length along longest watercourse measured to centroid = 610.60(Ft.)
Length along longest watercourse = 0.209 Mi.
Length along longest watercourse measured to centroid = 0.116 Mi.
Difference in elevation = 9.50(Ft.)
Slope along watercourse = 45.3567 Ft./Mi.
Average Manning's 'N' = 0.040
Lag time = 0.113 Hr.
Lag time = 6.79 Min.
25% of lag time = 1.70 Min.
40% of lag time = 2.71 Min.
Unit time = 5.00 Min.
Duration of storm = 24 Hour(s)
User Entered Base Flow = 0.00(CFS)

2 YEAR Area rainfall data:

Area(Ac.) [1]	Rainfall(In) [2]	Weighting[1*2]
12.23	2.79	34.12

100 YEAR Area rainfall data:

Area(Ac.) [1]	Rainfall(In) [2]	Weighting[1*2]
12.23	7.72	94.42

STORM EVENT (YEAR) = 2.00
 Area Averaged 2-Year Rainfall = 2.790 (In)
 Area Averaged 100-Year Rainfall = 7.720 (In)

Point rain (area averaged) = 2.790 (In)
 Areal adjustment factor = 100.00 %
 Adjusted average point rain = 2.790 (In)

Sub-Area Data:

Area (Ac.)	Runoff Index	Impervious %
12.230	46.00	0.000
Total Area Entered = 12.23 (Ac.)		

RI	RI	Infil. Rate	Impervious	Adj. Infil. Rate	Area%	F
AMC2	AMC-2	(In/Hr)	(Dec.%)	(In/Hr)	(Dec.)	(In/Hr)
46.0	46.0	0.611	0.000	0.611	1.000	0.611
						Sum (F) = 0.611

Area averaged mean soil loss (F) (In/Hr) = 0.611
 Minimum soil loss rate ((In/Hr)) = 0.306
 (for 24 hour storm duration)
 Soil loss rate (decimal) = 0.900

 Unit Hydrograph
 MOUNTAIN S-Curve

Unit Hydrograph Data

Unit time period (hrs)	Time % of lag	Distribution Graph %	Unit Hydrograph (CFS)
1	0.083	73.674	15.042
2	0.167	147.347	36.939
3	0.250	221.021	14.894
4	0.333	294.694	8.503
5	0.417	368.368	5.644
6	0.500	442.041	3.929
7	0.583	515.715	3.090
8	0.667	589.388	2.520
9	0.750	663.062	2.116
10	0.833	736.736	1.784
11	0.917	810.409	1.461
12	1.000	884.083	1.329
13	1.083	957.756	1.326
14	1.167	1031.430	1.423
		Sum = 100.000	Sum= 12.326

The following loss rate calculations reflect use of the minimum calculated loss rate subtracted from the Storm Rain to produce the maximum Effective Rain value

Unit	Time (Hr.)	Pattern Percent	Storm Rain (In/Hr)	Loss rate (In./Hr)		Effective (In/Hr)
				Max	Low	
1	0.08	0.07	0.022	(1.083)	0.020	0.002
2	0.17	0.07	0.022	(1.079)	0.020	0.002
3	0.25	0.07	0.022	(1.075)	0.020	0.002
4	0.33	0.10	0.033	(1.071)	0.030	0.003
5	0.42	0.10	0.033	(1.067)	0.030	0.003

6	0.50	0.10	0.033	(1.063)	0.030	0.003
7	0.58	0.10	0.033	(1.058)	0.030	0.003
8	0.67	0.10	0.033	(1.054)	0.030	0.003
9	0.75	0.10	0.033	(1.050)	0.030	0.003
10	0.83	0.13	0.045	(1.046)	0.040	0.004
11	0.92	0.13	0.045	(1.042)	0.040	0.004
12	1.00	0.13	0.045	(1.038)	0.040	0.004
13	1.08	0.10	0.033	(1.034)	0.030	0.003
14	1.17	0.10	0.033	(1.030)	0.030	0.003
15	1.25	0.10	0.033	(1.026)	0.030	0.003
16	1.33	0.10	0.033	(1.021)	0.030	0.003
17	1.42	0.10	0.033	(1.017)	0.030	0.003
18	1.50	0.10	0.033	(1.013)	0.030	0.003
19	1.58	0.10	0.033	(1.009)	0.030	0.003
20	1.67	0.10	0.033	(1.005)	0.030	0.003
21	1.75	0.10	0.033	(1.001)	0.030	0.003
22	1.83	0.13	0.045	(0.997)	0.040	0.004
23	1.92	0.13	0.045	(0.993)	0.040	0.004
24	2.00	0.13	0.045	(0.989)	0.040	0.004
25	2.08	0.13	0.045	(0.985)	0.040	0.004
26	2.17	0.13	0.045	(0.981)	0.040	0.004
27	2.25	0.13	0.045	(0.977)	0.040	0.004
28	2.33	0.13	0.045	(0.973)	0.040	0.004
29	2.42	0.13	0.045	(0.969)	0.040	0.004
30	2.50	0.13	0.045	(0.965)	0.040	0.004
31	2.58	0.17	0.056	(0.961)	0.050	0.006
32	2.67	0.17	0.056	(0.957)	0.050	0.006
33	2.75	0.17	0.056	(0.953)	0.050	0.006
34	2.83	0.17	0.056	(0.950)	0.050	0.006
35	2.92	0.17	0.056	(0.946)	0.050	0.006
36	3.00	0.17	0.056	(0.942)	0.050	0.006
37	3.08	0.17	0.056	(0.938)	0.050	0.006
38	3.17	0.17	0.056	(0.934)	0.050	0.006
39	3.25	0.17	0.056	(0.930)	0.050	0.006
40	3.33	0.17	0.056	(0.926)	0.050	0.006
41	3.42	0.17	0.056	(0.922)	0.050	0.006
42	3.50	0.17	0.056	(0.918)	0.050	0.006
43	3.58	0.17	0.056	(0.915)	0.050	0.006
44	3.67	0.17	0.056	(0.911)	0.050	0.006
45	3.75	0.17	0.056	(0.907)	0.050	0.006
46	3.83	0.20	0.067	(0.903)	0.060	0.007
47	3.92	0.20	0.067	(0.899)	0.060	0.007
48	4.00	0.20	0.067	(0.895)	0.060	0.007
49	4.08	0.20	0.067	(0.892)	0.060	0.007
50	4.17	0.20	0.067	(0.888)	0.060	0.007
51	4.25	0.20	0.067	(0.884)	0.060	0.007
52	4.33	0.23	0.078	(0.880)	0.070	0.008
53	4.42	0.23	0.078	(0.877)	0.070	0.008
54	4.50	0.23	0.078	(0.873)	0.070	0.008
55	4.58	0.23	0.078	(0.869)	0.070	0.008
56	4.67	0.23	0.078	(0.865)	0.070	0.008
57	4.75	0.23	0.078	(0.862)	0.070	0.008
58	4.83	0.27	0.089	(0.858)	0.080	0.009
59	4.92	0.27	0.089	(0.854)	0.080	0.009
60	5.00	0.27	0.089	(0.850)	0.080	0.009
61	5.08	0.20	0.067	(0.847)	0.060	0.007
62	5.17	0.20	0.067	(0.843)	0.060	0.007
63	5.25	0.20	0.067	(0.839)	0.060	0.007
64	5.33	0.23	0.078	(0.836)	0.070	0.008
65	5.42	0.23	0.078	(0.832)	0.070	0.008

66	5.50	0.23	0.078	(0.828)	0.070	0.008
67	5.58	0.27	0.089	(0.825)	0.080	0.009
68	5.67	0.27	0.089	(0.821)	0.080	0.009
69	5.75	0.27	0.089	(0.818)	0.080	0.009
70	5.83	0.27	0.089	(0.814)	0.080	0.009
71	5.92	0.27	0.089	(0.810)	0.080	0.009
72	6.00	0.27	0.089	(0.807)	0.080	0.009
73	6.08	0.30	0.100	(0.803)	0.090	0.010
74	6.17	0.30	0.100	(0.800)	0.090	0.010
75	6.25	0.30	0.100	(0.796)	0.090	0.010
76	6.33	0.30	0.100	(0.792)	0.090	0.010
77	6.42	0.30	0.100	(0.789)	0.090	0.010
78	6.50	0.30	0.100	(0.785)	0.090	0.010
79	6.58	0.33	0.112	(0.782)	0.100	0.011
80	6.67	0.33	0.112	(0.778)	0.100	0.011
81	6.75	0.33	0.112	(0.775)	0.100	0.011
82	6.83	0.33	0.112	(0.771)	0.100	0.011
83	6.92	0.33	0.112	(0.768)	0.100	0.011
84	7.00	0.33	0.112	(0.764)	0.100	0.011
85	7.08	0.33	0.112	(0.761)	0.100	0.011
86	7.17	0.33	0.112	(0.757)	0.100	0.011
87	7.25	0.33	0.112	(0.754)	0.100	0.011
88	7.33	0.37	0.123	(0.751)	0.110	0.012
89	7.42	0.37	0.123	(0.747)	0.110	0.012
90	7.50	0.37	0.123	(0.744)	0.110	0.012
91	7.58	0.40	0.134	(0.740)	0.121	0.013
92	7.67	0.40	0.134	(0.737)	0.121	0.013
93	7.75	0.40	0.134	(0.733)	0.121	0.013
94	7.83	0.43	0.145	(0.730)	0.131	0.015
95	7.92	0.43	0.145	(0.727)	0.131	0.015
96	8.00	0.43	0.145	(0.723)	0.131	0.015
97	8.08	0.50	0.167	(0.720)	0.151	0.017
98	8.17	0.50	0.167	(0.717)	0.151	0.017
99	8.25	0.50	0.167	(0.713)	0.151	0.017
100	8.33	0.50	0.167	(0.710)	0.151	0.017
101	8.42	0.50	0.167	(0.707)	0.151	0.017
102	8.50	0.50	0.167	(0.703)	0.151	0.017
103	8.58	0.53	0.179	(0.700)	0.161	0.018
104	8.67	0.53	0.179	(0.697)	0.161	0.018
105	8.75	0.53	0.179	(0.693)	0.161	0.018
106	8.83	0.57	0.190	(0.690)	0.171	0.019
107	8.92	0.57	0.190	(0.687)	0.171	0.019
108	9.00	0.57	0.190	(0.684)	0.171	0.019
109	9.08	0.63	0.212	(0.680)	0.191	0.021
110	9.17	0.63	0.212	(0.677)	0.191	0.021
111	9.25	0.63	0.212	(0.674)	0.191	0.021
112	9.33	0.67	0.223	(0.671)	0.201	0.022
113	9.42	0.67	0.223	(0.668)	0.201	0.022
114	9.50	0.67	0.223	(0.664)	0.201	0.022
115	9.58	0.70	0.234	(0.661)	0.211	0.023
116	9.67	0.70	0.234	(0.658)	0.211	0.023
117	9.75	0.70	0.234	(0.655)	0.211	0.023
118	9.83	0.73	0.246	(0.652)	0.221	0.025
119	9.92	0.73	0.246	(0.649)	0.221	0.025
120	10.00	0.73	0.246	(0.645)	0.221	0.025
121	10.08	0.50	0.167	(0.642)	0.151	0.017
122	10.17	0.50	0.167	(0.639)	0.151	0.017
123	10.25	0.50	0.167	(0.636)	0.151	0.017
124	10.33	0.50	0.167	(0.633)	0.151	0.017
125	10.42	0.50	0.167	(0.630)	0.151	0.017

126	10.50	0.50	0.167	(0.627)	0.151	0.017
127	10.58	0.67	0.223	(0.624)	0.201	0.022
128	10.67	0.67	0.223	(0.621)	0.201	0.022
129	10.75	0.67	0.223	(0.618)	0.201	0.022
130	10.83	0.67	0.223	(0.615)	0.201	0.022
131	10.92	0.67	0.223	(0.612)	0.201	0.022
132	11.00	0.67	0.223	(0.609)	0.201	0.022
133	11.08	0.63	0.212	(0.606)	0.191	0.021
134	11.17	0.63	0.212	(0.603)	0.191	0.021
135	11.25	0.63	0.212	(0.600)	0.191	0.021
136	11.33	0.63	0.212	(0.597)	0.191	0.021
137	11.42	0.63	0.212	(0.594)	0.191	0.021
138	11.50	0.63	0.212	(0.591)	0.191	0.021
139	11.58	0.57	0.190	(0.588)	0.171	0.019
140	11.67	0.57	0.190	(0.585)	0.171	0.019
141	11.75	0.57	0.190	(0.582)	0.171	0.019
142	11.83	0.60	0.201	(0.579)	0.181	0.020
143	11.92	0.60	0.201	(0.576)	0.181	0.020
144	12.00	0.60	0.201	(0.573)	0.181	0.020
145	12.08	0.83	0.279	(0.571)	0.251	0.028
146	12.17	0.83	0.279	(0.568)	0.251	0.028
147	12.25	0.83	0.279	(0.565)	0.251	0.028
148	12.33	0.87	0.290	(0.562)	0.261	0.029
149	12.42	0.87	0.290	(0.559)	0.261	0.029
150	12.50	0.87	0.290	(0.556)	0.261	0.029
151	12.58	0.93	0.312	(0.554)	0.281	0.031
152	12.67	0.93	0.312	(0.551)	0.281	0.031
153	12.75	0.93	0.312	(0.548)	0.281	0.031
154	12.83	0.97	0.324	(0.545)	0.291	0.032
155	12.92	0.97	0.324	(0.542)	0.291	0.032
156	13.00	0.97	0.324	(0.540)	0.291	0.032
157	13.08	1.13	0.379	(0.537)	0.341	0.038
158	13.17	1.13	0.379	(0.534)	0.341	0.038
159	13.25	1.13	0.379	(0.532)	0.341	0.038
160	13.33	1.13	0.379	(0.529)	0.341	0.038
161	13.42	1.13	0.379	(0.526)	0.341	0.038
162	13.50	1.13	0.379	(0.524)	0.341	0.038
163	13.58	0.77	0.257	(0.521)	0.231	0.026
164	13.67	0.77	0.257	(0.518)	0.231	0.026
165	13.75	0.77	0.257	(0.516)	0.231	0.026
166	13.83	0.77	0.257	(0.513)	0.231	0.026
167	13.92	0.77	0.257	(0.510)	0.231	0.026
168	14.00	0.77	0.257	(0.508)	0.231	0.026
169	14.08	0.90	0.301	(0.505)	0.271	0.030
170	14.17	0.90	0.301	(0.503)	0.271	0.030
171	14.25	0.90	0.301	(0.500)	0.271	0.030
172	14.33	0.87	0.290	(0.497)	0.261	0.029
173	14.42	0.87	0.290	(0.495)	0.261	0.029
174	14.50	0.87	0.290	(0.492)	0.261	0.029
175	14.58	0.87	0.290	(0.490)	0.261	0.029
176	14.67	0.87	0.290	(0.487)	0.261	0.029
177	14.75	0.87	0.290	(0.485)	0.261	0.029
178	14.83	0.83	0.279	(0.482)	0.251	0.028
179	14.92	0.83	0.279	(0.480)	0.251	0.028
180	15.00	0.83	0.279	(0.477)	0.251	0.028
181	15.08	0.80	0.268	(0.475)	0.241	0.027
182	15.17	0.80	0.268	(0.472)	0.241	0.027
183	15.25	0.80	0.268	(0.470)	0.241	0.027
184	15.33	0.77	0.257	(0.468)	0.231	0.026
185	15.42	0.77	0.257	(0.465)	0.231	0.026

186	15.50	0.77	0.257	(0.463)	0.231	0.026
187	15.58	0.63	0.212	(0.460)	0.191	0.021
188	15.67	0.63	0.212	(0.458)	0.191	0.021
189	15.75	0.63	0.212	(0.456)	0.191	0.021
190	15.83	0.63	0.212	(0.453)	0.191	0.021
191	15.92	0.63	0.212	(0.451)	0.191	0.021
192	16.00	0.63	0.212	(0.449)	0.191	0.021
193	16.08	0.13	0.045	(0.447)	0.040	0.004
194	16.17	0.13	0.045	(0.444)	0.040	0.004
195	16.25	0.13	0.045	(0.442)	0.040	0.004
196	16.33	0.13	0.045	(0.440)	0.040	0.004
197	16.42	0.13	0.045	(0.437)	0.040	0.004
198	16.50	0.13	0.045	(0.435)	0.040	0.004
199	16.58	0.10	0.033	(0.433)	0.030	0.003
200	16.67	0.10	0.033	(0.431)	0.030	0.003
201	16.75	0.10	0.033	(0.429)	0.030	0.003
202	16.83	0.10	0.033	(0.426)	0.030	0.003
203	16.92	0.10	0.033	(0.424)	0.030	0.003
204	17.00	0.10	0.033	(0.422)	0.030	0.003
205	17.08	0.17	0.056	(0.420)	0.050	0.006
206	17.17	0.17	0.056	(0.418)	0.050	0.006
207	17.25	0.17	0.056	(0.416)	0.050	0.006
208	17.33	0.17	0.056	(0.414)	0.050	0.006
209	17.42	0.17	0.056	(0.412)	0.050	0.006
210	17.50	0.17	0.056	(0.410)	0.050	0.006
211	17.58	0.17	0.056	(0.408)	0.050	0.006
212	17.67	0.17	0.056	(0.406)	0.050	0.006
213	17.75	0.17	0.056	(0.404)	0.050	0.006
214	17.83	0.13	0.045	(0.402)	0.040	0.004
215	17.92	0.13	0.045	(0.400)	0.040	0.004
216	18.00	0.13	0.045	(0.398)	0.040	0.004
217	18.08	0.13	0.045	(0.396)	0.040	0.004
218	18.17	0.13	0.045	(0.394)	0.040	0.004
219	18.25	0.13	0.045	(0.392)	0.040	0.004
220	18.33	0.13	0.045	(0.390)	0.040	0.004
221	18.42	0.13	0.045	(0.388)	0.040	0.004
222	18.50	0.13	0.045	(0.386)	0.040	0.004
223	18.58	0.10	0.033	(0.384)	0.030	0.003
224	18.67	0.10	0.033	(0.382)	0.030	0.003
225	18.75	0.10	0.033	(0.380)	0.030	0.003
226	18.83	0.07	0.022	(0.379)	0.020	0.002
227	18.92	0.07	0.022	(0.377)	0.020	0.002
228	19.00	0.07	0.022	(0.375)	0.020	0.002
229	19.08	0.10	0.033	(0.373)	0.030	0.003
230	19.17	0.10	0.033	(0.372)	0.030	0.003
231	19.25	0.10	0.033	(0.370)	0.030	0.003
232	19.33	0.13	0.045	(0.368)	0.040	0.004
233	19.42	0.13	0.045	(0.366)	0.040	0.004
234	19.50	0.13	0.045	(0.365)	0.040	0.004
235	19.58	0.10	0.033	(0.363)	0.030	0.003
236	19.67	0.10	0.033	(0.361)	0.030	0.003
237	19.75	0.10	0.033	(0.360)	0.030	0.003
238	19.83	0.07	0.022	(0.358)	0.020	0.002
239	19.92	0.07	0.022	(0.356)	0.020	0.002
240	20.00	0.07	0.022	(0.355)	0.020	0.002
241	20.08	0.10	0.033	(0.353)	0.030	0.003
242	20.17	0.10	0.033	(0.352)	0.030	0.003
243	20.25	0.10	0.033	(0.350)	0.030	0.003
244	20.33	0.10	0.033	(0.349)	0.030	0.003
245	20.42	0.10	0.033	(0.347)	0.030	0.003

246	20.50	0.10	0.033	(0.346)	0.030	0.003
247	20.58	0.10	0.033	(0.344)	0.030	0.003
248	20.67	0.10	0.033	(0.343)	0.030	0.003
249	20.75	0.10	0.033	(0.341)	0.030	0.003
250	20.83	0.07	0.022	(0.340)	0.020	0.002
251	20.92	0.07	0.022	(0.339)	0.020	0.002
252	21.00	0.07	0.022	(0.337)	0.020	0.002
253	21.08	0.10	0.033	(0.336)	0.030	0.003
254	21.17	0.10	0.033	(0.335)	0.030	0.003
255	21.25	0.10	0.033	(0.333)	0.030	0.003
256	21.33	0.07	0.022	(0.332)	0.020	0.002
257	21.42	0.07	0.022	(0.331)	0.020	0.002
258	21.50	0.07	0.022	(0.330)	0.020	0.002
259	21.58	0.10	0.033	(0.328)	0.030	0.003
260	21.67	0.10	0.033	(0.327)	0.030	0.003
261	21.75	0.10	0.033	(0.326)	0.030	0.003
262	21.83	0.07	0.022	(0.325)	0.020	0.002
263	21.92	0.07	0.022	(0.324)	0.020	0.002
264	22.00	0.07	0.022	(0.323)	0.020	0.002
265	22.08	0.10	0.033	(0.322)	0.030	0.003
266	22.17	0.10	0.033	(0.321)	0.030	0.003
267	22.25	0.10	0.033	(0.320)	0.030	0.003
268	22.33	0.07	0.022	(0.319)	0.020	0.002
269	22.42	0.07	0.022	(0.318)	0.020	0.002
270	22.50	0.07	0.022	(0.317)	0.020	0.002
271	22.58	0.07	0.022	(0.316)	0.020	0.002
272	22.67	0.07	0.022	(0.315)	0.020	0.002
273	22.75	0.07	0.022	(0.314)	0.020	0.002
274	22.83	0.07	0.022	(0.313)	0.020	0.002
275	22.92	0.07	0.022	(0.312)	0.020	0.002
276	23.00	0.07	0.022	(0.312)	0.020	0.002
277	23.08	0.07	0.022	(0.311)	0.020	0.002
278	23.17	0.07	0.022	(0.310)	0.020	0.002
279	23.25	0.07	0.022	(0.310)	0.020	0.002
280	23.33	0.07	0.022	(0.309)	0.020	0.002
281	23.42	0.07	0.022	(0.308)	0.020	0.002
282	23.50	0.07	0.022	(0.308)	0.020	0.002
283	23.58	0.07	0.022	(0.307)	0.020	0.002
284	23.67	0.07	0.022	(0.307)	0.020	0.002
285	23.75	0.07	0.022	(0.306)	0.020	0.002
286	23.83	0.07	0.022	(0.306)	0.020	0.002
287	23.92	0.07	0.022	(0.306)	0.020	0.002
288	24.00	0.07	0.022	(0.306)	0.020	0.002

(Loss Rate Not Used)

Sum = 100.0 Sum = 3.3

Flood volume = Effective rainfall 0.28 (In)
times area 12.2 (Ac.) / [(In) / (Ft.)] = 0.3 (Ac.Ft)
Total soil loss = 2.51 (In)
Total soil loss = 2.559 (Ac.Ft)
Total rainfall = 2.79 (In)
Flood volume = 12385.9 Cubic Feet
Total soil loss = 111472.9 Cubic Feet

Peak flow rate of this hydrograph = 0.456 (CFS)

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24 - H O U R S T O R M
R u n o f f H y d r o g r a p h

Hydrograph in 5 Minute intervals ((CFS))

Time(h+m)	Volume	Ac.Ft	Q(CFS)	0	2.5	5.0	7.5	10.0
0+ 5	0.0000		0.00	Q				
0+10	0.0001		0.01	Q				
0+15	0.0003		0.02	Q				
0+20	0.0004		0.02	Q				
0+25	0.0006		0.03	Q				
0+30	0.0008		0.03	Q				
0+35	0.0011		0.03	Q				
0+40	0.0013		0.04	Q				
0+45	0.0016		0.04	Q				
0+50	0.0019		0.04	Q				
0+55	0.0022		0.05	Q				
1+ 0	0.0025		0.05	Q				
1+ 5	0.0028		0.05	Q				
1+10	0.0032		0.04	Q				
1+15	0.0035		0.04	Q				
1+20	0.0037		0.04	Q				
1+25	0.0040		0.04	Q				
1+30	0.0043		0.04	Q				
1+35	0.0046		0.04	Q				
1+40	0.0049		0.04	Q				
1+45	0.0052		0.04	Q				
1+50	0.0055		0.04	Q				
1+55	0.0058		0.05	Q				
2+ 0	0.0062		0.05	Q				
2+ 5	0.0065		0.05	Q				
2+10	0.0069		0.05	Q				
2+15	0.0073		0.05	QV				
2+20	0.0076		0.05	QV				
2+25	0.0080		0.05	QV				
2+30	0.0084		0.05	QV				
2+35	0.0088		0.06	QV				
2+40	0.0092		0.06	QV				
2+45	0.0096		0.06	QV				
2+50	0.0101		0.07	QV				
2+55	0.0105		0.07	QV				
3+ 0	0.0110		0.07	QV				
3+ 5	0.0115		0.07	QV				
3+10	0.0119		0.07	QV				
3+15	0.0124		0.07	QV				
3+20	0.0129		0.07	QV				
3+25	0.0133		0.07	QV				
3+30	0.0138		0.07	QV				
3+35	0.0143		0.07	Q V				
3+40	0.0148		0.07	Q V				
3+45	0.0152		0.07	Q V				
3+50	0.0157		0.07	Q V				
3+55	0.0162		0.08	Q V				
4+ 0	0.0168		0.08	Q V				
4+ 5	0.0173		0.08	Q V				
4+10	0.0179		0.08	Q V				
4+15	0.0184		0.08	Q V				
4+20	0.0190		0.08	Q V				
4+25	0.0196		0.09	Q V				
4+30	0.0202		0.09	Q V				
4+35	0.0209		0.09	Q V				
4+40	0.0215		0.09	Q V				

4+45	0.0222	0.09	Q	V
4+50	0.0228	0.10	Q	V
4+55	0.0235	0.10	Q	V
5+ 0	0.0242	0.10	Q	V
5+ 5	0.0249	0.10	Q	V
5+10	0.0256	0.09	Q	V
5+15	0.0262	0.09	Q	V
5+20	0.0268	0.09	Q	V
5+25	0.0275	0.09	Q	V
5+30	0.0281	0.09	Q	V
5+35	0.0288	0.10	Q	V
5+40	0.0295	0.10	Q	V
5+45	0.0302	0.11	Q	V
5+50	0.0309	0.11	Q	V
5+55	0.0317	0.11	Q	V
6+ 0	0.0324	0.11	Q	V
6+ 5	0.0332	0.11	Q	V
6+10	0.0340	0.12	Q	V
6+15	0.0348	0.12	Q	V
6+20	0.0356	0.12	Q	V
6+25	0.0364	0.12	Q	V
6+30	0.0373	0.12	Q	V
6+35	0.0381	0.12	Q	V
6+40	0.0390	0.13	Q	V
6+45	0.0399	0.13	Q	V
6+50	0.0409	0.13	Q	V
6+55	0.0418	0.13	Q	V
7+ 0	0.0427	0.14	Q	V
7+ 5	0.0437	0.14	Q	V
7+10	0.0446	0.14	Q	V
7+15	0.0455	0.14	Q	V
7+20	0.0465	0.14	Q	V
7+25	0.0475	0.14	Q	V
7+30	0.0485	0.15	Q	V
7+35	0.0495	0.15	Q	V
7+40	0.0506	0.16	Q	V
7+45	0.0517	0.16	Q	V
7+50	0.0528	0.16	Q	V
7+55	0.0540	0.17	Q	V
8+ 0	0.0551	0.17	Q	V
8+ 5	0.0564	0.18	Q	V
8+10	0.0577	0.19	Q	V
8+15	0.0590	0.19	Q	V
8+20	0.0604	0.20	Q	V
8+25	0.0617	0.20	Q	V
8+30	0.0631	0.20	Q	V
8+35	0.0645	0.20	Q	V
8+40	0.0660	0.21	Q	V
8+45	0.0674	0.21	Q	V
8+50	0.0689	0.22	Q	V
8+55	0.0705	0.22	Q	V
9+ 0	0.0720	0.23	Q	V
9+ 5	0.0736	0.23	Q	V
9+10	0.0753	0.24	Q	V
9+15	0.0770	0.25	Q	V
9+20	0.0788	0.25	Q	V
9+25	0.0806	0.26	Q	V
9+30	0.0824	0.27	Q	V
9+35	0.0843	0.27	Q	V
9+40	0.0862	0.28	Q	V

9+45	0.0881	0.28	Q	V			
9+50	0.0901	0.28	Q	V			
9+55	0.0921	0.29	Q	V			
10+ 0	0.0941	0.29	Q	V			
10+ 5	0.0960	0.28	Q	V			
10+10	0.0978	0.25	Q	V			
10+15	0.0994	0.23	Q	V			
10+20	0.1009	0.23	Q	V			
10+25	0.1025	0.22	Q	V			
10+30	0.1040	0.22	Q	V			
10+35	0.1055	0.23	Q	V			
10+40	0.1073	0.25	Q	V			
10+45	0.1091	0.26	Q	V			
10+50	0.1109	0.26	Q	V			
10+55	0.1127	0.27	Q	V			
11+ 0	0.1145	0.27	Q	V			
11+ 5	0.1164	0.27	Q	V			
11+10	0.1182	0.26	Q	V			
11+15	0.1200	0.26	Q	V			
11+20	0.1218	0.26	Q	V			
11+25	0.1236	0.26	Q	V			
11+30	0.1254	0.26	Q	V			
11+35	0.1272	0.26	Q	V			
11+40	0.1289	0.25	Q	V			
11+45	0.1306	0.24	Q	V			
11+50	0.1322	0.24	Q	V			
11+55	0.1339	0.25	Q	V			
12+ 0	0.1356	0.25	Q	V			
12+ 5	0.1374	0.26	Q	V			
12+10	0.1395	0.30	Q	V			
12+15	0.1416	0.31	Q	V			
12+20	0.1439	0.32	Q	V			
12+25	0.1462	0.33	Q	V			
12+30	0.1485	0.34	Q	V			
12+35	0.1509	0.35	Q	V			
12+40	0.1533	0.36	Q	V			
12+45	0.1559	0.37	Q	V			
12+50	0.1584	0.37	Q	V			
12+55	0.1611	0.38	Q	V			
13+ 0	0.1637	0.39	Q	V			
13+ 5	0.1665	0.40	Q	V			
13+10	0.1695	0.43	Q	V			
13+15	0.1725	0.44	Q	V			
13+20	0.1756	0.45	Q	V			
13+25	0.1787	0.45	Q	V			
13+30	0.1818	0.46	Q	V			
13+35	0.1848	0.44	Q	V			
13+40	0.1875	0.38	Q	V			
13+45	0.1899	0.36	Q	V			
13+50	0.1924	0.35	Q	V			
13+55	0.1947	0.34	Q	V			
14+ 0	0.1970	0.34	Q	V			
14+ 5	0.1994	0.34	Q	V			
14+10	0.2019	0.36	Q	V			
14+15	0.2044	0.36	Q	V			
14+20	0.2069	0.36	Q	V			
14+25	0.2094	0.36	Q	V			
14+30	0.2118	0.36	Q	V			
14+35	0.2143	0.36	Q	V			
14+40	0.2167	0.36	Q	V			

14+45	0.2192	0.36	Q	V
14+50	0.2216	0.35	Q	V
14+55	0.2240	0.35	Q	V
15+ 0	0.2264	0.35	Q	V
15+ 5	0.2288	0.35	Q	V
15+10	0.2312	0.34	Q	V
15+15	0.2335	0.34	Q	V
15+20	0.2358	0.33	Q	V
15+25	0.2380	0.33	Q	V
15+30	0.2403	0.32	Q	V
15+35	0.2424	0.31	Q	V
15+40	0.2444	0.29	Q	V
15+45	0.2464	0.28	Q	V
15+50	0.2483	0.28	Q	V
15+55	0.2502	0.27	Q	V
16+ 0	0.2521	0.27	Q	V
16+ 5	0.2537	0.24	Q	V
16+10	0.2548	0.16	Q	V
16+15	0.2557	0.13	Q	V
16+20	0.2564	0.11	Q	V
16+25	0.2571	0.10	Q	V
16+30	0.2577	0.09	Q	V
16+35	0.2582	0.08	Q	V
16+40	0.2587	0.07	Q	V
16+45	0.2591	0.06	Q	V
16+50	0.2595	0.06	Q	V
16+55	0.2599	0.05	Q	V
17+ 0	0.2602	0.05	Q	V
17+ 5	0.2605	0.05	Q	V
17+10	0.2609	0.06	Q	V
17+15	0.2614	0.06	Q	V
17+20	0.2618	0.06	Q	V
17+25	0.2622	0.06	Q	V
17+30	0.2627	0.07	Q	V
17+35	0.2631	0.07	Q	V
17+40	0.2636	0.07	Q	V
17+45	0.2640	0.07	Q	V
17+50	0.2645	0.07	Q	V
17+55	0.2649	0.06	Q	V
18+ 0	0.2653	0.06	Q	V
18+ 5	0.2657	0.06	Q	V
18+10	0.2661	0.06	Q	V
18+15	0.2665	0.06	Q	V
18+20	0.2669	0.06	Q	V
18+25	0.2673	0.06	Q	V
18+30	0.2677	0.06	Q	V
18+35	0.2680	0.05	Q	V
18+40	0.2684	0.05	Q	V
18+45	0.2687	0.05	Q	V
18+50	0.2690	0.04	Q	V
18+55	0.2692	0.04	Q	V
19+ 0	0.2695	0.03	Q	V
19+ 5	0.2697	0.03	Q	V
19+10	0.2700	0.04	Q	V
19+15	0.2703	0.04	Q	V
19+20	0.2706	0.04	Q	V
19+25	0.2709	0.05	Q	V
19+30	0.2712	0.05	Q	V
19+35	0.2716	0.05	Q	V
19+40	0.2719	0.04	Q	V

19+45	0.2722	0.04	Q	V
19+50	0.2724	0.04	Q	V
19+55	0.2727	0.03	Q	V
20+ 0	0.2729	0.03	Q	V
20+ 5	0.2731	0.03	Q	V
20+10	0.2734	0.04	Q	V
20+15	0.2737	0.04	Q	V
20+20	0.2739	0.04	Q	V
20+25	0.2742	0.04	Q	V
20+30	0.2745	0.04	Q	V
20+35	0.2748	0.04	Q	V
20+40	0.2751	0.04	Q	V
20+45	0.2753	0.04	Q	V
20+50	0.2756	0.04	Q	V
20+55	0.2758	0.03	Q	V
21+ 0	0.2761	0.03	Q	V
21+ 5	0.2763	0.03	Q	V
21+10	0.2765	0.04	Q	V
21+15	0.2768	0.04	Q	V
21+20	0.2771	0.04	Q	V
21+25	0.2773	0.03	Q	V
21+30	0.2775	0.03	Q	V
21+35	0.2777	0.03	Q	V
21+40	0.2780	0.04	Q	V
21+45	0.2782	0.04	Q	V
21+50	0.2785	0.04	Q	V
21+55	0.2787	0.03	Q	V
22+ 0	0.2789	0.03	Q	V
22+ 5	0.2792	0.03	Q	V
22+10	0.2794	0.04	Q	V
22+15	0.2797	0.04	Q	V
22+20	0.2799	0.04	Q	V
22+25	0.2801	0.03	Q	V
22+30	0.2804	0.03	Q	V
22+35	0.2806	0.03	Q	V
22+40	0.2808	0.03	Q	V
22+45	0.2810	0.03	Q	V
22+50	0.2812	0.03	Q	V
22+55	0.2814	0.03	Q	V
23+ 0	0.2815	0.03	Q	V
23+ 5	0.2817	0.03	Q	V
23+10	0.2819	0.03	Q	V
23+15	0.2821	0.03	Q	V
23+20	0.2823	0.03	Q	V
23+25	0.2825	0.03	Q	V
23+30	0.2827	0.03	Q	V
23+35	0.2829	0.03	Q	V
23+40	0.2831	0.03	Q	V
23+45	0.2833	0.03	Q	V
23+50	0.2835	0.03	Q	V
23+55	0.2836	0.03	Q	V
24+ 0	0.2838	0.03	Q	V
24+ 5	0.2840	0.02	Q	V
24+10	0.2841	0.01	Q	V
24+15	0.2841	0.01	Q	V
24+20	0.2842	0.01	Q	V
24+25	0.2842	0.01	Q	V
24+30	0.2843	0.00	Q	V
24+35	0.2843	0.00	Q	V
24+40	0.2843	0.00	Q	V

24+45	0.2843	0.00	Q				V
24+50	0.2843	0.00	Q				V
24+55	0.2843	0.00	Q				V
25+ 0	0.2843	0.00	Q				V
25+ 5	0.2843	0.00	Q				V

10-YEAR, 24-HOUR STORM EVENT

Unit Hydrograph Analysis

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Study date 02/16/23 File: ara2410.out

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Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978

Program License Serial Number 6279

English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used

English Units used in output format

302.01.21
AREA A
10 YR STORM
PRE PROJECT

Drainage Area = 12.23(Ac.) = 0.019 Sq. Mi.
Drainage Area for Depth-Area Areal Adjustment = 12.23(Ac.) = 0.019
Sq. Mi.
Length along longest watercourse = 1105.90(Ft.)
Length along longest watercourse measured to centroid = 610.60(Ft.)
Length along longest watercourse = 0.209 Mi.
Length along longest watercourse measured to centroid = 0.116 Mi.
Difference in elevation = 9.50(Ft.)
Slope along watercourse = 45.3567 Ft./Mi.
Average Manning's 'N' = 0.040
Lag time = 0.113 Hr.
Lag time = 6.79 Min.
25% of lag time = 1.70 Min.
40% of lag time = 2.71 Min.
Unit time = 5.00 Min.
Duration of storm = 24 Hour(s)
User Entered Base Flow = 0.00(CFS)

2 YEAR Area rainfall data:

Area(Ac.) [1]	Rainfall(In) [2]	Weighting[1*2]
12.23	2.79	34.12

100 YEAR Area rainfall data:

Area(Ac.) [1]	Rainfall(In) [2]	Weighting[1*2]
12.23	7.72	94.42

STORM EVENT (YEAR) = 10.00
 Area Averaged 2-Year Rainfall = 2.790 (In)
 Area Averaged 100-Year Rainfall = 7.720 (In)

Point rain (area averaged) = 4.818 (In)
 Areal adjustment factor = 100.00 %
 Adjusted average point rain = 4.818 (In)

Sub-Area Data:

Area (Ac.)	Runoff Index	Impervious %
12.230	46.00	0.000
Total Area Entered = 12.23 (Ac.)		

RI	RI	Infil. Rate	Impervious	Adj. Infil. Rate	Area%	F
AMC2	AMC-2	(In/Hr)	(Dec.%)	(In/Hr)	(Dec.)	(In/Hr)
46.0	46.0	0.611	0.000	0.611	1.000	0.611
						Sum (F) = 0.611

Area averaged mean soil loss (F) (In/Hr) = 0.611
 Minimum soil loss rate ((In/Hr)) = 0.306
 (for 24 hour storm duration)
 Soil loss rate (decimal) = 0.900

 U n i t H y d r o g r a p h
 MOUNTAIN S-Curve

Unit Hydrograph Data

Unit time period (hrs)	Time % of lag	Distribution Graph %	Unit Hydrograph (CFS)
1	0.083	73.674	15.042
2	0.167	147.347	36.939
3	0.250	221.021	14.894
4	0.333	294.694	8.503
5	0.417	368.368	5.644
6	0.500	442.041	3.929
7	0.583	515.715	3.090
8	0.667	589.388	2.520
9	0.750	663.062	2.116
10	0.833	736.736	1.784
11	0.917	810.409	1.461
12	1.000	884.083	1.329
13	1.083	957.756	1.326
14	1.167	1031.430	1.423
		Sum = 100.000	Sum= 12.326

The following loss rate calculations reflect use of the minimum calculated loss rate subtracted from the Storm Rain to produce the maximum Effective Rain value

Unit Time (Hr.)	Pattern Percent	Storm Rain (In/Hr)	Loss rate (In./Hr) Max Low	Effective (In/Hr)
1	0.08	0.039	(1.083)	0.035
2	0.17	0.039	(1.079)	0.035
3	0.25	0.039	(1.075)	0.035
4	0.33	0.058	(1.071)	0.052
5	0.42	0.058	(1.067)	0.052

6	0.50	0.10	0.058	(1.063)	0.052	0.006
7	0.58	0.10	0.058	(1.058)	0.052	0.006
8	0.67	0.10	0.058	(1.054)	0.052	0.006
9	0.75	0.10	0.058	(1.050)	0.052	0.006
10	0.83	0.13	0.077	(1.046)	0.069	0.008
11	0.92	0.13	0.077	(1.042)	0.069	0.008
12	1.00	0.13	0.077	(1.038)	0.069	0.008
13	1.08	0.10	0.058	(1.034)	0.052	0.006
14	1.17	0.10	0.058	(1.030)	0.052	0.006
15	1.25	0.10	0.058	(1.026)	0.052	0.006
16	1.33	0.10	0.058	(1.021)	0.052	0.006
17	1.42	0.10	0.058	(1.017)	0.052	0.006
18	1.50	0.10	0.058	(1.013)	0.052	0.006
19	1.58	0.10	0.058	(1.009)	0.052	0.006
20	1.67	0.10	0.058	(1.005)	0.052	0.006
21	1.75	0.10	0.058	(1.001)	0.052	0.006
22	1.83	0.13	0.077	(0.997)	0.069	0.008
23	1.92	0.13	0.077	(0.993)	0.069	0.008
24	2.00	0.13	0.077	(0.989)	0.069	0.008
25	2.08	0.13	0.077	(0.985)	0.069	0.008
26	2.17	0.13	0.077	(0.981)	0.069	0.008
27	2.25	0.13	0.077	(0.977)	0.069	0.008
28	2.33	0.13	0.077	(0.973)	0.069	0.008
29	2.42	0.13	0.077	(0.969)	0.069	0.008
30	2.50	0.13	0.077	(0.965)	0.069	0.008
31	2.58	0.17	0.096	(0.961)	0.087	0.010
32	2.67	0.17	0.096	(0.957)	0.087	0.010
33	2.75	0.17	0.096	(0.953)	0.087	0.010
34	2.83	0.17	0.096	(0.950)	0.087	0.010
35	2.92	0.17	0.096	(0.946)	0.087	0.010
36	3.00	0.17	0.096	(0.942)	0.087	0.010
37	3.08	0.17	0.096	(0.938)	0.087	0.010
38	3.17	0.17	0.096	(0.934)	0.087	0.010
39	3.25	0.17	0.096	(0.930)	0.087	0.010
40	3.33	0.17	0.096	(0.926)	0.087	0.010
41	3.42	0.17	0.096	(0.922)	0.087	0.010
42	3.50	0.17	0.096	(0.918)	0.087	0.010
43	3.58	0.17	0.096	(0.915)	0.087	0.010
44	3.67	0.17	0.096	(0.911)	0.087	0.010
45	3.75	0.17	0.096	(0.907)	0.087	0.010
46	3.83	0.20	0.116	(0.903)	0.104	0.012
47	3.92	0.20	0.116	(0.899)	0.104	0.012
48	4.00	0.20	0.116	(0.895)	0.104	0.012
49	4.08	0.20	0.116	(0.892)	0.104	0.012
50	4.17	0.20	0.116	(0.888)	0.104	0.012
51	4.25	0.20	0.116	(0.884)	0.104	0.012
52	4.33	0.23	0.135	(0.880)	0.121	0.013
53	4.42	0.23	0.135	(0.877)	0.121	0.013
54	4.50	0.23	0.135	(0.873)	0.121	0.013
55	4.58	0.23	0.135	(0.869)	0.121	0.013
56	4.67	0.23	0.135	(0.865)	0.121	0.013
57	4.75	0.23	0.135	(0.862)	0.121	0.013
58	4.83	0.27	0.154	(0.858)	0.139	0.015
59	4.92	0.27	0.154	(0.854)	0.139	0.015
60	5.00	0.27	0.154	(0.850)	0.139	0.015
61	5.08	0.20	0.116	(0.847)	0.104	0.012
62	5.17	0.20	0.116	(0.843)	0.104	0.012
63	5.25	0.20	0.116	(0.839)	0.104	0.012
64	5.33	0.23	0.135	(0.836)	0.121	0.013
65	5.42	0.23	0.135	(0.832)	0.121	0.013

66	5.50	0.23	0.135	(0.828)	0.121	0.013
67	5.58	0.27	0.154	(0.825)	0.139	0.015
68	5.67	0.27	0.154	(0.821)	0.139	0.015
69	5.75	0.27	0.154	(0.818)	0.139	0.015
70	5.83	0.27	0.154	(0.814)	0.139	0.015
71	5.92	0.27	0.154	(0.810)	0.139	0.015
72	6.00	0.27	0.154	(0.807)	0.139	0.015
73	6.08	0.30	0.173	(0.803)	0.156	0.017
74	6.17	0.30	0.173	(0.800)	0.156	0.017
75	6.25	0.30	0.173	(0.796)	0.156	0.017
76	6.33	0.30	0.173	(0.792)	0.156	0.017
77	6.42	0.30	0.173	(0.789)	0.156	0.017
78	6.50	0.30	0.173	(0.785)	0.156	0.017
79	6.58	0.33	0.193	(0.782)	0.173	0.019
80	6.67	0.33	0.193	(0.778)	0.173	0.019
81	6.75	0.33	0.193	(0.775)	0.173	0.019
82	6.83	0.33	0.193	(0.771)	0.173	0.019
83	6.92	0.33	0.193	(0.768)	0.173	0.019
84	7.00	0.33	0.193	(0.764)	0.173	0.019
85	7.08	0.33	0.193	(0.761)	0.173	0.019
86	7.17	0.33	0.193	(0.757)	0.173	0.019
87	7.25	0.33	0.193	(0.754)	0.173	0.019
88	7.33	0.37	0.212	(0.751)	0.191	0.021
89	7.42	0.37	0.212	(0.747)	0.191	0.021
90	7.50	0.37	0.212	(0.744)	0.191	0.021
91	7.58	0.40	0.231	(0.740)	0.208	0.023
92	7.67	0.40	0.231	(0.737)	0.208	0.023
93	7.75	0.40	0.231	(0.733)	0.208	0.023
94	7.83	0.43	0.251	(0.730)	0.225	0.025
95	7.92	0.43	0.251	(0.727)	0.225	0.025
96	8.00	0.43	0.251	(0.723)	0.225	0.025
97	8.08	0.50	0.289	(0.720)	0.260	0.029
98	8.17	0.50	0.289	(0.717)	0.260	0.029
99	8.25	0.50	0.289	(0.713)	0.260	0.029
100	8.33	0.50	0.289	(0.710)	0.260	0.029
101	8.42	0.50	0.289	(0.707)	0.260	0.029
102	8.50	0.50	0.289	(0.703)	0.260	0.029
103	8.58	0.53	0.308	(0.700)	0.278	0.031
104	8.67	0.53	0.308	(0.697)	0.278	0.031
105	8.75	0.53	0.308	(0.693)	0.278	0.031
106	8.83	0.57	0.328	(0.690)	0.295	0.033
107	8.92	0.57	0.328	(0.687)	0.295	0.033
108	9.00	0.57	0.328	(0.684)	0.295	0.033
109	9.08	0.63	0.366	(0.680)	0.330	0.037
110	9.17	0.63	0.366	(0.677)	0.330	0.037
111	9.25	0.63	0.366	(0.674)	0.330	0.037
112	9.33	0.67	0.385	(0.671)	0.347	0.039
113	9.42	0.67	0.385	(0.668)	0.347	0.039
114	9.50	0.67	0.385	(0.664)	0.347	0.039
115	9.58	0.70	0.405	(0.661)	0.364	0.040
116	9.67	0.70	0.405	(0.658)	0.364	0.040
117	9.75	0.70	0.405	(0.655)	0.364	0.040
118	9.83	0.73	0.424	(0.652)	0.382	0.042
119	9.92	0.73	0.424	(0.649)	0.382	0.042
120	10.00	0.73	0.424	(0.645)	0.382	0.042
121	10.08	0.50	0.289	(0.642)	0.260	0.029
122	10.17	0.50	0.289	(0.639)	0.260	0.029
123	10.25	0.50	0.289	(0.636)	0.260	0.029
124	10.33	0.50	0.289	(0.633)	0.260	0.029
125	10.42	0.50	0.289	(0.630)	0.260	0.029

126	10.50	0.50	0.289	(0.627)	0.260	0.029
127	10.58	0.67	0.385	(0.624)	0.347	0.039
128	10.67	0.67	0.385	(0.621)	0.347	0.039
129	10.75	0.67	0.385	(0.618)	0.347	0.039
130	10.83	0.67	0.385	(0.615)	0.347	0.039
131	10.92	0.67	0.385	(0.612)	0.347	0.039
132	11.00	0.67	0.385	(0.609)	0.347	0.039
133	11.08	0.63	0.366	(0.606)	0.330	0.037
134	11.17	0.63	0.366	(0.603)	0.330	0.037
135	11.25	0.63	0.366	(0.600)	0.330	0.037
136	11.33	0.63	0.366	(0.597)	0.330	0.037
137	11.42	0.63	0.366	(0.594)	0.330	0.037
138	11.50	0.63	0.366	(0.591)	0.330	0.037
139	11.58	0.57	0.328	(0.588)	0.295	0.033
140	11.67	0.57	0.328	(0.585)	0.295	0.033
141	11.75	0.57	0.328	(0.582)	0.295	0.033
142	11.83	0.60	0.347	(0.579)	0.312	0.035
143	11.92	0.60	0.347	(0.576)	0.312	0.035
144	12.00	0.60	0.347	(0.573)	0.312	0.035
145	12.08	0.83	0.482	(0.571)	0.434	0.048
146	12.17	0.83	0.482	(0.568)	0.434	0.048
147	12.25	0.83	0.482	(0.565)	0.434	0.048
148	12.33	0.87	0.501	(0.562)	0.451	0.050
149	12.42	0.87	0.501	(0.559)	0.451	0.050
150	12.50	0.87	0.501	(0.556)	0.451	0.050
151	12.58	0.93	0.540	(0.554)	0.486	0.054
152	12.67	0.93	0.540	(0.551)	0.486	0.054
153	12.75	0.93	0.540	(0.548)	0.486	0.054
154	12.83	0.97	0.559	(0.545)	0.503	0.056
155	12.92	0.97	0.559	(0.542)	0.503	0.056
156	13.00	0.97	0.559	(0.540)	0.503	0.056
157	13.08	1.13	0.655	0.537 (0.590)		0.118
158	13.17	1.13	0.655	0.534 (0.590)		0.121
159	13.25	1.13	0.655	0.532 (0.590)		0.124
160	13.33	1.13	0.655	0.529 (0.590)		0.126
161	13.42	1.13	0.655	0.526 (0.590)		0.129
162	13.50	1.13	0.655	0.524 (0.590)		0.132
163	13.58	0.77	0.443	(0.521)	0.399	0.044
164	13.67	0.77	0.443	(0.518)	0.399	0.044
165	13.75	0.77	0.443	(0.516)	0.399	0.044
166	13.83	0.77	0.443	(0.513)	0.399	0.044
167	13.92	0.77	0.443	(0.510)	0.399	0.044
168	14.00	0.77	0.443	(0.508)	0.399	0.044
169	14.08	0.90	0.520	(0.505)	0.468	0.052
170	14.17	0.90	0.520	(0.503)	0.468	0.052
171	14.25	0.90	0.520	(0.500)	0.468	0.052
172	14.33	0.87	0.501	(0.497)	0.451	0.050
173	14.42	0.87	0.501	(0.495)	0.451	0.050
174	14.50	0.87	0.501	(0.492)	0.451	0.050
175	14.58	0.87	0.501	(0.490)	0.451	0.050
176	14.67	0.87	0.501	(0.487)	0.451	0.050
177	14.75	0.87	0.501	(0.485)	0.451	0.050
178	14.83	0.83	0.482	(0.482)	0.434	0.048
179	14.92	0.83	0.482	(0.480)	0.434	0.048
180	15.00	0.83	0.482	(0.477)	0.434	0.048
181	15.08	0.80	0.463	(0.475)	0.416	0.046
182	15.17	0.80	0.463	(0.472)	0.416	0.046
183	15.25	0.80	0.463	(0.470)	0.416	0.046
184	15.33	0.77	0.443	(0.468)	0.399	0.044
185	15.42	0.77	0.443	(0.465)	0.399	0.044

186	15.50	0.77	0.443	(0.463)	0.399	0.044
187	15.58	0.63	0.366	(0.460)	0.330	0.037
188	15.67	0.63	0.366	(0.458)	0.330	0.037
189	15.75	0.63	0.366	(0.456)	0.330	0.037
190	15.83	0.63	0.366	(0.453)	0.330	0.037
191	15.92	0.63	0.366	(0.451)	0.330	0.037
192	16.00	0.63	0.366	(0.449)	0.330	0.037
193	16.08	0.13	0.077	(0.447)	0.069	0.008
194	16.17	0.13	0.077	(0.444)	0.069	0.008
195	16.25	0.13	0.077	(0.442)	0.069	0.008
196	16.33	0.13	0.077	(0.440)	0.069	0.008
197	16.42	0.13	0.077	(0.437)	0.069	0.008
198	16.50	0.13	0.077	(0.435)	0.069	0.008
199	16.58	0.10	0.058	(0.433)	0.052	0.006
200	16.67	0.10	0.058	(0.431)	0.052	0.006
201	16.75	0.10	0.058	(0.429)	0.052	0.006
202	16.83	0.10	0.058	(0.426)	0.052	0.006
203	16.92	0.10	0.058	(0.424)	0.052	0.006
204	17.00	0.10	0.058	(0.422)	0.052	0.006
205	17.08	0.17	0.096	(0.420)	0.087	0.010
206	17.17	0.17	0.096	(0.418)	0.087	0.010
207	17.25	0.17	0.096	(0.416)	0.087	0.010
208	17.33	0.17	0.096	(0.414)	0.087	0.010
209	17.42	0.17	0.096	(0.412)	0.087	0.010
210	17.50	0.17	0.096	(0.410)	0.087	0.010
211	17.58	0.17	0.096	(0.408)	0.087	0.010
212	17.67	0.17	0.096	(0.406)	0.087	0.010
213	17.75	0.17	0.096	(0.404)	0.087	0.010
214	17.83	0.13	0.077	(0.402)	0.069	0.008
215	17.92	0.13	0.077	(0.400)	0.069	0.008
216	18.00	0.13	0.077	(0.398)	0.069	0.008
217	18.08	0.13	0.077	(0.396)	0.069	0.008
218	18.17	0.13	0.077	(0.394)	0.069	0.008
219	18.25	0.13	0.077	(0.392)	0.069	0.008
220	18.33	0.13	0.077	(0.390)	0.069	0.008
221	18.42	0.13	0.077	(0.388)	0.069	0.008
222	18.50	0.13	0.077	(0.386)	0.069	0.008
223	18.58	0.10	0.058	(0.384)	0.052	0.006
224	18.67	0.10	0.058	(0.382)	0.052	0.006
225	18.75	0.10	0.058	(0.380)	0.052	0.006
226	18.83	0.07	0.039	(0.379)	0.035	0.004
227	18.92	0.07	0.039	(0.377)	0.035	0.004
228	19.00	0.07	0.039	(0.375)	0.035	0.004
229	19.08	0.10	0.058	(0.373)	0.052	0.006
230	19.17	0.10	0.058	(0.372)	0.052	0.006
231	19.25	0.10	0.058	(0.370)	0.052	0.006
232	19.33	0.13	0.077	(0.368)	0.069	0.008
233	19.42	0.13	0.077	(0.366)	0.069	0.008
234	19.50	0.13	0.077	(0.365)	0.069	0.008
235	19.58	0.10	0.058	(0.363)	0.052	0.006
236	19.67	0.10	0.058	(0.361)	0.052	0.006
237	19.75	0.10	0.058	(0.360)	0.052	0.006
238	19.83	0.07	0.039	(0.358)	0.035	0.004
239	19.92	0.07	0.039	(0.356)	0.035	0.004
240	20.00	0.07	0.039	(0.355)	0.035	0.004
241	20.08	0.10	0.058	(0.353)	0.052	0.006
242	20.17	0.10	0.058	(0.352)	0.052	0.006
243	20.25	0.10	0.058	(0.350)	0.052	0.006
244	20.33	0.10	0.058	(0.349)	0.052	0.006
245	20.42	0.10	0.058	(0.347)	0.052	0.006

246	20.50	0.10	0.058	(0.346)	0.052	0.006
247	20.58	0.10	0.058	(0.344)	0.052	0.006
248	20.67	0.10	0.058	(0.343)	0.052	0.006
249	20.75	0.10	0.058	(0.341)	0.052	0.006
250	20.83	0.07	0.039	(0.340)	0.035	0.004
251	20.92	0.07	0.039	(0.339)	0.035	0.004
252	21.00	0.07	0.039	(0.337)	0.035	0.004
253	21.08	0.10	0.058	(0.336)	0.052	0.006
254	21.17	0.10	0.058	(0.335)	0.052	0.006
255	21.25	0.10	0.058	(0.333)	0.052	0.006
256	21.33	0.07	0.039	(0.332)	0.035	0.004
257	21.42	0.07	0.039	(0.331)	0.035	0.004
258	21.50	0.07	0.039	(0.330)	0.035	0.004
259	21.58	0.10	0.058	(0.328)	0.052	0.006
260	21.67	0.10	0.058	(0.327)	0.052	0.006
261	21.75	0.10	0.058	(0.326)	0.052	0.006
262	21.83	0.07	0.039	(0.325)	0.035	0.004
263	21.92	0.07	0.039	(0.324)	0.035	0.004
264	22.00	0.07	0.039	(0.323)	0.035	0.004
265	22.08	0.10	0.058	(0.322)	0.052	0.006
266	22.17	0.10	0.058	(0.321)	0.052	0.006
267	22.25	0.10	0.058	(0.320)	0.052	0.006
268	22.33	0.07	0.039	(0.319)	0.035	0.004
269	22.42	0.07	0.039	(0.318)	0.035	0.004
270	22.50	0.07	0.039	(0.317)	0.035	0.004
271	22.58	0.07	0.039	(0.316)	0.035	0.004
272	22.67	0.07	0.039	(0.315)	0.035	0.004
273	22.75	0.07	0.039	(0.314)	0.035	0.004
274	22.83	0.07	0.039	(0.313)	0.035	0.004
275	22.92	0.07	0.039	(0.312)	0.035	0.004
276	23.00	0.07	0.039	(0.312)	0.035	0.004
277	23.08	0.07	0.039	(0.311)	0.035	0.004
278	23.17	0.07	0.039	(0.310)	0.035	0.004
279	23.25	0.07	0.039	(0.310)	0.035	0.004
280	23.33	0.07	0.039	(0.309)	0.035	0.004
281	23.42	0.07	0.039	(0.308)	0.035	0.004
282	23.50	0.07	0.039	(0.308)	0.035	0.004
283	23.58	0.07	0.039	(0.307)	0.035	0.004
284	23.67	0.07	0.039	(0.307)	0.035	0.004
285	23.75	0.07	0.039	(0.306)	0.035	0.004
286	23.83	0.07	0.039	(0.306)	0.035	0.004
287	23.92	0.07	0.039	(0.306)	0.035	0.004
288	24.00	0.07	0.039	(0.306)	0.035	0.004

(Loss Rate Not Used)

Sum = 100.0 Sum = 6.1

Flood volume = Effective rainfall 0.51 (In)
times area 12.2 (Ac.) / [(In) / (Ft.)] = 0.5 (Ac.Ft)
Total soil loss = 4.31 (In)
Total soil loss = 4.389 (Ac.Ft)
Total rainfall = 4.82 (In)
Flood volume = 22710.9 Cubic Feet
Total soil loss = 191189.4 Cubic Feet

Peak flow rate of this hydrograph = 1.436 (CFS)

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24 - H O U R S T O R M
R u n o f f H y d r o g r a p h

Hydrograph in 5 Minute intervals ((CFS))

Time(h+m)	Volume	Ac.Ft	Q(CFS)	0	2.5	5.0	7.5	10.0
0+ 5	0.0000		0.01	Q				
0+10	0.0002		0.02	Q				
0+15	0.0004		0.03	Q				
0+20	0.0007		0.04	Q				
0+25	0.0011		0.05	Q				
0+30	0.0014		0.06	Q				
0+35	0.0019		0.06	Q				
0+40	0.0023		0.06	Q				
0+45	0.0027		0.06	Q				
0+50	0.0032		0.07	Q				
0+55	0.0038		0.08	Q				
1+ 0	0.0043		0.08	Q				
1+ 5	0.0049		0.08	Q				
1+10	0.0054		0.08	Q				
1+15	0.0060		0.07	Q				
1+20	0.0065		0.07	Q				
1+25	0.0070		0.07	Q				
1+30	0.0075		0.07	Q				
1+35	0.0080		0.07	Q				
1+40	0.0085		0.07	Q				
1+45	0.0090		0.07	Q				
1+50	0.0095		0.08	Q				
1+55	0.0101		0.08	Q				
2+ 0	0.0107		0.09	Q				
2+ 5	0.0113		0.09	Q				
2+10	0.0119		0.09	Q				
2+15	0.0126		0.09	Q				
2+20	0.0132		0.09	QV				
2+25	0.0138		0.09	QV				
2+30	0.0145		0.09	QV				
2+35	0.0151		0.10	QV				
2+40	0.0159		0.11	QV				
2+45	0.0166		0.11	QV				
2+50	0.0174		0.11	QV				
2+55	0.0182		0.11	QV				
3+ 0	0.0190		0.12	QV				
3+ 5	0.0198		0.12	QV				
3+10	0.0206		0.12	QV				
3+15	0.0214		0.12	QV				
3+20	0.0222		0.12	QV				
3+25	0.0230		0.12	QV				
3+30	0.0238		0.12	QV				
3+35	0.0247		0.12	QV				
3+40	0.0255		0.12	QV				
3+45	0.0263		0.12	Q V				
3+50	0.0271		0.12	Q V				
3+55	0.0280		0.13	Q V				
4+ 0	0.0290		0.13	Q V				
4+ 5	0.0299		0.14	Q V				
4+10	0.0309		0.14	Q V				
4+15	0.0318		0.14	Q V				
4+20	0.0328		0.14	Q V				
4+25	0.0339		0.15	Q V				
4+30	0.0349		0.16	Q V				
4+35	0.0360		0.16	Q V				
4+40	0.0371		0.16	Q V				

4+45	0.0383	0.16	Q	V
4+50	0.0394	0.17	Q	V
4+55	0.0406	0.18	Q	V
5+ 0	0.0419	0.18	Q	V
5+ 5	0.0431	0.18	Q	V
5+10	0.0442	0.16	Q	V
5+15	0.0452	0.15	Q	V
5+20	0.0463	0.15	Q	V
5+25	0.0474	0.16	Q	V
5+30	0.0485	0.16	Q	V
5+35	0.0497	0.17	Q	V
5+40	0.0509	0.18	Q	V
5+45	0.0522	0.18	Q	V
5+50	0.0534	0.18	Q	V
5+55	0.0547	0.19	Q	V
6+ 0	0.0560	0.19	Q	V
6+ 5	0.0573	0.19	Q	V
6+10	0.0587	0.20	Q	V
6+15	0.0601	0.20	Q	V
6+20	0.0615	0.21	Q	V
6+25	0.0629	0.21	Q	V
6+30	0.0644	0.21	Q	V
6+35	0.0659	0.21	Q	V
6+40	0.0674	0.22	Q	V
6+45	0.0690	0.23	Q	V
6+50	0.0706	0.23	Q	V
6+55	0.0722	0.23	Q	V
7+ 0	0.0738	0.23	Q	V
7+ 5	0.0754	0.23	Q	V
7+10	0.0770	0.24	Q	V
7+15	0.0786	0.24	Q	V
7+20	0.0803	0.24	Q	V
7+25	0.0820	0.25	Q	V
7+30	0.0837	0.25	Q	V
7+35	0.0855	0.26	Q	V
7+40	0.0874	0.27	Q	V
7+45	0.0893	0.27	Q	V
7+50	0.0912	0.28	Q	V
7+55	0.0932	0.29	Q	V
8+ 0	0.0952	0.30	Q	V
8+ 5	0.0973	0.31	Q	V
8+10	0.0996	0.33	Q	V
8+15	0.1019	0.33	Q	V
8+20	0.1042	0.34	Q	V
8+25	0.1066	0.34	Q	V
8+30	0.1090	0.35	Q	V
8+35	0.1114	0.35	Q	V
8+40	0.1139	0.36	Q	V
8+45	0.1165	0.37	Q	V
8+50	0.1190	0.38	Q	V
8+55	0.1217	0.39	Q	V
9+ 0	0.1244	0.39	Q	V
9+ 5	0.1272	0.40	Q	V
9+10	0.1301	0.42	Q	V
9+15	0.1330	0.43	Q	V
9+20	0.1361	0.44	Q	V
9+25	0.1392	0.45	Q	V
9+30	0.1423	0.46	Q	V
9+35	0.1455	0.47	Q	V
9+40	0.1488	0.48	Q	V

9+45	0.1522	0.48	Q	V					
9+50	0.1555	0.49	Q	V					
9+55	0.1590	0.50	Q	V					
10+ 0	0.1625	0.51	Q	V					
10+ 5	0.1659	0.49	Q	V					
10+10	0.1688	0.43	Q	V					
10+15	0.1716	0.41	Q	V					
10+20	0.1743	0.39	Q	V					
10+25	0.1770	0.38	Q	V					
10+30	0.1796	0.38	Q	V					
10+35	0.1823	0.39	Q	V					
10+40	0.1853	0.43	Q	V					
10+45	0.1883	0.45	Q	V					
10+50	0.1915	0.45	Q	V					
10+55	0.1946	0.46	Q	V					
11+ 0	0.1978	0.46	Q	V					
11+ 5	0.2010	0.46	Q	V					
11+10	0.2041	0.45	Q	V					
11+15	0.2072	0.45	Q	V					
11+20	0.2103	0.45	Q	V					
11+25	0.2134	0.45	Q	V					
11+30	0.2165	0.45	Q	V					
11+35	0.2196	0.45	Q	V					
11+40	0.2226	0.43	Q	V					
11+45	0.2255	0.42	Q	V					
11+50	0.2284	0.42	Q	V					
11+55	0.2313	0.43	Q	V					
12+ 0	0.2342	0.43	Q	V					
12+ 5	0.2374	0.45	Q	V					
12+10	0.2409	0.51	Q	V					
12+15	0.2446	0.54	Q	V					
12+20	0.2484	0.56	Q	V					
12+25	0.2524	0.57	Q	V					
12+30	0.2564	0.58	Q	V					
12+35	0.2605	0.60	Q	V					
12+40	0.2648	0.62	Q	V					
12+45	0.2692	0.63	Q	V					
12+50	0.2736	0.64	Q	V					
12+55	0.2782	0.66	Q	V					
13+ 0	0.2828	0.67	Q	V					
13+ 5	0.2882	0.79	Q	V					
13+10	0.2957	1.08	Q	V					
13+15	0.3041	1.22	Q	V					
13+20	0.3131	1.31	Q	V					
13+25	0.3226	1.38	Q	V					
13+30	0.3325	1.44	Q	V					
13+35	0.3416	1.32	Q	V					
13+40	0.3482	0.96	Q	V					
13+45	0.3538	0.82	Q	V					
13+50	0.3590	0.75	Q	V					
13+55	0.3638	0.70	Q	V					
14+ 0	0.3685	0.68	Q	V					
14+ 5	0.3731	0.67	Q	V					
14+10	0.3778	0.69	Q	V					
14+15	0.3825	0.68	Q	V					
14+20	0.3872	0.67	Q	V					
14+25	0.3917	0.65	Q	V					
14+30	0.3961	0.64	Q	V					
14+35	0.4004	0.63	Q	V					
14+40	0.4046	0.61	Q	V					

14+45	0.4089	0.61	Q	V
14+50	0.4131	0.61	Q	V
14+55	0.4172	0.60	Q	V
15+ 0	0.4214	0.60	Q	V
15+ 5	0.4255	0.60	Q	V
15+10	0.4295	0.59	Q	V
15+15	0.4335	0.58	Q	V
15+20	0.4375	0.58	Q	V
15+25	0.4414	0.56	Q	V
15+30	0.4453	0.56	Q	V
15+35	0.4490	0.54	Q	V
15+40	0.4525	0.50	Q	V
15+45	0.4558	0.49	Q	V
15+50	0.4591	0.48	Q	V
15+55	0.4624	0.47	Q	V
16+ 0	0.4656	0.47	Q	V
16+ 5	0.4684	0.41	Q	V
16+10	0.4703	0.28	Q	V
16+15	0.4719	0.22	Q	V
16+20	0.4732	0.19	Q	V
16+25	0.4743	0.17	Q	V
16+30	0.4754	0.15	Q	V
16+35	0.4763	0.14	Q	V
16+40	0.4771	0.12	Q	V
16+45	0.4778	0.11	Q	V
16+50	0.4785	0.10	Q	V
16+55	0.4791	0.09	Q	V
17+ 0	0.4797	0.08	Q	V
17+ 5	0.4803	0.09	Q	V
17+10	0.4810	0.10	Q	V
17+15	0.4817	0.10	Q	V
17+20	0.4824	0.11	Q	V
17+25	0.4832	0.11	Q	V
17+30	0.4840	0.11	Q	V
17+35	0.4847	0.11	Q	V
17+40	0.4855	0.11	Q	V
17+45	0.4863	0.12	Q	V
17+50	0.4871	0.11	Q	V
17+55	0.4878	0.10	Q	V
18+ 0	0.4885	0.10	Q	V
18+ 5	0.4892	0.10	Q	V
18+10	0.4899	0.10	Q	V
18+15	0.4906	0.10	Q	V
18+20	0.4913	0.10	Q	V
18+25	0.4919	0.10	Q	V
18+30	0.4926	0.10	Q	V
18+35	0.4932	0.09	Q	V
18+40	0.4938	0.08	Q	V
18+45	0.4944	0.08	Q	V
18+50	0.4949	0.07	Q	V
18+55	0.4953	0.06	Q	V
19+ 0	0.4957	0.06	Q	V
19+ 5	0.4961	0.06	Q	V
19+10	0.4966	0.07	Q	V
19+15	0.4971	0.07	Q	V
19+20	0.4976	0.07	Q	V
19+25	0.4981	0.08	Q	V
19+30	0.4987	0.09	Q	V
19+35	0.4993	0.08	Q	V
19+40	0.4998	0.08	Q	V

19+45	0.5003	0.07	Q	V
19+50	0.5008	0.07	Q	V
19+55	0.5012	0.06	Q	V
20+ 0	0.5016	0.06	Q	V
20+ 5	0.5020	0.06	Q	V
20+10	0.5025	0.07	Q	V
20+15	0.5029	0.07	Q	V
20+20	0.5034	0.07	Q	V
20+25	0.5039	0.07	Q	V
20+30	0.5044	0.07	Q	V
20+35	0.5049	0.07	Q	V
20+40	0.5054	0.07	Q	V
20+45	0.5058	0.07	Q	V
20+50	0.5063	0.07	Q	V
20+55	0.5067	0.06	Q	V
21+ 0	0.5071	0.05	Q	V
21+ 5	0.5075	0.06	Q	V
21+10	0.5079	0.06	Q	V
21+15	0.5084	0.07	Q	V
21+20	0.5088	0.06	Q	V
21+25	0.5092	0.06	Q	V
21+30	0.5096	0.05	Q	V
21+35	0.5100	0.06	Q	V
21+40	0.5104	0.06	Q	V
21+45	0.5108	0.07	Q	V
21+50	0.5113	0.06	Q	V
21+55	0.5117	0.06	Q	V
22+ 0	0.5120	0.05	Q	V
22+ 5	0.5124	0.06	Q	V
22+10	0.5128	0.06	Q	V
22+15	0.5133	0.07	Q	V
22+20	0.5137	0.06	Q	V
22+25	0.5141	0.06	Q	V
22+30	0.5145	0.05	Q	V
22+35	0.5148	0.05	Q	V
22+40	0.5152	0.05	Q	V
22+45	0.5155	0.05	Q	V
22+50	0.5159	0.05	Q	V
22+55	0.5162	0.05	Q	V
23+ 0	0.5165	0.05	Q	V
23+ 5	0.5169	0.05	Q	V
23+10	0.5172	0.05	Q	V
23+15	0.5175	0.05	Q	V
23+20	0.5179	0.05	Q	V
23+25	0.5182	0.05	Q	V
23+30	0.5185	0.05	Q	V
23+35	0.5189	0.05	Q	V
23+40	0.5192	0.05	Q	V
23+45	0.5195	0.05	Q	V
23+50	0.5198	0.05	Q	V
23+55	0.5202	0.05	Q	V
24+ 0	0.5205	0.05	Q	V
24+ 5	0.5208	0.04	Q	V
24+10	0.5209	0.02	Q	V
24+15	0.5210	0.02	Q	V
24+20	0.5211	0.01	Q	V
24+25	0.5212	0.01	Q	V
24+30	0.5212	0.01	Q	V
24+35	0.5213	0.01	Q	V
24+40	0.5213	0.00	Q	V

24+45	0.5213	0.00	Q				V
24+50	0.5213	0.00	Q				V
24+55	0.5214	0.00	Q				V
25+ 0	0.5214	0.00	Q				V
25+ 5	0.5214	0.00	Q				V

APPENDIX C: POST-PROJECT UNIT HYDROGRAPH METHOD HYDROLOGY

APPENDIX C.1: UNIT HYDROGRAPH ANALYSIS, AREAS “A”

2-YEAR, 24-HOUR STORM EVENT

Unit Hydrograph Analysis

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Study date 02/16/23 File: ara242.out

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Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978

Program License Serial Number 6279

English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used

English Units used in output format

302.01.21
AREA A
2 YR STORM
POST PROJECT

Drainage Area = 12.23(Ac.) = 0.019 Sq. Mi.
Drainage Area for Depth-Area Areal Adjustment = 12.23(Ac.) = 0.019
Sq. Mi.
Length along longest watercourse = 1105.90(Ft.)
Length along longest watercourse measured to centroid = 610.60(Ft.)
Length along longest watercourse = 0.209 Mi.
Length along longest watercourse measured to centroid = 0.116 Mi.
Difference in elevation = 9.50(Ft.)
Slope along watercourse = 45.3567 Ft./Mi.
Average Manning's 'N' = 0.025
Lag time = 0.071 Hr.
Lag time = 4.24 Min.
25% of lag time = 1.06 Min.
40% of lag time = 1.70 Min.
Unit time = 5.00 Min.
Duration of storm = 24 Hour(s)
User Entered Base Flow = 0.00(CFS)

2 YEAR Area rainfall data:

Area(Ac.) [1]	Rainfall(In) [2]	Weighting[1*2]
12.23	2.79	34.12

100 YEAR Area rainfall data:

Area(Ac.) [1]	Rainfall(In) [2]	Weighting[1*2]
12.23	7.72	94.42

STORM EVENT (YEAR) = 2.00
 Area Averaged 2-Year Rainfall = 2.790 (In)
 Area Averaged 100-Year Rainfall = 7.720 (In)

Point rain (area averaged) = 2.790 (In)
 Areal adjustment factor = 100.00 %
 Adjusted average point rain = 2.790 (In)

Sub-Area Data:

Area (Ac.)	Runoff Index	Impervious %
12.230	33.60	0.350
Total Area Entered = 12.23 (Ac.)		

RI	RI	Infil. Rate	Impervious	Adj. Infil. Rate	Area%	F
AMC2	AMC-2	(In/Hr)	(Dec.%)	(In/Hr)	(Dec.)	(In/Hr)
33.6	33.6	0.728	0.350	0.498	1.000	0.498
						Sum (F) = 0.498

Area averaged mean soil loss (F) (In/Hr) = 0.498
 Minimum soil loss rate ((In/Hr)) = 0.249
 (for 24 hour storm duration)
 Soil low loss rate (decimal) = 0.620

 U n i t H y d r o g r a p h
 MOUNTAIN S-Curve

Unit Hydrograph Data

Unit time period (hrs)	Time % of lag	Distribution Graph %	Unit Hydrograph (CFS)
1	0.083	117.878	27.331
2	0.167	235.755	38.091
3	0.250	353.633	12.913
4	0.333	471.511	6.898
5	0.417	589.388	4.601
6	0.500	707.266	3.392
7	0.583	825.144	2.550
8	0.667	943.021	2.137
9	0.750	1060.899	2.087
		Sum = 100.000	Sum= 12.326

The following loss rate calculations reflect use of the minimum calculated loss rate subtracted from the Storm Rain to produce the maximum Effective Rain value

Unit Time (Hr.)	Pattern Percent	Storm Rain (In/Hr)	Loss rate (In./Hr) Max Low	Effective (In/Hr)
1	0.08	0.07	(0.884) 0.014	0.008
2	0.17	0.07	(0.880) 0.014	0.008
3	0.25	0.07	(0.877) 0.014	0.008
4	0.33	0.10	(0.873) 0.021	0.013
5	0.42	0.10	(0.870) 0.021	0.013
6	0.50	0.10	(0.867) 0.021	0.013
7	0.58	0.10	(0.863) 0.021	0.013
8	0.67	0.10	(0.860) 0.021	0.013
9	0.75	0.10	(0.856) 0.021	0.013
10	0.83	0.13	(0.853) 0.028	0.017

11	0.92	0.13	0.045	(0.850)	0.028	0.017
12	1.00	0.13	0.045	(0.846)	0.028	0.017
13	1.08	0.10	0.033	(0.843)	0.021	0.013
14	1.17	0.10	0.033	(0.840)	0.021	0.013
15	1.25	0.10	0.033	(0.836)	0.021	0.013
16	1.33	0.10	0.033	(0.833)	0.021	0.013
17	1.42	0.10	0.033	(0.830)	0.021	0.013
18	1.50	0.10	0.033	(0.826)	0.021	0.013
19	1.58	0.10	0.033	(0.823)	0.021	0.013
20	1.67	0.10	0.033	(0.820)	0.021	0.013
21	1.75	0.10	0.033	(0.816)	0.021	0.013
22	1.83	0.13	0.045	(0.813)	0.028	0.017
23	1.92	0.13	0.045	(0.810)	0.028	0.017
24	2.00	0.13	0.045	(0.807)	0.028	0.017
25	2.08	0.13	0.045	(0.803)	0.028	0.017
26	2.17	0.13	0.045	(0.800)	0.028	0.017
27	2.25	0.13	0.045	(0.797)	0.028	0.017
28	2.33	0.13	0.045	(0.794)	0.028	0.017
29	2.42	0.13	0.045	(0.790)	0.028	0.017
30	2.50	0.13	0.045	(0.787)	0.028	0.017
31	2.58	0.17	0.056	(0.784)	0.035	0.021
32	2.67	0.17	0.056	(0.781)	0.035	0.021
33	2.75	0.17	0.056	(0.778)	0.035	0.021
34	2.83	0.17	0.056	(0.774)	0.035	0.021
35	2.92	0.17	0.056	(0.771)	0.035	0.021
36	3.00	0.17	0.056	(0.768)	0.035	0.021
37	3.08	0.17	0.056	(0.765)	0.035	0.021
38	3.17	0.17	0.056	(0.762)	0.035	0.021
39	3.25	0.17	0.056	(0.758)	0.035	0.021
40	3.33	0.17	0.056	(0.755)	0.035	0.021
41	3.42	0.17	0.056	(0.752)	0.035	0.021
42	3.50	0.17	0.056	(0.749)	0.035	0.021
43	3.58	0.17	0.056	(0.746)	0.035	0.021
44	3.67	0.17	0.056	(0.743)	0.035	0.021
45	3.75	0.17	0.056	(0.740)	0.035	0.021
46	3.83	0.20	0.067	(0.736)	0.042	0.025
47	3.92	0.20	0.067	(0.733)	0.042	0.025
48	4.00	0.20	0.067	(0.730)	0.042	0.025
49	4.08	0.20	0.067	(0.727)	0.042	0.025
50	4.17	0.20	0.067	(0.724)	0.042	0.025
51	4.25	0.20	0.067	(0.721)	0.042	0.025
52	4.33	0.23	0.078	(0.718)	0.048	0.030
53	4.42	0.23	0.078	(0.715)	0.048	0.030
54	4.50	0.23	0.078	(0.712)	0.048	0.030
55	4.58	0.23	0.078	(0.709)	0.048	0.030
56	4.67	0.23	0.078	(0.706)	0.048	0.030
57	4.75	0.23	0.078	(0.703)	0.048	0.030
58	4.83	0.27	0.089	(0.700)	0.055	0.034
59	4.92	0.27	0.089	(0.697)	0.055	0.034
60	5.00	0.27	0.089	(0.694)	0.055	0.034
61	5.08	0.20	0.067	(0.691)	0.042	0.025
62	5.17	0.20	0.067	(0.688)	0.042	0.025
63	5.25	0.20	0.067	(0.685)	0.042	0.025
64	5.33	0.23	0.078	(0.682)	0.048	0.030
65	5.42	0.23	0.078	(0.679)	0.048	0.030
66	5.50	0.23	0.078	(0.676)	0.048	0.030
67	5.58	0.27	0.089	(0.673)	0.055	0.034
68	5.67	0.27	0.089	(0.670)	0.055	0.034
69	5.75	0.27	0.089	(0.667)	0.055	0.034
70	5.83	0.27	0.089	(0.664)	0.055	0.034

71	5.92	0.27	0.089	(0.661)	0.055	0.034
72	6.00	0.27	0.089	(0.658)	0.055	0.034
73	6.08	0.30	0.100	(0.655)	0.062	0.038
74	6.17	0.30	0.100	(0.652)	0.062	0.038
75	6.25	0.30	0.100	(0.649)	0.062	0.038
76	6.33	0.30	0.100	(0.646)	0.062	0.038
77	6.42	0.30	0.100	(0.643)	0.062	0.038
78	6.50	0.30	0.100	(0.640)	0.062	0.038
79	6.58	0.33	0.112	(0.638)	0.069	0.042
80	6.67	0.33	0.112	(0.635)	0.069	0.042
81	6.75	0.33	0.112	(0.632)	0.069	0.042
82	6.83	0.33	0.112	(0.629)	0.069	0.042
83	6.92	0.33	0.112	(0.626)	0.069	0.042
84	7.00	0.33	0.112	(0.623)	0.069	0.042
85	7.08	0.33	0.112	(0.620)	0.069	0.042
86	7.17	0.33	0.112	(0.618)	0.069	0.042
87	7.25	0.33	0.112	(0.615)	0.069	0.042
88	7.33	0.37	0.123	(0.612)	0.076	0.047
89	7.42	0.37	0.123	(0.609)	0.076	0.047
90	7.50	0.37	0.123	(0.606)	0.076	0.047
91	7.58	0.40	0.134	(0.604)	0.083	0.051
92	7.67	0.40	0.134	(0.601)	0.083	0.051
93	7.75	0.40	0.134	(0.598)	0.083	0.051
94	7.83	0.43	0.145	(0.595)	0.090	0.055
95	7.92	0.43	0.145	(0.593)	0.090	0.055
96	8.00	0.43	0.145	(0.590)	0.090	0.055
97	8.08	0.50	0.167	(0.587)	0.104	0.064
98	8.17	0.50	0.167	(0.584)	0.104	0.064
99	8.25	0.50	0.167	(0.582)	0.104	0.064
100	8.33	0.50	0.167	(0.579)	0.104	0.064
101	8.42	0.50	0.167	(0.576)	0.104	0.064
102	8.50	0.50	0.167	(0.574)	0.104	0.064
103	8.58	0.53	0.179	(0.571)	0.111	0.068
104	8.67	0.53	0.179	(0.568)	0.111	0.068
105	8.75	0.53	0.179	(0.565)	0.111	0.068
106	8.83	0.57	0.190	(0.563)	0.118	0.072
107	8.92	0.57	0.190	(0.560)	0.118	0.072
108	9.00	0.57	0.190	(0.557)	0.118	0.072
109	9.08	0.63	0.212	(0.555)	0.131	0.081
110	9.17	0.63	0.212	(0.552)	0.131	0.081
111	9.25	0.63	0.212	(0.550)	0.131	0.081
112	9.33	0.67	0.223	(0.547)	0.138	0.085
113	9.42	0.67	0.223	(0.544)	0.138	0.085
114	9.50	0.67	0.223	(0.542)	0.138	0.085
115	9.58	0.70	0.234	(0.539)	0.145	0.089
116	9.67	0.70	0.234	(0.537)	0.145	0.089
117	9.75	0.70	0.234	(0.534)	0.145	0.089
118	9.83	0.73	0.246	(0.531)	0.152	0.093
119	9.92	0.73	0.246	(0.529)	0.152	0.093
120	10.00	0.73	0.246	(0.526)	0.152	0.093
121	10.08	0.50	0.167	(0.524)	0.104	0.064
122	10.17	0.50	0.167	(0.521)	0.104	0.064
123	10.25	0.50	0.167	(0.519)	0.104	0.064
124	10.33	0.50	0.167	(0.516)	0.104	0.064
125	10.42	0.50	0.167	(0.514)	0.104	0.064
126	10.50	0.50	0.167	(0.511)	0.104	0.064
127	10.58	0.67	0.223	(0.509)	0.138	0.085
128	10.67	0.67	0.223	(0.506)	0.138	0.085
129	10.75	0.67	0.223	(0.504)	0.138	0.085
130	10.83	0.67	0.223	(0.501)	0.138	0.085

131	10.92	0.67	0.223	(0.499)	0.138	0.085
132	11.00	0.67	0.223	(0.496)	0.138	0.085
133	11.08	0.63	0.212	(0.494)	0.131	0.081
134	11.17	0.63	0.212	(0.491)	0.131	0.081
135	11.25	0.63	0.212	(0.489)	0.131	0.081
136	11.33	0.63	0.212	(0.487)	0.131	0.081
137	11.42	0.63	0.212	(0.484)	0.131	0.081
138	11.50	0.63	0.212	(0.482)	0.131	0.081
139	11.58	0.57	0.190	(0.479)	0.118	0.072
140	11.67	0.57	0.190	(0.477)	0.118	0.072
141	11.75	0.57	0.190	(0.475)	0.118	0.072
142	11.83	0.60	0.201	(0.472)	0.125	0.076
143	11.92	0.60	0.201	(0.470)	0.125	0.076
144	12.00	0.60	0.201	(0.468)	0.125	0.076
145	12.08	0.83	0.279	(0.465)	0.173	0.106
146	12.17	0.83	0.279	(0.463)	0.173	0.106
147	12.25	0.83	0.279	(0.461)	0.173	0.106
148	12.33	0.87	0.290	(0.458)	0.180	0.110
149	12.42	0.87	0.290	(0.456)	0.180	0.110
150	12.50	0.87	0.290	(0.454)	0.180	0.110
151	12.58	0.93	0.312	(0.451)	0.194	0.119
152	12.67	0.93	0.312	(0.449)	0.194	0.119
153	12.75	0.93	0.312	(0.447)	0.194	0.119
154	12.83	0.97	0.324	(0.445)	0.201	0.123
155	12.92	0.97	0.324	(0.442)	0.201	0.123
156	13.00	0.97	0.324	(0.440)	0.201	0.123
157	13.08	1.13	0.379	(0.438)	0.235	0.144
158	13.17	1.13	0.379	(0.436)	0.235	0.144
159	13.25	1.13	0.379	(0.433)	0.235	0.144
160	13.33	1.13	0.379	(0.431)	0.235	0.144
161	13.42	1.13	0.379	(0.429)	0.235	0.144
162	13.50	1.13	0.379	(0.427)	0.235	0.144
163	13.58	0.77	0.257	(0.425)	0.159	0.098
164	13.67	0.77	0.257	(0.423)	0.159	0.098
165	13.75	0.77	0.257	(0.420)	0.159	0.098
166	13.83	0.77	0.257	(0.418)	0.159	0.098
167	13.92	0.77	0.257	(0.416)	0.159	0.098
168	14.00	0.77	0.257	(0.414)	0.159	0.098
169	14.08	0.90	0.301	(0.412)	0.187	0.114
170	14.17	0.90	0.301	(0.410)	0.187	0.114
171	14.25	0.90	0.301	(0.408)	0.187	0.114
172	14.33	0.87	0.290	(0.406)	0.180	0.110
173	14.42	0.87	0.290	(0.404)	0.180	0.110
174	14.50	0.87	0.290	(0.401)	0.180	0.110
175	14.58	0.87	0.290	(0.399)	0.180	0.110
176	14.67	0.87	0.290	(0.397)	0.180	0.110
177	14.75	0.87	0.290	(0.395)	0.180	0.110
178	14.83	0.83	0.279	(0.393)	0.173	0.106
179	14.92	0.83	0.279	(0.391)	0.173	0.106
180	15.00	0.83	0.279	(0.389)	0.173	0.106
181	15.08	0.80	0.268	(0.387)	0.166	0.102
182	15.17	0.80	0.268	(0.385)	0.166	0.102
183	15.25	0.80	0.268	(0.383)	0.166	0.102
184	15.33	0.77	0.257	(0.381)	0.159	0.098
185	15.42	0.77	0.257	(0.379)	0.159	0.098
186	15.50	0.77	0.257	(0.377)	0.159	0.098
187	15.58	0.63	0.212	(0.376)	0.131	0.081
188	15.67	0.63	0.212	(0.374)	0.131	0.081
189	15.75	0.63	0.212	(0.372)	0.131	0.081
190	15.83	0.63	0.212	(0.370)	0.131	0.081

191	15.92	0.63	0.212	(0.368)	0.131	0.081
192	16.00	0.63	0.212	(0.366)	0.131	0.081
193	16.08	0.13	0.045	(0.364)	0.028	0.017
194	16.17	0.13	0.045	(0.362)	0.028	0.017
195	16.25	0.13	0.045	(0.360)	0.028	0.017
196	16.33	0.13	0.045	(0.359)	0.028	0.017
197	16.42	0.13	0.045	(0.357)	0.028	0.017
198	16.50	0.13	0.045	(0.355)	0.028	0.017
199	16.58	0.10	0.033	(0.353)	0.021	0.013
200	16.67	0.10	0.033	(0.351)	0.021	0.013
201	16.75	0.10	0.033	(0.350)	0.021	0.013
202	16.83	0.10	0.033	(0.348)	0.021	0.013
203	16.92	0.10	0.033	(0.346)	0.021	0.013
204	17.00	0.10	0.033	(0.344)	0.021	0.013
205	17.08	0.17	0.056	(0.343)	0.035	0.021
206	17.17	0.17	0.056	(0.341)	0.035	0.021
207	17.25	0.17	0.056	(0.339)	0.035	0.021
208	17.33	0.17	0.056	(0.337)	0.035	0.021
209	17.42	0.17	0.056	(0.336)	0.035	0.021
210	17.50	0.17	0.056	(0.334)	0.035	0.021
211	17.58	0.17	0.056	(0.332)	0.035	0.021
212	17.67	0.17	0.056	(0.331)	0.035	0.021
213	17.75	0.17	0.056	(0.329)	0.035	0.021
214	17.83	0.13	0.045	(0.327)	0.028	0.017
215	17.92	0.13	0.045	(0.326)	0.028	0.017
216	18.00	0.13	0.045	(0.324)	0.028	0.017
217	18.08	0.13	0.045	(0.323)	0.028	0.017
218	18.17	0.13	0.045	(0.321)	0.028	0.017
219	18.25	0.13	0.045	(0.319)	0.028	0.017
220	18.33	0.13	0.045	(0.318)	0.028	0.017
221	18.42	0.13	0.045	(0.316)	0.028	0.017
222	18.50	0.13	0.045	(0.315)	0.028	0.017
223	18.58	0.10	0.033	(0.313)	0.021	0.013
224	18.67	0.10	0.033	(0.312)	0.021	0.013
225	18.75	0.10	0.033	(0.310)	0.021	0.013
226	18.83	0.07	0.022	(0.309)	0.014	0.008
227	18.92	0.07	0.022	(0.307)	0.014	0.008
228	19.00	0.07	0.022	(0.306)	0.014	0.008
229	19.08	0.10	0.033	(0.304)	0.021	0.013
230	19.17	0.10	0.033	(0.303)	0.021	0.013
231	19.25	0.10	0.033	(0.302)	0.021	0.013
232	19.33	0.13	0.045	(0.300)	0.028	0.017
233	19.42	0.13	0.045	(0.299)	0.028	0.017
234	19.50	0.13	0.045	(0.297)	0.028	0.017
235	19.58	0.10	0.033	(0.296)	0.021	0.013
236	19.67	0.10	0.033	(0.295)	0.021	0.013
237	19.75	0.10	0.033	(0.293)	0.021	0.013
238	19.83	0.07	0.022	(0.292)	0.014	0.008
239	19.92	0.07	0.022	(0.291)	0.014	0.008
240	20.00	0.07	0.022	(0.289)	0.014	0.008
241	20.08	0.10	0.033	(0.288)	0.021	0.013
242	20.17	0.10	0.033	(0.287)	0.021	0.013
243	20.25	0.10	0.033	(0.286)	0.021	0.013
244	20.33	0.10	0.033	(0.284)	0.021	0.013
245	20.42	0.10	0.033	(0.283)	0.021	0.013
246	20.50	0.10	0.033	(0.282)	0.021	0.013
247	20.58	0.10	0.033	(0.281)	0.021	0.013
248	20.67	0.10	0.033	(0.280)	0.021	0.013
249	20.75	0.10	0.033	(0.278)	0.021	0.013
250	20.83	0.07	0.022	(0.277)	0.014	0.008

251	20.92	0.07	0.022	(0.276)	0.014	0.008
252	21.00	0.07	0.022	(0.275)	0.014	0.008
253	21.08	0.10	0.033	(0.274)	0.021	0.013
254	21.17	0.10	0.033	(0.273)	0.021	0.013
255	21.25	0.10	0.033	(0.272)	0.021	0.013
256	21.33	0.07	0.022	(0.271)	0.014	0.008
257	21.42	0.07	0.022	(0.270)	0.014	0.008
258	21.50	0.07	0.022	(0.269)	0.014	0.008
259	21.58	0.10	0.033	(0.268)	0.021	0.013
260	21.67	0.10	0.033	(0.267)	0.021	0.013
261	21.75	0.10	0.033	(0.266)	0.021	0.013
262	21.83	0.07	0.022	(0.265)	0.014	0.008
263	21.92	0.07	0.022	(0.264)	0.014	0.008
264	22.00	0.07	0.022	(0.263)	0.014	0.008
265	22.08	0.10	0.033	(0.262)	0.021	0.013
266	22.17	0.10	0.033	(0.261)	0.021	0.013
267	22.25	0.10	0.033	(0.261)	0.021	0.013
268	22.33	0.07	0.022	(0.260)	0.014	0.008
269	22.42	0.07	0.022	(0.259)	0.014	0.008
270	22.50	0.07	0.022	(0.258)	0.014	0.008
271	22.58	0.07	0.022	(0.257)	0.014	0.008
272	22.67	0.07	0.022	(0.257)	0.014	0.008
273	22.75	0.07	0.022	(0.256)	0.014	0.008
274	22.83	0.07	0.022	(0.255)	0.014	0.008
275	22.92	0.07	0.022	(0.255)	0.014	0.008
276	23.00	0.07	0.022	(0.254)	0.014	0.008
277	23.08	0.07	0.022	(0.254)	0.014	0.008
278	23.17	0.07	0.022	(0.253)	0.014	0.008
279	23.25	0.07	0.022	(0.252)	0.014	0.008
280	23.33	0.07	0.022	(0.252)	0.014	0.008
281	23.42	0.07	0.022	(0.251)	0.014	0.008
282	23.50	0.07	0.022	(0.251)	0.014	0.008
283	23.58	0.07	0.022	(0.251)	0.014	0.008
284	23.67	0.07	0.022	(0.250)	0.014	0.008
285	23.75	0.07	0.022	(0.250)	0.014	0.008
286	23.83	0.07	0.022	(0.250)	0.014	0.008
287	23.92	0.07	0.022	(0.249)	0.014	0.008
288	24.00	0.07	0.022	(0.249)	0.014	0.008

(Loss Rate Not Used)

Sum = 100.0

Sum = 12.7

Flood volume = Effective rainfall 1.06(In)
times area 12.2(Ac.) / [(In)/(Ft.)] = 1.1(Ac.Ft)
Total soil loss = 1.73(In)
Total soil loss = 1.763(Ac.Ft)
Total rainfall = 2.79(In)
Flood volume = 47066.3 Cubic Feet
Total soil loss = 76792.5 Cubic Feet

Peak flow rate of this hydrograph = 1.760(CFS)

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24 - H O U R S T O R M
R u n o f f H y d r o g r a p h

Hydrograph in 5 Minute intervals ((CFS))

Time(h+m)	Volume Ac.Ft	Q(CFS)	0	2.5	5.0	7.5	10.0
0+ 5	0.0002	0.03	Q				

0+10	0.0007	0.07	Q
0+15	0.0012	0.08	Q
0+20	0.0019	0.10	Q
0+25	0.0028	0.13	Q
0+30	0.0038	0.14	Q
0+35	0.0048	0.14	Q
0+40	0.0058	0.15	Q
0+45	0.0069	0.15	Q
0+50	0.0080	0.17	Q
0+55	0.0093	0.19	Q
1+ 0	0.0107	0.20	Q
1+ 5	0.0120	0.19	Q
1+10	0.0132	0.17	Q
1+15	0.0143	0.16	Q
1+20	0.0154	0.16	Q
1+25	0.0165	0.16	Q
1+30	0.0176	0.16	Q
1+35	0.0187	0.16	Q
1+40	0.0198	0.16	Q
1+45	0.0209	0.16	Q
1+50	0.0221	0.17	Q
1+55	0.0234	0.19	Q
2+ 0	0.0247	0.20	Q
2+ 5	0.0261	0.20	Q
2+10	0.0275	0.20	QV
2+15	0.0290	0.21	QV
2+20	0.0304	0.21	QV
2+25	0.0318	0.21	QV
2+30	0.0333	0.21	QV
2+35	0.0348	0.22	QV
2+40	0.0365	0.24	QV
2+45	0.0382	0.25	Q
2+50	0.0399	0.25	Q
2+55	0.0417	0.26	Q
3+ 0	0.0435	0.26	Q
3+ 5	0.0453	0.26	Q
3+10	0.0471	0.26	Q
3+15	0.0489	0.26	Q
3+20	0.0507	0.26	Q
3+25	0.0525	0.26	Q
3+30	0.0543	0.26	QV
3+35	0.0561	0.26	QV
3+40	0.0579	0.26	QV
3+45	0.0597	0.26	QV
3+50	0.0616	0.28	QV
3+55	0.0636	0.30	QV
4+ 0	0.0657	0.30	QV
4+ 5	0.0678	0.31	QV
4+10	0.0699	0.31	QV
4+15	0.0721	0.31	QV
4+20	0.0743	0.33	QV
4+25	0.0767	0.35	QV
4+30	0.0791	0.35	QV
4+35	0.0816	0.36	Q V
4+40	0.0841	0.36	Q V
4+45	0.0866	0.36	Q V
4+50	0.0892	0.38	Q V
4+55	0.0919	0.40	Q V
5+ 0	0.0947	0.41	Q V
5+ 5	0.0974	0.38	Q V

5+10	0.0997	0.34	Q	V			
5+15	0.1020	0.33	Q	V			
5+20	0.1044	0.34	Q	V			
5+25	0.1068	0.36	Q	V			
5+30	0.1093	0.36	Q	V			
5+35	0.1119	0.38	Q	V			
5+40	0.1147	0.40	Q	V			
5+45	0.1174	0.40	Q	V			
5+50	0.1203	0.41	Q	V			
5+55	0.1231	0.41	Q	V			
6+ 0	0.1260	0.41	Q	V			
6+ 5	0.1289	0.43	Q	V			
6+10	0.1320	0.45	Q	V			
6+15	0.1352	0.46	Q	V			
6+20	0.1384	0.46	Q	V			
6+25	0.1416	0.47	Q	V			
6+30	0.1448	0.47	Q	V			
6+35	0.1481	0.48	Q	V			
6+40	0.1516	0.50	Q	V			
6+45	0.1551	0.51	Q	V			
6+50	0.1587	0.52	Q	V			
6+55	0.1622	0.52	Q	V			
7+ 0	0.1658	0.52	Q	V			
7+ 5	0.1694	0.52	Q	V			
7+10	0.1730	0.52	Q	V			
7+15	0.1766	0.52	Q	V			
7+20	0.1803	0.54	Q	V			
7+25	0.1841	0.56	Q	V			
7+30	0.1880	0.56	Q	V			
7+35	0.1920	0.58	Q	V			
7+40	0.1962	0.60	Q	V			
7+45	0.2004	0.61	Q	V			
7+50	0.2048	0.63	Q	V			
7+55	0.2093	0.66	Q	V			
8+ 0	0.2138	0.66	Q	V			
8+ 5	0.2187	0.70	Q	V			
8+10	0.2238	0.74	Q	V			
8+15	0.2290	0.76	Q	V			
8+20	0.2343	0.77	Q	V			
8+25	0.2396	0.77	Q	V			
8+30	0.2449	0.78	Q	V			
8+35	0.2504	0.79	Q	V			
8+40	0.2560	0.82	Q	V			
8+45	0.2617	0.83	Q	V			
8+50	0.2675	0.84	Q	V			
8+55	0.2735	0.87	Q	V			
9+ 0	0.2795	0.87	Q	V			
9+ 5	0.2858	0.91	Q	V			
9+10	0.2923	0.95	Q	V			
9+15	0.2990	0.97	Q	V			
9+20	0.3058	0.99	Q	V			
9+25	0.3128	1.02	Q	V			
9+30	0.3199	1.03	Q	V			
9+35	0.3271	1.05	Q	V			
9+40	0.3345	1.07	Q	V			
9+45	0.3419	1.08	Q	V			
9+50	0.3495	1.10	Q	V			
9+55	0.3573	1.13	Q	V			
10+ 0	0.3651	1.14	Q	V			
10+ 5	0.3723	1.04	Q	V			

10+10	0.3785	0.90	Q	V		
10+15	0.3844	0.86	Q	V		
10+20	0.3902	0.84	Q	V		
10+25	0.3958	0.82	Q	V		
10+30	0.4014	0.81	Q	V		
10+35	0.4074	0.87	Q	V		
10+40	0.4140	0.96	Q	V		
10+45	0.4209	0.99	Q	V		
10+50	0.4278	1.01	Q	V		
10+55	0.4348	1.02	Q	V		
11+ 0	0.4419	1.03	Q	V		
11+ 5	0.4489	1.02	Q	V		
11+10	0.4559	1.01	Q	V		
11+15	0.4628	1.00	Q	V		
11+20	0.4697	1.00	Q	V		
11+25	0.4766	1.00	Q	V		
11+30	0.4834	1.00	Q	V		
11+35	0.4901	0.97	Q	V		
11+40	0.4965	0.93	Q	V		
11+45	0.5027	0.91	Q	V		
11+50	0.5091	0.92	Q	V		
11+55	0.5155	0.93	Q	V		
12+ 0	0.5220	0.94	Q	V		
12+ 5	0.5291	1.04	Q	V		
12+10	0.5372	1.18	Q	V		
12+15	0.5456	1.22	Q	V		
12+20	0.5544	1.27	Q	V		
12+25	0.5633	1.30	Q	V		
12+30	0.5725	1.32	Q	V		
12+35	0.5819	1.37	Q	V		
12+40	0.5916	1.42	Q	V		
12+45	0.6015	1.44	Q	V		
12+50	0.6116	1.46	Q	V		
12+55	0.6218	1.49	Q	V		
13+ 0	0.6321	1.50	Q	V		
13+ 5	0.6430	1.58	Q	V		
13+10	0.6545	1.68	Q	V		
13+15	0.6664	1.72	Q	V		
13+20	0.6783	1.74	Q	V		
13+25	0.6904	1.75	Q	V		
13+30	0.7025	1.76	Q	V		
13+35	0.7136	1.61	Q	V		
13+40	0.7232	1.40	Q	V		
13+45	0.7324	1.33	Q	V		
13+50	0.7412	1.29	Q	V		
13+55	0.7499	1.26	Q	V		
14+ 0	0.7585	1.24	Q	V		
14+ 5	0.7673	1.28	Q	V		
14+10	0.7766	1.35	Q	V		
14+15	0.7860	1.37	Q	V		
14+20	0.7955	1.37	Q	V		
14+25	0.8048	1.36	Q	V		
14+30	0.8141	1.36	Q	V		
14+35	0.8235	1.36	Q	V		
14+40	0.8329	1.36	Q	V		
14+45	0.8423	1.36	Q	V		
14+50	0.8515	1.35	Q	V		
14+55	0.8607	1.33	Q	V		
15+ 0	0.8698	1.32	Q	V		
15+ 5	0.8787	1.30	Q	V		

15+10	0.8875	1.28	Q			V
15+15	0.8963	1.27	Q			V
15+20	0.9049	1.25	Q			V
15+25	0.9133	1.23	Q			V
15+30	0.9217	1.22	Q			V
15+35	0.9297	1.16	Q			V
15+40	0.9371	1.07	Q			V
15+45	0.9442	1.04	Q			V
15+50	0.9513	1.03	Q			V
15+55	0.9583	1.02	Q			V
16+ 0	0.9653	1.01	Q			V
16+ 5	0.9707	0.79	Q			V
16+10	0.9740	0.48	Q			V
16+15	0.9766	0.38	Q			V
16+20	0.9789	0.33	Q			V
16+25	0.9809	0.29	Q			V
16+30	0.9827	0.26	Q			V
16+35	0.9842	0.23	Q			V
16+40	0.9856	0.19	Q			V
16+45	0.9867	0.17	Q			V
16+50	0.9878	0.16	Q			V
16+55	0.9890	0.16	Q			V
17+ 0	0.9901	0.16	Q			V
17+ 5	0.9914	0.19	Q			V
17+10	0.9929	0.23	Q			V
17+15	0.9946	0.24	Q			V
17+20	0.9963	0.25	Q			V
17+25	0.9980	0.25	Q			V
17+30	0.9997	0.25	Q			V
17+35	1.0015	0.26	Q			V
17+40	1.0033	0.26	Q			V
17+45	1.0051	0.26	Q			V
17+50	1.0068	0.25	Q			V
17+55	1.0084	0.23	Q			V
18+ 0	1.0099	0.22	Q			V
18+ 5	1.0114	0.22	Q			V
18+10	1.0129	0.21	Q			V
18+15	1.0143	0.21	Q			V
18+20	1.0158	0.21	Q			V
18+25	1.0172	0.21	Q			V
18+30	1.0187	0.21	Q			V
18+35	1.0200	0.19	Q			V
18+40	1.0212	0.17	Q			V
18+45	1.0224	0.17	Q			V
18+50	1.0234	0.15	Q			V
18+55	1.0243	0.13	Q			V
19+ 0	1.0251	0.12	Q			V
19+ 5	1.0260	0.13	Q			V
19+10	1.0270	0.15	Q			V
19+15	1.0280	0.15	Q			V
19+20	1.0292	0.17	Q			V
19+25	1.0304	0.19	Q			V
19+30	1.0318	0.19	Q			V
19+35	1.0331	0.18	Q			V
19+40	1.0342	0.17	Q			V
19+45	1.0354	0.16	Q			V
19+50	1.0364	0.15	Q			V
19+55	1.0373	0.13	Q			V
20+ 0	1.0381	0.12	Q			V
20+ 5	1.0390	0.13	Q			V

20+10	1.0400	0.15	Q	V
20+15	1.0410	0.15	Q	V
20+20	1.0420	0.15	Q	V
20+25	1.0431	0.15	Q	V
20+30	1.0441	0.15	Q	V
20+35	1.0452	0.15	Q	V
20+40	1.0463	0.16	Q	V
20+45	1.0474	0.16	Q	V
20+50	1.0483	0.14	Q	V
20+55	1.0492	0.12	Q	V
21+ 0	1.0500	0.12	Q	V
21+ 5	1.0509	0.13	Q	V
21+10	1.0518	0.14	Q	V
21+15	1.0529	0.15	Q	V
21+20	1.0538	0.14	Q	V
21+25	1.0546	0.12	Q	V
21+30	1.0554	0.11	Q	V
21+35	1.0563	0.12	Q	V
21+40	1.0572	0.14	Q	V
21+45	1.0583	0.15	Q	V
21+50	1.0592	0.14	Q	V
21+55	1.0600	0.12	Q	V
22+ 0	1.0608	0.11	Q	V
22+ 5	1.0617	0.12	Q	V
22+10	1.0627	0.14	Q	V
22+15	1.0637	0.15	Q	V
22+20	1.0646	0.14	Q	V
22+25	1.0654	0.12	Q	V
22+30	1.0662	0.11	Q	V
22+35	1.0670	0.11	Q	V
22+40	1.0677	0.11	Q	V
22+45	1.0685	0.11	Q	V
22+50	1.0692	0.11	Q	V
22+55	1.0699	0.11	Q	V
23+ 0	1.0706	0.10	Q	V
23+ 5	1.0714	0.10	Q	V
23+10	1.0721	0.10	Q	V
23+15	1.0728	0.10	Q	V
23+20	1.0735	0.10	Q	V
23+25	1.0742	0.10	Q	V
23+30	1.0750	0.10	Q	V
23+35	1.0757	0.10	Q	V
23+40	1.0764	0.10	Q	V
23+45	1.0771	0.10	Q	V
23+50	1.0779	0.10	Q	V
23+55	1.0786	0.10	Q	V
24+ 0	1.0793	0.10	Q	V
24+ 5	1.0798	0.08	Q	V
24+10	1.0801	0.04	Q	V
24+15	1.0802	0.02	Q	V
24+20	1.0803	0.02	Q	V
24+25	1.0804	0.01	Q	V
24+30	1.0804	0.01	Q	V
24+35	1.0805	0.00	Q	V
24+40	1.0805	0.00	Q	V

10-YEAR, 24-HOUR STORM EVENT

Unit Hydrograph Analysis

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Study date 02/16/23 File: ara2410.out

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Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978

Program License Serial Number 6279

English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used

English Units used in output format

302.01.21
AREA A
10 YR STORM
POST PROJECT

Drainage Area = 12.23(Ac.) = 0.019 Sq. Mi.
Drainage Area for Depth-Area Areal Adjustment = 12.23(Ac.) = 0.019
Sq. Mi.
Length along longest watercourse = 1105.90(Ft.)
Length along longest watercourse measured to centroid = 610.60(Ft.)
Length along longest watercourse = 0.209 Mi.
Length along longest watercourse measured to centroid = 0.116 Mi.
Difference in elevation = 9.50(Ft.)
Slope along watercourse = 45.3567 Ft./Mi.
Average Manning's 'N' = 0.025
Lag time = 0.071 Hr.
Lag time = 4.24 Min.
25% of lag time = 1.06 Min.
40% of lag time = 1.70 Min.
Unit time = 5.00 Min.
Duration of storm = 24 Hour(s)
User Entered Base Flow = 0.00(CFS)

2 YEAR Area rainfall data:

Area(Ac.) [1]	Rainfall(In) [2]	Weighting[1*2]
12.23	2.79	34.12

100 YEAR Area rainfall data:

Area(Ac.) [1]	Rainfall(In) [2]	Weighting[1*2]
12.23	7.72	94.42

STORM EVENT (YEAR) = 10.00
 Area Averaged 2-Year Rainfall = 2.790 (In)
 Area Averaged 100-Year Rainfall = 7.720 (In)

Point rain (area averaged) = 4.818 (In)
 Areal adjustment factor = 100.00 %
 Adjusted average point rain = 4.818 (In)

Sub-Area Data:

Area (Ac.)	Runoff Index	Impervious %
12.230	33.60	0.350
Total Area Entered = 12.23 (Ac.)		

RI	RI	Infil. Rate	Impervious	Adj. Infil. Rate	Area%	F
AMC2	AMC-2	(In/Hr)	(Dec.%)	(In/Hr)	(Dec.)	(In/Hr)
33.6	33.6	0.728	0.350	0.498	1.000	0.498
						Sum (F) = 0.498

Area averaged mean soil loss (F) (In/Hr) = 0.498
 Minimum soil loss rate ((In/Hr)) = 0.249
 (for 24 hour storm duration)
 Soil loss rate (decimal) = 0.620

Unit Hydrograph
 MOUNTAIN S-Curve

Unit Hydrograph Data

Unit time period (hrs)	Time % of lag	Distribution Graph %	Unit Hydrograph (CFS)
1	0.083	117.878	27.331
2	0.167	235.755	38.091
3	0.250	353.633	12.913
4	0.333	471.511	6.898
5	0.417	589.388	4.601
6	0.500	707.266	3.392
7	0.583	825.144	2.550
8	0.667	943.021	2.137
9	0.750	1060.899	2.087
		Sum = 100.000	Sum= 12.326

The following loss rate calculations reflect use of the minimum calculated loss rate subtracted from the Storm Rain to produce the maximum Effective Rain value

Unit Time (Hr.)	Pattern Percent	Storm Rain (In/Hr)	Loss rate (In./Hr) Max Low	Effective (In/Hr)
1	0.08	0.07	(0.884) 0.024	0.015
2	0.17	0.07	(0.880) 0.024	0.015
3	0.25	0.07	(0.877) 0.024	0.015
4	0.33	0.10	(0.873) 0.036	0.022
5	0.42	0.10	(0.870) 0.036	0.022
6	0.50	0.10	(0.867) 0.036	0.022
7	0.58	0.10	(0.863) 0.036	0.022
8	0.67	0.10	(0.860) 0.036	0.022
9	0.75	0.10	(0.856) 0.036	0.022
10	0.83	0.13	(0.853) 0.048	0.029

11	0.92	0.13	0.077	(0.850)	0.048	0.029
12	1.00	0.13	0.077	(0.846)	0.048	0.029
13	1.08	0.10	0.058	(0.843)	0.036	0.022
14	1.17	0.10	0.058	(0.840)	0.036	0.022
15	1.25	0.10	0.058	(0.836)	0.036	0.022
16	1.33	0.10	0.058	(0.833)	0.036	0.022
17	1.42	0.10	0.058	(0.830)	0.036	0.022
18	1.50	0.10	0.058	(0.826)	0.036	0.022
19	1.58	0.10	0.058	(0.823)	0.036	0.022
20	1.67	0.10	0.058	(0.820)	0.036	0.022
21	1.75	0.10	0.058	(0.816)	0.036	0.022
22	1.83	0.13	0.077	(0.813)	0.048	0.029
23	1.92	0.13	0.077	(0.810)	0.048	0.029
24	2.00	0.13	0.077	(0.807)	0.048	0.029
25	2.08	0.13	0.077	(0.803)	0.048	0.029
26	2.17	0.13	0.077	(0.800)	0.048	0.029
27	2.25	0.13	0.077	(0.797)	0.048	0.029
28	2.33	0.13	0.077	(0.794)	0.048	0.029
29	2.42	0.13	0.077	(0.790)	0.048	0.029
30	2.50	0.13	0.077	(0.787)	0.048	0.029
31	2.58	0.17	0.096	(0.784)	0.060	0.037
32	2.67	0.17	0.096	(0.781)	0.060	0.037
33	2.75	0.17	0.096	(0.778)	0.060	0.037
34	2.83	0.17	0.096	(0.774)	0.060	0.037
35	2.92	0.17	0.096	(0.771)	0.060	0.037
36	3.00	0.17	0.096	(0.768)	0.060	0.037
37	3.08	0.17	0.096	(0.765)	0.060	0.037
38	3.17	0.17	0.096	(0.762)	0.060	0.037
39	3.25	0.17	0.096	(0.758)	0.060	0.037
40	3.33	0.17	0.096	(0.755)	0.060	0.037
41	3.42	0.17	0.096	(0.752)	0.060	0.037
42	3.50	0.17	0.096	(0.749)	0.060	0.037
43	3.58	0.17	0.096	(0.746)	0.060	0.037
44	3.67	0.17	0.096	(0.743)	0.060	0.037
45	3.75	0.17	0.096	(0.740)	0.060	0.037
46	3.83	0.20	0.116	(0.736)	0.072	0.044
47	3.92	0.20	0.116	(0.733)	0.072	0.044
48	4.00	0.20	0.116	(0.730)	0.072	0.044
49	4.08	0.20	0.116	(0.727)	0.072	0.044
50	4.17	0.20	0.116	(0.724)	0.072	0.044
51	4.25	0.20	0.116	(0.721)	0.072	0.044
52	4.33	0.23	0.135	(0.718)	0.084	0.051
53	4.42	0.23	0.135	(0.715)	0.084	0.051
54	4.50	0.23	0.135	(0.712)	0.084	0.051
55	4.58	0.23	0.135	(0.709)	0.084	0.051
56	4.67	0.23	0.135	(0.706)	0.084	0.051
57	4.75	0.23	0.135	(0.703)	0.084	0.051
58	4.83	0.27	0.154	(0.700)	0.096	0.059
59	4.92	0.27	0.154	(0.697)	0.096	0.059
60	5.00	0.27	0.154	(0.694)	0.096	0.059
61	5.08	0.20	0.116	(0.691)	0.072	0.044
62	5.17	0.20	0.116	(0.688)	0.072	0.044
63	5.25	0.20	0.116	(0.685)	0.072	0.044
64	5.33	0.23	0.135	(0.682)	0.084	0.051
65	5.42	0.23	0.135	(0.679)	0.084	0.051
66	5.50	0.23	0.135	(0.676)	0.084	0.051
67	5.58	0.27	0.154	(0.673)	0.096	0.059
68	5.67	0.27	0.154	(0.670)	0.096	0.059
69	5.75	0.27	0.154	(0.667)	0.096	0.059
70	5.83	0.27	0.154	(0.664)	0.096	0.059

71	5.92	0.27	0.154	(0.661)	0.096	0.059
72	6.00	0.27	0.154	(0.658)	0.096	0.059
73	6.08	0.30	0.173	(0.655)	0.108	0.066
74	6.17	0.30	0.173	(0.652)	0.108	0.066
75	6.25	0.30	0.173	(0.649)	0.108	0.066
76	6.33	0.30	0.173	(0.646)	0.108	0.066
77	6.42	0.30	0.173	(0.643)	0.108	0.066
78	6.50	0.30	0.173	(0.640)	0.108	0.066
79	6.58	0.33	0.193	(0.638)	0.119	0.073
80	6.67	0.33	0.193	(0.635)	0.119	0.073
81	6.75	0.33	0.193	(0.632)	0.119	0.073
82	6.83	0.33	0.193	(0.629)	0.119	0.073
83	6.92	0.33	0.193	(0.626)	0.119	0.073
84	7.00	0.33	0.193	(0.623)	0.119	0.073
85	7.08	0.33	0.193	(0.620)	0.119	0.073
86	7.17	0.33	0.193	(0.618)	0.119	0.073
87	7.25	0.33	0.193	(0.615)	0.119	0.073
88	7.33	0.37	0.212	(0.612)	0.131	0.081
89	7.42	0.37	0.212	(0.609)	0.131	0.081
90	7.50	0.37	0.212	(0.606)	0.131	0.081
91	7.58	0.40	0.231	(0.604)	0.143	0.088
92	7.67	0.40	0.231	(0.601)	0.143	0.088
93	7.75	0.40	0.231	(0.598)	0.143	0.088
94	7.83	0.43	0.251	(0.595)	0.155	0.095
95	7.92	0.43	0.251	(0.593)	0.155	0.095
96	8.00	0.43	0.251	(0.590)	0.155	0.095
97	8.08	0.50	0.289	(0.587)	0.179	0.110
98	8.17	0.50	0.289	(0.584)	0.179	0.110
99	8.25	0.50	0.289	(0.582)	0.179	0.110
100	8.33	0.50	0.289	(0.579)	0.179	0.110
101	8.42	0.50	0.289	(0.576)	0.179	0.110
102	8.50	0.50	0.289	(0.574)	0.179	0.110
103	8.58	0.53	0.308	(0.571)	0.191	0.117
104	8.67	0.53	0.308	(0.568)	0.191	0.117
105	8.75	0.53	0.308	(0.565)	0.191	0.117
106	8.83	0.57	0.328	(0.563)	0.203	0.125
107	8.92	0.57	0.328	(0.560)	0.203	0.125
108	9.00	0.57	0.328	(0.557)	0.203	0.125
109	9.08	0.63	0.366	(0.555)	0.227	0.139
110	9.17	0.63	0.366	(0.552)	0.227	0.139
111	9.25	0.63	0.366	(0.550)	0.227	0.139
112	9.33	0.67	0.385	(0.547)	0.239	0.146
113	9.42	0.67	0.385	(0.544)	0.239	0.146
114	9.50	0.67	0.385	(0.542)	0.239	0.146
115	9.58	0.70	0.405	(0.539)	0.251	0.154
116	9.67	0.70	0.405	(0.537)	0.251	0.154
117	9.75	0.70	0.405	(0.534)	0.251	0.154
118	9.83	0.73	0.424	(0.531)	0.263	0.161
119	9.92	0.73	0.424	(0.529)	0.263	0.161
120	10.00	0.73	0.424	(0.526)	0.263	0.161
121	10.08	0.50	0.289	(0.524)	0.179	0.110
122	10.17	0.50	0.289	(0.521)	0.179	0.110
123	10.25	0.50	0.289	(0.519)	0.179	0.110
124	10.33	0.50	0.289	(0.516)	0.179	0.110
125	10.42	0.50	0.289	(0.514)	0.179	0.110
126	10.50	0.50	0.289	(0.511)	0.179	0.110
127	10.58	0.67	0.385	(0.509)	0.239	0.146
128	10.67	0.67	0.385	(0.506)	0.239	0.146
129	10.75	0.67	0.385	(0.504)	0.239	0.146
130	10.83	0.67	0.385	(0.501)	0.239	0.146

131	10.92	0.67	0.385	(0.499)	0.239	0.146
132	11.00	0.67	0.385	(0.496)	0.239	0.146
133	11.08	0.63	0.366	(0.494)	0.227	0.139
134	11.17	0.63	0.366	(0.491)	0.227	0.139
135	11.25	0.63	0.366	(0.489)	0.227	0.139
136	11.33	0.63	0.366	(0.487)	0.227	0.139
137	11.42	0.63	0.366	(0.484)	0.227	0.139
138	11.50	0.63	0.366	(0.482)	0.227	0.139
139	11.58	0.57	0.328	(0.479)	0.203	0.125
140	11.67	0.57	0.328	(0.477)	0.203	0.125
141	11.75	0.57	0.328	(0.475)	0.203	0.125
142	11.83	0.60	0.347	(0.472)	0.215	0.132
143	11.92	0.60	0.347	(0.470)	0.215	0.132
144	12.00	0.60	0.347	(0.468)	0.215	0.132
145	12.08	0.83	0.482	(0.465)	0.299	0.183
146	12.17	0.83	0.482	(0.463)	0.299	0.183
147	12.25	0.83	0.482	(0.461)	0.299	0.183
148	12.33	0.87	0.501	(0.458)	0.311	0.190
149	12.42	0.87	0.501	(0.456)	0.311	0.190
150	12.50	0.87	0.501	(0.454)	0.311	0.190
151	12.58	0.93	0.540	(0.451)	0.335	0.205
152	12.67	0.93	0.540	(0.449)	0.335	0.205
153	12.75	0.93	0.540	(0.447)	0.335	0.205
154	12.83	0.97	0.559	(0.445)	0.347	0.212
155	12.92	0.97	0.559	(0.442)	0.347	0.212
156	13.00	0.97	0.559	(0.440)	0.347	0.212
157	13.08	1.13	0.655	(0.438)	0.406	0.249
158	13.17	1.13	0.655	(0.436)	0.406	0.249
159	13.25	1.13	0.655	(0.433)	0.406	0.249
160	13.33	1.13	0.655	(0.431)	0.406	0.249
161	13.42	1.13	0.655	(0.429)	0.406	0.249
162	13.50	1.13	0.655	(0.427)	0.406	0.249
163	13.58	0.77	0.443	(0.425)	0.275	0.168
164	13.67	0.77	0.443	(0.423)	0.275	0.168
165	13.75	0.77	0.443	(0.420)	0.275	0.168
166	13.83	0.77	0.443	(0.418)	0.275	0.168
167	13.92	0.77	0.443	(0.416)	0.275	0.168
168	14.00	0.77	0.443	(0.414)	0.275	0.168
169	14.08	0.90	0.520	(0.412)	0.323	0.198
170	14.17	0.90	0.520	(0.410)	0.323	0.198
171	14.25	0.90	0.520	(0.408)	0.323	0.198
172	14.33	0.87	0.501	(0.406)	0.311	0.190
173	14.42	0.87	0.501	(0.404)	0.311	0.190
174	14.50	0.87	0.501	(0.401)	0.311	0.190
175	14.58	0.87	0.501	(0.399)	0.311	0.190
176	14.67	0.87	0.501	(0.397)	0.311	0.190
177	14.75	0.87	0.501	(0.395)	0.311	0.190
178	14.83	0.83	0.482	(0.393)	0.299	0.183
179	14.92	0.83	0.482	(0.391)	0.299	0.183
180	15.00	0.83	0.482	(0.389)	0.299	0.183
181	15.08	0.80	0.463	(0.387)	0.287	0.176
182	15.17	0.80	0.463	(0.385)	0.287	0.176
183	15.25	0.80	0.463	(0.383)	0.287	0.176
184	15.33	0.77	0.443	(0.381)	0.275	0.168
185	15.42	0.77	0.443	(0.379)	0.275	0.168
186	15.50	0.77	0.443	(0.377)	0.275	0.168
187	15.58	0.63	0.366	(0.376)	0.227	0.139
188	15.67	0.63	0.366	(0.374)	0.227	0.139
189	15.75	0.63	0.366	(0.372)	0.227	0.139
190	15.83	0.63	0.366	(0.370)	0.227	0.139

191	15.92	0.63	0.366	(0.368)	0.227	0.139
192	16.00	0.63	0.366	(0.366)	0.227	0.139
193	16.08	0.13	0.077	(0.364)	0.048	0.029
194	16.17	0.13	0.077	(0.362)	0.048	0.029
195	16.25	0.13	0.077	(0.360)	0.048	0.029
196	16.33	0.13	0.077	(0.359)	0.048	0.029
197	16.42	0.13	0.077	(0.357)	0.048	0.029
198	16.50	0.13	0.077	(0.355)	0.048	0.029
199	16.58	0.10	0.058	(0.353)	0.036	0.022
200	16.67	0.10	0.058	(0.351)	0.036	0.022
201	16.75	0.10	0.058	(0.350)	0.036	0.022
202	16.83	0.10	0.058	(0.348)	0.036	0.022
203	16.92	0.10	0.058	(0.346)	0.036	0.022
204	17.00	0.10	0.058	(0.344)	0.036	0.022
205	17.08	0.17	0.096	(0.343)	0.060	0.037
206	17.17	0.17	0.096	(0.341)	0.060	0.037
207	17.25	0.17	0.096	(0.339)	0.060	0.037
208	17.33	0.17	0.096	(0.337)	0.060	0.037
209	17.42	0.17	0.096	(0.336)	0.060	0.037
210	17.50	0.17	0.096	(0.334)	0.060	0.037
211	17.58	0.17	0.096	(0.332)	0.060	0.037
212	17.67	0.17	0.096	(0.331)	0.060	0.037
213	17.75	0.17	0.096	(0.329)	0.060	0.037
214	17.83	0.13	0.077	(0.327)	0.048	0.029
215	17.92	0.13	0.077	(0.326)	0.048	0.029
216	18.00	0.13	0.077	(0.324)	0.048	0.029
217	18.08	0.13	0.077	(0.323)	0.048	0.029
218	18.17	0.13	0.077	(0.321)	0.048	0.029
219	18.25	0.13	0.077	(0.319)	0.048	0.029
220	18.33	0.13	0.077	(0.318)	0.048	0.029
221	18.42	0.13	0.077	(0.316)	0.048	0.029
222	18.50	0.13	0.077	(0.315)	0.048	0.029
223	18.58	0.10	0.058	(0.313)	0.036	0.022
224	18.67	0.10	0.058	(0.312)	0.036	0.022
225	18.75	0.10	0.058	(0.310)	0.036	0.022
226	18.83	0.07	0.039	(0.309)	0.024	0.015
227	18.92	0.07	0.039	(0.307)	0.024	0.015
228	19.00	0.07	0.039	(0.306)	0.024	0.015
229	19.08	0.10	0.058	(0.304)	0.036	0.022
230	19.17	0.10	0.058	(0.303)	0.036	0.022
231	19.25	0.10	0.058	(0.302)	0.036	0.022
232	19.33	0.13	0.077	(0.300)	0.048	0.029
233	19.42	0.13	0.077	(0.299)	0.048	0.029
234	19.50	0.13	0.077	(0.297)	0.048	0.029
235	19.58	0.10	0.058	(0.296)	0.036	0.022
236	19.67	0.10	0.058	(0.295)	0.036	0.022
237	19.75	0.10	0.058	(0.293)	0.036	0.022
238	19.83	0.07	0.039	(0.292)	0.024	0.015
239	19.92	0.07	0.039	(0.291)	0.024	0.015
240	20.00	0.07	0.039	(0.289)	0.024	0.015
241	20.08	0.10	0.058	(0.288)	0.036	0.022
242	20.17	0.10	0.058	(0.287)	0.036	0.022
243	20.25	0.10	0.058	(0.286)	0.036	0.022
244	20.33	0.10	0.058	(0.284)	0.036	0.022
245	20.42	0.10	0.058	(0.283)	0.036	0.022
246	20.50	0.10	0.058	(0.282)	0.036	0.022
247	20.58	0.10	0.058	(0.281)	0.036	0.022
248	20.67	0.10	0.058	(0.280)	0.036	0.022
249	20.75	0.10	0.058	(0.278)	0.036	0.022
250	20.83	0.07	0.039	(0.277)	0.024	0.015

251	20.92	0.07	0.039	(0.276)	0.024	0.015
252	21.00	0.07	0.039	(0.275)	0.024	0.015
253	21.08	0.10	0.058	(0.274)	0.036	0.022
254	21.17	0.10	0.058	(0.273)	0.036	0.022
255	21.25	0.10	0.058	(0.272)	0.036	0.022
256	21.33	0.07	0.039	(0.271)	0.024	0.015
257	21.42	0.07	0.039	(0.270)	0.024	0.015
258	21.50	0.07	0.039	(0.269)	0.024	0.015
259	21.58	0.10	0.058	(0.268)	0.036	0.022
260	21.67	0.10	0.058	(0.267)	0.036	0.022
261	21.75	0.10	0.058	(0.266)	0.036	0.022
262	21.83	0.07	0.039	(0.265)	0.024	0.015
263	21.92	0.07	0.039	(0.264)	0.024	0.015
264	22.00	0.07	0.039	(0.263)	0.024	0.015
265	22.08	0.10	0.058	(0.262)	0.036	0.022
266	22.17	0.10	0.058	(0.261)	0.036	0.022
267	22.25	0.10	0.058	(0.261)	0.036	0.022
268	22.33	0.07	0.039	(0.260)	0.024	0.015
269	22.42	0.07	0.039	(0.259)	0.024	0.015
270	22.50	0.07	0.039	(0.258)	0.024	0.015
271	22.58	0.07	0.039	(0.257)	0.024	0.015
272	22.67	0.07	0.039	(0.257)	0.024	0.015
273	22.75	0.07	0.039	(0.256)	0.024	0.015
274	22.83	0.07	0.039	(0.255)	0.024	0.015
275	22.92	0.07	0.039	(0.255)	0.024	0.015
276	23.00	0.07	0.039	(0.254)	0.024	0.015
277	23.08	0.07	0.039	(0.254)	0.024	0.015
278	23.17	0.07	0.039	(0.253)	0.024	0.015
279	23.25	0.07	0.039	(0.252)	0.024	0.015
280	23.33	0.07	0.039	(0.252)	0.024	0.015
281	23.42	0.07	0.039	(0.251)	0.024	0.015
282	23.50	0.07	0.039	(0.251)	0.024	0.015
283	23.58	0.07	0.039	(0.251)	0.024	0.015
284	23.67	0.07	0.039	(0.250)	0.024	0.015
285	23.75	0.07	0.039	(0.250)	0.024	0.015
286	23.83	0.07	0.039	(0.250)	0.024	0.015
287	23.92	0.07	0.039	(0.249)	0.024	0.015
288	24.00	0.07	0.039	(0.249)	0.024	0.015

(Loss Rate Not Used)

Sum = 100.0 Sum = 22.0

Flood volume = Effective rainfall 1.83(In)
times area 12.2(Ac.)/[(In)/(Ft.)] = 1.9(Ac.Ft)
Total soil loss = 2.99(In)
Total soil loss = 3.044(Ac.Ft)
Total rainfall = 4.82(In)
Flood volume = 81282.1 Cubic Feet
Total soil loss = 132618.2 Cubic Feet

Peak flow rate of this hydrograph = 3.040(CFS)

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24 - H O U R S T O R M
R u n o f f H y d r o g r a p h

Hydrograph in 5 Minute intervals ((CFS))

Time(h+m) Volume Ac.Ft Q(CFS) 0 2.5 5.0 7.5 10.0

0+ 5 0.0003 0.05 Q | | | | |

0+10	0.0012	0.12	Q
0+15	0.0021	0.14	Q
0+20	0.0034	0.18	Q
0+25	0.0049	0.22	Q
0+30	0.0065	0.24	Q
0+35	0.0083	0.25	Q
0+40	0.0100	0.26	VQ
0+45	0.0119	0.26	VQ
0+50	0.0139	0.29	VQ
0+55	0.0161	0.33	VQ
1+ 0	0.0185	0.34	VQ
1+ 5	0.0207	0.32	VQ
1+10	0.0227	0.29	VQ
1+15	0.0247	0.28	VQ
1+20	0.0266	0.28	VQ
1+25	0.0285	0.28	VQ
1+30	0.0304	0.28	VQ
1+35	0.0323	0.27	VQ
1+40	0.0342	0.27	VQ
1+45	0.0361	0.27	VQ
1+50	0.0381	0.30	VQ
1+55	0.0404	0.33	VQ
2+ 0	0.0427	0.34	VQ
2+ 5	0.0451	0.35	VQ
2+10	0.0476	0.35	Q
2+15	0.0500	0.36	Q
2+20	0.0525	0.36	Q
2+25	0.0549	0.36	Q
2+30	0.0574	0.36	Q
2+35	0.0601	0.39	Q
2+40	0.0630	0.42	Q
2+45	0.0660	0.43	Q
2+50	0.0690	0.44	Q
2+55	0.0720	0.44	Q
3+ 0	0.0751	0.45	Q
3+ 5	0.0782	0.45	Q
3+10	0.0813	0.45	Q
3+15	0.0844	0.45	Q
3+20	0.0875	0.45	Q
3+25	0.0906	0.45	Q
3+30	0.0937	0.45	QV
3+35	0.0968	0.45	QV
3+40	0.0999	0.45	QV
3+45	0.1030	0.45	QV
3+50	0.1063	0.48	QV
3+55	0.1098	0.51	Q
4+ 0	0.1134	0.52	Q
4+ 5	0.1171	0.53	Q
4+10	0.1207	0.53	Q
4+15	0.1244	0.54	Q
4+20	0.1283	0.56	Q
4+25	0.1324	0.60	Q
4+30	0.1366	0.61	Q
4+35	0.1409	0.62	QV
4+40	0.1452	0.62	QV
4+45	0.1495	0.63	QV
4+50	0.1540	0.65	QV
4+55	0.1588	0.69	QV
5+ 0	0.1636	0.70	QV
5+ 5	0.1681	0.66	QV

5+10	0.1722	0.60	QV			
5+15	0.1762	0.57	QV			
5+20	0.1803	0.59	QV			
5+25	0.1845	0.62	QV			
5+30	0.1888	0.62	Q V			
5+35	0.1933	0.65	Q V			
5+40	0.1980	0.69	Q V			
5+45	0.2028	0.70	Q V			
5+50	0.2077	0.71	Q V			
5+55	0.2126	0.71	Q V			
6+ 0	0.2175	0.72	Q V			
6+ 5	0.2226	0.74	Q V			
6+10	0.2280	0.78	QV			
6+15	0.2335	0.79	Q V			
6+20	0.2390	0.80	Q V			
6+25	0.2445	0.80	Q V			
6+30	0.2501	0.81	Q V			
6+35	0.2558	0.83	Q V			
6+40	0.2618	0.87	Q V			
6+45	0.2679	0.88	Q V			
6+50	0.2740	0.89	Q V			
6+55	0.2802	0.89	Q V			
7+ 0	0.2863	0.90	Q V			
7+ 5	0.2925	0.90	Q V			
7+10	0.2987	0.90	Q V			
7+15	0.3050	0.90	Q V			
7+20	0.3114	0.93	Q V			
7+25	0.3180	0.96	Q V			
7+30	0.3247	0.97	Q V			
7+35	0.3316	1.00	Q V			
7+40	0.3388	1.04	Q V			
7+45	0.3461	1.06	Q V			
7+50	0.3536	1.09	Q V			
7+55	0.3614	1.13	Q V			
8+ 0	0.3693	1.15	Q V			
8+ 5	0.3776	1.21	Q V			
8+10	0.3864	1.28	Q V			
8+15	0.3955	1.31	Q V			
8+20	0.4046	1.32	Q V			
8+25	0.4138	1.33	Q V			
8+30	0.4230	1.34	Q V			
8+35	0.4325	1.37	Q V			
8+40	0.4422	1.41	Q V			
8+45	0.4520	1.43	Q V			
8+50	0.4620	1.46	Q V			
8+55	0.4723	1.49	Q V			
9+ 0	0.4827	1.51	Q V			
9+ 5	0.4935	1.57	Q V			
9+10	0.5048	1.64	Q V			
9+15	0.5163	1.67	Q V			
9+20	0.5281	1.71	Q V			
9+25	0.5402	1.75	Q V			
9+30	0.5524	1.77	Q V			
9+35	0.5649	1.81	Q V			
9+40	0.5776	1.85	Q V			
9+45	0.5905	1.87	Q V			
9+50	0.6036	1.90	Q V			
9+55	0.6170	1.94	Q V			
10+ 0	0.6305	1.96	Q V			
10+ 5	0.6429	1.80	Q V			

10+10	0.6537	1.56	Q	V			
10+15	0.6639	1.49	Q	V			
10+20	0.6738	1.44	Q	V			
10+25	0.6836	1.42	Q	V			
10+30	0.6932	1.40	Q	V			
10+35	0.7036	1.50	Q	V			
10+40	0.7150	1.66	Q	V			
10+45	0.7268	1.71	Q	V			
10+50	0.7388	1.74	Q	V			
10+55	0.7509	1.76	Q	V			
11+ 0	0.7631	1.78	Q	V			
11+ 5	0.7753	1.76	Q	V			
11+10	0.7872	1.74	Q	V			
11+15	0.7992	1.74	Q	V			
11+20	0.8111	1.73	Q	V			
11+25	0.8230	1.73	Q	V			
11+30	0.8349	1.72	Q	V			
11+35	0.8464	1.67	Q	V			
11+40	0.8574	1.60	Q	V			
11+45	0.8682	1.57	Q	V			
11+50	0.8791	1.59	Q	V			
11+55	0.8903	1.61	Q	V			
12+ 0	0.9014	1.62	Q	V			
12+ 5	0.9137	1.79	Q	V			
12+10	0.9278	2.03	Q	V			
12+15	0.9423	2.11	Q	V			
12+20	0.9574	2.19	Q	V			
12+25	0.9729	2.25	Q	V			
12+30	0.9886	2.29	Q	V			
12+35	1.0048	2.36	Q	V			
12+40	1.0217	2.44	Q	V			
12+45	1.0388	2.48	Q	V			
12+50	1.0562	2.52	Q	V			
12+55	1.0738	2.57	Q	V			
13+ 0	1.0917	2.59	Q	V			
13+ 5	1.1104	2.72	Q	V			
13+10	1.1304	2.90	Q	V			
13+15	1.1508	2.97	Q	V			
13+20	1.1715	3.00	Q	V			
13+25	1.1923	3.02	Q	V			
13+30	1.2132	3.04	Q	V			
13+35	1.2324	2.78	Q	V			
13+40	1.2490	2.41	Q	V			
13+45	1.2648	2.29	Q	V			
13+50	1.2801	2.22	Q	V			
13+55	1.2951	2.18	Q	V			
14+ 0	1.3099	2.14	Q	V			
14+ 5	1.3251	2.22	Q	V			
14+10	1.3412	2.33	Q	V			
14+15	1.3575	2.36	Q	V			
14+20	1.3737	2.36	Q	V			
14+25	1.3899	2.34	Q	V			
14+30	1.4060	2.34	Q	V			
14+35	1.4222	2.35	Q	V			
14+40	1.4383	2.35	Q	V			
14+45	1.4546	2.35	Q	V			
14+50	1.4706	2.33	Q	V			
14+55	1.4864	2.29	Q	V			
15+ 0	1.5020	2.28	Q	V			
15+ 5	1.5175	2.25	Q	V			

15+10	1.5327	2.21		Q		V
15+15	1.5478	2.19		Q		V
15+20	1.5627	2.16		Q		V
15+25	1.5773	2.12		Q		V
15+30	1.5918	2.10		Q		V
15+35	1.6055	2.00		Q		V
15+40	1.6183	1.85		Q		V
15+45	1.6307	1.80		Q		V
15+50	1.6429	1.77		Q		V
15+55	1.6550	1.75		Q		V
16+ 0	1.6670	1.74		Q		V
16+ 5	1.6763	1.36		Q		V
16+10	1.6821	0.84		Q		V
16+15	1.6866	0.65		Q		V
16+20	1.6905	0.56		Q		V
16+25	1.6939	0.50		Q		V
16+30	1.6970	0.45		Q		V
16+35	1.6997	0.39		Q		V
16+40	1.7020	0.33		Q		V
16+45	1.7040	0.29		Q		V
16+50	1.7060	0.28		Q		V
16+55	1.7079	0.28		Q		V
17+ 0	1.7098	0.28		Q		V
17+ 5	1.7120	0.32		Q		V
17+10	1.7147	0.39		Q		V
17+15	1.7176	0.41		Q		V
17+20	1.7205	0.42		Q		V
17+25	1.7235	0.43		Q		V
17+30	1.7265	0.44		Q		V
17+35	1.7296	0.44		Q		V
17+40	1.7327	0.45		Q		V
17+45	1.7358	0.45		Q		V
17+50	1.7387	0.43		Q		V
17+55	1.7414	0.39		Q		V
18+ 0	1.7440	0.38		Q		V
18+ 5	1.7466	0.37		Q		V
18+10	1.7492	0.37		Q		V
18+15	1.7517	0.37		Q		V
18+20	1.7542	0.37		Q		V
18+25	1.7567	0.36		Q		V
18+30	1.7592	0.36		Q		V
18+35	1.7615	0.34		Q		V
18+40	1.7636	0.30		Q		V
18+45	1.7656	0.29		Q		V
18+50	1.7674	0.26		Q		V
18+55	1.7689	0.22		Q		V
19+ 0	1.7703	0.21		Q		V
19+ 5	1.7719	0.22		Q		V
19+10	1.7736	0.25		Q		V
19+15	1.7754	0.26		Q		V
19+20	1.7773	0.29		Q		V
19+25	1.7796	0.32		Q		V
19+30	1.7819	0.34		Q		V
19+35	1.7841	0.32		Q		V
19+40	1.7861	0.29		Q		V
19+45	1.7880	0.28		Q		V
19+50	1.7898	0.26		Q		V
19+55	1.7913	0.22		Q		V
20+ 0	1.7927	0.21		Q		V
20+ 5	1.7943	0.22		Q		V

20+10	1.7960	0.25	Q	V
20+15	1.7978	0.26	Q	V
20+20	1.7996	0.26	Q	V
20+25	1.8014	0.26	Q	V
20+30	1.8032	0.26	Q	V
20+35	1.8050	0.27	Q	V
20+40	1.8069	0.27	Q	V
20+45	1.8088	0.27	Q	V
20+50	1.8104	0.25	Q	V
20+55	1.8119	0.21	Q	V
21+ 0	1.8133	0.20	Q	V
21+ 5	1.8148	0.22	Q	V
21+10	1.8165	0.25	Q	V
21+15	1.8183	0.26	Q	V
21+20	1.8199	0.24	Q	V
21+25	1.8213	0.20	Q	V
21+30	1.8227	0.19	Q	V
21+35	1.8241	0.21	Q	V
21+40	1.8258	0.25	Q	V
21+45	1.8276	0.26	Q	V
21+50	1.8292	0.24	Q	V
21+55	1.8306	0.20	Q	V
22+ 0	1.8320	0.19	Q	V
22+ 5	1.8335	0.21	Q	V
22+10	1.8352	0.25	Q	V
22+15	1.8369	0.26	Q	V
22+20	1.8386	0.24	Q	V
22+25	1.8400	0.20	Q	V
22+30	1.8413	0.19	Q	V
22+35	1.8426	0.19	Q	V
22+40	1.8439	0.19	Q	V
22+45	1.8452	0.19	Q	V
22+50	1.8465	0.18	Q	V
22+55	1.8477	0.18	Q	V
23+ 0	1.8490	0.18	Q	V
23+ 5	1.8502	0.18	Q	V
23+10	1.8515	0.18	Q	V
23+15	1.8527	0.18	Q	V
23+20	1.8540	0.18	Q	V
23+25	1.8552	0.18	Q	V
23+30	1.8564	0.18	Q	V
23+35	1.8577	0.18	Q	V
23+40	1.8589	0.18	Q	V
23+45	1.8602	0.18	Q	V
23+50	1.8614	0.18	Q	V
23+55	1.8627	0.18	Q	V
24+ 0	1.8639	0.18	Q	V
24+ 5	1.8648	0.13	Q	V
24+10	1.8652	0.06	Q	V
24+15	1.8655	0.04	Q	V
24+20	1.8657	0.03	Q	V
24+25	1.8658	0.02	Q	V
24+30	1.8659	0.01	Q	V
24+35	1.8660	0.01	Q	V
24+40	1.8660	0.00	Q	V

APPENDIX D: BASIN VOLUME CALCULATIONS

APPENDIX D.1: Basin 1 Volume Calculations

THE RIDGE - BASIN 1 VOLUME CALCULATION

Contour Elevation	Contour Area (sf)	Contour Area (ac)	Average Area (ac)	Incremental Volume (ac-ft) (ac-ft)	Total Basin Volume (ac-ft)
4362.0	16304.60	0.374		0	RESERVED FOR WQ UP TO 4362.5
			0.386		
4362.5	17299.05	0.397			0
			0.409		
4363.0	18293.50	0.420		0.409	0.41
			0.444		
4364.0	20359.2	0.467		0.444	0.85
			0.492		
4365.0	22504.40	0.517		0.492	1.34
			0.542		
4366.0	24728.4	0.568		0.542	1.89
			0.594		
4367.0	27031.26	0.621		0.594	2.48
			0.647		
4367	29342.5	0.674		0.647	3.13

WATER QUALITY VOLUME FOR DMA 1, 2 & DMA 4=12,200, REQUIRES BOTTOM AREA OF 9,038 SF.
 BASIN 2 BOTTOM AREA IS 16,304.6 BASED ON GRADING PLAN
 MITIGATION VOLUME = 1.3 AC-FT. PONDS AT 4365.0 FT

APPENDIX E: OFFSITE RATIONAL METHOD HYDROLOGY

APPENDIX E.1: OFFSITE RATIONAL METHOD ANALYSIS, AREAS “A”

Riverside County Rational Hydrology Program

CIVILCADD/CIVILDESIGN Engineering Software, (c) 1989 - 2001 Version 6.4
Rational Hydrology Study Date: 10/26/21 File:ARA100.out

302.01.21
100-YEAR RATIONAL METHOD FOR AREA A
OFFSITE AREA
FN: ARA100.RRV

***** Hydrology Study Control Information *****

English (in-lb) Units used in input data file

TRI-8 Builders - S/N 615

Rational Method Hydrology Program based on
Riverside County Flood Control & Water Conservation District
1978 hydrology manual

Storm event (year) = 100.00 Antecedent Moisture Condition = 2

2 year, 1 hour precipitation = 0.720(In.)
100 year, 1 hour precipitation = 2.100(In.)

Storm event year = 100.0
Calculated rainfall intensity data:
1 hour intensity = 2.100(In/Hr)
Slope of intensity duration curve = 0.5500

+++++
Process from Point/Station 101.000 to Point/Station 102.000
**** INITIAL AREA EVALUATION ****

Initial area flow distance = 974.000(Ft.)
Top (of initial area) elevation = 4636.000(Ft.)
Bottom (of initial area) elevation = 4511.000(Ft.)
Difference in elevation = 125.000(Ft.)
Slope = 0.12834 s(percent)= 12.83
TC = $k(0.710)*[(\text{length}^3)/(\text{elevation change})]^{0.2}$
Initial area time of concentration = 16.788 min.
Rainfall intensity = 4.231(In/Hr) for a 100.0 year storm
UNDEVELOPED (fair cover) subarea
Runoff Coefficient = 0.849
Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 1.000
RI index for soil(AMC 2) = 84.00
Pervious area fraction = 1.000; Impervious fraction = 0.000
Initial subarea runoff = 11.855(CFS)
Total initial stream area = 3.300(Ac.)
Pervious area fraction = 1.000

+++++
Process from Point/Station 102.000 to Point/Station 103.000
**** NATURAL CHANNEL TIME + SUBAREA FLOW ADDITION ****

Top of natural channel elevation = 4511.000 (Ft.)
End of natural channel elevation = 4423.000 (Ft.)
Length of natural channel = 623.000 (Ft.)
Estimated mean flow rate at midpoint of channel = 21.733 (CFS)

Natural valley channel type used
L.A. County flood control district formula for channel velocity:
Velocity(ft/s) = $(7 + 8(q(\text{English Units})^{.352}) (\text{slope}^{0.5}))$
Velocity using mean channel flow = 11.52 (Ft/s)

Correction to map slope used on extremely rugged channels with
drops and waterfalls (Plate D-6.2)
Normal channel slope = 0.1413
Corrected/adjusted channel slope = 0.1413
Travel time = 0.90 min. TC = 17.69 min.

Adding area flow to channel
UNDEVELOPED (fair cover) subarea
Runoff Coefficient = 0.848
Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 1.000
RI index for soil(AMC 2) = 84.00
Pervious area fraction = 1.000; Impervious fraction = 0.000
Rainfall intensity = 4.111 (In/Hr) for a 100.0 year storm
Subarea runoff = 19.166 (CFS) for 5.500 (Ac.)
Total runoff = 31.020 (CFS) Total area = 8.800 (Ac.)

+++++
Process from Point/Station 103.000 to Point/Station 104.000
**** NATURAL CHANNEL TIME + SUBAREA FLOW ADDITION ****

Top of natural channel elevation = 4423.000 (Ft.)
End of natural channel elevation = 4401.000 (Ft.)
Length of natural channel = 558.000 (Ft.)
Estimated mean flow rate at midpoint of channel = 39.480 (CFS)

Natural valley channel type used
L.A. County flood control district formula for channel velocity:
Velocity(ft/s) = $(7 + 8(q(\text{English Units})^{.352}) (\text{slope}^{0.5}))$
Velocity using mean channel flow = 7.18 (Ft/s)

Correction to map slope used on extremely rugged channels with
drops and waterfalls (Plate D-6.2)
Normal channel slope = 0.0394
Corrected/adjusted channel slope = 0.0394
Travel time = 1.29 min. TC = 18.98 min.

Adding area flow to channel
UNDEVELOPED (fair cover) subarea
Runoff Coefficient = 0.779

Decimal fraction soil group A = 0.460
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.540
RI index for soil(AMC 2) = 68.36
Pervious area fraction = 1.000; Impervious fraction = 0.000
Rainfall intensity = 3.954(In/Hr) for a 100.0 year storm
Subarea runoff = 14.777(CFS) for 4.800(Ac.)
Total runoff = 45.797(CFS) Total area = 13.600(Ac.)

++++
Process from Point/Station 104.000 to Point/Station 105.000
**** NATURAL CHANNEL TIME + SUBAREA FLOW ADDITION ****

Top of natural channel elevation = 4401.000(Ft.)
End of natural channel elevation = 4382.000(Ft.)
Length of natural channel = 1137.000(Ft.)
Estimated mean flow rate at midpoint of channel = 107.253(CFS)

Natural valley channel type used
L.A. County flood control district formula for channel velocity:
Velocity(ft/s) = $(7 + 8(q(\text{English Units})^{.352}) (\text{slope}^{0.5}))$
Velocity using mean channel flow = 6.27(Ft/s)

Correction to map slope used on extremely rugged channels with
drops and waterfalls (Plate D-6.2)
Normal channel slope = 0.0167
Corrected/adjusted channel slope = 0.0167
Travel time = 3.02 min. TC = 22.01 min.

Adding area flow to channel
UNDEVELOPED (fair cover) subarea
Runoff Coefficient = 0.713
Decimal fraction soil group A = 0.760
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.240
RI index for soil(AMC 2) = 58.16
Pervious area fraction = 1.000; Impervious fraction = 0.000
Rainfall intensity = 3.646(In/Hr) for a 100.0 year storm
Subarea runoff = 94.814(CFS) for 36.500(Ac.)
Total runoff = 140.611(CFS) Total area = 50.100(Ac.)

++++
Process from Point/Station 105.000 to Point/Station 106.000
**** NATURAL CHANNEL TIME + SUBAREA FLOW ADDITION ****

Top of natural channel elevation = 4382.000(Ft.)
End of natural channel elevation = 4380.000(Ft.)
Length of natural channel = 491.000(Ft.)
Estimated mean flow rate at midpoint of channel = 164.327(CFS)

Natural valley channel type used
L.A. County flood control district formula for channel velocity:
Velocity(ft/s) = $(7 + 8(q(\text{English Units})^{.352}) (\text{slope}^{0.5}))$
Velocity using mean channel flow = 3.52(Ft/s)

Correction to map slope used on extremely rugged channels with drops and waterfalls (Plate D-6.2)

Normal channel slope = 0.0041
Corrected/adjusted channel slope = 0.0041
Travel time = 2.32 min. TC = 24.33 min.

Adding area flow to channel

UNDEVELOPED (fair cover) subarea
Runoff Coefficient = 0.649
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil(AMC 2) = 50.00
Pervious area fraction = 1.000; Impervious fraction = 0.000
Rainfall intensity = 3.450(In/Hr) for a 100.0 year storm
Subarea runoff = 37.846(CFS) for 16.900(Ac.)
Total runoff = 178.457(CFS) Total area = 67.000(Ac.)

++++
Process from Point/Station 106.000 to Point/Station 109.000
**** NATURAL CHANNEL TIME + SUBAREA FLOW ADDITION ****

Top of natural channel elevation = 4380.000(Ft.)
End of natural channel elevation = 4378.500(Ft.)
Length of natural channel = 518.000(Ft.)
Estimated mean flow rate at midpoint of channel = 206.690(CFS)

Natural valley channel type used

L.A. County flood control district formula for channel velocity:
Velocity(ft/s) = $(7 + 8(q(\text{English Units})^{.352})(\text{slope}^{.5}))$
Velocity using mean channel flow = 3.19(Ft/s)

Correction to map slope used on extremely rugged channels with drops and waterfalls (Plate D-6.2)

Normal channel slope = 0.0029
Corrected/adjusted channel slope = 0.0029
Travel time = 2.71 min. TC = 27.04 min.

Adding area flow to channel

USER INPUT of soil data for subarea
Runoff Coefficient = 0.622
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil(AMC 2) = 46.00
Pervious area fraction = 0.950; Impervious fraction = 0.050
Rainfall intensity = 3.255(In/Hr) for a 100.0 year storm
Subarea runoff = 42.954(CFS) for 21.200(Ac.)
Total runoff = 221.411(CFS) Total area = 88.200(Ac.)

++++
Process from Point/Station 106.000 to Point/Station 109.000
**** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 1

Stream flow area = 88.200 (Ac.)

Runoff from this stream = 221.411 (CFS)

Time of concentration = 27.04 min.

Rainfall intensity = 3.255 (In/Hr)

Program is now starting with Main Stream No. 2

++++
Process from Point/Station 107.000 to Point/Station 108.000
**** INITIAL AREA EVALUATION ****

Initial area flow distance = 554.000 (Ft.)

Top (of initial area) elevation = 4389.000 (Ft.)

Bottom (of initial area) elevation = 4382.000 (Ft.)

Difference in elevation = 7.000 (Ft.)

Slope = 0.01264 s(percent) = 1.26

$TC = k(0.710) * [(length^3) / (elevation\ change)]^{0.2}$

Initial area time of concentration = 21.298 min.

Rainfall intensity = 3.712 (In/Hr) for a 100.0 year storm

UNDEVELOPED (fair cover) subarea

Runoff Coefficient = 0.662

Decimal fraction soil group A = 1.000

Decimal fraction soil group B = 0.000

Decimal fraction soil group C = 0.000

Decimal fraction soil group D = 0.000

RI index for soil (AMC 2) = 50.00

Pervious area fraction = 1.000; Impervious fraction = 0.000

Initial subarea runoff = 5.899 (CFS)

Total initial stream area = 2.400 (Ac.)

Pervious area fraction = 1.000

++++
Process from Point/Station 108.000 to Point/Station 109.000
**** NATURAL CHANNEL TIME + SUBAREA FLOW ADDITION ****

Top of natural channel elevation = 4382.000 (Ft.)

End of natural channel elevation = 4378.500 (Ft.)

Length of natural channel = 724.000 (Ft.)

Estimated mean flow rate at midpoint of channel = 10.201 (CFS)

Natural valley channel type used

L.A. County flood control district formula for channel velocity:

Velocity (ft/s) = $(7 + 8(q(\text{English Units})^{0.352}) (\text{slope}^{0.5}))$

Velocity using mean channel flow = 1.75 (Ft/s)

Correction to map slope used on extremely rugged channels with
drops and waterfalls (Plate D-6.2)

Normal channel slope = 0.0048

Corrected/adjusted channel slope = 0.0048

Travel time = 6.91 min. TC = 28.21 min.

Adding area flow to channel

USER INPUT of soil data for subarea

Runoff Coefficient = 0.618

Decimal fraction soil group A = 1.000

Decimal fraction soil group B = 0.000

Decimal fraction soil group C = 0.000
 Decimal fraction soil group D = 0.000
 RI index for soil(AMC 2) = 46.00
 Pervious area fraction = 0.950; Impervious fraction = 0.050
 Rainfall intensity = 3.181(In/Hr) for a 100.0 year storm
 Subarea runoff = 6.880(CFS) for 3.500(Ac.)
 Total runoff = 12.779(CFS) Total area = 5.900(Ac.)

++++++
 Process from Point/Station 108.000 to Point/Station 109.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 2
 Stream flow area = 5.900(Ac.)
 Runoff from this stream = 12.779(CFS)
 Time of concentration = 28.21 min.
 Rainfall intensity = 3.181(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	221.411	27.04	3.255
2	12.779	28.21	3.181

Largest stream flow has longer or shorter time of concentration

$Q_p = 221.411 + \text{sum of}$
 $\frac{Q_a}{T_b/T_a}$
 $12.779 * 0.959 = 12.250$
 $Q_p = 233.660$

Total of 2 main streams to confluence:

Flow rates before confluence point:

221.411 12.779

Area of streams before confluence:

88.200 5.900

Results of confluence:

Total flow rate = 233.660(CFS)

Time of concentration = 27.039 min.

Effective stream area after confluence = 94.100(Ac.)

End of computations, total study area = 94.10 (Ac.)

The following figures may

be used for a unit hydrograph study of the same area.

Area averaged pervious area fraction(A_p) = 0.987

Area averaged RI index number = 56.2

Riverside County Rational Hydrology Program

CIVILCADD/CIVILDESIGN Engineering Software, (c) 1989 - 2001 Version 6.4
Rational Hydrology Study Date: 10/26/21 File:ARA10.out

302.01.21
10-YEAR RATIONAL METHOD FOR AREA A
OFFSITE AREA
FN: ARA10.RRV

***** Hydrology Study Control Information *****

English (in-lb) Units used in input data file

TRI-8 Builders - S/N 615

Rational Method Hydrology Program based on
Riverside County Flood Control & Water Conservation District
1978 hydrology manual

Storm event (year) = 10.00 Antecedent Moisture Condition = 2

2 year, 1 hour precipitation = 0.720(In.)
100 year, 1 hour precipitation = 2.100(In.)

Storm event year = 10.0
Calculated rainfall intensity data:
1 hour intensity = 1.288(In/Hr)
Slope of intensity duration curve = 0.5500

+++++
Process from Point/Station 101.000 to Point/Station 102.000
**** INITIAL AREA EVALUATION ****

Initial area flow distance = 974.000(Ft.)
Top (of initial area) elevation = 4636.000(Ft.)
Bottom (of initial area) elevation = 4511.000(Ft.)
Difference in elevation = 125.000(Ft.)
Slope = 0.12834 s(percent)= 12.83
TC = $k(0.710)*[(\text{length}^3)/(\text{elevation change})]^{0.2}$
Initial area time of concentration = 16.788 min.
Rainfall intensity = 2.595(In/Hr) for a 10.0 year storm
UNDEVELOPED (fair cover) subarea
Runoff Coefficient = 0.820
Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 1.000
RI index for soil(AMC 2) = 84.00
Pervious area fraction = 1.000; Impervious fraction = 0.000
Initial subarea runoff = 7.019(CFS)
Total initial stream area = 3.300(Ac.)
Pervious area fraction = 1.000

+++++
Process from Point/Station 102.000 to Point/Station 103.000
**** NATURAL CHANNEL TIME + SUBAREA FLOW ADDITION ****

Top of natural channel elevation = 4511.000 (Ft.)
End of natural channel elevation = 4423.000 (Ft.)
Length of natural channel = 623.000 (Ft.)
Estimated mean flow rate at midpoint of channel = 12.868 (CFS)

Natural valley channel type used
L.A. County flood control district formula for channel velocity:
Velocity(ft/s) = $(7 + 8(q(\text{English Units})^{.352}) (\text{slope}^{0.5}))$
Velocity using mean channel flow = 10.02 (Ft/s)

Correction to map slope used on extremely rugged channels with
drops and waterfalls (Plate D-6.2)
Normal channel slope = 0.1413
Corrected/adjusted channel slope = 0.1413
Travel time = 1.04 min. TC = 17.82 min.

Adding area flow to channel
UNDEVELOPED (fair cover) subarea
Runoff Coefficient = 0.817
Decimal fraction soil group A = 0.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 1.000
RI index for soil(AMC 2) = 84.00
Pervious area fraction = 1.000; Impervious fraction = 0.000
Rainfall intensity = 2.510 (In/Hr) for a 10.0 year storm
Subarea runoff = 11.285 (CFS) for 5.500 (Ac.)
Total runoff = 18.304 (CFS) Total area = 8.800 (Ac.)

+++++
Process from Point/Station 103.000 to Point/Station 104.000
**** NATURAL CHANNEL TIME + SUBAREA FLOW ADDITION ****

Top of natural channel elevation = 4423.000 (Ft.)
End of natural channel elevation = 4401.000 (Ft.)
Length of natural channel = 558.000 (Ft.)
Estimated mean flow rate at midpoint of channel = 23.296 (CFS)

Natural valley channel type used
L.A. County flood control district formula for channel velocity:
Velocity(ft/s) = $(7 + 8(q(\text{English Units})^{.352}) (\text{slope}^{0.5}))$
Velocity using mean channel flow = 6.20 (Ft/s)

Correction to map slope used on extremely rugged channels with
drops and waterfalls (Plate D-6.2)
Normal channel slope = 0.0394
Corrected/adjusted channel slope = 0.0394
Travel time = 1.50 min. TC = 19.32 min.

Adding area flow to channel
UNDEVELOPED (fair cover) subarea
Runoff Coefficient = 0.716

Decimal fraction soil group A = 0.460
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.540
RI index for soil(AMC 2) = 68.36
Pervious area fraction = 1.000; Impervious fraction = 0.000
Rainfall intensity = 2.401(In/Hr) for a 10.0 year storm
Subarea runoff = 8.253(CFS) for 4.800(Ac.)
Total runoff = 26.557(CFS) Total area = 13.600(Ac.)

++++
Process from Point/Station 104.000 to Point/Station 105.000
**** NATURAL CHANNEL TIME + SUBAREA FLOW ADDITION ****

Top of natural channel elevation = 4401.000(Ft.)
End of natural channel elevation = 4382.000(Ft.)
Length of natural channel = 1137.000(Ft.)
Estimated mean flow rate at midpoint of channel = 62.193(CFS)

Natural valley channel type used
L.A. County flood control district formula for channel velocity:
Velocity(ft/s) = $(7 + 8(q(\text{English Units})^{.352}) (\text{slope}^{.5}))$
Velocity using mean channel flow = 5.33(Ft/s)

Correction to map slope used on extremely rugged channels with
drops and waterfalls (Plate D-6.2)
Normal channel slope = 0.0167
Corrected/adjusted channel slope = 0.0167
Travel time = 3.55 min. TC = 22.88 min.

Adding area flow to channel
UNDEVELOPED (fair cover) subarea
Runoff Coefficient = 0.626
Decimal fraction soil group A = 0.760
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.240
RI index for soil(AMC 2) = 58.16
Pervious area fraction = 1.000; Impervious fraction = 0.000
Rainfall intensity = 2.188(In/Hr) for a 10.0 year storm
Subarea runoff = 49.980(CFS) for 36.500(Ac.)
Total runoff = 76.537(CFS) Total area = 50.100(Ac.)

++++
Process from Point/Station 105.000 to Point/Station 106.000
**** NATURAL CHANNEL TIME + SUBAREA FLOW ADDITION ****

Top of natural channel elevation = 4382.000(Ft.)
End of natural channel elevation = 4380.000(Ft.)
Length of natural channel = 491.000(Ft.)
Estimated mean flow rate at midpoint of channel = 89.446(CFS)

Natural valley channel type used
L.A. County flood control district formula for channel velocity:
Velocity(ft/s) = $(7 + 8(q(\text{English Units})^{.352}) (\text{slope}^{.5}))$
Velocity using mean channel flow = 2.93(Ft/s)

Correction to map slope used on extremely rugged channels with drops and waterfalls (Plate D-6.2)

Normal channel slope = 0.0041
Corrected/adjusted channel slope = 0.0041
Travel time = 2.79 min. TC = 25.67 min.

Adding area flow to channel

UNDEVELOPED (fair cover) subarea
Runoff Coefficient = 0.546
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil(AMC 2) = 50.00
Pervious area fraction = 1.000; Impervious fraction = 0.000
Rainfall intensity = 2.054(In/Hr) for a 10.0 year storm
Subarea runoff = 18.944(CFS) for 16.900(Ac.)
Total runoff = 95.481(CFS) Total area = 67.000(Ac.)

++++
Process from Point/Station 106.000 to Point/Station 109.000
**** NATURAL CHANNEL TIME + SUBAREA FLOW ADDITION ****

Top of natural channel elevation = 4380.000(Ft.)
End of natural channel elevation = 4378.500(Ft.)
Length of natural channel = 518.000(Ft.)
Estimated mean flow rate at midpoint of channel = 110.587(CFS)

Natural valley channel type used

L.A. County flood control district formula for channel velocity:
Velocity(ft/s) = $(7 + 8(q(\text{English Units})^{.352})(\text{slope}^{.5}))$
Velocity using mean channel flow = 2.63(Ft/s)

Correction to map slope used on extremely rugged channels with drops and waterfalls (Plate D-6.2)

Normal channel slope = 0.0029
Corrected/adjusted channel slope = 0.0029
Travel time = 3.28 min. TC = 28.95 min.

Adding area flow to channel

USER INPUT of soil data for subarea
Runoff Coefficient = 0.516
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil(AMC 2) = 46.00
Pervious area fraction = 0.950; Impervious fraction = 0.050
Rainfall intensity = 1.923(In/Hr) for a 10.0 year storm
Subarea runoff = 21.044(CFS) for 21.200(Ac.)
Total runoff = 116.526(CFS) Total area = 88.200(Ac.)

++++
Process from Point/Station 106.000 to Point/Station 109.000
**** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 1

Stream flow area = 88.200 (Ac.)

Runoff from this stream = 116.526 (CFS)

Time of concentration = 28.95 min.

Rainfall intensity = 1.923 (In/Hr)

Program is now starting with Main Stream No. 2

++++
Process from Point/Station 107.000 to Point/Station 108.000
**** INITIAL AREA EVALUATION ****

Initial area flow distance = 554.000 (Ft.)
Top (of initial area) elevation = 4389.000 (Ft.)
Bottom (of initial area) elevation = 4382.000 (Ft.)
Difference in elevation = 7.000 (Ft.)
Slope = 0.01264 s(percent) = 1.26
TC = $k(0.710) * [(length^3) / (elevation\ change)]^{0.2}$
Initial area time of concentration = 21.298 min.
Rainfall intensity = 2.276 (In/Hr) for a 10.0 year storm
UNDEVELOPED (fair cover) subarea
Runoff Coefficient = 0.568
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000
Decimal fraction soil group C = 0.000
Decimal fraction soil group D = 0.000
RI index for soil (AMC 2) = 50.00
Pervious area fraction = 1.000; Impervious fraction = 0.000
Initial subarea runoff = 3.101 (CFS)
Total initial stream area = 2.400 (Ac.)
Pervious area fraction = 1.000

++++
Process from Point/Station 108.000 to Point/Station 109.000
**** NATURAL CHANNEL TIME + SUBAREA FLOW ADDITION ****

Top of natural channel elevation = 4382.000 (Ft.)
End of natural channel elevation = 4378.500 (Ft.)
Length of natural channel = 724.000 (Ft.)
Estimated mean flow rate at midpoint of channel = 5.361 (CFS)

Natural valley channel type used
L.A. County flood control district formula for channel velocity:
Velocity (ft/s) = $(7 + 8(q(\text{English Units})^{0.352}) (\text{slope}^{0.5}))$
Velocity using mean channel flow = 1.49 (Ft/s)

Correction to map slope used on extremely rugged channels with
drops and waterfalls (Plate D-6.2)
Normal channel slope = 0.0048
Corrected/adjusted channel slope = 0.0048
Travel time = 8.09 min. TC = 29.39 min.

Adding area flow to channel
USER INPUT of soil data for subarea
Runoff Coefficient = 0.515
Decimal fraction soil group A = 1.000
Decimal fraction soil group B = 0.000

Decimal fraction soil group C = 0.000
 Decimal fraction soil group D = 0.000
 RI index for soil(AMC 2) = 46.00
 Pervious area fraction = 0.950; Impervious fraction = 0.050
 Rainfall intensity = 1.907(In/Hr) for a 10.0 year storm
 Subarea runoff = 3.434(CFS) for 3.500(Ac.)
 Total runoff = 6.535(CFS) Total area = 5.900(Ac.)

++++++
 Process from Point/Station 108.000 to Point/Station 109.000
 **** CONFLUENCE OF MAIN STREAMS ****

The following data inside Main Stream is listed:

In Main Stream number: 2
 Stream flow area = 5.900(Ac.)
 Runoff from this stream = 6.535(CFS)
 Time of concentration = 29.39 min.
 Rainfall intensity = 1.907(In/Hr)
 Summary of stream data:

Stream No.	Flow rate (CFS)	TC (min)	Rainfall Intensity (In/Hr)
1	116.526	28.95	1.923
2	6.535	29.39	1.907

Largest stream flow has longer or shorter time of concentration

$Q_p = 116.526 + \text{sum of}$
 $Q_a \quad T_b/T_a$
 $6.535 * 0.985 = 6.437$
 $Q_p = 122.963$

Total of 2 main streams to confluence:

Flow rates before confluence point:

116.526 6.535

Area of streams before confluence:

88.200 5.900

Results of confluence:

Total flow rate = 122.963(CFS)

Time of concentration = 28.951 min.

Effective stream area after confluence = 94.100(Ac.)

End of computations, total study area = 94.10 (Ac.)

The following figures may

be used for a unit hydrograph study of the same area.

Area averaged pervious area fraction(A_p) = 0.987

Area averaged RI index number = 56.2

APPENDIX F: OFFSITE CROSS SECTION CALCULATIONS

APPENDIX F.1: CROSS SECTION A CALCULATION

Worksheet for SECTION A

Results

Critical Depth	0.41	ft
Critical Slope	0.02530	ft/ft
Velocity	3.46	ft/s
Velocity Head	0.19	ft
Specific Energy	0.59	ft
Froude Number	1.04	
Flow Type	Supercritical	

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.40	ft
Critical Depth	0.41	ft
Channel Slope	0.02770	ft/ft
Critical Slope	0.02530	ft/ft

Messages

Notes

Water Surface Elevation is set
at Crown Line of Apple Canyon Road

Cross Section for SECTION A

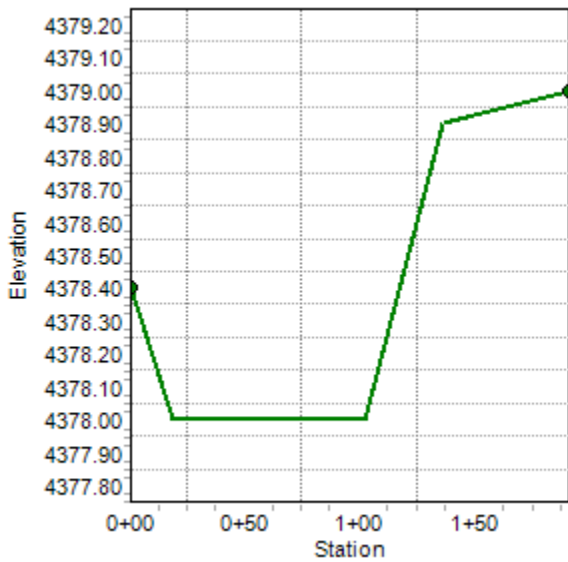
Project Description

Friction Method Manning Formula
Solve For Discharge

Input Data

Channel Slope	0.02770	ft/ft
Normal Depth	0.40	ft
Discharge	138.74	ft ³ /s

Cross Section Image



APPENDIX F.2: CROSS SECTION B CALCULATION

Worksheet for SECTION B

Results

Critical Depth	0.64	ft
Critical Slope	0.02170	ft/ft
Velocity	3.27	ft/s
Velocity Head	0.17	ft
Specific Energy	0.97	ft
Froude Number	0.70	
Flow Type	Subcritical	

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.80	ft
Critical Depth	0.64	ft
Channel Slope	0.01000	ft/ft
Critical Slope	0.02170	ft/ft

Messages

Notes

Water Surface Elevation is set
at Crown Line of Apple Canyon Road

Cross Section for SECTION B

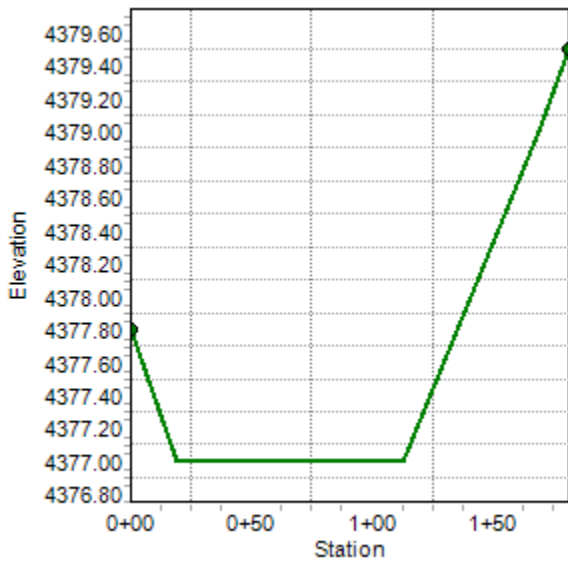
Project Description

Friction Method	Manning Formula
Solve For	Discharge

Input Data

Channel Slope	0.01000	ft/ft
Normal Depth	0.80	ft
Discharge	299.08	ft ³ /s

Cross Section Image



APPENDIX F.3: CROSS SECTION C CALCULATION

Worksheet for SECTION C

Results

Critical Slope	0.02381	ft/ft
Velocity	3.02	ft/s
Velocity Head	0.14	ft
Specific Energy	0.84	ft
Froude Number	0.78	
Flow Type	Subcritical	

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.70	ft
Critical Depth	0.61	ft
Channel Slope	0.01380	ft/ft
Critical Slope	0.02381	ft/ft

Messages

Notes

Water Surface Elevation is set
at Crown Line of Apple Canyon Road

Cross Section for SECTION C

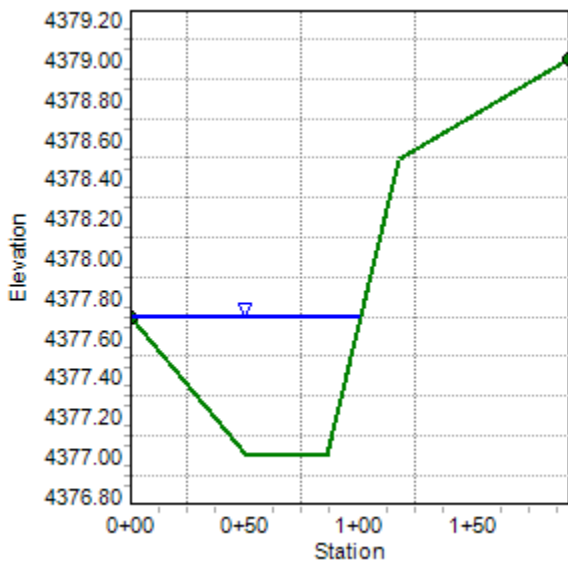
Project Description

Friction Method	Manning Formula
Solve For	Discharge

Input Data

Channel Slope	0.01380	ft/ft
Normal Depth	0.70	ft
Discharge	143.06	ft ³ /s

Cross Section Image



APPENDIX F.4: CROSS SECTION D CALCULATION

Worksheet for SECTION D

Results

Critical Slope	0.02787	ft/ft
Velocity	2.23	ft/s
Velocity Head	0.08	ft
Specific Energy	0.68	ft
Froude Number	0.72	
Flow Type	Subcritical	

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.60	ft
Critical Depth	0.53	ft
Channel Slope	0.01380	ft/ft
Critical Slope	0.02787	ft/ft

Messages

Notes

Water Surface Elevation is set
at Crown Line of Apple Canyon Road

Cross Section for SECTION D

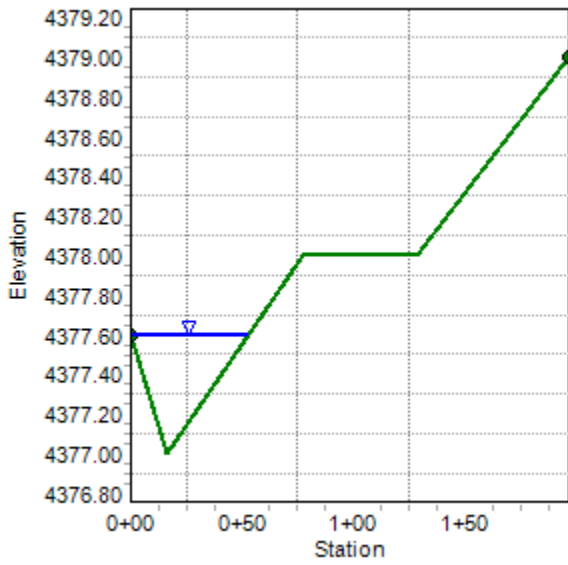
Project Description

Friction Method	Manning Formula
Solve For	Discharge

Input Data

Channel Slope	0.01380	ft/ft
Normal Depth	0.60	ft
Discharge	35.56	ft ³ /s

Cross Section Image



APPENDIX F.5: CROSS SECTION E CALCULATION

Worksheet for SECTION E

Project Description

Friction Method Manning Formula
Solve For Discharge

Input Data

Channel Slope 0.00400 ft/ft
Normal Depth 0.60 ft
Section Definitions

Station (ft)	Elevation (ft)
0+00	4377.60
0+31	4377.00
0+71	4377.00
1+15	4377.50

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00, 4377.60)	(1+15, 4377.50)	0.035

Options

Current Roughness Weighted Method Pavlovskii's Method
Open Channel Weighting Method Pavlovskii's Method
Closed Channel Weighting Method Pavlovskii's Method

Results

Discharge 74.10 ft³/s
Elevation Range 4377.00 to 4377.60 ft
Flow Area 48.88 ft²
Wetted Perimeter 115.19 ft
Hydraulic Radius 0.42 ft
Top Width 115.08 ft
Normal Depth 0.60 ft
Critical Depth 0.38 ft
Critical Slope 0.02760 ft/ft

Worksheet for SECTION E

Results

Velocity	1.52	ft/s
Velocity Head	0.04	ft
Specific Energy	0.64	ft
Froude Number	0.41	
Flow Type	Subcritical	

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.60	ft
Critical Depth	0.38	ft
Channel Slope	0.00400	ft/ft
Critical Slope	0.02760	ft/ft

Messages

Notes

Water Surface Elevation is set
at Crown Line of Apple Canyon Road

Cross Section for SECTION E

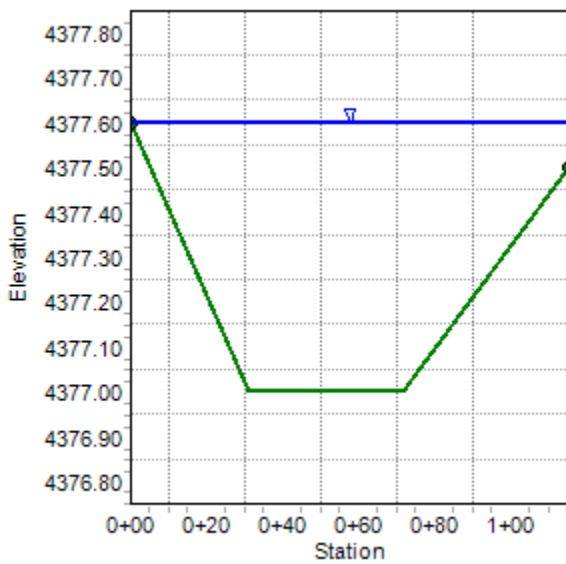
Project Description

Friction Method	Manning Formula
Solve For	Discharge

Input Data

Channel Slope	0.00400	ft/ft
Normal Depth	0.60	ft
Discharge	74.10	ft ³ /s

Cross Section Image



APPENDIX F.6: CROSS SECTION H CALCULATION

Worksheet for SECTION H

Project Description

Friction Method Manning Formula
Solve For Discharge

Input Data

Channel Slope 0.00400 ft/ft
Normal Depth 2.08 ft
Section Definitions

Station (ft)	Elevation (ft)
0+00	4383.00
1+11	4382.00
1+84	4382.00
2+45	4381.00
3+83	4380.00
4+06	4381.00
5+21	4382.00
5+95	4383.00

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00, 4383.00)	(5+95, 4383.00)	0.035

Options

Current Roughness Weighted Method Pavlovskii's Method
Open Channel Weighting Method Pavlovskii's Method
Closed Channel Weighting Method Pavlovskii's Method

Results

Discharge 873.24 ft³/s
Elevation Range 4380.00 to 4383.00 ft
Flow Area 361.57 ft²
Wetted Perimeter 423.86 ft
Hydraulic Radius 0.85 ft

Worksheet for SECTION H

Results

Top Width	423.82	ft
Normal Depth	2.08	ft
Critical Depth	1.49	ft
Critical Slope	0.01983	ft/ft
Velocity	2.42	ft/s
Velocity Head	0.09	ft
Specific Energy	2.17	ft
Froude Number	0.46	
Flow Type	Subcritical	

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	2.08	ft
Critical Depth	1.49	ft
Channel Slope	0.00400	ft/ft
Critical Slope	0.01983	ft/ft

Messages

Notes

Water Surface Elevation is set
at Crown Line of Apple Canyon Road

Cross Section for SECTION H

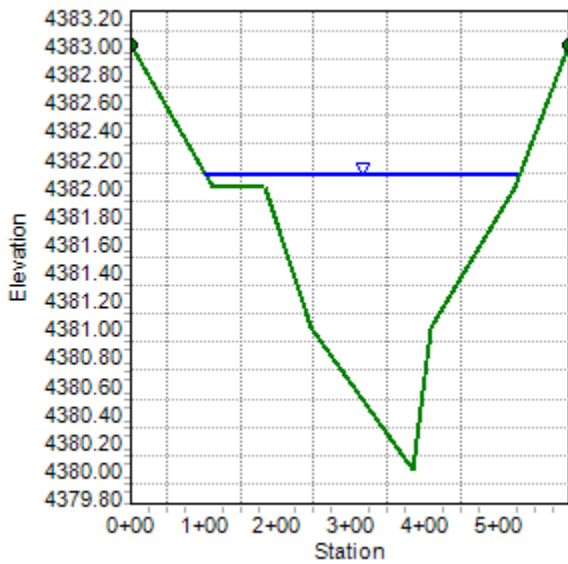
Project Description

Friction Method	Manning Formula
Solve For	Discharge

Input Data

Channel Slope	0.00400	ft/ft
Normal Depth	2.08	ft
Discharge	873.24	ft ³ /s

Cross Section Image



APPENDIX F.7: CROSS SECTION I CALCULATION

Worksheet for SECTION I

Project Description

Friction Method	Manning Formula
Solve For	Discharge

Input Data

Channel Slope	0.00370	ft/ft
Normal Depth	1.00	ft
Section Definitions		

Station (ft)	Elevation (ft)
0+00	4381.00
1+62	4380.00
2+18	4380.00
2+86	4381.00
3+43	4382.00
4+53	4383.00
5+08	4383.00

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00, 4381.00)	(5+08, 4383.00)	0.035

Options

Current Roughness weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

Results

Discharge	314.03	ft ³ /s
Elevation Range	4380.00 to 4383.00 ft	
Flow Area	171.22	ft ²
Wetted Perimeter	286.07	ft
Hydraulic Radius	0.60	ft
Top Width	286.06	ft

Worksheet for SECTION I

Results

Normal Depth	1.00	ft
Critical Depth	0.65	ft
Critical Slope	0.02392	ft/ft
Velocity	1.83	ft/s
Velocity Head	0.05	ft
Specific Energy	1.05	ft
Froude Number	0.42	
Flow Type	Subcritical	

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	1.00	ft
Critical Depth	0.65	ft
Channel Slope	0.00370	ft/ft
Critical Slope	0.02392	ft/ft

Messages

Notes

Water Surface Elevation is set
at Crown Line of Apple Canyon Road

Cross Section for SECTION I

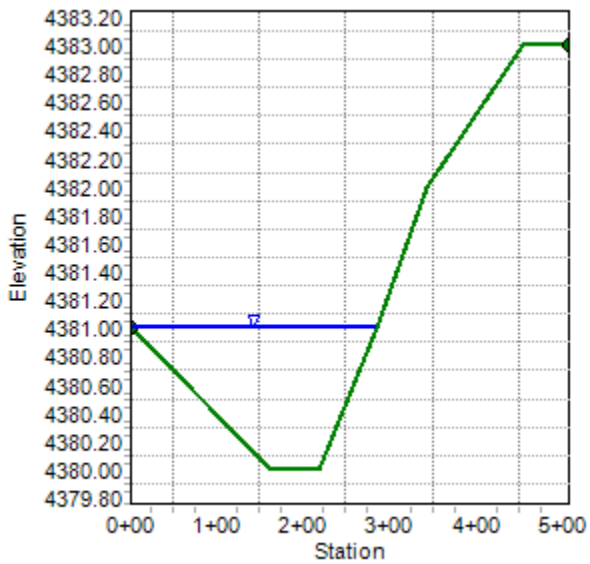
Project Description

Friction Method	Manning Formula
Solve For	Discharge

Input Data

Channel Slope	0.00370	ft/ft
Normal Depth	1.00	ft
Discharge	314.03	ft ³ /s

Cross Section Image



APPENDIX F.8: CROSS SECTION J CALCULATION

Worksheet for SECTION J

Project Description

Friction Method	Manning Formula
Solve For	Discharge

Input Data

Channel Slope	0.00600	ft/ft
Normal Depth	1.00	ft

Section Definitions

Station (ft)	Elevation (ft)
0+00	4380.00
1+14	4379.00
1+35	4379.00
1+91	4380.00
3+34	4380.00
4+21	4381.00
4+61	4382.00
5+25	4382.00

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00, 4380.00)	(5+25, 4382.00)	0.035

Options

Current Roughness Weighted Method	Pavlovskii's Method
Open Channel Weighting Method	Pavlovskii's Method
Closed Channel Weighting Method	Pavlovskii's Method

Results

Discharge	235.27	ft ³ /s
Elevation Range	4379.00 to 4382.00 ft	
Flow Area	105.99	ft ²
Wetted Perimeter	191.10	ft
Hydraulic Radius	0.55	ft

Worksheet for SECTION J

Results

Top Width	191.09	ft
Normal Depth	1.00	ft
Critical Depth	0.75	ft
Critical Slope	0.02370	ft/ft
Velocity	2.22	ft/s
Velocity Head	0.08	ft
Specific Energy	1.08	ft
Froude Number	0.53	
Flow Type	Subcritical	

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	1.00	ft
Critical Depth	0.75	ft
Channel Slope	0.00600	ft/ft
Critical Slope	0.02370	ft/ft

Messages

Notes

Water Surface Elevation is set
at Crown Line of Apple Canyon Road

Cross Section for SECTION J

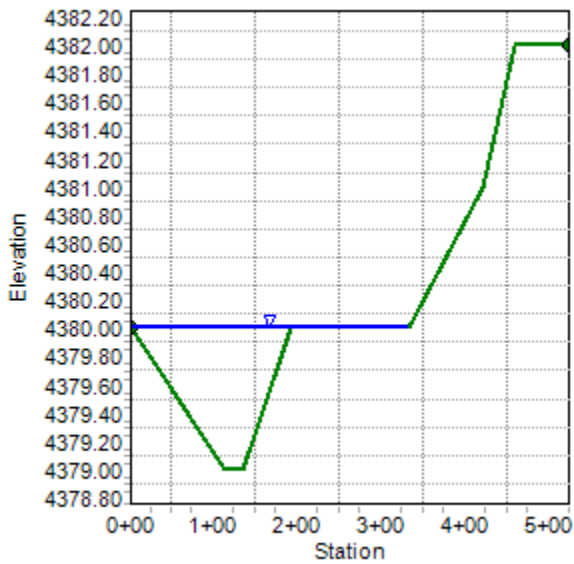
Project Description

Friction Method	Manning Formula
Solve For	Discharge

Input Data

Channel Slope	0.00600	ft/ft
Normal Depth	1.00	ft
Discharge	235.27	ft ³ /s

Cross Section Image



EXCERPTS

EXCERPT A: WEBB & ASSOCIATES STUDY



Technical Memorandum

To: Caroline Legrand – The Ridge
 From: Joseph C. Caldwell, PE – Water Resources Practice Leader, Albert A. Webb Associates
 Date: August 10th, 2020
 Re: Preliminary Hydrology and Floodplain Analysis for The Ridge

Introduction

Albert A. Webb Associates (WEBB) has been contracted by The Ridge to provide a preliminary hydrology and hydraulics study for the Idyllwild Guest Ranch project. This technical memorandum highlights the methodology and results for a preliminary floodplain analysis and is intended to be utilized for due diligence and entitlement purposes.

The Idyllwild Guest Ranch project is the proposed guest ranch facility to be developed in Idyllwild, CA. The general site location can be seen in the attached Appendix A. The site is approximately 37 acres, borders Apple Canyon Road to the north and Highway 74 to the south. The parcel’s potential floodplain is influenced by the adjacent creek, Hurkey Creek.

Hydrology

The existing 100-Year tributary flow rates were calculated utilizing RCFC&WCD’s Synthetic Unit Hydrograph methodology. The associated watershed has been delineated and highlighted in Appendix B. The geometric parameters of the existing model are tabulated in Table 1. The hydrologic model was created with Civil Design v.8.1 and can be referenced in Appendix D.

Parameter	Value	Units
Drainage Area	7,115	ac
Length	37,171	ft
Length from centroid	18,322	ft
Elevation Delta (ΔZ)	4,483	ft
Average Manning’s Value	0.05	NA

Table 1 - Geometric Parameters

Rainfall data for the existing model was obtained from the NOAA 14 atlas and can be referenced in Appendix C. The peak runoffs from various storm events and durations are tabulated in Table 2.

Sub Area	100-Year 3 Hour		100-Year 6 Hour		100-Year 24 Hour	
	Q [cfs]	V [ac-ft]	Q [cfs]	V [ac-ft]	Q [cfs]	V [ac-ft]
Hurkey Creek Watershed	5,769	1,299	6,074	1,548	4,547	2,700

Table 2 – Hurkey Creek Unit Hydrograph Results

The 6-hour storm duration produces the largest peak flows and governs the existing hydraulic model.

Hydraulics

The existing hydraulic model for Hurkey Creek was modeled using HEC-RAS v 5.0.7. The conditions were modeled as a 2D finite volume mesh with an average Manning’s value of 0.06. This Manning’s value provides a conservatively high estimate for the existing floodplain. The 2D mesh was elevated utilizing aerial topography from the project site. The synthetic unit hydrograph from the 100-Year 6-Hour storm event was input at the upstream end of the model. The model was executed for a simulation period of 10 hours to allow the unit hydrograph to peak and recede across the model domain.

The existing Highway 74 bridge was not considered in the 2D model for several reasons; (a) the existing bridge does not have adequate capacity, (b) the 100-year flow is likely to top the Highway 74 road surface, (c) the conveyance associated with the Highway 74 bridge will not affect water surface elevations at the proposed development site. During final design, a more detailed analysis that includes the bridge should be prepared.

The results of the 2D model are documented in several exhibits, all found in Appendix E. The exhibits highlight the peak depth, velocities, and the 100-Year floodplain affecting the development site.

Conclusion

The proposed development is associated with a large offsite watershed that generates significant flows during a 100-Year storm event. Based on 2D RAS modeling, a portion of the parcel is expected to experience some light to moderate flooding. However, the existing flooding does not impact the locations where the proposed development is located.

Further analysis should be prepared as part of preliminary and final engineering.

Appendices

Appendix A – Project Location

Appendix B – Hurkey Creek Delineated Watershed

Appendix C – Hurkey Creek NOAA 14 Rainfall Data

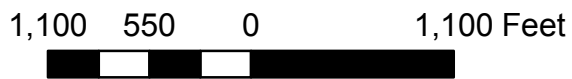
Appendix D – Hurkey Creek Hydrology via Civil Design

Appendix E – 2D RAS Exhibits

APPENDIX A

Project Location

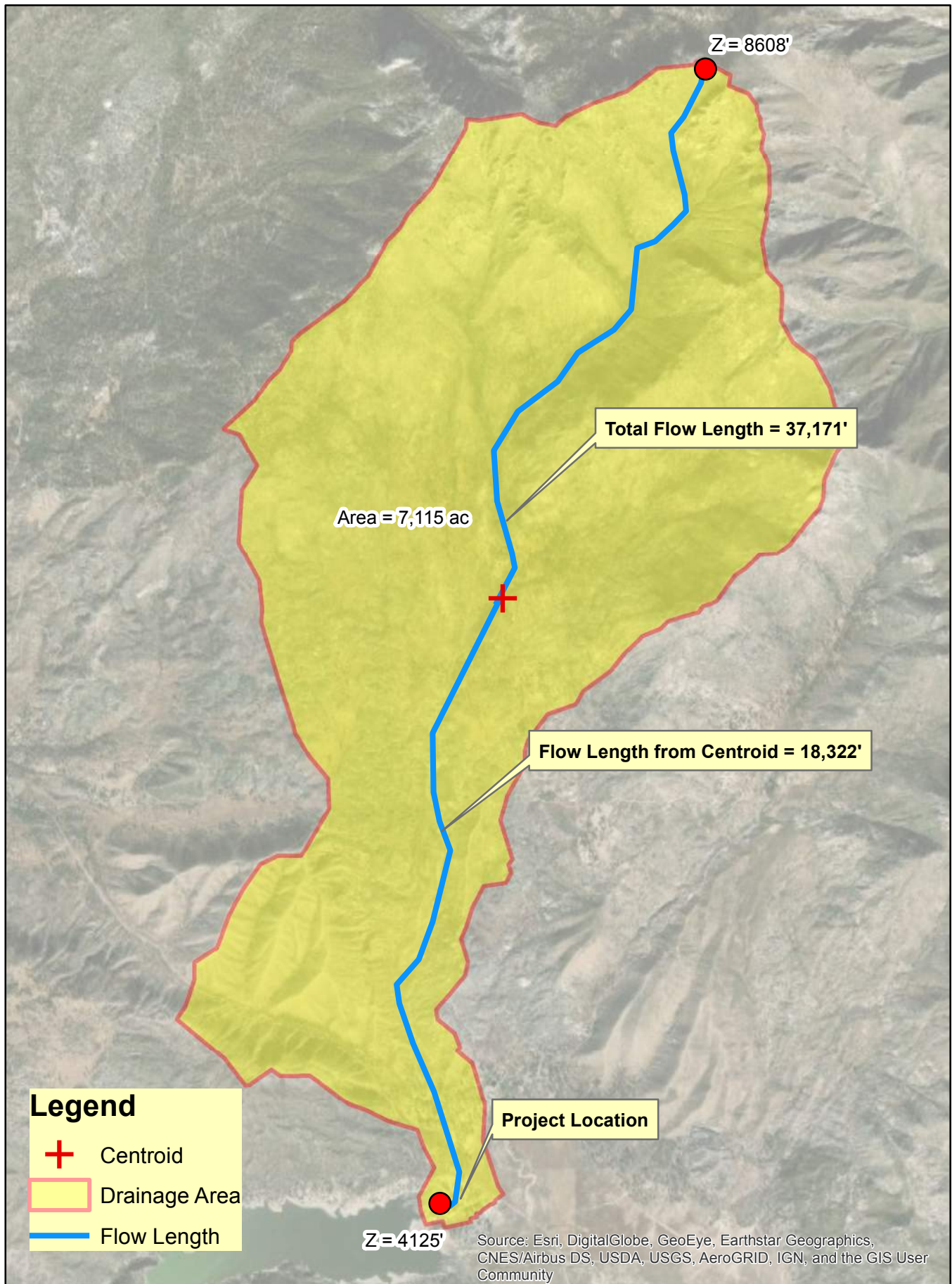
Project Location



APPENDIX B

Hurkey Creek Delineated Watershed

Hurkey Creek Watershed



APPENDIX C

Hurkey Creek NOAA 14 Rainfall Data



NOAA Atlas 14, Volume 6, Version 2
Location name: Mountain Center, California, USA*
Latitude: 33.6753°, Longitude: -116.6793°
Elevation: 4373.1 ft**



* source: ESRI Maps
 ** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

[PF_tabular](#) | [PF_graphical](#) | [Maps_&_aerials](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.163 (0.137-0.197)	0.212 (0.177-0.256)	0.282 (0.235-0.342)	0.344 (0.284-0.421)	0.437 (0.349-0.554)	0.516 (0.402-0.667)	0.602 (0.458-0.799)	0.698 (0.516-0.954)	0.841 (0.595-1.20)	0.963 (0.658-1.42)
10-min	0.234 (0.196-0.283)	0.304 (0.254-0.368)	0.404 (0.337-0.491)	0.494 (0.408-0.604)	0.627 (0.500-0.794)	0.739 (0.577-0.957)	0.863 (0.656-1.15)	1.00 (0.739-1.37)	1.21 (0.854-1.72)	1.38 (0.943-2.04)
15-min	0.283 (0.237-0.342)	0.368 (0.307-0.445)	0.489 (0.407-0.593)	0.597 (0.493-0.730)	0.758 (0.604-0.960)	0.894 (0.698-1.16)	1.04 (0.794-1.39)	1.21 (0.894-1.65)	1.46 (1.03-2.08)	1.67 (1.14-2.47)
30-min	0.412 (0.345-0.498)	0.535 (0.447-0.647)	0.712 (0.593-0.864)	0.869 (0.718-1.06)	1.10 (0.880-1.40)	1.30 (1.01-1.68)	1.52 (1.16-2.02)	1.76 (1.30-2.41)	2.12 (1.50-3.03)	2.43 (1.66-3.60)
60-min	0.556 (0.465-0.672)	0.722 (0.603-0.873)	0.961 (0.800-1.17)	1.17 (0.968-1.43)	1.49 (1.19-1.89)	1.76 (1.37-2.27)	2.05 (1.56-2.72)	2.38 (1.76-3.25)	2.86 (2.03-4.09)	3.28 (2.24-4.85)
2-hr	0.709 (0.593-0.856)	0.922 (0.770-1.12)	1.23 (1.02-1.49)	1.49 (1.23-1.82)	1.88 (1.50-2.38)	2.21 (1.72-2.86)	2.56 (1.95-3.40)	2.95 (2.18-4.04)	3.53 (2.50-5.03)	4.01 (2.74-5.93)
3-hr	0.818 (0.685-0.988)	1.07 (0.890-1.29)	1.41 (1.18-1.72)	1.72 (1.42-2.10)	2.16 (1.73-2.74)	2.53 (1.97-3.27)	2.93 (2.23-3.88)	3.36 (2.48-4.59)	3.99 (2.83-5.70)	4.52 (3.09-6.69)
6-hr	1.12 (0.937-1.35)	1.46 (1.22-1.77)	1.94 (1.62-2.35)	2.35 (1.94-2.88)	2.95 (2.35-3.73)	3.44 (2.68-4.45)	3.96 (3.02-5.26)	4.53 (3.35-6.20)	5.36 (3.79-7.65)	6.04 (4.13-8.94)
12-hr	1.52 (1.27-1.83)	1.98 (1.66-2.40)	2.64 (2.19-3.20)	3.20 (2.64-3.91)	4.01 (3.20-5.08)	4.67 (3.65-6.05)	5.39 (4.10-7.15)	6.16 (4.55-8.42)	7.28 (5.15-10.4)	8.21 (5.61-12.1)
24-hr	2.13 (1.88-2.46)	2.79 (2.47-3.22)	3.72 (3.28-4.31)	4.53 (3.96-5.28)	5.70 (4.83-6.87)	6.67 (5.54-8.21)	7.72 (6.26-9.72)	8.86 (6.99-11.5)	10.5 (7.98-14.2)	11.9 (8.73-16.6)
2-day	2.57 (2.27-2.97)	3.41 (3.02-3.94)	4.61 (4.06-5.33)	5.65 (4.94-6.59)	7.18 (6.08-8.65)	8.45 (7.01-10.4)	9.82 (7.97-12.4)	11.3 (8.95-14.7)	13.6 (10.3-18.2)	15.4 (11.3-21.4)
3-day	2.70 (2.39-3.11)	3.65 (3.22-4.22)	4.99 (4.40-5.78)	6.17 (5.39-7.20)	7.89 (6.68-9.50)	9.31 (7.73-11.4)	10.9 (8.80-13.7)	12.5 (9.90-16.2)	15.0 (11.4-20.2)	17.1 (12.5-23.8)
4-day	2.84 (2.51-3.28)	3.87 (3.42-4.47)	5.34 (4.71-6.18)	6.62 (5.79-7.73)	8.51 (7.21-10.3)	10.1 (8.37-12.4)	11.8 (9.55-14.8)	13.7 (10.8-17.7)	16.4 (12.4-22.1)	18.7 (13.7-26.0)
7-day	3.26 (2.88-3.76)	4.44 (3.92-5.13)	6.15 (5.42-7.12)	7.67 (6.70-8.95)	9.95 (8.42-12.0)	11.9 (9.86-14.6)	14.0 (11.4-17.6)	16.4 (12.9-21.2)	20.0 (15.1-26.9)	23.0 (16.9-32.0)
10-day	3.54 (3.13-4.08)	4.82 (4.25-5.56)	6.69 (5.89-7.74)	8.37 (7.32-9.77)	10.9 (9.24-13.2)	13.1 (10.9-16.1)	15.5 (12.6-19.5)	18.3 (14.4-23.6)	22.4 (17.0-30.2)	26.0 (19.1-36.1)
20-day	4.31 (3.81-4.97)	5.90 (5.21-6.81)	8.23 (7.25-9.53)	10.4 (9.05-12.1)	13.6 (11.5-16.4)	16.4 (13.6-20.2)	19.6 (15.9-24.6)	23.1 (18.3-29.9)	28.6 (21.7-38.5)	33.4 (24.5-46.5)
30-day	5.13 (4.53-5.91)	7.04 (6.22-8.13)	9.86 (8.69-11.4)	12.4 (10.8-14.5)	16.3 (13.8-19.6)	19.6 (16.3-24.1)	23.4 (19.0-29.4)	27.6 (21.8-35.7)	34.1 (25.9-45.9)	39.7 (29.1-55.2)
45-day	5.92 (5.23-6.83)	8.17 (7.22-9.44)	11.5 (10.1-13.3)	14.4 (12.6-16.8)	18.8 (15.9-22.6)	22.5 (18.7-27.7)	26.7 (21.6-33.6)	31.3 (24.7-40.5)	38.3 (29.1-51.6)	44.3 (32.5-61.7)
60-day	6.90 (6.10-7.96)	9.56 (8.45-11.0)	13.4 (11.8-15.5)	16.7 (14.6-19.5)	21.7 (18.4-26.1)	25.9 (21.5-31.8)	30.4 (24.7-38.3)	35.4 (28.0-45.8)	42.9 (32.5-57.7)	49.2 (36.1-68.4)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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APPENDIX D

Hurkey Creek Hydrology via Civil Design

hurkey3100.out

Unit Hydrograph Analysis

Copyright (c) CIVILCADD/CIVILDESIGN, 1989 - 2008, Version 8.1
Study date 08/12/20 File: hurkey3100.out

+++++

Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978

Program License Serial Number 4010

English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used

English Units used in output format

Hurkey Creek Title Revision RNR Aug122020

Drainage Area = 7115.00(Ac.) = 11.117 Sq. Mi.
11.117 Sq. Mi. Drainage Area for Depth-Area Area Adjustment = 7115.00(Ac.) =
11.117 Sq. Mi.
Length along longest watercourse = 37172.00(Ft.)
18294.00(Ft.) Length along longest watercourse measured to centroid =
Length along longest watercourse = 7.040 Mi.
Length along longest watercourse measured to centroid = 3.465 Mi.
Difference in elevation = 4483.00(Ft.)
Slope along watercourse = 636.7761 Ft./Mi.
Average Manning's 'N' = 0.050
Lag time = 1.185 Hr.
Lag time = 71.08 Min.
25% of lag time = 17.77 Min.
40% of lag time = 28.43 Min.
Unit time = 15.00 Min.
Duration of storm = 3 Hour(s)
User Entered Base Flow = 0.00(CFS)

2 YEAR Area rainfall data:

Area(Ac.)[1]	Rainfall(In)[2]	weighting[1*2]
7115.00	1.87	13305.05

100 YEAR Area rainfall data:

Area(Ac.)[1]	Rainfall(In)[2]	weighting[1*2]
7115.00	3.21	22839.15

STORM EVENT (YEAR) = 100.00
Area Averaged 2-Year Rainfall = 1.870(In)
Area Averaged 100-Year Rainfall = 3.210(In)

hurkey3100.out

Point rain (area averaged) = 3.210(In)
 Areal adjustment factor = 96.93 %
 Adjusted average point rain = 3.111(In)

Sub-Area Data:

Area(Ac.)	Runoff Index	Impervious %
2134.000	45.00	0.000
4981.000	83.00	0.200
Total Area Entered = 7115.00(Ac.)		

RI	RI	Infil. Rate	Impervious	Adj. Infil. Rate	Area%	F
AMC2	AMC-2	(In/Hr)	(Dec.%)	(In/Hr)	(Dec.)	(In/Hr)
45.0	45.0	0.621	0.000	0.621	0.300	0.186
83.0	83.0	0.210	0.200	0.172	0.700	0.120
Sum (F) =						0.307

Area averaged mean soil loss (F) (In/Hr) = 0.307
 Minimum soil loss rate ((In/Hr)) = 0.153
 (for 24 hour storm duration)
 Soil loss rate (decimal) = 0.900

Unit Hydrograph
 MOUNTAIN S-Curve

Unit Hydrograph Data

Unit time period (hrs)	Time % of lag	Distribution Graph %	Unit Hydrograph (CFS)
1	0.250	21.104	2.427
2	0.500	42.207	7.119
3	0.750	63.311	13.719
4	1.000	84.415	14.964
5	1.250	105.519	9.772
6	1.500	126.622	6.400
7	1.750	147.726	4.806
8	2.000	168.830	3.913
9	2.250	189.933	3.255
10	2.500	211.037	2.846
11	2.750	232.141	2.552
12	3.000	253.244	2.265
13	3.250	274.348	1.978
14	3.500	295.452	1.778
15	3.750	316.556	1.610
16	4.000	337.659	1.411
17	4.250	358.763	1.265
18	4.500	379.867	1.126
19	4.750	400.970	1.097
20	5.000	422.074	0.982
21	5.250	443.178	0.886
22	5.500	464.281	0.877
23	5.750	485.385	0.846
24	6.000	506.489	0.836
25	6.250	527.593	0.716
26	6.500	548.696	0.675
27	6.750	569.800	0.657
28	7.000	590.904	0.633
29	7.250	612.007	0.619
30	7.500	633.111	0.557
31	7.750	654.215	0.548
32	8.000	675.319	0.520
33	8.250	696.422	0.506
34	8.500	717.526	0.477
35	8.750	738.630	0.423

hurkey3100.out

36	9.000	759.733	0.418	29.943
37	9.250	780.837	0.386	27.678
38	9.500	801.941	0.380	27.239
39	9.750	823.044	0.380	27.239
40	10.000	844.148	0.380	27.239
41	10.250	865.252	0.380	27.239
42	10.500	886.356	0.380	27.239
43	10.750	907.459	0.380	27.239
44	11.000	928.563	0.380	27.239
45	11.250	949.667	0.380	27.239
46	11.500	970.770	0.380	27.239
47	11.750	991.874	0.380	27.239
48	12.000	1012.978	0.336	24.108
			Sum = 100.000	Sum= 7170.586

The following loss rate calculations reflect use of the minimum calculated loss rate subtracted from the Storm Rain to produce the maximum Effective Rain value

Unit	Time (Hr.)	Pattern Percent	Storm Rain (In/Hr)	Loss rate(In./Hr)		Effective (In/Hr)
				Max	Low	
1	0.25	3.70	0.460	0.307	(0.414)	0.154
2	0.50	4.80	0.597	0.307	(0.538)	0.291
3	0.75	5.10	0.635	0.307	(0.571)	0.328
4	1.00	4.90	0.610	0.307	(0.549)	0.303
5	1.25	6.60	0.821	0.307	(0.739)	0.515
6	1.50	7.30	0.909	0.307	(0.818)	0.602
7	1.75	8.40	1.045	0.307	(0.941)	0.739
8	2.00	9.00	1.120	0.307	(1.008)	0.813
9	2.25	12.30	1.531	0.307	(1.378)	1.224
10	2.50	17.60	2.190	0.307	(1.971)	1.884
11	2.75	16.10	2.004	0.307	(1.803)	1.697
12	3.00	4.20	0.523	0.307	(0.470)	0.216

Sum = 100.0 (Loss Rate Not Used) Sum = 8.8

Flood volume = Effective rainfall 2.19(In)
times area 7115.0(Ac.)/[(In)/(Ft.)] = 1299.2(Ac.Ft)
Total soil loss = 0.92(In)
Total soil loss = 545.531(Ac.Ft)
Total rainfall = 3.11(In)
Flood volume = 56594475.5 Cubic Feet
Total soil loss = 23763344.7 Cubic Feet

Peak flow rate of this hydrograph = 5769.287(CFS)

++++
3 - H O U R S T O R M
R u n o f f H y d r o g r a p h

Hydrograph in 15 Minute intervals ((CFS))

Time(h+m)	Volume Ac.Ft	Q(CFS)	0	1450.0	2900.0	4350.0	5800.0
0+15	0.5533	26.78	Q				
0+30	3.2219	129.16	Q				
0+45	10.5966	356.94	V Q				
1+ 0	24.4708	671.51	V Q				
1+15	44.8670	987.18	V Q				
1+30	71.5730	1292.57	V Q				
1+45	106.3759	1684.46	V Q				
2+ 0	151.2174	2170.33	V Q				
2+15	207.6711	2732.36	V Q				

			hurkey3100.out			
2+30	280.0041	3500.92		V		Q
2+45	374.1449	4556.41			V	
3+ 0	488.8097	5549.78				V
3+15	608.0099	5769.29				V
3+30	707.5999	4820.16				V
3+45	779.4404	3477.08				V
4+ 0	832.9980	2592.19				V
4+15	876.1986	2090.91				V
4+30	912.7195	1767.61				V
4+45	944.4297	1534.78				V
5+ 0	972.5204	1359.59				V
5+15	997.5577	1211.80				V
5+30	1019.8467	1078.79				V
5+45	1039.8045	965.96				V
6+ 0	1057.8327	872.56				V
6+15	1074.1278	788.69				V
6+30	1088.8751	713.77				V
6+45	1102.3920	654.21				V
7+ 0	1114.9750	609.02				V
7+15	1126.7593	570.36				V
7+30	1137.7339	531.17				V
7+45	1148.1073	502.08				V
8+ 0	1158.0517	481.31				V
8+15	1167.5216	458.34				V
8+30	1176.4084	430.12				V
8+45	1184.6921	400.94				V
9+ 0	1192.5817	381.85				V
9+15	1200.1513	366.37				V
9+30	1207.3858	350.15				V
9+45	1214.2507	332.26				V
10+ 0	1220.7602	315.06				V
10+15	1226.9991	301.97				V
10+30	1232.9704	289.01				V
10+45	1238.6723	275.98				V
11+ 0	1244.0906	262.24				V
11+15	1249.2828	251.30				V
11+30	1254.3440	244.96				V
11+45	1259.3069	240.20				V
12+ 0	1264.2342	238.48				V
12+15	1269.0641	233.77				V
12+30	1273.7280	225.73				V
12+45	1278.2087	216.87				V
13+ 0	1282.5051	207.94				V
13+15	1286.5061	193.64				V
13+30	1290.1593	176.81				V
13+45	1293.3917	156.45				V
14+ 0	1296.1395	133.00				V
14+15	1298.1554	97.57				V
14+30	1299.1227	46.82				V
14+45	1299.2304	5.21				V

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Unit Hydrograph Analysis

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Study date 08/12/20 File: hurkey6100.out

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Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978

Program License Serial Number 4010

English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used

English Units used in output format

Hurkey Creek Title Revision RNR Aug122020

Drainage Area = 7115.00(Ac.) = 11.117 Sq. Mi.
11.117 Sq. Mi. Drainage Area for Depth-Area Area Adjustment = 7115.00(Ac.) =
11.117 Sq. Mi.
Length along longest watercourse = 37172.00(Ft.)
18294.00(Ft.) Length along longest watercourse measured to centroid =
Length along longest watercourse = 7.040 Mi.
Length along longest watercourse measured to centroid = 3.465 Mi.
Difference in elevation = 4483.00(Ft.)
Slope along watercourse = 636.7761 Ft./Mi.
Average Manning's 'N' = 0.050
Lag time = 1.185 Hr.
Lag time = 71.08 Min.
25% of lag time = 17.77 Min.
40% of lag time = 28.43 Min.
Unit time = 15.00 Min.
Duration of storm = 6 Hour(s)
User Entered Base Flow = 0.00(CFS)

2 YEAR Area rainfall data:

Area(Ac.)[1]	Rainfall(In)[2]	weighting[1*2]
7115.00	2.65	18854.75

100 YEAR Area rainfall data:

Area(Ac.)[1]	Rainfall(In)[2]	weighting[1*2]
7115.00	4.51	32088.65

STORM EVENT (YEAR) = 100.00
Area Averaged 2-Year Rainfall = 2.650(In)
Area Averaged 100-Year Rainfall = 4.510(In)

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Point rain (area averaged) = 4.510(In)
 Areal adjustment factor = 97.62 %
 Adjusted average point rain = 4.402(In)

Sub-Area Data:

Area(Ac.)	Runoff Index	Impervious %
2134.000	45.00	0.000
4981.000	83.00	0.200
Total Area Entered = 7115.00(Ac.)		

RI	RI	Infil. Rate	Impervious	Adj. Infil. Rate	Area%	F
AMC2	AMC-2	(In/Hr)	(Dec.%)	(In/Hr)	(Dec.)	(In/Hr)
45.0	45.0	0.621	0.000	0.621	0.300	0.186
83.0	83.0	0.210	0.200	0.172	0.700	0.120
Sum (F) =						0.307

Area averaged mean soil loss (F) (In/Hr) = 0.307
 Minimum soil loss rate ((In/Hr)) = 0.153
 (for 24 hour storm duration)
 Soil loss rate (decimal) = 0.900

Unit Hydrograph
 MOUNTAIN S-Curve

Unit Hydrograph Data

Unit time period (hrs)	Time % of lag	Distribution Graph %	Unit Hydrograph (CFS)
1	0.250	21.104	2.427
2	0.500	42.207	7.119
3	0.750	63.311	13.719
4	1.000	84.415	14.964
5	1.250	105.519	9.772
6	1.500	126.622	6.400
7	1.750	147.726	4.806
8	2.000	168.830	3.913
9	2.250	189.933	3.255
10	2.500	211.037	2.846
11	2.750	232.141	2.552
12	3.000	253.244	2.265
13	3.250	274.348	1.978
14	3.500	295.452	1.778
15	3.750	316.556	1.610
16	4.000	337.659	1.411
17	4.250	358.763	1.265
18	4.500	379.867	1.126
19	4.750	400.970	1.097
20	5.000	422.074	0.982
21	5.250	443.178	0.886
22	5.500	464.281	0.877
23	5.750	485.385	0.846
24	6.000	506.489	0.836
25	6.250	527.593	0.716
26	6.500	548.696	0.675
27	6.750	569.800	0.657
28	7.000	590.904	0.633
29	7.250	612.007	0.619
30	7.500	633.111	0.557
31	7.750	654.215	0.548
32	8.000	675.319	0.520
33	8.250	696.422	0.506
34	8.500	717.526	0.477
35	8.750	738.630	0.423

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36	9.000	759.733	0.418	29.943
37	9.250	780.837	0.386	27.678
38	9.500	801.941	0.380	27.239
39	9.750	823.044	0.380	27.239
40	10.000	844.148	0.380	27.239
41	10.250	865.252	0.380	27.239
42	10.500	886.356	0.380	27.239
43	10.750	907.459	0.380	27.239
44	11.000	928.563	0.380	27.239
45	11.250	949.667	0.380	27.239
46	11.500	970.770	0.380	27.239
47	11.750	991.874	0.380	27.239
48	12.000	1012.978	0.336	24.108
			Sum = 100.000	Sum= 7170.586

The following loss rate calculations reflect use of the minimum calculated loss rate subtracted from the Storm Rain to produce the maximum Effective Rain value

Unit	Time (Hr.)	Pattern Percent	Storm Rain (In/Hr)	Loss rate(In./Hr)		Effective (In/Hr)
				Max	Low	
1	0.25	1.70	0.299	(0.307)	0.269	0.030
2	0.50	1.90	0.335	(0.307)	0.301	0.033
3	0.75	2.10	0.370	0.307	(0.333)	0.063
4	1.00	2.20	0.387	0.307	(0.349)	0.081
5	1.25	2.40	0.423	0.307	(0.380)	0.116
6	1.50	2.40	0.423	0.307	(0.380)	0.116
7	1.75	2.40	0.423	0.307	(0.380)	0.116
8	2.00	2.50	0.440	0.307	(0.396)	0.134
9	2.25	2.60	0.458	0.307	(0.412)	0.151
10	2.50	2.70	0.475	0.307	(0.428)	0.169
11	2.75	2.80	0.493	0.307	(0.444)	0.186
12	3.00	3.00	0.528	0.307	(0.475)	0.222
13	3.25	3.20	0.564	0.307	(0.507)	0.257
14	3.50	3.60	0.634	0.307	(0.571)	0.327
15	3.75	4.30	0.757	0.307	(0.682)	0.451
16	4.00	4.70	0.828	0.307	(0.745)	0.521
17	4.25	5.40	0.951	0.307	(0.856)	0.644
18	4.50	6.20	1.092	0.307	(0.983)	0.785
19	4.75	6.90	1.215	0.307	(1.094)	0.908
20	5.00	7.50	1.321	0.307	(1.189)	1.014
21	5.25	10.60	1.867	0.307	(1.680)	1.560
22	5.50	14.50	2.553	0.307	(2.298)	2.247
23	5.75	3.40	0.599	0.307	(0.539)	0.292
24	6.00	1.00	0.176	(0.307)	0.158	0.018

Sum = 100.0 (Loss Rate Not Used) Sum = 10.4

Flood volume = Effective rainfall 2.61(In)
times area 7115.0(Ac.)/[(In)/(Ft.)] = 1547.5(Ac.Ft)
Total soil loss = 1.79(In)
Total soil loss = 1062.746(Ac.Ft)
Total rainfall = 4.40(In)
Flood volume = 67411263.3 Cubic Feet
Total soil loss = 46293218.8 Cubic Feet

Peak flow rate of this hydrograph = 6074.802(CFS)

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6 - H O U R S T O R M
R u n o f f H y d r o g r a p h

Hydrograph in 15 Minute intervals ((CFS))

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Time(h+m)	Volume	Ac.Ft	Q(CFS)	0	1525.0	3050.0	4575.0	6100.0
0+15	0.1077	5.21	Q					
0+30	0.5440	21.12	Q					
0+45	1.7329	57.54	Q					
1+ 0	4.0338	111.36	Q					
1+15	7.7619	180.44	VQ					
1+30	13.2127	263.81	VQ					
1+45	20.4464	350.11	V Q					
2+ 0	29.2599	426.57	V Q					
2+15	39.3758	489.61	V Q					
2+30	50.8739	556.50	V Q					
2+45	63.9939	635.01	V Q					
3+ 0	78.9585	724.29	V Q					
3+15	96.0482	827.14	V Q					
3+30	115.8214	957.02	V Q					
3+45	139.2884	1135.81	V Q					
4+ 0	167.8321	1381.51	V Q					
4+15	203.1467	1709.23	V Q					
4+30	246.6224	2104.22	V Q					
4+45	299.5223	2560.36	V Q					
5+ 0	363.2250	3083.21	V Q					
5+15	440.1092	3721.19	V Q					
5+30	535.5293	4618.33	V Q					
5+45	650.5010	5564.63	V Q					
6+ 0	776.0135	6074.80	V Q					
6+15	886.7301	5358.69	V Q					
6+30	967.4464	3906.67	V Q					
6+45	1027.2629	2895.12	V Q					
7+ 0	1075.2753	2323.80	V Q					
7+15	1115.8360	1963.14	V Q					
7+30	1150.9804	1700.99	V Q					
7+45	1182.1203	1507.17	V Q					
8+ 0	1209.9832	1348.56	V Q					
8+15	1234.9042	1206.18	V Q					
8+30	1257.2254	1080.34	V Q					
8+45	1277.4279	977.80	V Q					
9+ 0	1295.7806	888.27	V Q					
9+15	1312.4433	806.47	V Q					
9+30	1327.7336	740.05	V Q					
9+45	1341.9135	686.31	V Q					
10+ 0	1355.2743	646.66	V Q					
10+15	1367.7167	602.21	V Q					
10+30	1379.4292	566.88	V Q					
10+45	1390.6514	543.16	V Q					
11+ 0	1401.3608	518.33	V Q					
11+15	1411.5109	491.26	V Q					
11+30	1420.9540	457.05	V Q					
11+45	1429.9543	435.61	V Q					
12+ 0	1438.6031	418.61	V Q					
12+15	1446.8886	401.02	V Q					
12+30	1454.7941	382.63	V Q					
12+45	1462.2527	361.00	V Q					
13+ 0	1469.3863	345.26	V Q					
13+15	1476.1671	328.19	V Q					
13+30	1482.6263	312.63	V Q					
13+45	1488.7323	295.53	V Q					
14+ 0	1494.5000	279.15	V Q					
14+15	1500.0559	268.91	V Q					
14+30	1505.3925	258.29	V Q					
14+45	1510.5876	251.44	V Q					
15+ 0	1515.6521	245.12	V Q					
15+15	1520.5673	237.90	V Q					
15+30	1525.2903	228.59	V Q					
15+45	1529.7550	216.09	V Q					

				hurkey6100.out			
16+ 0	1533.9184	201.51	Q				V
16+15	1537.7100	183.51	Q				V
16+30	1541.0514	161.73	Q				V
16+45	1543.8746	136.64	Q				V
17+ 0	1546.0914	107.29	Q				V
17+15	1547.3854	62.63	Q				V
17+30	1547.5409	7.52	Q				V
17+45	1547.5497	0.42	Q				V

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Unit Hydrograph Analysis

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Study date 08/12/20 File: hurkey24100.out

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Riverside County Synthetic Unit Hydrology Method
RCFC & WCD Manual date - April 1978

Program License Serial Number 4010

English (in-lb) Input Units Used
English Rainfall Data (Inches) Input Values Used

English Units used in output format

Hurkey Creek Title Revision RNR Aug122020

Drainage Area = 7115.00(Ac.) = 11.117 Sq. Mi.
11.117 Sq. Mi. Drainage Area for Depth-Area Area Adjustment = 7115.00(Ac.) =
11.117 Sq. Mi.
Length along longest watercourse = 37172.00(Ft.)
18294.00(Ft.) Length along longest watercourse measured to centroid =
Length along longest watercourse = 7.040 Mi.
Length along longest watercourse measured to centroid = 3.465 Mi.
Difference in elevation = 4483.00(Ft.)
Slope along watercourse = 636.7761 Ft./Mi.
Average Manning's 'N' = 0.050
Lag time = 1.185 Hr.
Lag time = 71.08 Min.
25% of lag time = 17.77 Min.
40% of lag time = 28.43 Min.
Unit time = 15.00 Min.
Duration of storm = 24 Hour(s)
User Entered Base Flow = 0.00(CFS)

2 YEAR Area rainfall data:

Area(Ac.)[1]	Rainfall(In)[2]	weighting[1*2]
7115.00	5.63	40057.45

100 YEAR Area rainfall data:

Area(Ac.)[1]	Rainfall(In)[2]	weighting[1*2]
7115.00	9.51	67663.65

STORM EVENT (YEAR) = 100.00
Area Averaged 2-Year Rainfall = 5.630(In)
Area Averaged 100-Year Rainfall = 9.510(In)

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Point rain (area averaged) = 9.510(In)
Areal adjustment factor = 98.63 %
Adjusted average point rain = 9.380(In)

Sub-Area Data:

Area(Ac.)	Runoff Index	Impervious %
2134.000	45.00	0.000
4981.000	83.00	0.200
Total Area Entered =	7115.00(Ac.)	

RI	RI	Infil. Rate	Impervious	Adj. Infil. Rate	Area%	F
AMC2	AMC-2	(In/Hr)	(Dec.%)	(In/Hr)	(Dec.)	(In/Hr)
45.0	45.0	0.621	0.000	0.621	0.300	0.186
83.0	83.0	0.210	0.200	0.172	0.700	0.120
					Sum (F) =	0.307

Area averaged mean soil loss (F) (In/Hr) = 0.307
Minimum soil loss rate ((In/Hr)) = 0.153
(for 24 hour storm duration)
Soil loss rate (decimal) = 0.900

Unit Hydrograph
MOUNTAIN S-Curve

Unit Hydrograph Data

Unit time period (hrs)	Time % of lag	Distribution Graph %	Unit Hydrograph (CFS)
1	0.250	21.104	2.427
2	0.500	42.207	7.119
3	0.750	63.311	13.719
4	1.000	84.415	14.964
5	1.250	105.519	9.772
6	1.500	126.622	6.400
7	1.750	147.726	4.806
8	2.000	168.830	3.913
9	2.250	189.933	3.255
10	2.500	211.037	2.846
11	2.750	232.141	2.552
12	3.000	253.244	2.265
13	3.250	274.348	1.978
14	3.500	295.452	1.778
15	3.750	316.556	1.610
16	4.000	337.659	1.411
17	4.250	358.763	1.265
18	4.500	379.867	1.126
19	4.750	400.970	1.097
20	5.000	422.074	0.982
21	5.250	443.178	0.886
22	5.500	464.281	0.877
23	5.750	485.385	0.846
24	6.000	506.489	0.836
25	6.250	527.593	0.716
26	6.500	548.696	0.675
27	6.750	569.800	0.657
28	7.000	590.904	0.633
29	7.250	612.007	0.619
30	7.500	633.111	0.557
31	7.750	654.215	0.548
32	8.000	675.319	0.520
33	8.250	696.422	0.506
34	8.500	717.526	0.477
35	8.750	738.630	0.423

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36	9.000	759.733	0.418	29.943		
37	9.250	780.837	0.386	27.678		
38	9.500	801.941	0.380	27.239		
39	9.750	823.044	0.380	27.239		
40	10.000	844.148	0.380	27.239		
41	10.250	865.252	0.380	27.239		
42	10.500	886.356	0.380	27.239		
43	10.750	907.459	0.380	27.239		
44	11.000	928.563	0.380	27.239		
45	11.250	949.667	0.380	27.239		
46	11.500	970.770	0.380	27.239		
47	11.750	991.874	0.380	27.239		
48	12.000	1012.978	0.336	24.108		
			Sum = 100.000	Sum=	7170.586	

The following loss rate calculations reflect use of the minimum calculated loss rate subtracted from the Storm Rain to produce the maximum Effective Rain value

Unit	Time (Hr.)	Pattern Percent	Storm Rain (In/Hr)	Loss rate(In./Hr)		Effective (In/Hr)
				Max	Low	
1	0.25	0.20	0.075	(0.542)	0.068	0.008
2	0.50	0.30	0.113	(0.535)	0.101	0.011
3	0.75	0.30	0.113	(0.529)	0.101	0.011
4	1.00	0.40	0.150	(0.523)	0.135	0.015
5	1.25	0.30	0.113	(0.517)	0.101	0.011
6	1.50	0.30	0.113	(0.511)	0.101	0.011
7	1.75	0.30	0.113	(0.504)	0.101	0.011
8	2.00	0.40	0.150	(0.498)	0.135	0.015
9	2.25	0.40	0.150	(0.492)	0.135	0.015
10	2.50	0.40	0.150	(0.486)	0.135	0.015
11	2.75	0.50	0.188	(0.480)	0.169	0.019
12	3.00	0.50	0.188	(0.475)	0.169	0.019
13	3.25	0.50	0.188	(0.469)	0.169	0.019
14	3.50	0.50	0.188	(0.463)	0.169	0.019
15	3.75	0.50	0.188	(0.457)	0.169	0.019
16	4.00	0.60	0.225	(0.451)	0.203	0.023
17	4.25	0.60	0.225	(0.446)	0.203	0.023
18	4.50	0.70	0.263	(0.440)	0.236	0.026
19	4.75	0.70	0.263	(0.434)	0.236	0.026
20	5.00	0.80	0.300	(0.429)	0.270	0.030
21	5.25	0.60	0.225	(0.423)	0.203	0.023
22	5.50	0.70	0.263	(0.418)	0.236	0.026
23	5.75	0.80	0.300	(0.412)	0.270	0.030
24	6.00	0.80	0.300	(0.407)	0.270	0.030
25	6.25	0.90	0.338	(0.401)	0.304	0.034
26	6.50	0.90	0.338	(0.396)	0.304	0.034
27	6.75	1.00	0.375	(0.391)	0.338	0.038
28	7.00	1.00	0.375	(0.385)	0.338	0.038
29	7.25	1.00	0.375	(0.380)	0.338	0.038
30	7.50	1.10	0.413	(0.375)	0.371	0.041
31	7.75	1.20	0.450	0.370	(0.405)	0.080
32	8.00	1.30	0.488	0.365	(0.439)	0.123
33	8.25	1.50	0.563	0.360	(0.507)	0.203
34	8.50	1.50	0.563	0.355	(0.507)	0.208
35	8.75	1.60	0.600	0.350	(0.540)	0.251
36	9.00	1.70	0.638	0.345	(0.574)	0.293
37	9.25	1.90	0.713	0.340	(0.642)	0.373
38	9.50	2.00	0.750	0.335	(0.675)	0.415
39	9.75	2.10	0.788	0.330	(0.709)	0.458
40	10.00	2.20	0.825	0.325	(0.743)	0.500
41	10.25	1.50	0.563	0.321	(0.507)	0.242
42	10.50	1.50	0.563	0.316	(0.507)	0.247

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43	10.75	2.00	0.750	0.311	(0.675)	0.439
44	11.00	2.00	0.750	0.307	(0.675)	0.443
45	11.25	1.90	0.713	0.302	(0.642)	0.410
46	11.50	1.90	0.713	0.298	(0.642)	0.415
47	11.75	1.70	0.638	0.294	(0.574)	0.344
48	12.00	1.80	0.675	0.289	(0.608)	0.386
49	12.25	2.50	0.938	0.285	(0.844)	0.653
50	12.50	2.60	0.976	0.281	(0.878)	0.695
51	12.75	2.80	1.051	0.276	(0.946)	0.774
52	13.00	2.90	1.088	0.272	(0.979)	0.816
53	13.25	3.40	1.276	0.268	(1.148)	1.008
54	13.50	3.40	1.276	0.264	(1.148)	1.012
55	13.75	2.30	0.863	0.260	(0.777)	0.603
56	14.00	2.30	0.863	0.256	(0.777)	0.607
57	14.25	2.70	1.013	0.252	(0.912)	0.761
58	14.50	2.60	0.976	0.248	(0.878)	0.727
59	14.75	2.60	0.976	0.245	(0.878)	0.731
60	15.00	2.50	0.938	0.241	(0.844)	0.697
61	15.25	2.40	0.900	0.237	(0.810)	0.663
62	15.50	2.30	0.863	0.233	(0.777)	0.629
63	15.75	1.90	0.713	0.230	(0.642)	0.483
64	16.00	1.90	0.713	0.226	(0.642)	0.486
65	16.25	0.40	0.150	(0.223)	0.135	0.015
66	16.50	0.40	0.150	(0.220)	0.135	0.015
67	16.75	0.30	0.113	(0.216)	0.101	0.011
68	17.00	0.30	0.113	(0.213)	0.101	0.011
69	17.25	0.50	0.188	(0.210)	0.169	0.019
70	17.50	0.50	0.188	(0.207)	0.169	0.019
71	17.75	0.50	0.188	(0.203)	0.169	0.019
72	18.00	0.40	0.150	(0.200)	0.135	0.015
73	18.25	0.40	0.150	(0.198)	0.135	0.015
74	18.50	0.40	0.150	(0.195)	0.135	0.015
75	18.75	0.30	0.113	(0.192)	0.101	0.011
76	19.00	0.20	0.075	(0.189)	0.068	0.008
77	19.25	0.30	0.113	(0.186)	0.101	0.011
78	19.50	0.40	0.150	(0.184)	0.135	0.015
79	19.75	0.30	0.113	(0.181)	0.101	0.011
80	20.00	0.20	0.075	(0.179)	0.068	0.008
81	20.25	0.30	0.113	(0.177)	0.101	0.011
82	20.50	0.30	0.113	(0.174)	0.101	0.011
83	20.75	0.30	0.113	(0.172)	0.101	0.011
84	21.00	0.20	0.075	(0.170)	0.068	0.008
85	21.25	0.30	0.113	(0.168)	0.101	0.011
86	21.50	0.20	0.075	(0.166)	0.068	0.008
87	21.75	0.30	0.113	(0.164)	0.101	0.011
88	22.00	0.20	0.075	(0.162)	0.068	0.008
89	22.25	0.30	0.113	(0.161)	0.101	0.011
90	22.50	0.20	0.075	(0.159)	0.068	0.008
91	22.75	0.20	0.075	(0.158)	0.068	0.008
92	23.00	0.20	0.075	(0.157)	0.068	0.008
93	23.25	0.20	0.075	(0.156)	0.068	0.008
94	23.50	0.20	0.075	(0.155)	0.068	0.008
95	23.75	0.20	0.075	(0.154)	0.068	0.008
96	24.00	0.20	0.075	(0.153)	0.068	0.008

(Loss Rate Not Used)

Sum = 100.0 Sum = 18.2

Flood volume = Effective rainfall 4.55(In)
times area 7115.0(Ac.)/[(In)/(Ft.)] = 2699.8(Ac.Ft)
Total soil loss = 4.83(In)
Total soil loss = 2861.767(Ac.Ft)
Total rainfall = 9.38(In)
Flood volume = 117602133.4 Cubic Feet
Total soil loss = 124658555.3 Cubic Feet

Peak flow rate of this hydrograph = 4547.067(CFS)

+++++

24 - H O U R S T O R M
R u n o f f H y d r o g r a p h

Hydrograph in 15 Minute intervals ((CFS))

Time(h+m)	Volume	Ac.Ft	Q(CFS)	0	1150.0	2300.0	3450.0	4600.0
0+15	0.0270		1.31	Q				
0+30	0.1467		5.79	Q				
0+45	0.4585		15.09	Q				
1+ 0	1.0267		27.50	Q				
1+15	1.8128		38.05	Q				
1+30	2.7612		45.90	Q				
1+45	3.8055		50.55	Q				
2+ 0	4.9047		53.20	Q				
2+15	6.0827		57.02	Q				
2+30	7.3779		62.69	Q				
2+45	8.8091		69.27	Q				
3+ 0	10.3699		75.54	Q				
3+15	12.0749		82.52	Q				
3+30	13.9190		89.26	Q				
3+45	15.8655		94.21	Q				
4+ 0	17.9022		98.58	Q				
4+15	20.0418		103.56	Q				
4+30	22.3258		110.55	Q				
4+45	24.7807		118.82	VQ				
5+ 0	27.4228		127.88	VQ				
5+15	30.2349		136.11	VQ				
5+30	33.1748		142.29	VQ				
5+45	36.1882		145.85	VQ				
6+ 0	39.2807		149.67	VQ				
6+15	42.5384		157.67	VQ				
6+30	45.9864		166.88	VQ				
6+45	49.6344		176.56	VQ				
7+ 0	53.4885		186.54	VQ				
7+15	57.5450		196.34	VQ				
7+30	61.7959		205.74	VQ				
7+45	66.3631		221.05	VQ				
8+ 0	71.6805		257.36	VQ				
8+15	78.7068		340.07	VQ				
8+30	88.4671		472.40	V	Q			
8+45	101.6805		639.53	V	Q			
9+ 0	118.4534		811.81	V	Q			
9+15	138.8603		987.69	V	Q			
9+30	163.5033		1192.72	V	Q			
9+45	193.0465		1429.89	V	Q	Q		
10+ 0	227.7705		1680.64	V	Q	Q		
10+15	266.4743		1873.27	V	Q	Q		
10+30	306.8961		1956.41	V	Q	Q		
10+45	346.4146		1912.70	V	Q	Q		
11+ 0	385.0729		1871.06	V	Q	Q		
11+15	426.1083		1986.11	V	Q	Q		
11+30	470.6279		2154.75	V	Q	Q		
11+45	516.9620		2242.57	V	Q	Q		
12+ 0	563.8939		2271.50	V	Q	Q		
12+15	611.8606		2321.59	V	Q	Q		
12+30	663.0360		2476.89	V	Q	Q		
12+45	721.0910		2809.86	V	Q	Q		
13+ 0	787.5728		3217.72	V	Q	Q		
13+15	862.1873		3611.34	V	Q	Q		
13+30	945.1179		4013.84	V	Q	Q		
13+45	1035.3706		4368.23	V	Q	Q		
14+ 0	1129.3183		4547.07	V	Q	Q		
14+15	1221.3372		4453.71	V	Q	Q		

14+30	1310.2516	4303.46			V		
14+45	1399.7060	4329.59			V		
15+ 0	1491.0764	4422.33			V		
15+15	1583.3682	4466.92			V		
15+30	1675.6114	4464.57			V		
15+45	1766.5694	4402.37			V		
16+ 0	1854.9730	4278.73			V		
16+15	1937.9767	4017.38			V		
16+30	2012.5246	3608.12			V		
16+45	2075.3994	3043.14			V		
17+ 0	2126.4798	2472.29			V		
17+15	2169.7539	2094.46			V		
17+30	2207.9033	1846.43			V		
17+45	2242.3459	1667.02			V		
18+ 0	2273.8337	1524.01			V		
18+15	2302.7549	1399.79			V		
18+30	2329.4345	1291.29			V		
18+45	2354.1681	1197.11			V		
19+ 0	2377.1576	1112.69			V		
19+15	2398.5762	1036.66			V		
19+30	2418.5895	968.64			V		
19+45	2437.4542	913.06			V		
20+ 0	2455.3751	867.37			V		
20+15	2472.3595	822.05			V		
20+30	2488.4544	778.99			V		
20+45	2503.6978	737.78			V		
21+ 0	2518.2008	701.95			V		
21+15	2531.9616	666.02			V		
21+30	2544.9288	627.61			V		
21+45	2557.1366	590.86			V		
22+ 0	2568.6171	555.66			V		
22+15	2579.6191	532.50			V		
22+30	2590.1626	510.31			V		
22+45	2600.1726	484.48			V		
23+ 0	2609.6755	459.94			V		
23+15	2618.6708	435.37			V		
23+30	2627.2328	414.40			V		
23+45	2635.4221	396.36			V		
24+ 0	2643.2387	378.32			V		
24+15	2650.5455	353.65			V		
24+30	2657.2895	326.41			V		
24+45	2663.4037	295.93			V		
25+ 0	2668.8479	263.50			V		
25+15	2673.6071	230.35			V		
25+30	2677.7474	200.39			V		
25+45	2681.4917	181.22			V		
26+ 0	2684.8390	162.01			V		
26+15	2687.7229	139.58			V		
26+30	2690.1646	118.18			V		
26+45	2692.1684	96.98			V		
27+ 0	2693.7577	76.92			V		
27+15	2694.9550	57.95			V		
27+30	2695.7900	40.41			V		
27+45	2696.3367	26.46			V		
28+ 0	2696.6268	14.04			V		
28+15	2696.8969	13.07			V		
28+30	2697.1489	12.20			V		
28+45	2697.3857	11.46			V		
29+ 0	2697.6077	10.75			V		
29+15	2697.8129	9.93			V		
29+30	2698.0010	9.11			V		
29+45	2698.1730	8.33			V		
30+ 0	2698.3311	7.65			V		
30+15	2698.4765	7.04			V		
30+30	2698.6101	6.47			V		
30+45	2698.7341	6.00			V		

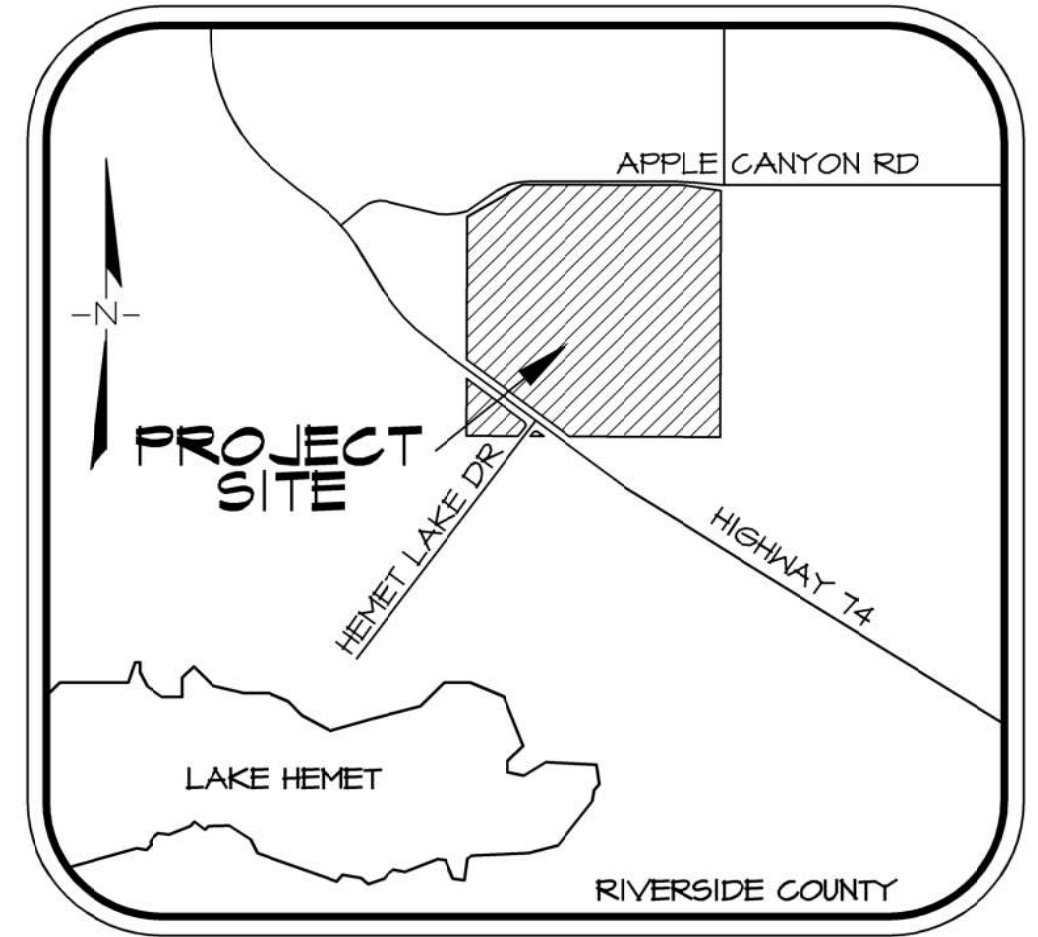
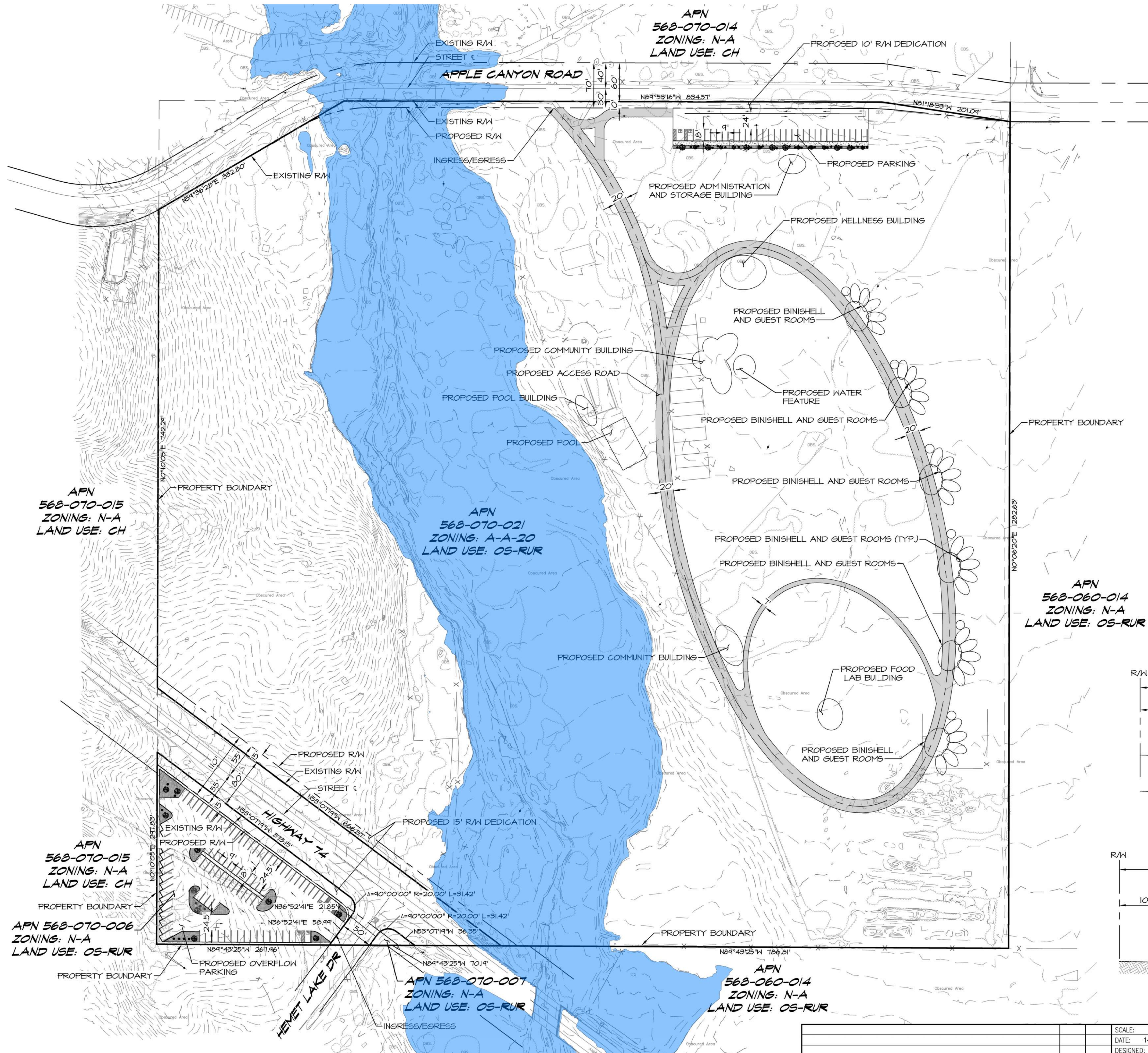
hurkey24100.out

31+ 0	2698.8507	5.65	Q				V
31+15	2698.9579	5.19	Q				V
31+30	2699.0550	4.70	Q				V
31+45	2699.1440	4.31	Q				V
32+ 0	2699.2270	4.02	Q				V
32+15	2699.3023	3.64	Q				V
32+30	2699.3701	3.28	Q				V
32+45	2699.4314	2.97	Q				V
33+ 0	2699.4878	2.73	Q				V
33+15	2699.5380	2.43	Q				V
33+30	2699.5837	2.21	Q				V
33+45	2699.6234	1.92	Q				V
34+ 0	2699.6586	1.70	Q				V
34+15	2699.6877	1.41	Q				V
34+30	2699.7125	1.20	Q				V
34+45	2699.7332	1.00	Q				V
35+ 0	2699.7496	0.79	Q				V
35+15	2699.7618	0.59	Q				V
35+30	2699.7698	0.39	Q				V
35+45	2699.7735	0.18	Q				V

APPENDIX E

2D RAS Exhibits

IN THE COUNTY OF RIVERSIDE, STATE OF CALIFORNIA
PRE-APPLICATION REVIEW
THE RIDGE



VICINITY MAP
N.T.S.

PROJECT DESCRIPTION
 THE RIDGE PROPOSES AN ECO-CONSCIOUS GUEST RANCH FACILITY ON 37.5± ACRES LOCATED AT 56475 APPLE CANYON ROAD IN THE COUNTY OF RIVERSIDE (APNs: 568-170-021, -006, & -007). THE RIDGE WILL BE DESIGNED TO FACILITATE A FULL IMMERSION NATURE EXPERIENCE IN THE BEAUTIFUL MOUNTAIN CENTER/IDYLLWILD SETTING. THE FACILITY WILL OFFER A VARIETY OF SELF-DEVELOPMENT THERAPIES AND ACTIVITIES SUCH AS YOGA, HIKING, HORSEBACK RIDING, ROCK CLIMBING AND WATER BASED ACTIVITIES.

THE PROJECT PROPOSES TO CONSTRUCT BINISHELLS WITH GUEST ROOMS, A RECEPTION AND DINING HALL WITH A POOL, A WELLNESS CENTER, AND AN AGRICULTURAL PRODUCTION/FOOD LAB. TWO PARKING LOTS ARE PROPOSED FOR THIS PROJECT: ONE ALONG APPLE CANYON ROAD AND AN OVERFLOW PARKING ALONG HIGHWAY 74 AND HIMET LAKE DRIVE. RIGHT OF WAY SHALL BE DEDICATED ALONG HIGHWAY 74 AND APPLE CANYON ROAD FOR ROAD IMPROVEMENT PURPOSES.

APPLICANT/PROPERTY OWNER
 THE RIDGE
 8401 WYNDHAM ROAD
 LOS ANGELES, CA 90046
 ATT: CAROLINE LEGRAND
 PHONE: (310) 666-3623
 EMAIL: LEGRAND.CAROLINE@GOOGLEMAIL.COM

TOPOGRAPHY
 INLAND AERIAL SURVEYS, INC.
 7117 ARLINGTON AVENUE SUITE A
 RIVERSIDE, CA 92503
 PHONE: (951) 681-4252
 DATED: 07-13-2020

ENGINEER
 ALBERT A. WEBB ASSOCIATES
 3788 MCGRAY STREET
 RIVERSIDE, CA 92506
 ATT: SANDY CHANDLER
 PHONE: (951) 686-1070
 FAX: (951) 788-1256
 EMAIL: SANDY.CHANDLER@WEBBASSOCIATES.COM

LEGAL DESCRIPTION
 THE SOUTHWEST QUARTER OF THE SOUTHWEST QUARTER OF SECTION 4, TOWNSHIP 6 SOUTH RANGE 3 EAST, SAN BERNARDINO BASE AND MERIDIAN, AS SHOWN BY UNITED STATES GOVERNMENT SURVEY.

ASSESSOR PARCEL NUMBER
 568-170-021
 568-170-006
 568-170-007

EXCEPTING THEREFROM ANY PORTION THEREOF LYING WITHIN STATE HIGHWAY 74, LAKE HIMET DRIVE AND APPLE CANYON ROAD;
 ALSO EXCEPTING THEREFROM THAT PORTION CONVEYED TO THE COUNTY OF RIVERSIDE BY DEED RECORDED NOVEMBER 8, 1955 IN BOOK 1017 PAGE 246 OF OFFICIAL RECORDS OF RIVERSIDE COUNTY, CALIFORNIA.

SITE ADDRESS
 56475 APPLE CANYON ROAD
 MOUNTAIN CENTER, CA 92561

GROSS ACREAGE
 37.98 ACRES

EXISTING LAND USE
 OS-RUR (OPEN SPACE-RURAL)

NET ACREAGE
 31.31 ACRES

PROPOSED LAND USE
 OS-RUR (OPEN SPACE-RURAL)

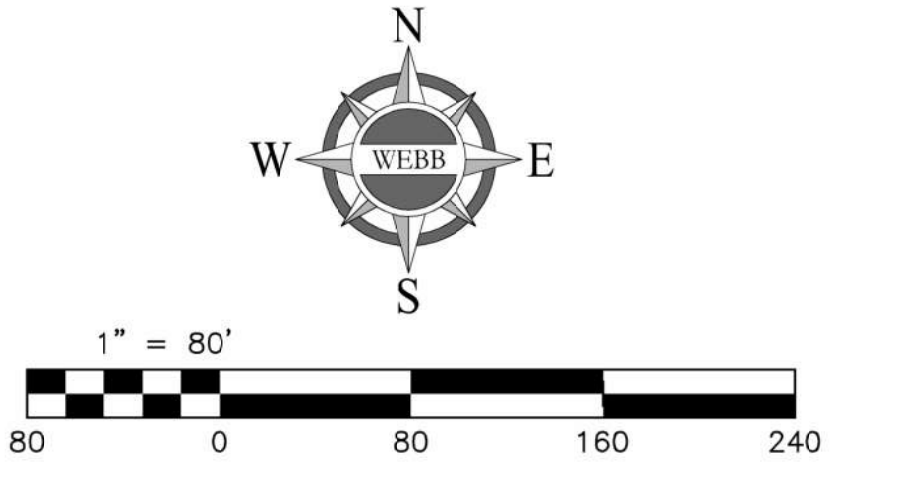
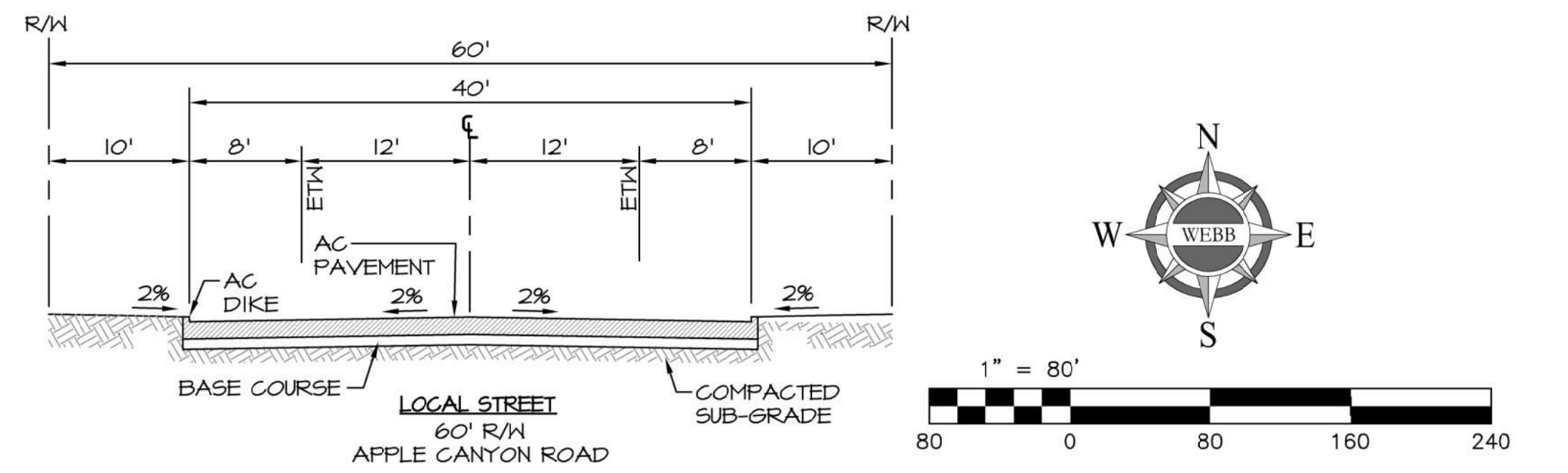
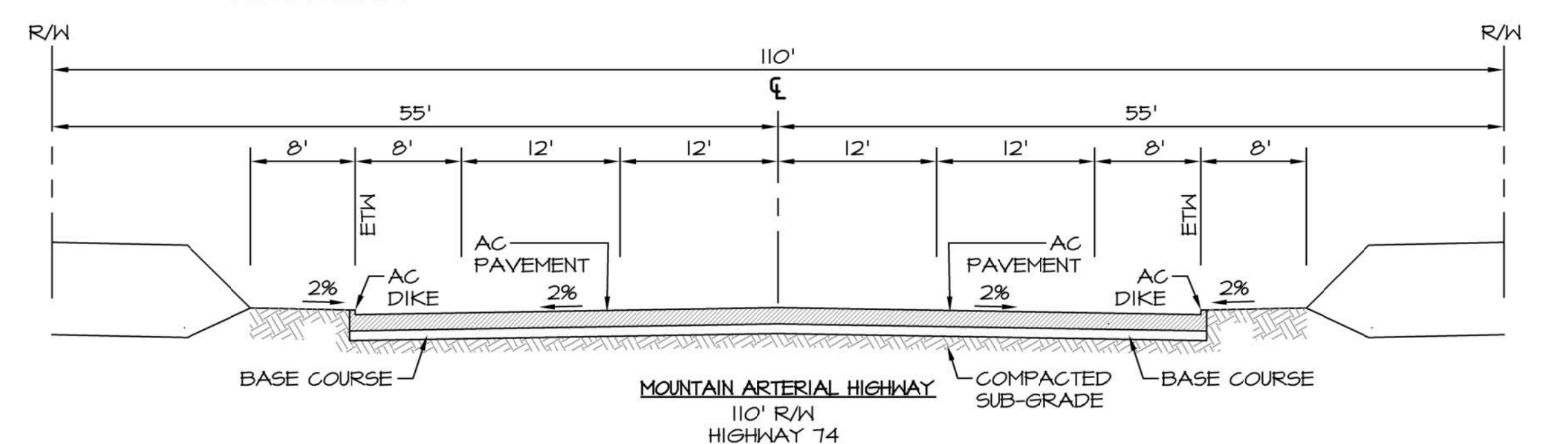
EXISTING ZONING
 A-1-20 (LIGHT AGRICULTURE)
 N-A (NATURAL ASSETS)

WASTE DISPOSAL SYSTEM
 ONSITE SEWAGE DISPOSAL

PROPOSED ZONING
 N-A (NATURAL ASSETS)

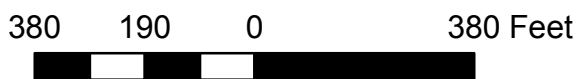
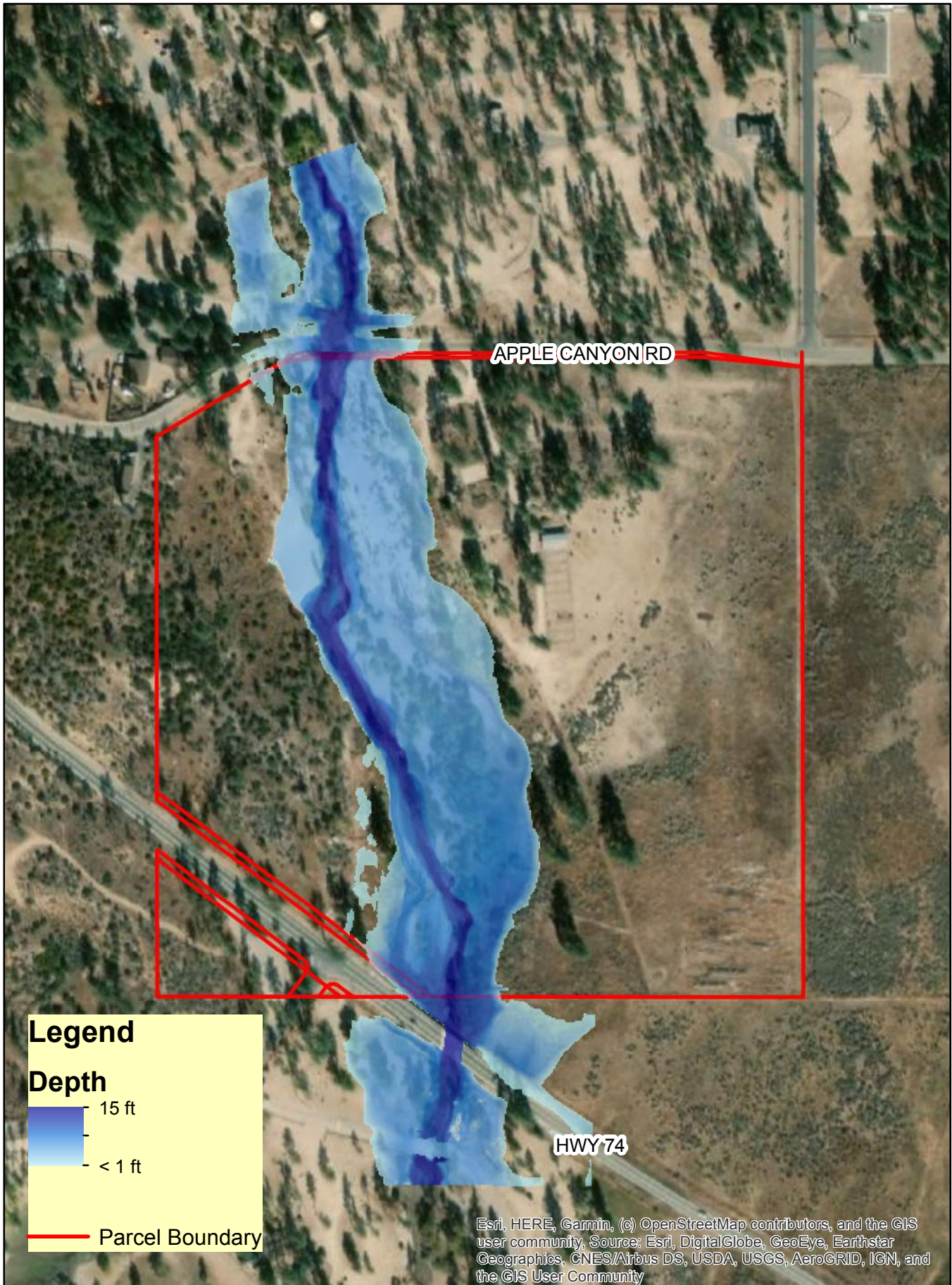
ONSITE WATER NOTE
 WATER FOR DOMESTIC, IRRIGATION AND FIRE USES SHALL BE PROVIDED BY ONSITE WELLS.

SETBACK REQUIREMENTS
 FRONT: 100 FEET
 REAR: 50 FEET
 SIDE: 50 FEET
 *PER N-A ZONING

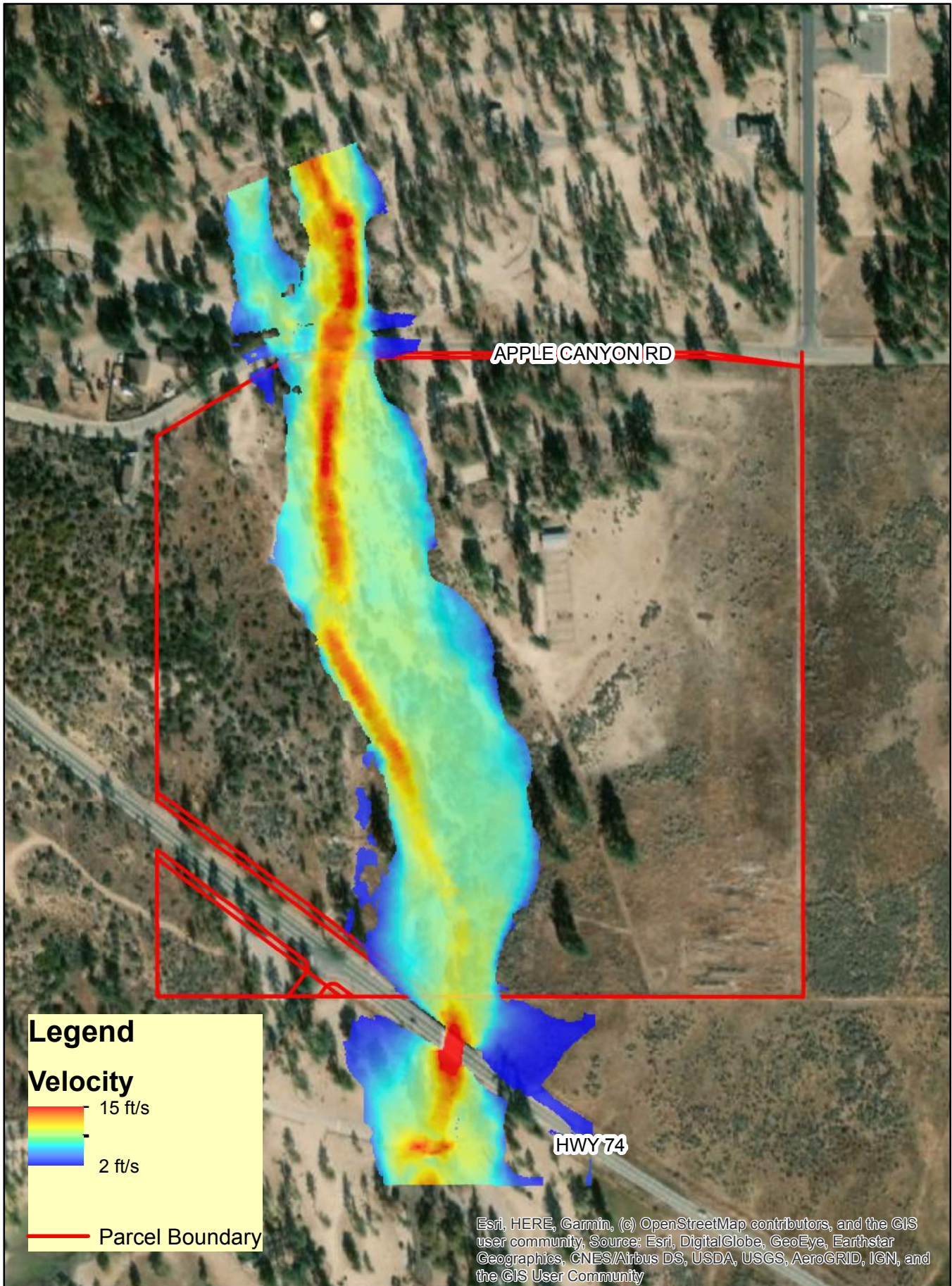


SCALE: 1" = 80'	ALBERT A. WEBB ASSOCIATES	ENGINEERING CONSULTANTS	W.O. 20-0120
DATE: 14-Aug-20	3788 MCGRAY STREET	3788 MCGRAY STREET	SHEET 1
DESIGNED: RH	RIVERSIDE, CA 92506	RIVERSIDE, CA 92506	OF 1 SHEETS
CHECKED: JRG	PH. (951) 686-1070	PH. (951) 686-1070	DWG. NO.
PLN CK REF:	FAX (951) 788-1256	FAX (951) 788-1256	
F.B.	PLOT DATE: 14-Aug-20		

Maximum Water Depth



Maximum Water Depth



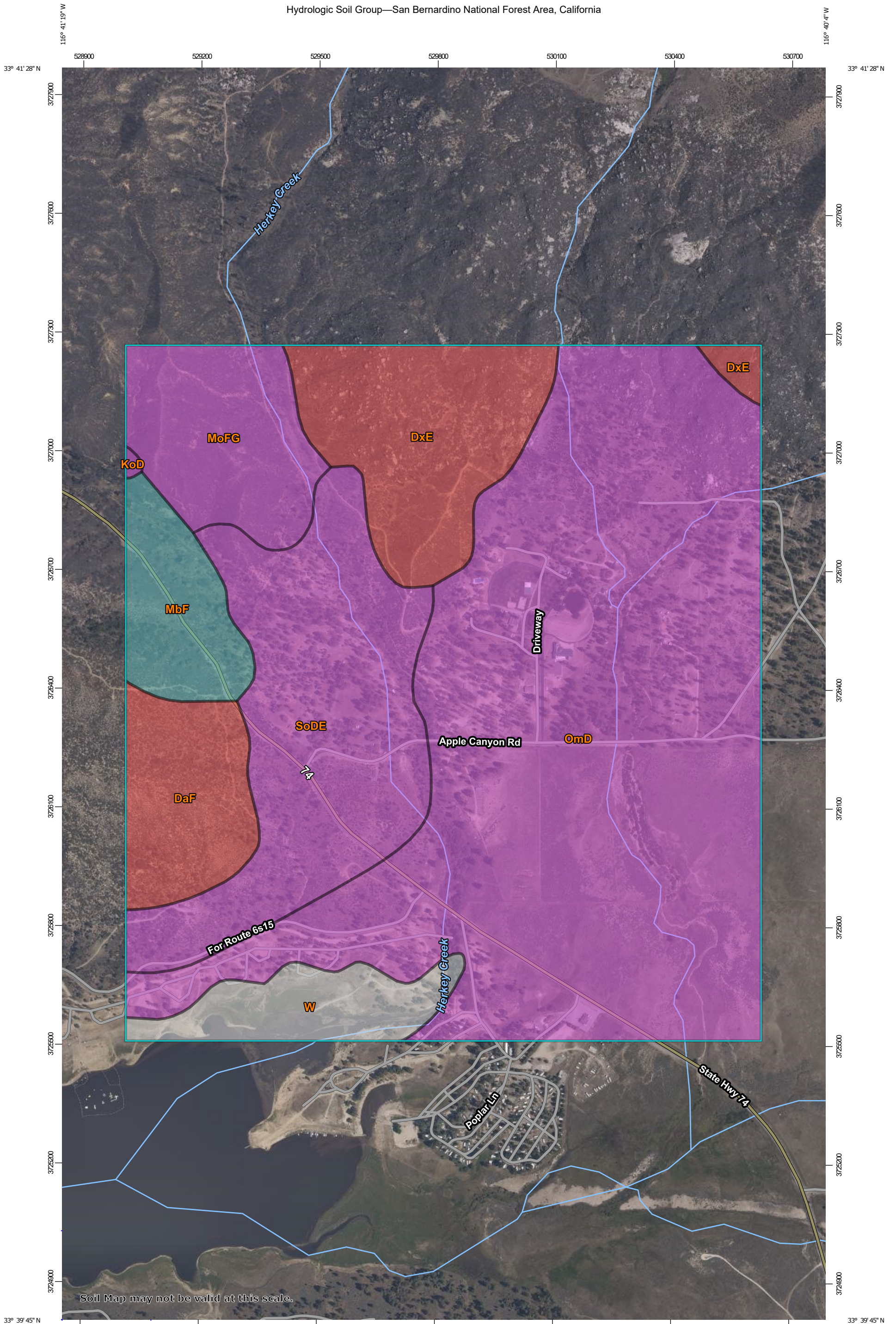
380 190 0 380 Feet



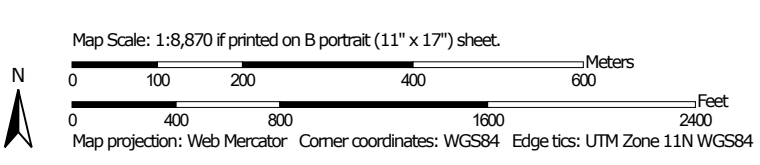
ALBERT A.
WEBB
ASSOCIATES

EXCERPT B: NRCS HYDRAULIC SOILS MAP

Hydrologic Soil Group—San Bernardino National Forest Area, California



Soil Map may not be valid at this scale.



MAP LEGEND

Area of Interest (AOI)









 Area of Interest (AOI)

Soils

Soil Rating Polygons





 A
 A/D
 B
 B/D
 C
 C/D
 D
 Not rated or not available

Soil Rating Lines


 A
 A/D
 B
 B/D
 C
 C/D
 D
 Not rated or not available

Soil Rating Points






 A
 A/D
 B
 B/D

 C
 C/D
 D
 Not rated or not available

Water Features

 Streams and Canals

Transportation

 Rails
 Interstate Highways
 US Routes
 Major Roads
 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: San Bernardino National Forest Area, California
 Survey Area Data: Version 12, May 27, 2020

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Aug 18, 2018—Aug 22, 2018

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
DaF	Pacifico-Wapi families complex, 30 to 50 percent slopes	D	39.1	5.6%
DxE	Wapi-Pacifico families, dry-Rock outcrop complex, 15 to 30 percent slopes	D	69.9	9.9%
KoD	Wind River-Oak Glen families association, 2 to 15 percent slopes	A	0.6	0.1%
MbF	Morical-Wind River families complex, 30 to 50 percent slopes	C	31.7	4.5%
MoFG	Typic Xerorthents-Morical family, dry association, 30 to 75 percent slopes	A	49.8	7.1%
OmD	Oak glen-rush families complex, 2 to 15 percent slopes	A	366.3	52.1%
SoDE	Oak Glen-Morical, very deep families complex, 2 to 30 percent slopes	A	116.9	16.6%
W	Water areas		29.1	4.1%
Totals for Area of Interest			703.4	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

EXHIBITS

EXHIBIT A: POST-PROJECT CONDITION RATIONAL METHOD HYDROLOGY MAP

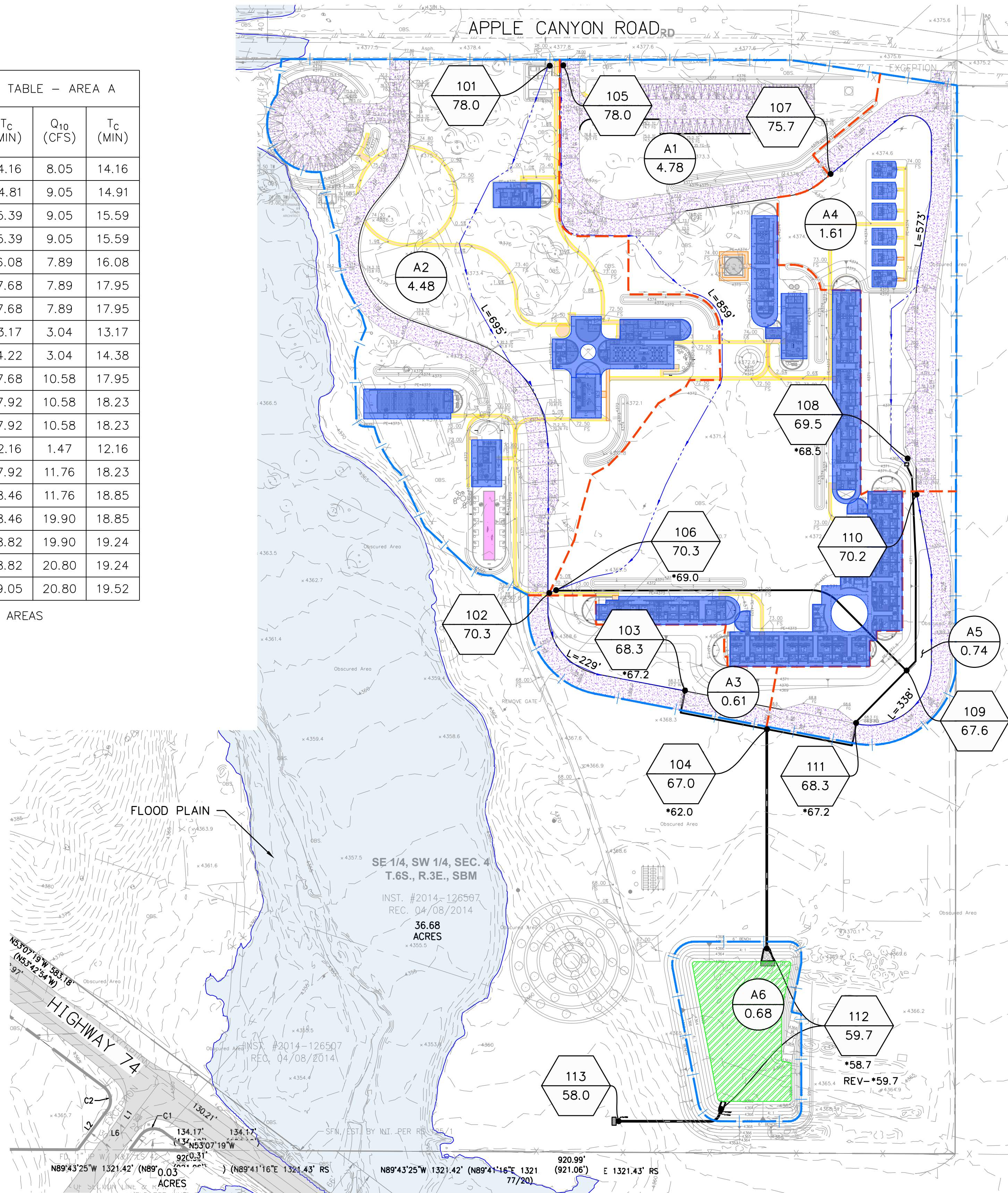
PRELIMINARY CONDITIONAL USE PERMIT 21-1021

COUNTY OF RIVERSIDE, STATE OF CALIFORNIA

POST-PROJECT CONDITION SITE HYDROLOGY MAP

NODES	Q ₁₀₀ (CFS)	T _c (MIN)	Q ₁₀ (CFS)	T _c (MIN)
101-102	14.48	14.16	8.05	14.16
102-103	16.31	14.81	9.05	14.91
103-104	16.31	15.39	9.05	15.59
*104-104	16.31	15.39	9.05	15.59
105-106	14.22	16.08	7.89	16.08
106-109	14.22	17.68	7.89	17.95
*109-109	14.22	17.68	7.89	17.95
107-108	5.45	13.17	3.04	13.17
108-109	5.45	14.22	3.04	14.38
*108-109	19.05	17.68	10.58	17.95
109-111	19.05	17.92	10.58	18.23
*111-111	19.05	17.92	10.58	18.23
110-111	2.64	12.16	1.47	12.16
*110-111	21.19	17.92	11.76	18.23
111-104	21.19	18.46	11.76	18.85
*111-104	35.94	18.46	19.90	18.85
104-112	35.94	18.82	19.90	19.24
112-112	37.69	18.82	20.80	19.24
112-113	37.69	19.05	20.80	19.52

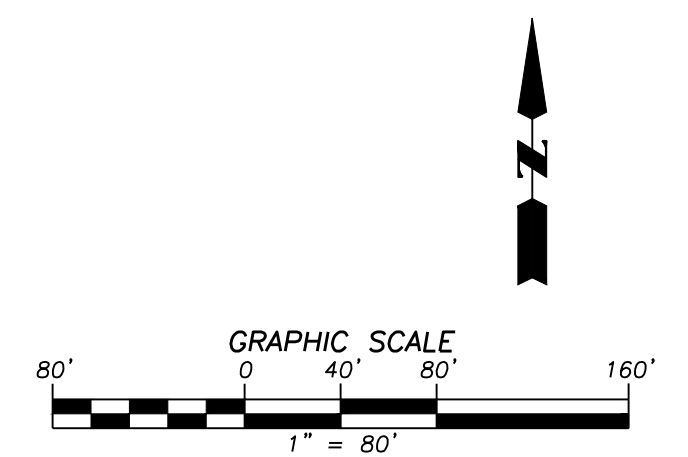
* DENOTES CONFLUENCE AREAS



LEGEND:

- NODE/CONCENTRATION POINT
- FLOWLINE ELEVATION
- APPROXIMATE INVERT ELEVATION
- SUB AREA
- FLOW DISTANCE
- FLOW PATH
- WATERSHED SUB-BOUNDARY
- WATERSHED BOUNDARY
- BASIN
- DRIVE AISLE
- PEDESTRIAN PATH
- POOL
- ROOF

SYMBOL	ROOF TOP AREA (SF)	SIDEWALK (SF)	ROAD	TOTAL IMPERVIOUS AREA (SF)	TOTAL NATURAL AREA (SF)	TOTAL AREA (AC)	IMPERVIOUS PERCENT
A1	36,002.00	2,926.00	16,175.00	55,103.00	153,114.00	4.78	26%
A2	17,725.00	15,042.00	30,696.00	63,463.00	131,686.00	4.48	33%
A3	0.00	0.00	8,418.00	8,418.00	18,153.00	0.61	32%
A4	3,507.00	1,190.00	17,708.00	22,405.00	47,726.00	1.61	32%
A5	0.00	0.00	11,981.00	11,981.00	20,253.00	0.74	37%
A6	0.00	0.00	0.00	0.00	38,768.00	0.89	0.00%
A7	0.00	0.00	0.00	0.00	29,621.00	0.68	0.00%



Drawing Name: Q:\2021\21\Engineering\Hydrology_Plan\Exhibits\Exhibit A - Post-Project Hydrology Map.dwg
Last Opened: May 26, 2023 - 1:30pm by mhunt.

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EXHIBIT "A"
CUP NO. 21-0121
POST-PROJECT RATIONAL
METHOD MAP

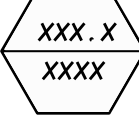
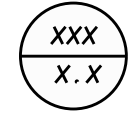



EXHIBIT B: PRE-PROJECT CONDITION UNIT HYDROGRAPH MAP

PRELIMINARY CONDITIONAL USE PERMIT 21-1021

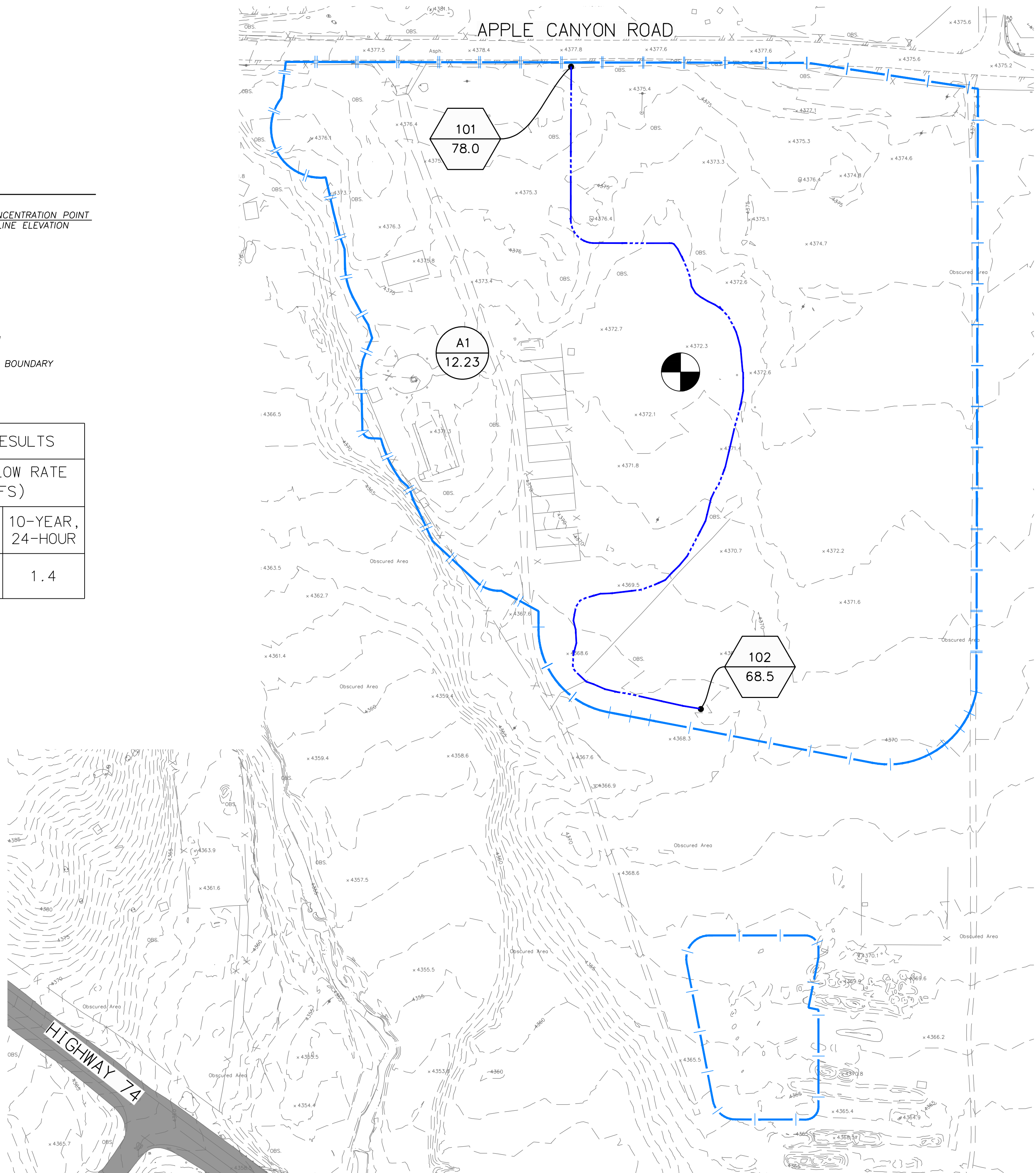
COUNTY OF RIVERSIDE, STATE OF CALIFORNIA

PRE-PROJECT CONDITION UNIT HYDROGRAPH MAP

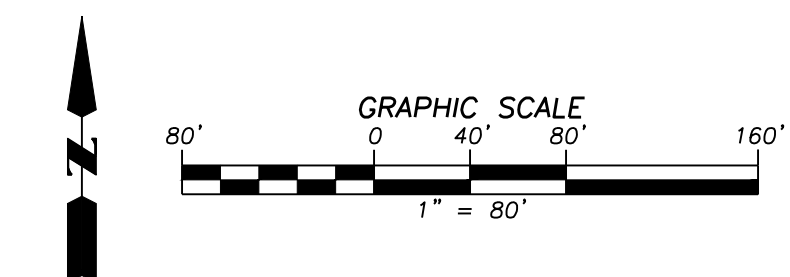
LEGEND:

	NODE/CONCENTRATION POINT FLOWLINE ELEVATION
	SUB AREA ACRES
	CENTROID
	FLOW PATH
	WATERSHED BOUNDARY

UNIT HYDROGRAPH RESULTS			
AREA	AREA (AC)	PEAK FLOW RATE (CFS)	
		2-YEAR, 24-HOUR	10-YEAR, 24-HOUR
A1	12.23	0.5	1.4



AREA INFORMATION							
AREA	AREA (AC)	FLOW LENGTH (FT)	CENTROID LENGTH (FT)	RUNOFF INDEX	IMPERVIOUS FRACTION	NODE TO NODE	
A1	12.23	1106	611	46.0	0.00	101	102

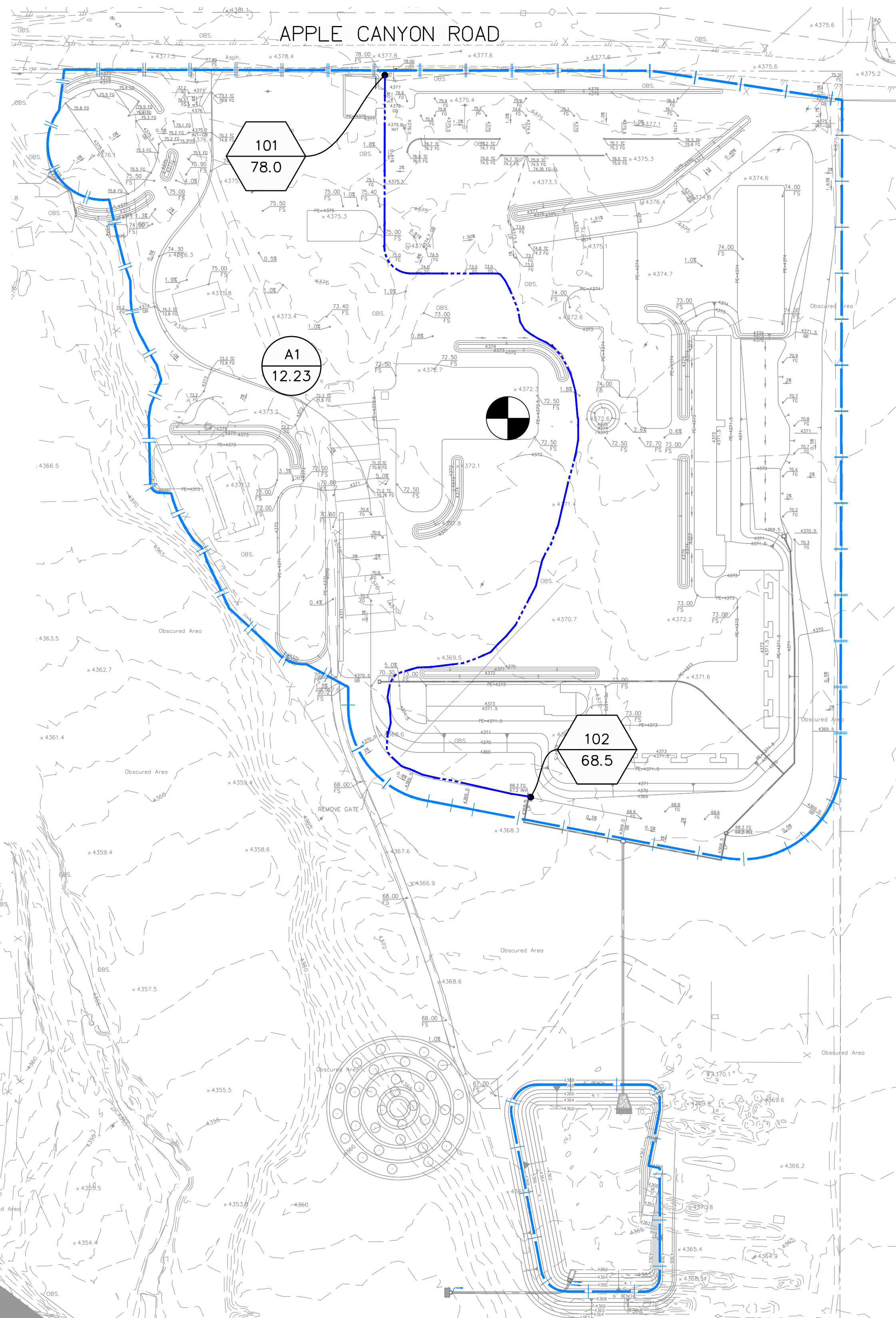


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EXHIBIT 'B'
CUP NO. 21-1021
PRE-PROJECT UNIT
HYDROGRAPH MAP

EXHIBIT C: POST-PROJECT CONDITION UNIT HYDROGRAPH MAP

PRELIMINARY CONDITIONAL USE PERMIT 21-1021
 COUNTY OF RIVERSIDE, STATE OF CALIFORNIA
POST-PROJECT CONDITION UNIT HYDROGRAPH MAP

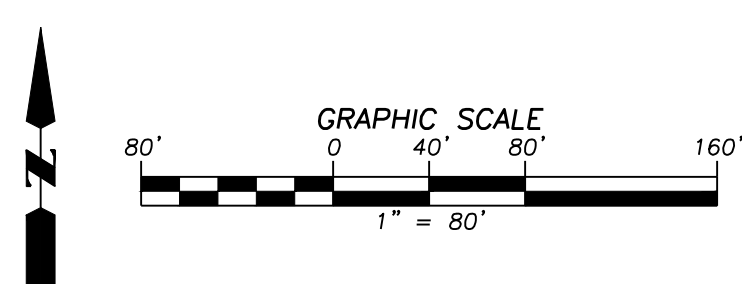


LEGEND:

	NODE/CONCENTRATION POINT FLOWLINE ELEVATION
	SUB AREA ACRES
	CENTROID
	FLOW PATH
	WATERSHED BOUNDARY

UNIT HYDROGRAPH RESULTS			
AREA	AREA (AC)	PEAK FLOW RATE (CFS)	
		2-YEAR, 24-HOUR	10-YEAR, 24-HOUR
A1	12.23	1.8	3.0

AREA INFORMATION						
AREA	AREA (AC)	FLOW LENGTH (FT)	CENTROID LENGTH (FT)	RUNOFF INDEX	IMPERVIOUS FRACTION	NODE TO NODE
A1	12.23	1106	611	33.6	0.35	101 102

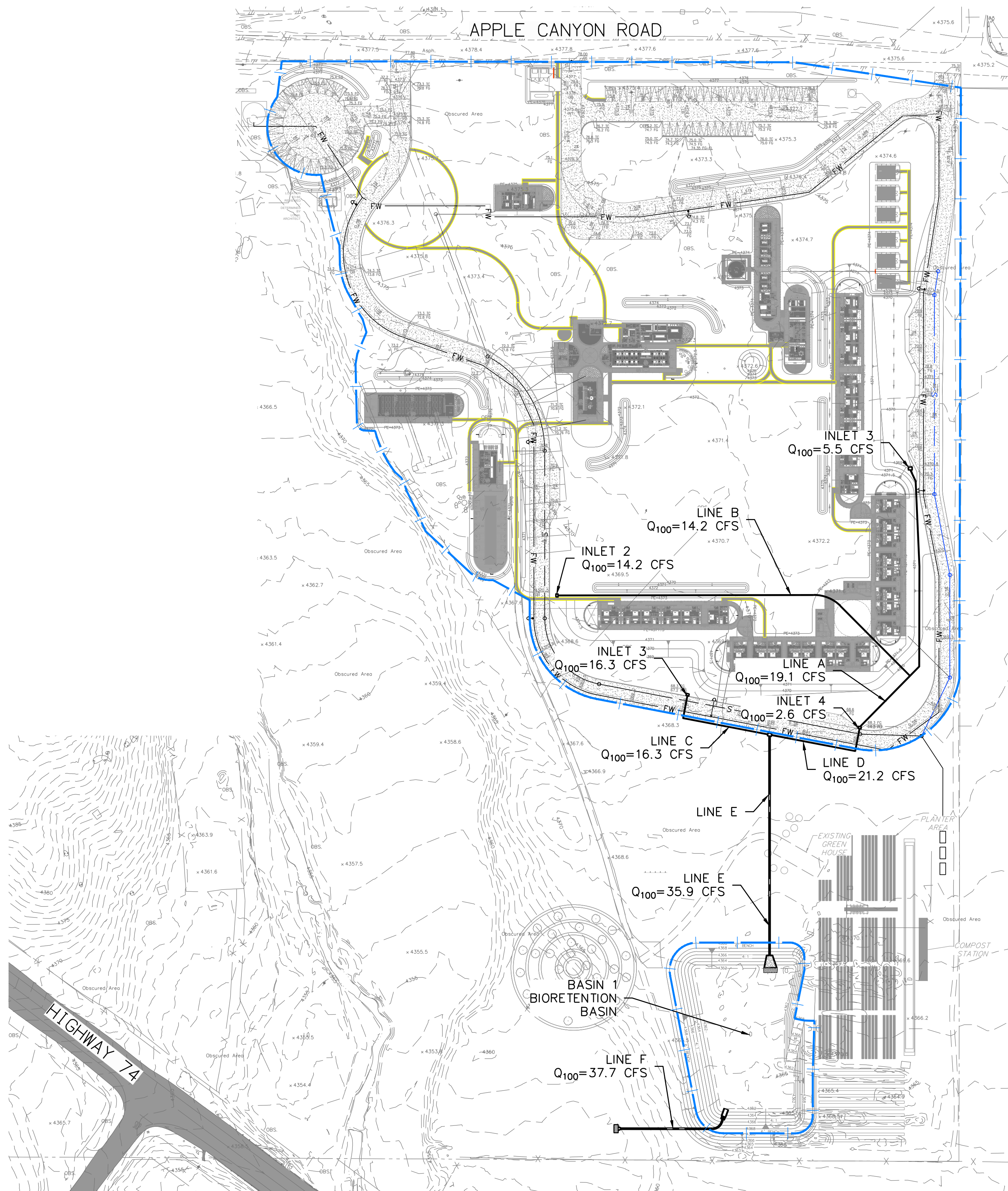


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EXHIBIT "C"
CUP NO. 21-1021
POST-PROJECT UNIT
HYDROGRAPH MAP

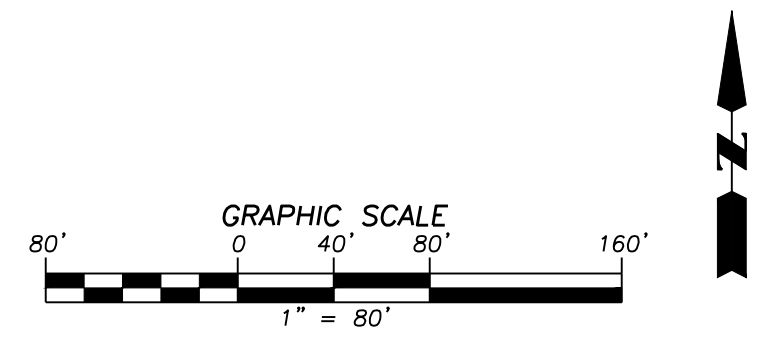
EXHIBIT D: DRAINAGE FACILITIES MAP

PRELIMINARY CONDITIONAL USE PERMIT 21-1021
 COUNTY OF RIVERSIDE, STATE OF CALIFORNIA
DRAINAGE FACILITIES MAP



LEGEND:

 WATERSHED BOUNDARY



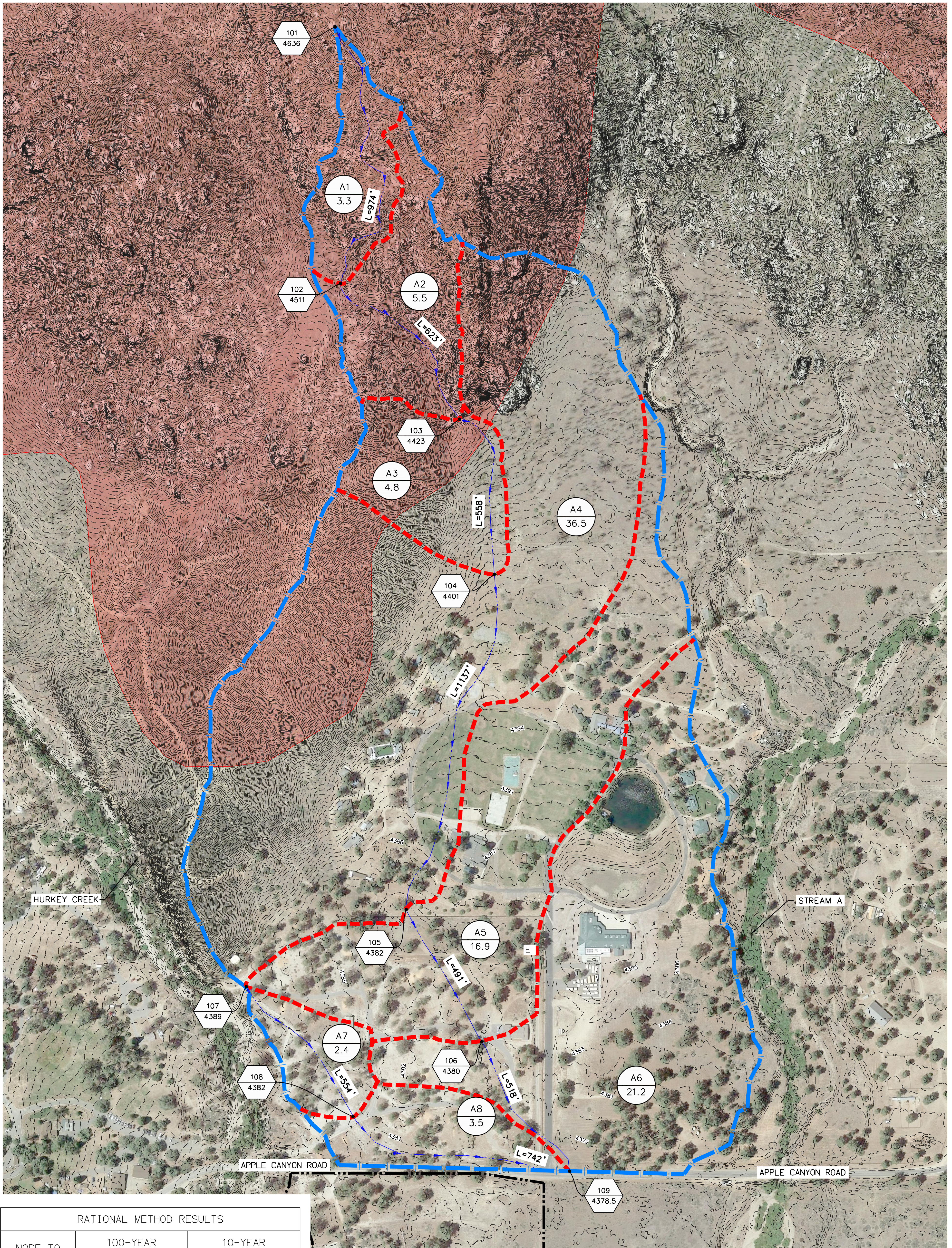
Drawing Name: Q:\2021\21\Engineering\Hydrology\Plan\Exhibits\Exhibit D - Drainage Facilities Map.dwg
 Last Opened: May 26, 2023 - 1:41pm by mhunt

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EXHIBIT 'D'
CUP NO. 21-1021
DRAINAGE FACILITIES
MAP

EXHIBIT E: OFFSITE RATIONAL METHOD HYDROLOGY

PRELIMINARY CONDITIONAL USE PERMIT 21-0121
 COUNTY OF RIVERSIDE, STATE OF CALIFORNIA
OFFSITE HYDROLOGY MAP

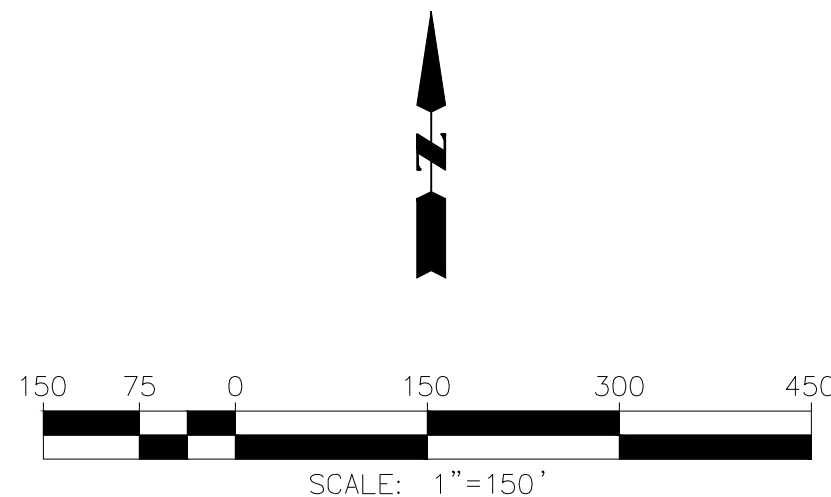


RATIONAL METHOD RESULTS

NODE TO NODE	100-YEAR		10-YEAR	
	T _c (MIN)	Q (CFS)	T _c (MIN)	Q (CFS)
101 102	16.79	11.86	16.79	7.02
102 103	17.69	31.02	17.82	18.30
103 104	18.98	45.80	19.32	26.56
104 105	22.01	140.61	22.88	76.54
105 106	24.33	178.46	25.67	95.48
106 109	27.04	221.41	28.95	116.53
107 108	21.30	5.90	21.30	3.10
108 109	28.21	12.78	29.39	6.54
108 109	27.04	233.66	28.95	122.96

LEGEND:

- NODE/CONCENTRATION POINT FLOWLINE ELEVATION
- APPROXIMATE INVERT ELEVATION
- SUB AREA ACRES
- FLOW DISTANCE
- FLOW PATH
- WATERSHED SUB-BOUNDARY
- WATERSHED BOUNDARY
- ONSITE PROJECT BOUNDARY



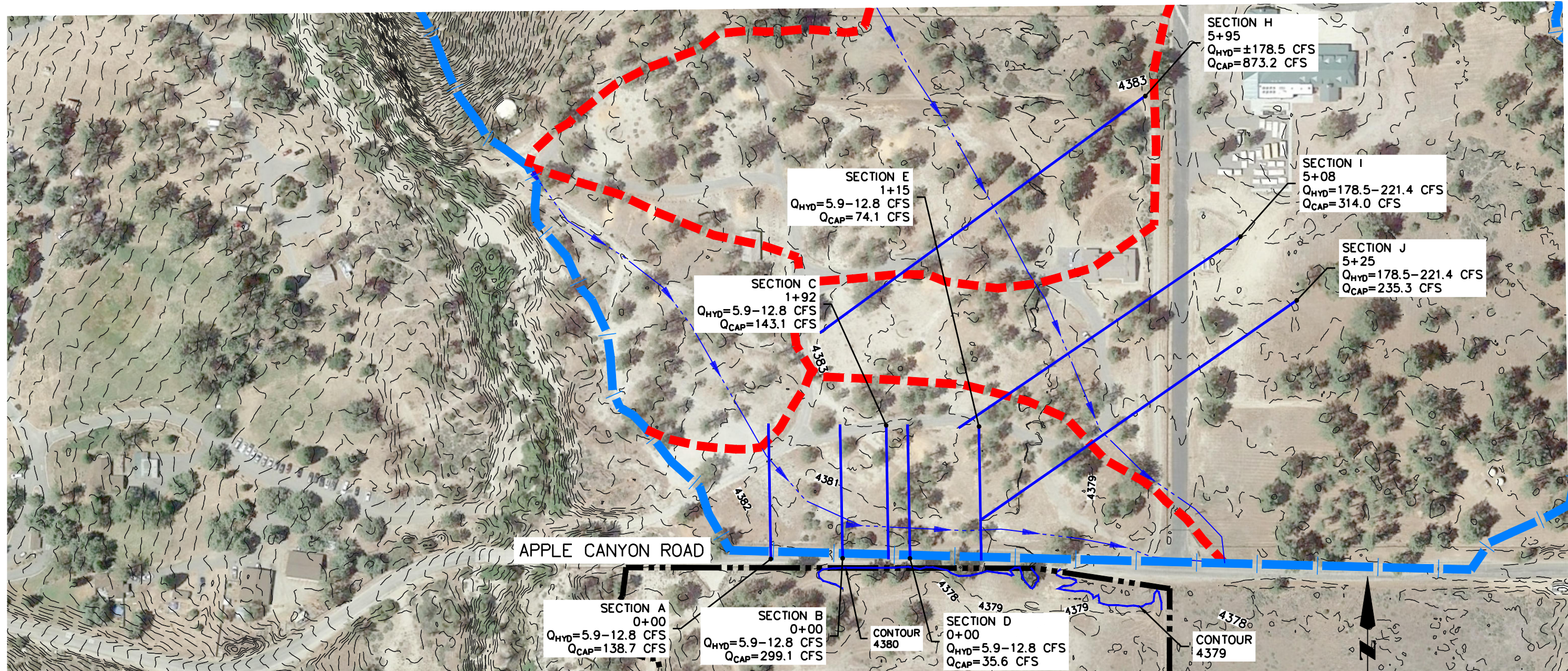
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EXHIBIT 'E'
CUP NO. 21-0121
OFFSITE
HYDROLOGY MAP

Drawing Name: O:\302.01.21\Engineering\Hydrology\Plan\Exhibits\302.01-Exhibit E - Offsite Hydrology Map.dwg
 Last Opened: Nov 18, 2022 - 7:04am by mhunt

EXHIBIT F: OFFSITE FLOW RATE ANALYSIS

Drawing Name: O:\302.01.21\Engineering\Hydrology\Plan\Exhibits\Exhibit E - F - Offsite Hydrology Map.dwg
Last Opened: Nov 18, 2022 - 2:57pm by mhunt

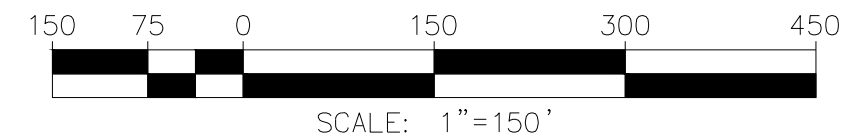


NOTE:
 Q_{HYD} = RATIONAL METHOD HYDROLOGY 100 YEAR FLOW RATE, FOUND IN EXHIBIT "E"
 Q_{CAP} = HYDRAULIC CAPACITY FLOW RATE OF THE CROSS SECTION, FOUND IN APPENDIX F.

CROSS SECTION DETAILS AND CALCULATIONS CAN BE FOUND IN APPENDIX F.

LEGEND:

- FLOW PATH
- WATERSHED SUB-BOUNDARY
- WATERSHED BOUNDARY
- CROSS SECTIONS
- ONSITE PROJECT BOUNDARY



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EXHIBIT "F"
CUP NO. 21-0121
OFFSITE FLOW RATE
ANALYSIS