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Ararat Homes
Traffic Impact Analysis
City of Los Angeles
August 1, 2020



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1.0 INTRODUCTION

This transportation assessment study is consistent with the City of Los Angeles Department of Transportation (LADOT), Transportation Assessment Guidelines (TAG). This study identifies and evaluates the potential project-specific transportation impacts of the proposed project. The analysis focuses on traditional mobility considerations as well as safety, sustainability, smart growth, and the reduction of greenhouse gas emissions.

The TAG conforms to the requirements of Senate Bill 743, and is consistent with the California Environmental Quality Act (CEQA), requiring the use of vehicle miles traveled (VMT) as the primary metric for evaluating a project's transportation impacts. The TAG also requires the traffic analysis to examine whether the proposed project conflicts with the City's plans, programs, ordinances, and policies. In addition, Non-CEQA transportation analysis is also required to assess the project's potential impacts on pedestrian, bicycle and transit facilities, project access, safety and circulation, project construction, and the potential for residential street intrusion.

2.0 PROJECT DESCRIPTION

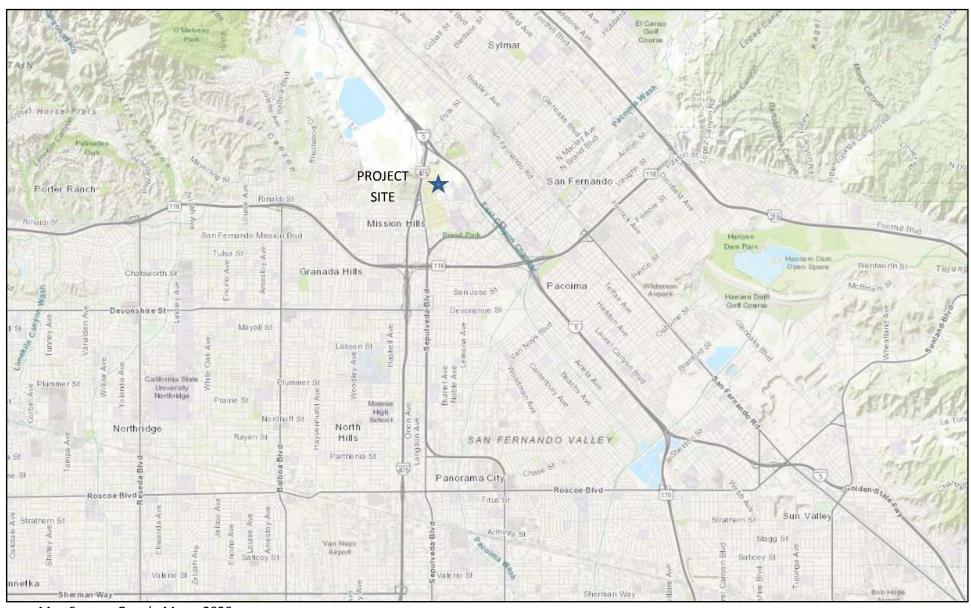
2.1 Project Location

As illustrated in **Figure 1**—*Regional Location Map*, the Project site (15105 Mission Hills Road) is located within the City of Los Angeles (the "City") in the Community of Mission Hills. Regional access to the Project site is provided by the I-5 (Golden State Freeway), the I-405 (San Diego Freeway) and the SR-118 (Ronald Reagan Freeway).

As illustrated in **Figure 2**—*Project Site and Surrounding Uses*, the Project site is bound on the east by Ararat Home of Los Angeles, an assisted living facility, on the west by Eden Memorial Park, a cemetery and on the south by Mission Hills Road. The Project site are currently occupied by three single-family housing units. The housing units will be demolished as part of the proposed project.

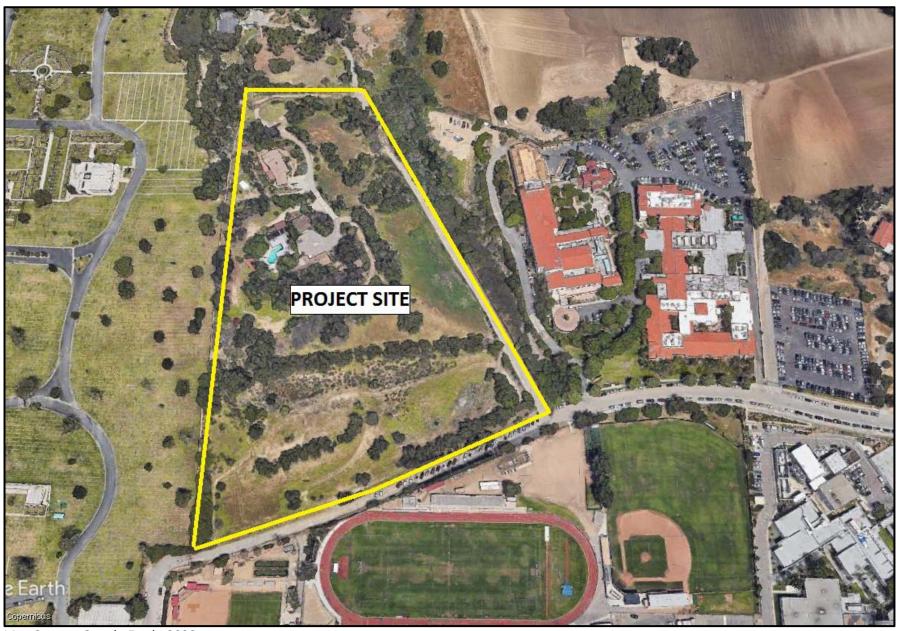
2.2 Project Characteristics

The proposed project consists of the expansion of the existing Ararat Home development. It will include the construction of 87-dwelling units of senior adult housing, a 100-bed assisted living facility and a 100-bed nursing home.



Map Source: Google Maps, 2020





Map Source: Google Earth, 2020



2.3 Existing Site Vehicular Access

Vehicle access to the existing land use is provided through a local access road that connects to Mission Hills Road.

2.4 Proposed Project Site Vehicular Access

Vehicular access to the proposed development will be provided via a driveway on Mission Hills Road at the western boundary of the project site and via the existing local access road along the eastern boundary of the site.

Please refer to **Figure 3**—*Proposed Site Plan* for an illustration of the proposed site layout. The following is a detailed description of the proposed site access points:

- Mission Hills Driveway: This driveway is planned to be 26 feet in width and located approximately 15 feet east of the western border of the Project site. This driveway will provide both ingress to and egress from the parking structure underneath the proposed assisted living and nursery facilities.
- Access Road: This access road which is approximately 24 feet in width, runs along the eastern border of the Project site and provides access to the proposed senior adult housing development to the north as well as the pick-up/drop-off area for the proposed assisted living and nursery facilities.

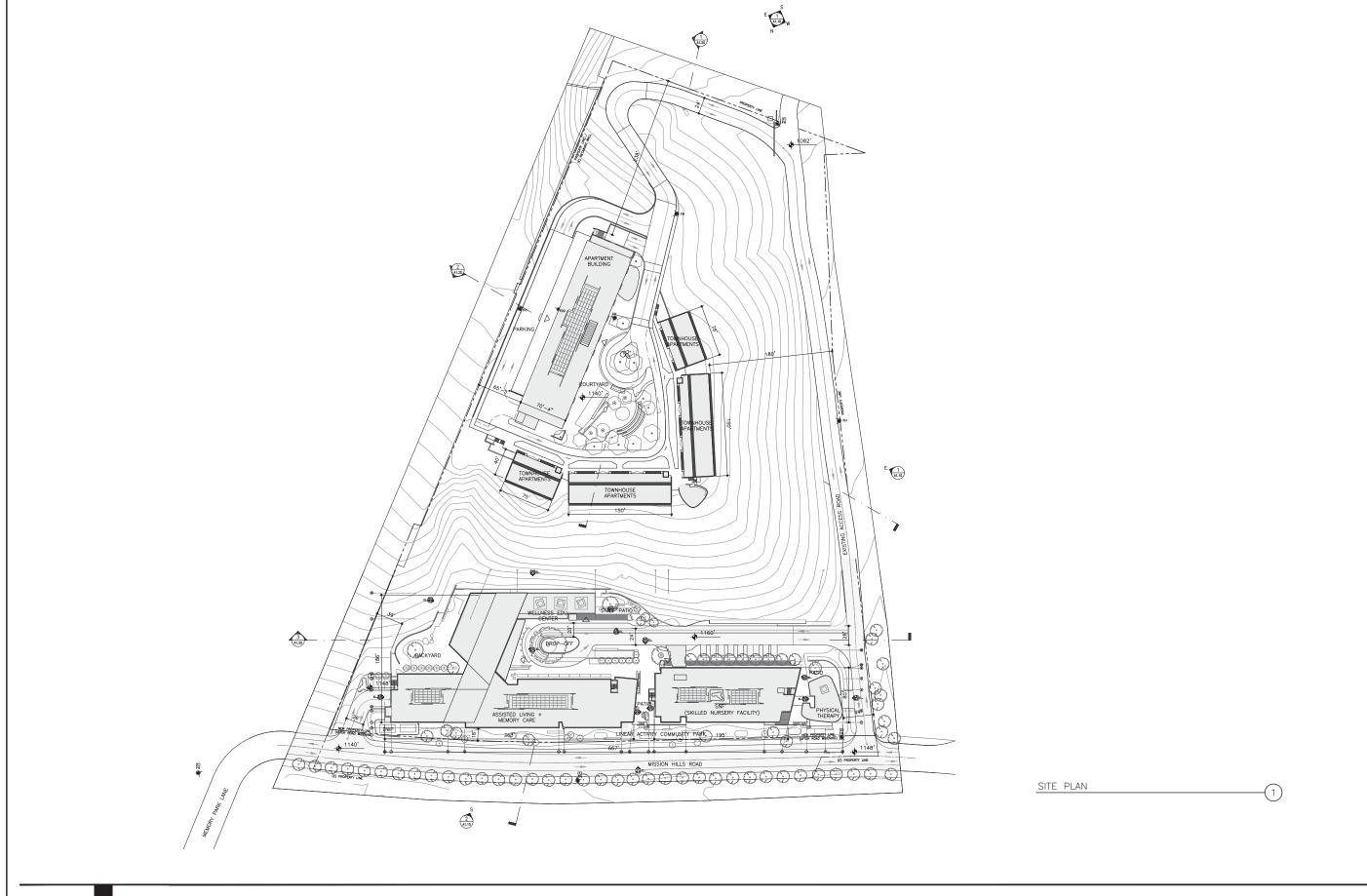
2.5 Proposed Project Parking

Project parking will be provided in a subterranean structure with 135 spaces.

2.6 Proposed Project Loading

Commercial Loading areas will be provided via the access road to either the drop-off area for the assisted living/nursery facility directly to the senior adult housing development to the north.

As described above, there is also a passenger drop-off and loading area in front of the assisted living facility.



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ARARAT HOME

SITE PLAN

EXPANSION ENTITLEMENTS APPLICATION LA CITY PLANNING DEPARTMENT

3.0 PROJECT CONTEXT

3.1 Non-vehicular Transportation System

3.1.1 Pedestrian

In the vicinity of the project site, within a quarter mile radius, the following pedestrian facilities are provided:

3.1.1.1 Sidewalk

Pedestrian sidewalks are provided at the following locations:

- Rinaldi Street—on the south side, there is a 15-foot sidewalk with tree wells west of Lev Avenue and a 10-foot sidewalk with tree wells east of Lev Avenue. On the north side, there is no sidewalk west of Memory Park Avenue, in front of the Eden Memorial Park. There is a 10-foot sidewalk between Memory Park Avenue and Alemany Drive and a 7-foot sidewalk east of Alemany Drive.
- Indian Hills Road—on the west side, there is a 6-foot sidewalk with 4-foot parkway between Rinaldi Street and Mission Hills Road. There is no sidewalk north of Mission Hills Road. On the east side, there is a 10-foot sidewalk between Rinaldi Street the Providence Holy Cross Medical Center Receiving driveway and a 6-foot sidewalk to the north.
- Mission Hills Road between Indian Hills Road and the driveway of the existing Ararat Home assisted living facility, there is a 10-foot sidewalk on the north side and a 5-foot sidewalk with 5-foot parkway on the southside. Between the existing Ararat Home driveway and Memory Park Avenue, Mission Hills Road does not have any sidewalk on both sides of the street.
- Memory Park Avenue—on the east side, there is a 6-foot sidewalk between Rinaldi Street and Mission Hills Road. There is no sidewalk on the west side of Memory Park Avenue.

3.1.1.2 Crosswalks, Curb Ramps and Pedestrian Push Buttons

In the vicinity of the project site, pedestrian crosswalks, curb ramps and pedestrian push buttons are available at the following locations:

- Rinaldi Street and Alemany Drive intersection—this is a two-phase signalized intersection with pedestrian push buttons for crossing Rinaldi Street. There are continental yellow school crosswalks crossing the east and the south legs of the intersection with accessible ramps.
- Rinaldi Street and Indian Hills Road intersection—this is also a two-phase signalized intersection with pedestrian push buttons for crossing Rinaldi Street. There are white traditional crosswalks crossing the east, west and south legs of the intersection. There is a white continental crosswalk crossing the north leg. Curb ramps are provided at all four corners.

- Mid-block location on Indian Hills Road just north of Providence Holy Cross Medical Center
 Receiving driveway—there is a mid-block white continental crosswalk with curb ramps on Indian
 Hills Road. The north and southbound vehicular traffic are controlled by STOP signs.
- Indian Hills Road and Mission Hills Road intersection—the eastbound approach of this three-legged intersection is controlled by a STOP sign. There are curb ramps provided for an unmarked crosswalk crossing the south leg of the intersection.

As part of the proposed project, Mission Hills Road in front of the project site will be widened and improved to provide a two-way roadway with one lane of traffic in each direction with sidewalks on both sides of the street in accordance to City standards.

According to the City of Los Angeles' Mobility Plan 2035, Rinaldi Street east of Indian Hills Road is a Pedestrian Enhanced District. Therefore, the project will comply with City requirements for Pedestrian Enhanced Districts, if any.

3.1.2 Bicycle

Within a quarter mile radius of the project site, bicycle facilities are provided at the following location:

 <u>Rinaldi Street</u>—there is a Class II Bicycle Lane for eastbound and westbound directions on Rinaldi Street.

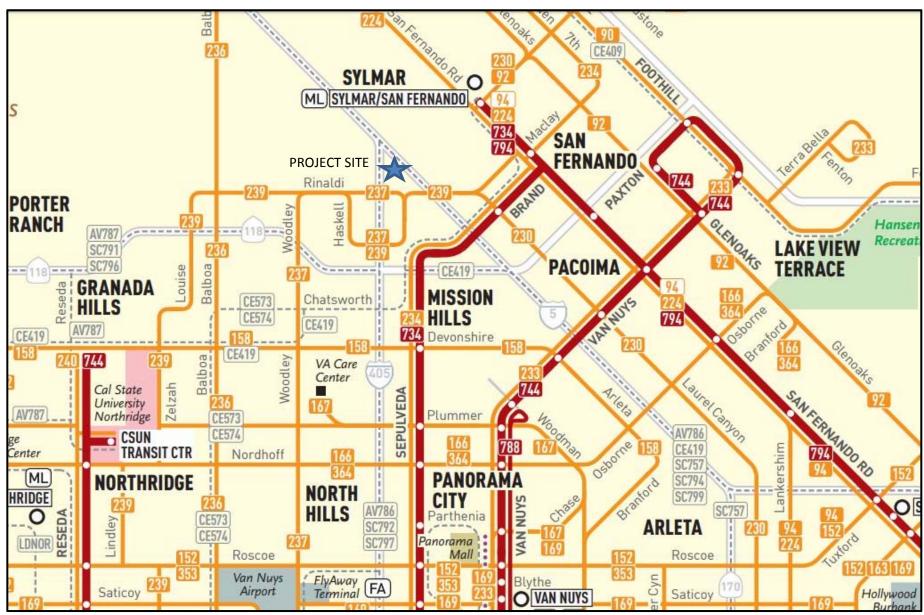
According to the City of Los Angeles' Mobility Plan 2035, the project site and its vicinity is not a part of the Bicycle Enhanced Network.

3.1.3 Transit

Transit services in the project area are provided by Los Angeles County Metropolitan Transportation Authority (Metro). LA Metro operates local bus Routes 237 and 239 within the study area along Sepulveda Boulevard and Rinaldi Street.

- Route 237 operates between the intersection of Highland Avenue and Santa Monica Boulevard in Hollywood and the intersection of Sepulveda Boulevard and Rinaldi Street in Mission Hills. Services are provided from Monday through Friday from 4:48 am to 1:42 am. During the peak periods, the headways are approximately 45 to 50 minutes.
- Route 239 operates between the Metrolink Sylmar Station in Sylmar and the intersection of Ventura Boulevard and Zelzah Avenue in Encino. Services are provided from Monday through Friday from 5:10 am to 10:34 pm. During the peak periods, the headways are approximately 50 minutes to an hour.

Please refer to **Figure 4**—*Map of Existing Public Transit* for illustrations of the locations of existing public transit routes. Rinaldi Street in the vicinity of the project site is identified as a Comprehensive Transit Enhanced Street in the City of Los Angeles' Mobility Plan 2035, Transit Enhanced Network.



Map Source: LA Metro, 2020



3.2 Vehicular Transportation System

3.2.1 Regional Freeway System

The Project area is served by the following freeways:

- The I-5 (Golden State) Freeway, is a north/south freeway that extends between northern and southern California. The nearest intersection freeways are the I-405 to the north and the SR-118 to the south. The segment of the I-5 freeway near the project site generally consists of three mixed-flow travel lanes in each direction plus one High Occupancy Vehicle (HOV) lane for the southbound direction. In the vicinity of the study area, there are north and southbound ramps at San Fernando Mission Boulevard.
- The I-405 (San Diego) Freeway, near the project site, is a north/south freeway that begins at the I-5 Freeway in the community of Mission Hills, in the San Fernando Valley, and extends southerly to the City of Irvine in Orange County. The nearest intersecting freeways are the SR-118 to the south and the I-5 to the north. The segment of the I-405 Freeway near the project site consists of three mixed-flow travel lanes in each direction. There is also one High Occupancy Vehicle (HOV) lane for the southbound direction. Near the study area, there are southbound ramps located at Rinaldi Street and northbound ramps at Sepulveda Boulevard.
- The SR-118 (Ronald Reagan) Freeway, near the project site, is an east/west freeway that extends between Pacoima in the San Fernando Valley to the east and the City of Moorpark to the west. The nearest intersecting freeways are the I-405 to the west and the I-5 to the east. The segment of the SR-118 freeway near the project site consists of four mixed-flow travel lanes and one High Occupancy Vehicle (HOV) lane for each direction. Near the study area, there are eastbound and westbound ramps located on Sepulveda Boulevard.

3.2.2 Surrounding Roadway Systems

The Project area is served by the following surrounding major roadways:

- Sepulveda Boulevard in the vicinity of the project site is a north-south "Boulevard II". It has three travel lanes in each direction separated by a continuous two-way left-turn lane. Parking is allowed on both sides of the street.
- Rinaldi Street is classified as an east-west "Avenue I". It has two travel lanes and a bike lane in each direction with a continuous two-way left-turn lane. On Rinaldi Street between

Sepulveda Boulevard and Lev Avenue, parking is prohibited on both sides of the street. East of Lev Avenue, there are parking on both sides of the street at various locations.

- Indian Hills Road is classified as a "Local Street" and it runs in a north-south direction in the vicinity of the project site. It has one lane in each direction with parking on both sides of the street.
- Mission Hills Road is an east-west "Local Street" that runs between Memory Park Avenue and Indian Hills Road. East of the project site, Mission Hills Road has one travel lane in each direction and parking is allowed on both sides of the street. In front of the project site, Mission Hills Road is currently an eastbound one-way street with angled parking on the north side of the street. As part of the off-site improvement for this project, Mission Hills Road will be widened along the project frontage to accommodate one lane of traffic and sidewalk in each direction with parking on both sides of the street.
- Memory Park Avenue is a north-south "Local Street" that connects Rinaldi Street and Mission Hills Road. It has one lane in each direction with parking on both sides of the street. It provides access to the Eden Memorial Park and the Bishop Alemany High School.

4.0 CEQA TRANSPORTATION IMPACT ASSESSMENT

In compliance with CEQA and/or in accordance with City regulations, LADOT may require applicants to analyze and assess project-specific transportation impacts based on the following criteria:

- If the Development Project is estimated to generate a net increase of 250 or more daily vehicle trips and requires discretionary action, a transportation assessment for a Development Project is required.
- If a Transportation Project is likely to either: (1) induce additional vehicle miles traveled by increasing vehicle capacity; or (2) reduce roadway through-lane capacity on a street that exceeds 750 vehicles per hour per lane for at least two (2) consecutive hours in a 24-hour period after the project is completed, a transportation assessment is generally required.
- A transportation assessment is required by City ordinance or regulation.

The preparation of a transportation impact assessment requires analysis and prediction of impacts or deficiencies to the circulation system generated by Development or Transportation Projects as well as the identification of feasible measures or corrective conditions to offset any impacts or deficiencies identified through a transportation assessment.

4.1 Project Daily Vehicle Trip Generation

LADOT's VMT calculator, Version 1.3, was used to determine if the project would exceed any of the Transportation Impact Assessment criteria which would require further transportation impact analysis. Based on the land use and size of existing and proposed project it was determined that the project would generate 978 Daily Vehicle Trips (see **Appendix A**). Since the project's Daily Vehicle Trips exceed the 250 threshold and the project requires discretionary action, as shown in the table below, further transportation impact assessment would be required:

Table 1: Project Daily Trip Generation

	Existing Land Use	Proposed Project	Net Increase
Daily Vehicle Trips	0	978	978
Daily VMT	0	9,161	9,161

4.2 Analysis Methodology

There are two categories of transportation impact analysis required by the LADOT's TAG. The first category, in Section 2, relates to potential transportation impacts under the California Environmental Quality Act (CEQA). Should a project exceed thresholds identified in the TAG, its impact would be considered significant under CEQA and thus would require any feasible mitigation measures be

implemented to reduce the impact below the threshold of significance, to the extent feasible. The CEQA thresholds identified in the TAG are consistent with City adopted thresholds and with State CEQA guidance.

The other category of analysis, non-CEQA transportation impact analysis found in Section 3 of the TAG, analyze transportation issues relating to safety, access, and circulation as they may be the result of the construction or operation of a project. The TAG identifies specific screening criteria in Sections 2 and 3 to determine whether each type of CEQA and non-CEQA transportation analysis is required depending on the size, use and daily vehicular trip generation of the project.

4.3 City of Los Angeles CEQA Thresholds

The City of Los Angeles TAG identifies three Thresholds to assess the project's transportation impacts, as follows:

- Threshold T-1: Conflicting with Plans, Programs, Ordinances, or Policies
 - o Impact Criteria: Would the project conflict with a program, plan, ordinance, or policy addressing the circulation system, including transit, roadways, bicycle, and pedestrian facilities?
- Threshold T-2.1: Causing Substantial Vehicle Miles Traveled
 - Impact Criteria: Development Projects The development project will have a potential impact if the project meets the following:
 - For residential projects, the project would generate household VMT per capita exceeding 15% below the existing average household VMT per capita for the Area Planning Commission (APC) area in which the project is located. (see Table 2.2-1)
 - For office projects, the project would generate work VMT per employee exceeding 15% below the existing average work VMT per employee for the APC in which the project is located. (see Table 2.2-1)
 - For regional serving retail projects, the project would result in a net increase in VMT.
 - For other land use types, measure VMT impacts for the work trip element using the criteria for office projects above. (see Table 2.2-1)

Table 2: VMT Impact Criteria (15% Below APC Average)

Area Planning Commission	Daily Household VMT per Capita	Daily Work VMT per Employee
Central	6.0	7.6
East LA	7.2	12.7
Harbor	9.2	12.3
North Valley	9.2	15.0
South LA	6.0	11.6
South Valley	9.4	11.6
West LA	7.4	11.1

- Threshold T-3: Substantially Increasing Hazards Due to a Geometric Design Feature or Incompatible Use
 - o **Impact Criteria:** Would the project substantially increase hazards due to a geometric design feature (e.g., sharp curves or dangerous intersections) or incompatible uses (e.g., farm equipment)?

4.4 CEQA Analysis of Transportation Impacts

4.4.1 Threshold T-1: Conflict with Plans, Programs, Ordinances, or Policies

According to the LADOT's TAG, "the City of Los Angeles has adopted programs, plans, ordinances and policies that establish the transportation planning regulatory framework for all travel modes. The overall goals of these policies are to achieve a safe, accessible and sustainable transportation system for all users".

4.4.1.1 Screening Criteria for T-1

The TAG Table 2.1-1 lists all policies that should be reviewed as part of the analysis to identify any potential conflicts with the proposed project. The TAG also provides a list of questions in Table 2.1-2 to guide the review of the documents in Table 2.1-1.

Table 3: Questions to Determine Project Applicability to Plans, Policies, and Programs

	Guiding Questions	Relevant Plans, Policies, and Programs	Supporting/Complementary City Plans, Policies, and Programs to consult	Project
		EXISTING PLAN APPLICAB	ILITY	
1	Does the project include additions or new construction along a street designated as a Boulevard I, and II, and/or Avenue I, II, or III on property zoned	LAMC Section 12.37		No

	for R3 or less restrictive			
	zone? (screening question)			
2	Is project site along any network identified in the City's Mobility Plan?	MP 2.3 through 2.7		No
3	Are dedications or improvements needed to serve long-term mobility needs identified in the Mobility Plan 2035?	MP - Street Classifications; MP - Street Designations and Standard Roadway Dimensions	MP - 2.17 Street Widenings	If needed, Project would comply
4	Does the project require placement of transit furniture in accordance with City's Coordinated Street Furniture and Bus Bench Program?			No
5	Is project site in an identified Transit Oriented Community (TOC)?	MP - TEN; MP - PED; MP - BEN; TOC Guidelines		No
6	Is project site on a roadway identified in City's High Injury Network?	Vision Zero	Mobility Plan 2035	No
7	Does project propose repurposing existing curb space? (Bike corral, carsharing, parklet, electric vehicle charging, loading zone, curb extension, etc.)	MP - 2.1 Adaptive Reuse of Streets; MP - 2.10 Loading Areas; MP - 3.5 Multi-Modal Features; MP - 3.8 Bicycle Parking; MP - 4.13 Parking and Land Use Management; MP - 5.4 Clean Fuels and Vehicles	MP - 2.3 Pedestrian Infrastructure; MP - 2.4 Neighborhood Enhanced Network; MP - 3.2 People with Disabilities; MP - 4.1 New Technologies; MP 5.1 Sustainable Transportation; MP - 5.5 Green Streets	No
8	Does project propose narrowing or shifting existing sidewalk placement?	MP 2.3 Pedestrian Infrastructure; MP 3.1 - Access for All; MP -PED; MP - ENG 19; MP 2.17 Street Widenings	Healthy LA; Vision Zero; Sustainability Plan	No
9	Does project propose paving, narrowing, shifting or removing an existing parkway?	MP - 5.5 Green Streets; Sustainability Plan		No
10	Does project propose modifying, removing or otherwise affect existing bicycle infrastructure? (ex: driveway proposed along street with bicycle facility)	MP - BEN; MP - 4.15 Public Hearing Process	Vision Zero	No
11	Is project site adjacent to an alley? If yes, will project make use of, modify, or restrict alley access?	MP - 3.9 Increased Network Access; MP - ENG.9; MP - PL.1; MP - PL.13; MP - PS.3		No No
12	Does project create a cul- de-sac or is project site located adjacent to	MP - 3.10 Cul-de-sacs		No

	existing cul-de-sac? If yes, is cul-de-sac consistent with design goal in Mobility Plan 2035 (maintain through bicycle and pedestrian access)?			
		ACCESS: DRIVEWAY AND LOA	-	
13	Does project site introduce a new driveway or loading access along an arterial (Avenue or Boulevard)?	MP - PL.1; MP - PK.10, CDG 4.1.02	Vision Zero	No
14	If yes to 13, Is a non- arterial frontage or alley access available to serve the driveway or loading access needs?	MP - PL.1; MPP 321	Vision Zero	N/A
15	Does project site include a corner lot? (avoid driveways too close to intersections)	CDG 4.1.01		No
16	Does project propose driveway width in excess of City standard?	MPP Sec. 321	Vision Zero, Sustainability plan MP - PED, MP - BEN CDG 4.1.04	No
17	Does project propose more driveways than required by City maximum standard?	MPP - Sec No. 321 Driveway Design	Vision Zero, MP, Healthy LA	No
18	Are loading zones proposed as a part of the project?	MP - 2.10 Loading Areas; MP - PK.1; MP - PK.7; MP - PK.8; MPP 321		No
19	Does project include "drop-off" zones or areas? If yes, are such areas located to the side or rear of the building?	MP - 2.10 Loading Areas		Yes, Onsite
20	Does project propose modifying, limiting/ restricting, or removing public access to a public right-of-way (e.g., vacating public right-of-way?)	MP - 2.3 Pedestrian Infrastructure; MP - 3.9 Increased Network Access		No

4.4.1.2 Project Consistency Analysis

Each of the documents listed in TAG Table 2.1-1 was reviewed for applicability to the Project, and the relevant transportation-related policies are described below, along with the Project's conformance.

4.4.1.3 Summary T-1

The Project is consistent with each of the City documents listed in Table 2.1-1 of the TAG. Therefore, the Project would not result in a significant impact under Threshold T-1 and no mitigation measures are required. The project, together with the Related Projects, is consistent with the documents and no significant cumulative impact would occur under Threshold T-1.

4.4.2 Threshold T-2: Causing Substantial Vehicle Miles Traveled

LADOT created a VMT Calculator tool which is specifically designed and intended to be used to develop project specific daily household VMT per capita and daily work VMT per employee for land use development projects in the City of Los Angeles. It implements the methodologies and significance thresholds described in Section 2.2 of the TAG for residential and employment projects. A proposed project's daily trips should be estimated using the VMT Calculator tool or the most recent version of the ITE Trip Generation Manual as described in the Section 2.2.4 of the Transportation Assessment Guidelines. TDM strategies should not be considered for the purpose of screening.

4.4.2.1 Screening Criteria for T-2

According to LADOT's TAG, if the project requires a discretionary action, and the answer is no to either T-2.1-1 or T-2.1-2, further analysis will not be required for Threshold T-2.1, and a "no impact" determination can be made for that threshold:

- T-2.1-1: Would the land use project generate a net increase of 250 or more daily vehicle trips?
- T-2.1-2: Would the project generate a net increase in daily VMT?

In addition to the above screening criteria, the portion of, or the entirety of a project that contains small- scale or local serving retail uses are assumed to have less than significant VMT impacts. If the answer to the following question is no, then that portion of the project meets the screening criteria and a no impact determination can be made for the portion of the project that contains retail uses. However, if the retail project is part of a larger mixed-use project, then the remaining portion of the project may be subject to further analysis in accordance with the above screening criteria. Projects that include retail uses in excess of the screening criteria would need to evaluate the entirety of the project's vehicle miles traveled, as specified in Section 2.2.4.

• If the project includes retail uses, does the portion of the project that contain retail uses exceed a net 50,000 square feet?

Independent of the above screening criteria, and the project requires a discretionary action, further analysis will be required if the following statement is true:

 Would the Project or Plan located within a one-half mile of a fixed-rail or fixed-guideway transit station replace an existing number of residential units with a smaller number of residential units?

For the purpose of screening for proposed change in housing units located near fixed-rail or fixed-guideway transit for development projects, the total number of housing units that exist on the project site should be counted and compared to the total number of housing units as proposed by the project to determine if the project would result in a net decrease in housing units. For the purposes of screening for proposed change in housing units that are in proximity to transit for land use plans, the total number of existing housing units within a 1/2 mile of a fixed-rail transit station that fall within the land use plan area should be counted and compared to the total housing capacity within the same area that could be built as a result of the land use plan to determine if the plan could result in a net decrease in housing.

4.4.2.2 Project VMT Analysis

LADOT's VMT Calculator was used to evaluate Project VMT and compare it to the VMT impact criteria. The VMT Calculator was set up with the Project's land uses and the respective sizes as the primary input. The VMT analysis results from the VMT Calculator are shown in the table below for before and after implementation of the Project's proposed TDM measures. The detailed output from the VMT Calculator is provided in **Appendix A**.

Table 4: VMT Analysis Summary

Land Use	Size				
Housing – Multi Family		87 DU			
Assisted Living & Nursing Home				200 B	eds
	VMT	Input			
Residential Population		<u> </u>		190	5
Employee Population				70	
Project Area Planning Commission				North \	/alley
VI	MT Calcul	ator Resi	ult		
	Before	TDM	After TI	MC	Reductions
Total Daily VMT	9,1	,161 8,397 764		764	
Home-Based Production VMT	2,0	2,055 1,795 260		260	
Home Based Work Attraction VMT	91	L8	853		65
Household VMT Per Capita	10	.5	9.2		1.3
Impact Threshold	9.	2	9.2		0
Significant Impact	es	No		Fully Mitigated	
Work VMT Per Employee 13		.1	13.1		0
Impact Threshold	15	15.0 15.0		0	
Significant Impact	N	lo No N/A			

4.4.2.3 VMT Before TDM Measures

As shown in the table above and **Appendix A** the Project is estimated to generate 9,161 daily VMT without incorporation of TDM measures. It would produce 2,055 home-based production VMT (used to calculate household VMT per capita) and 918 home-based work attraction VMT (used to calculate work VMT per employee). Based on the estimate of 196 population, the Project would generate average VMT per capita of 10.5, which is more than the North Valley APC impact threshold of 9.2 and, therefore, would result in a significant VMT impact. Additionally, based on the estimate of 70 employee population, the Project would generate average work VMT per employee of 13.1, which is less than the North Valley APC impact threshold of 15.0 and, therefore, would not result in a significant VMT impact.

4.4.2.4 VMT After TDM Measures

As shown in the table above and **Appendix A** with the TDM measures in place, the Project is estimated to generate 8,397 daily VMT (a reduction of 764 daily VMT compared to pre TDM mitigation measures). Based on the estimate of 196 population, the Project would generate average VMT per capita of 9.2. Therefore, with the TDM measures, the VMT impact of the Project would be reduced to the less-than-significant levels prior to incorporation of TDM.

4.4.2.5 TDM Mitigation Measures

In order to mitigate the Project VMT impacts for VMT per capita of 10.5 to less than North Valley APC impact threshold of 9.2, the following TDM strategies will be implemented to fully mitigate project impacts:

Table 5: TDM Strategy Input

Strate	еду Туре	Description	Proposed Project	Mitigations
Parking	Unbundle Parking	Monthly cost for parking (\$)	N/A	\$50
Education & Encouragement	Voluntary travel behavior change program	Employees and residents participating (%)	N/A	100%
Diguelo	Include Bike Parking Meets Ci Per LAMC Cod		N/A	Yes
Bicycle Infrastructure	Include Secure Bike Parking & Showers	Includes indoor bike parking/lockers, showers, & repair station (Yes/No)	N/A	Yes
Neighborhood Enhancement	hood Pedestrian Network Included (Within Project &		N/A	Within Project & connecting off-site

4.4.2.6 Cumulative Impacts

The Project does not demonstrate a project impact by applying an efficiency-based impact threshold (i.e. VMT per capita or VMT per employee) in the project impact analysis. Therefore, a less than significant project impact conclusion can be made which according to the TAG is enough in demonstrating there is no cumulative VMT impact.

4.4.2.7 Summary T-2

Using LADOT's VMT calculator, it was determined that the Project only generates VMT impacts for work VMT per employee at the rate of 9.8. However, by implementing TDM strategies, as shown in "TDM Strategy Input" table above, the Project mitigates all project related significant impacts to less than North Valley APC impact threshold of 9.2. The mitigation TDM strategies are listed in "TDM Strategy Input" table above.

4.4.3 Threshold T-3: Substantially Increasing Hazards Due to a Geometric Design Feature or Incompatible Use

LADOT's TAG indicates that impacts regarding the potential increase of hazards due to a geometric design feature generally relate to the design of access points to and from the project site, and may include safety, operational, or capacity impacts. Impacts can be related to vehicle/vehicle, vehicle/bicycle, or vehicle/pedestrian conflicts as well as to operational delays caused by vehicles slowing and/or queuing to access a project site. The analysis for T-3 should explore any potential conflicts that may be created by the driveway configuration or through the placement of project driveway(s) in areas of inadequate visibility, adjacent to bicycle or pedestrian facilities, or too close to busy or congested intersections.

4.4.3.1 Screening Criteria for T-3

If the project requires a discretionary action, and the answer is "yes" to either of the following questions, further analysis will be required to assess whether the project would result in impacts due to geometric design hazards or incompatible uses:

- Is the project proposing new driveways, or introducing new vehicle access to the property from the public right-of-way?
- Is the project proposing to, or required to make any voluntary or required, modifications to the public right-of-way (i.e., street dedications, reconfigurations of curb line, etc.)?

4.4.3.2 Project Hazards Analysis

Project access and circulation plans were reviewed considering commonly accepted traffic engineering design standards¹ to ascertain whether any deficiencies are apparent in the site access plans which would be considered significant. According to the TAG, the determination of significance shall be on a case-by-case basis, considering the following factors:

- The relative amount of pedestrian activity at project access points.
- Design features/physical configurations that affect the visibility of pedestrians and bicyclists to drivers entering and exiting the site, and the visibility of cars to pedestrians and bicyclists.
- The type of bicycle facilities the project driveway(s) crosses and the relative level of utilization.
- The physical conditions of the site and surrounding area, such as curves, slopes, walks, landscaping or other barriers, that could result in vehicle/pedestrian, vehicle/bicycle, or vehicle/vehicle impacts.
- The project location, or project-related changes to the public right-of-way, relative to proximity to the High Injury Network or a Safe Routes to School program area.
- Any other conditions, including the approximate location of incompatible uses that would substantially increase a transportation hazard.

As shown on the project's site plan for access and circulation in **Figure 3** the Project proposes two two-way driveways on Mission Hills Road for ingress and egress. One driveway at the easterly side of the project will serve the residential development and the nursing home. The driveway on the westerly side of the project will serve the Assisted living. There will be a third driveway for deliveries between the assisted living and the nursing home buildings.

4.4.3.3 Driveway Design

The driveway locations minimize the potential conflict between the parking facilities and pedestrians and vehicles on the street. Generally, each of the driveways would comply with LADOT's requirements in accordance with the Manual of Policies and Procedures.

¹One example of traffic engineering design standards includes but is not limited to Section 321 of LADOT's Manual of Policies and Procedures, which provides guidance on driveway design.

4.4.3.4 Project Site Pedestrian and Bicycle Access

The Project would provide pedestrian access points on Mission Hills Road. Bicycle parking would be provided on site in an area of the parking separate from the vehicular parking. Mission Hills Road adjacent to the Project site is not part of the High Injury Network. Therefore, it is less than likely to have injuries resulting from collisions between vehicles and pedestrians, bicyclists, or other vehicles on Mission Hills Road.

4.4.3.5 Cumulative Analysis

According to the LADOT's TAG, significant cumulative impact for Threshold T-3 may occur if the project, and other related projects combined with access points proposed along the same block(s), would create significant impacts. However, there are no related projects within the same blocks as the Project. The Project would not result in a significant impact with respect to Threshold T-3, neither would it contribute to a cumulatively significant impact.

4.4.3.6 Summary T-3

Based on the Project site plan review and design assumptions, the Project does not present any geometric design hazards related to traffic movement, mobility, or pedestrian accessibility, and no significant impact would occur with respect to Threshold T-3. Therefore, no mitigation measures would be required.

5.0 NON-CEQA TRANSPORTATION ANALYSIS

5.1 Pedestrian, Bicycle, and Transit Access Assessment

The pedestrian, bicycle, and transit facilities in the vicinity of the project were assessed to determine the potential impacts of the project on these facilities. The factors that were taken into consideration in assessing the potential project impacts, as required by the TAG, are listed below:

Assessment Factors	Project Impact
Removal or degradation of existing sidewalks, crosswalks, pedestrian refuge islands, and/or curb extensions/bulbouts	No
Removal or degradation of existing bikeways and/or supporting facilities (e.g., bikeshare stations, on-street bike racks/parking, bike corrals, etc.)	No
Removal or degradation of existing transit and/or local circulator facilities including stop, bench, shelter, concrete pad, bus lane, or other amenities	No
Removal of other existing transportation system elements supporting sustainable mobility	No
Increase street crossing distance for pedestrians; increase in number of travel/turning lanes; increase in turning radius or turning speeds	No
Removal, degradation, or narrowing of an existing sidewalk, path, crossing, or pedestrian access way	No
Removal or narrowing of existing sidewalk-street buffering elements (e.g., curb extension, parkway, planting strip, street trees, etc.)	No
Increase in pedestrian or vehicle volume, and thereby increase the need or attraction to cross a street at unmarked pedestrian crossings or unsignalized or uncontrolled intersections where a crossing is not available without significant rerouting. Refer to the Guidelines for Marked Crosswalks Across Uncontrolled Locations, in LADOT's Manual of Policies and Procedures (MPP) Section 344, or Guidelines for Traffic Signals in MPP Section 353 to determine approval and warrant criteria for an additional crossing.	No
Increase in pedestrian or vehicle volume, and thereby increase the need or attraction to cross a street at unmarked pedestrian crossings or unsignalized or uncontrolled intersections where a crossing is not available without significant rerouting. Refer to the Guidelines for Marked Crosswalks Across Uncontrolled Locations, in LADOT's Manual of Policies and Procedures (MPP) Section 344, or Guidelines for Traffic Signals in MPP Section 353 to determine approval and warrant criteria for an additional crossing.	No
Increase transit demand at bus stops that lack marked crossings, with insufficient sidewalks, or are in isolated, unshaded, or unlit areas.	No

5.2 Project Access, Safety and Circulation Evaluation

5.2.1. Study Intersections

After a consultation with the LADOT, it was determined that the following three (3) intersections would be analyzed and evaluated for operational assessment.

- (1) Rinaldi Street and Memory Park Avenue—a three-legged unsignalized intersection with STOP control on the southbound approach of Memory Park Avenue.
- (2) Rinaldi Street and Indian Hills Road—a four-legged two-phase signalized intersection
- (3) Indian Hills Road and Mission Hills Road—a three-legged unsignalized intersection with STOP control on the eastbound approach of Mission Hills Road

Refer to **Figure 5**—*Lane Configuration at Study Intersections* for a depiction of the configurations of traffic lanes at the approaches to the study intersections.

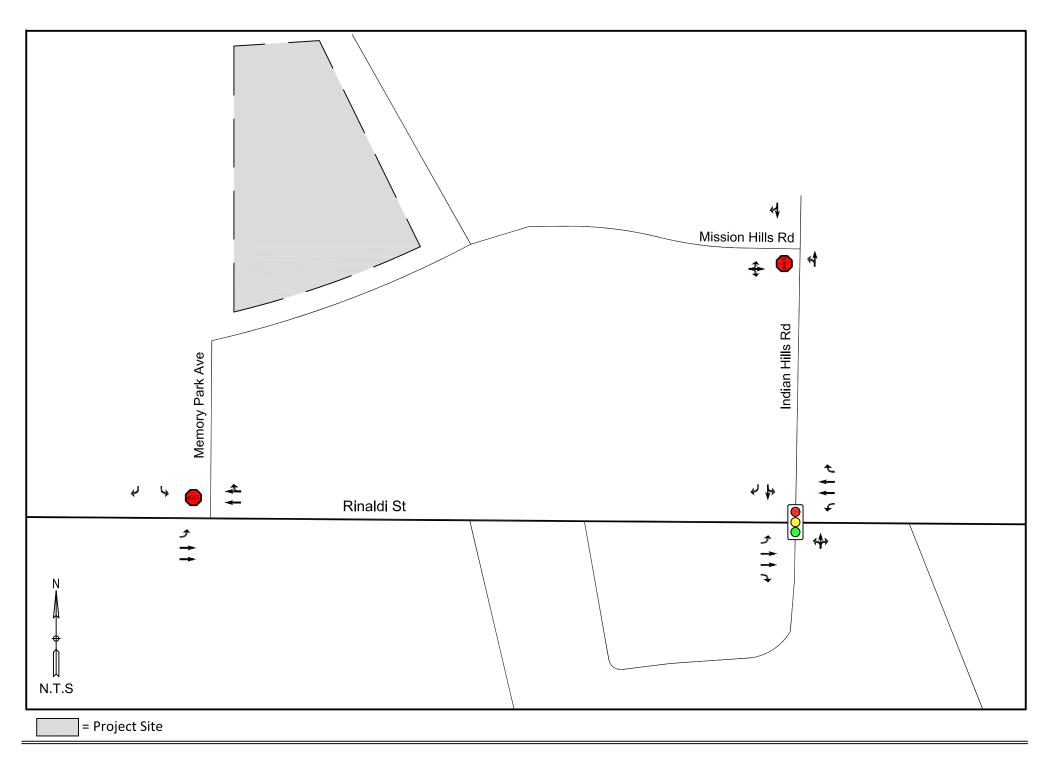
5.2.2 Existing Traffic Counts

Traffic counts were obtained for vehicular turning movements at the following three (3) intersections:

- (1) Rinaldi Street and Memory Park Avenue
- (2) Rinaldi Street and Indian Hills Road
- (3) Indian Hills Road and Mission Hills Road

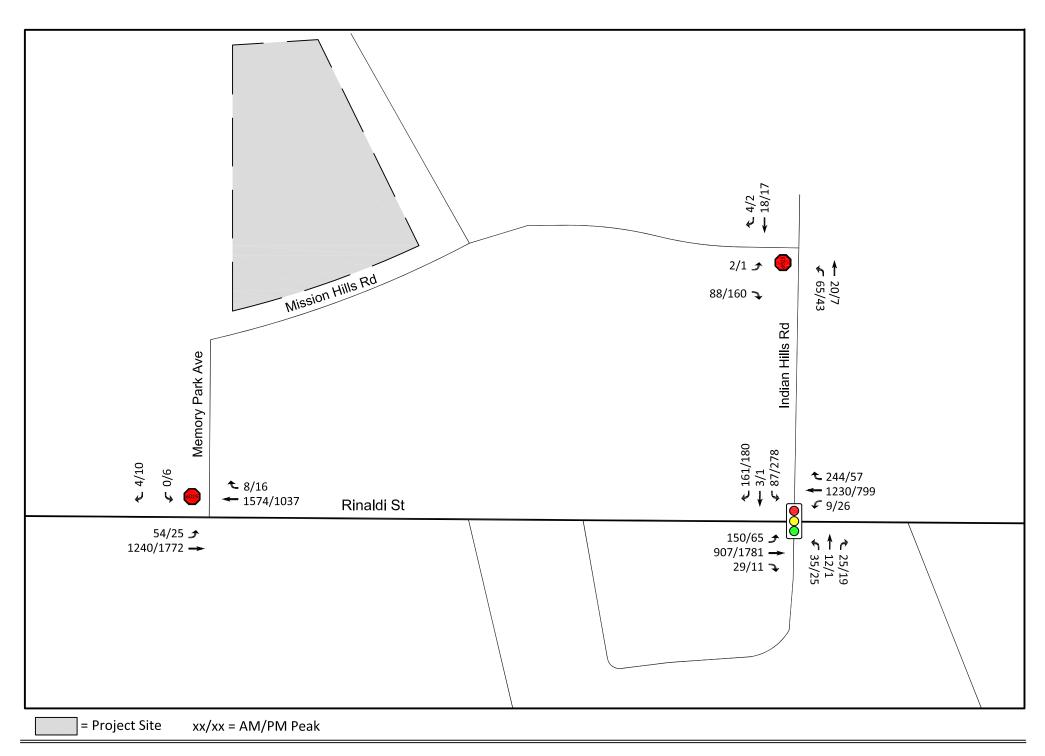
Vehicular turning movement counts were conducted on Wednesday, November 6, 2019, during the typical commuter hours of 7:00 AM to 10:00 AM and 3:00 PM to 6:00 PM, to obtain existing traffic volumes for the AM and PM peak hours.

Please refer to **Appendix B** for the manual traffic counts, and **Figure 6**—Existing (AM/PM Peak) Traffic Volumes for an illustration of the AM and PM peak-hour turning movement counts for the study intersections.



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Figure 5: Lane Configurations at Study Intersections



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Figure 6: Existing (AM/PM Peak) Traffic Volumes

5.3 Project Trip Generation Methodology

Trip rates from the Institute of Transportation Engineers (ITE) *Trip Generation Manual 10th Edition* were used in this analysis.

The trip generation calculations, and any reductions applied, are consistent with *the LADOT's TAG* and have been approved by City Staff.

The proposed Project consists of the following land uses:

- 87-dwelling units of attached senior adult housing
- 100-bed assisted Living
- 100-bed nursing home

As shown in **Table 6**—*Project Trip Generation*, the project is forecast to result in 53 net new AM peak hour trips, 71 net new PM peak hour trips, and 888 net new daily trips.

Table 6: Project Trip Generation

Land Use (ITE Code)	Size Units		AM	AM Peak Hour Trips			PM Peak Hour Trips				Daily Trips	
Lanu Ose (TE Code)	Size	Units	Rate	Total	In	Out	Rate	Total	In	Out	Rate	Total
New Project Land Use Added												
Senior Adult Housing-	87	du	0.20	17	35%	65%	0.26	23	55%	45%	2.70	322
-Attached (252)	07	uu	0.20	17	6	11	0.26	25	13	10	3.70	322
Assisted Living (254)	100	bed	0.19	19	63%	37%	0.26	26	38%	62%	2.60	260
Assisted Living (234)	100	beu	0.19	19	12	7	0.20	20	10	16	2.00	200
Nursing Home (620)	100	bod	0.17	17	72%	28%	0.22	22	33%	67%	2.06	306
Nursing Home (620)	rsing Home (620) 100 bed 0.17 17	1/	12	5	0.22	22	7	15	3.06	306		
Total Trip Generation				53	30	23		71	30	41		888

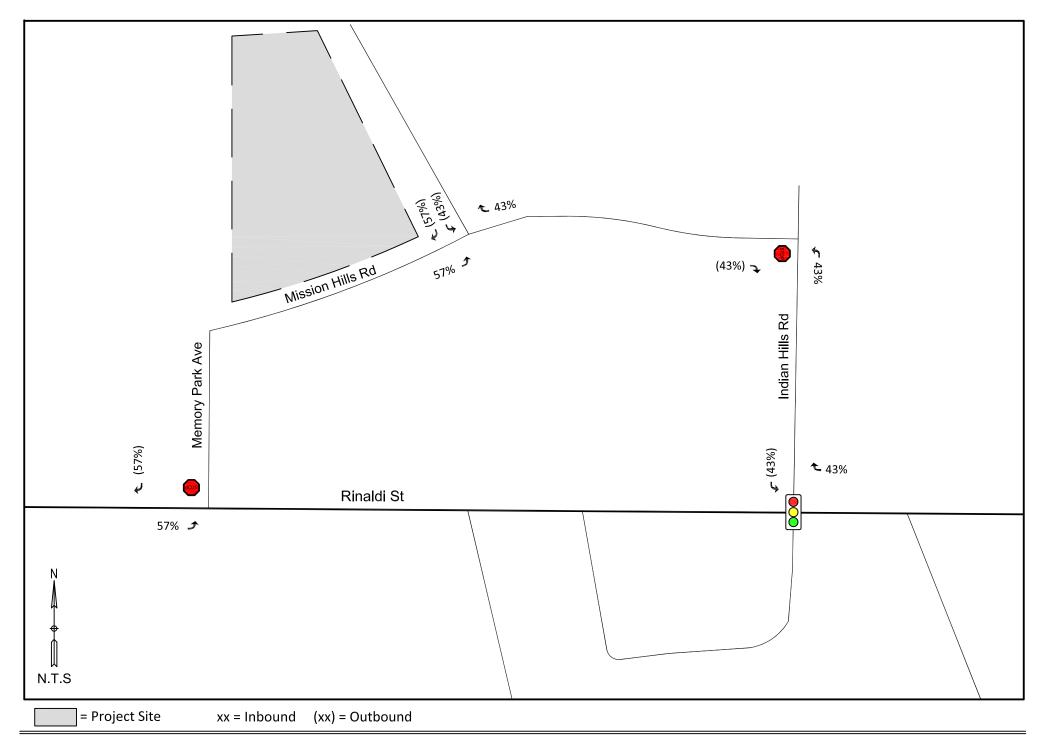
¹ ITE "Trip Generation" Manual, 10th Edition, 2017

5.4 Project Trip Distribution & Assignment

Trip distribution assumptions are used to determine the origin and destination of new vehicle trips associated with the Project. The geographic distribution of Project trips is based on the functional classification of streets in the vicinity, the magnitude of traffic volumes, as well as local knowledge of the roadway network. Based on the Project trip generation shown in **Table 6** and the regional trip distribution assumptions, a proposed study area for the traffic analysis was derived. The location and the number of the intersections to be analyzed was reviewed and approved by the LADOT staff.

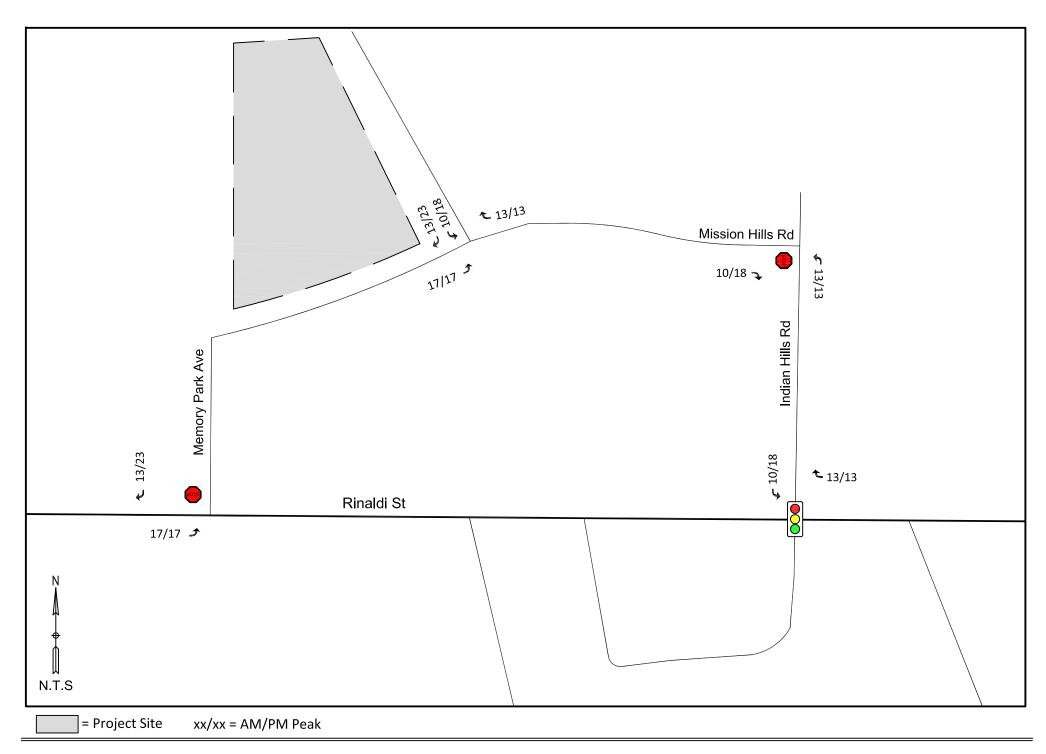
The following figures illustrate the Project's Trip Distributions and Assignments at the study intersections:

- **Figure 7**—Project Trip Distribution at Study Intersections
- **Figure 8**—*Project Trip Assignment at Study Intersections*



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Figure 7: Project Trip Distribution



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Figure 8: Project Trip Assignment

5.5 Traffic Forecasts

5.5.1 Related Projects

To understand the relative traffic impacts for the projected year of completion (2023), this traffic study analyzed potential traffic trips due to the development of related projects in the area. A list of related projects was provided by the City of Los Angeles and their associated trip volumes were calculated using the ITE Trip Generation Manual, 10th edition. A list of these projects and their locations relative to the Project site can be viewed in **Table 7** and **Figure 9** respectively. Moreover, the related project's trip assignment at the study intersections is shown in **Figure 10**.

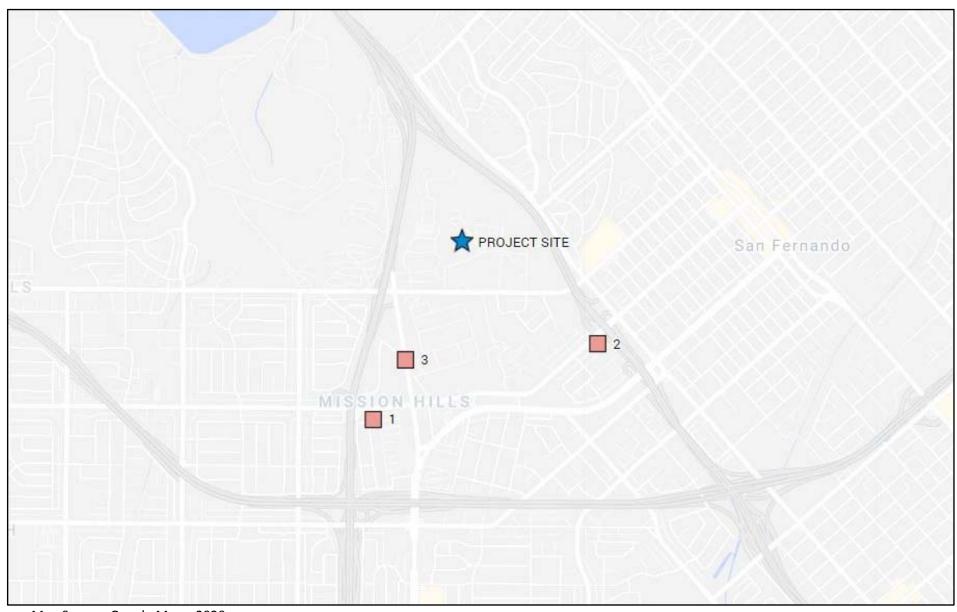
Only those projects with a potentially significant impact, and therefore a required traffic study, were included in the related projects list. Any other developing projects, that were considered small in nature, were encompassed in area wide ambient growth.

5.5.2 Ambient Traffic Growth

To account for the future traffic growth from intensification of existing developments, and other projects that are located further than a half mile from the project site, the existing traffic volumes were increased by an ambient growth rate of 2% per year to the anticipated year of completion (2023). These values were used to forecast future traffic volumes.

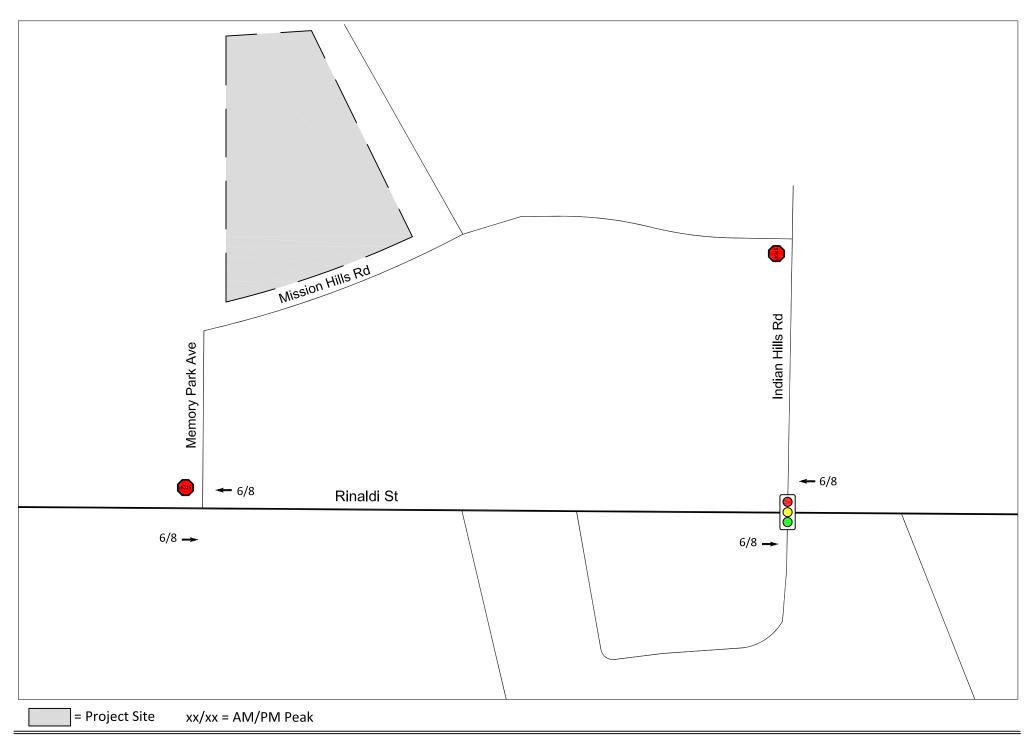
TABLE 7: RELATED PROJECT TRIP GENERATION

							Weekday Peak Hour						
Project #	Project Name	Location	Land Use	Size	Unit	Daily Trips	Morning			Evening			
							Inbound	Outbound	Total	Inbound	Outbound	Total	
1	New Office Bld	15530 San Fernando Mission Blvd	Office	28929	SF	319	36	6	42	7	36	43	
2	Senior Assisted Living	15530 West San Fernando Mission Blvd	Senior Assisted Living	191	Beds	497	23	13	36	19	31	50	
3	VTT-71105	11211 North Sepulveda Blvd	Single Family Home	65	DU	619	12	37	49	41	24	65	



Map Source: Google Maps, 2020





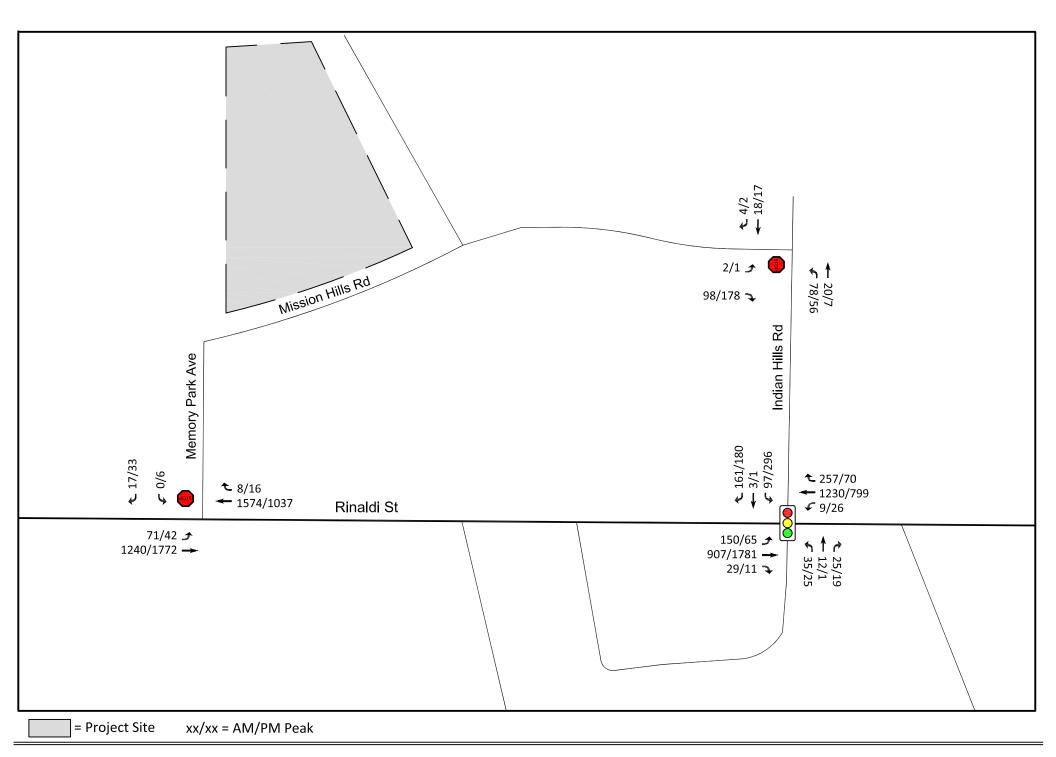
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Figure 10: Related Project Trip Assignment

5.6 Operational Evaluation

Operational analyses of vehicle average control delays, levels of service, and queueing were conducted at the three (3) study intersections for the following conditions and their traffic volumes are shown in the following figures:

1)	Existing Traffic Conditions	Figure 6
2)	Existing Plus Project Traffic Conditions	Figure 11
3)	Future (2023) Without Project Traffic Conditions	Figure 12
4)	Future (2023) With Project Traffic Conditions	Figure 13



JB & ASSOCIATES Figure 11: Existing + Project (AM/PM Peak) Traffic Volumes

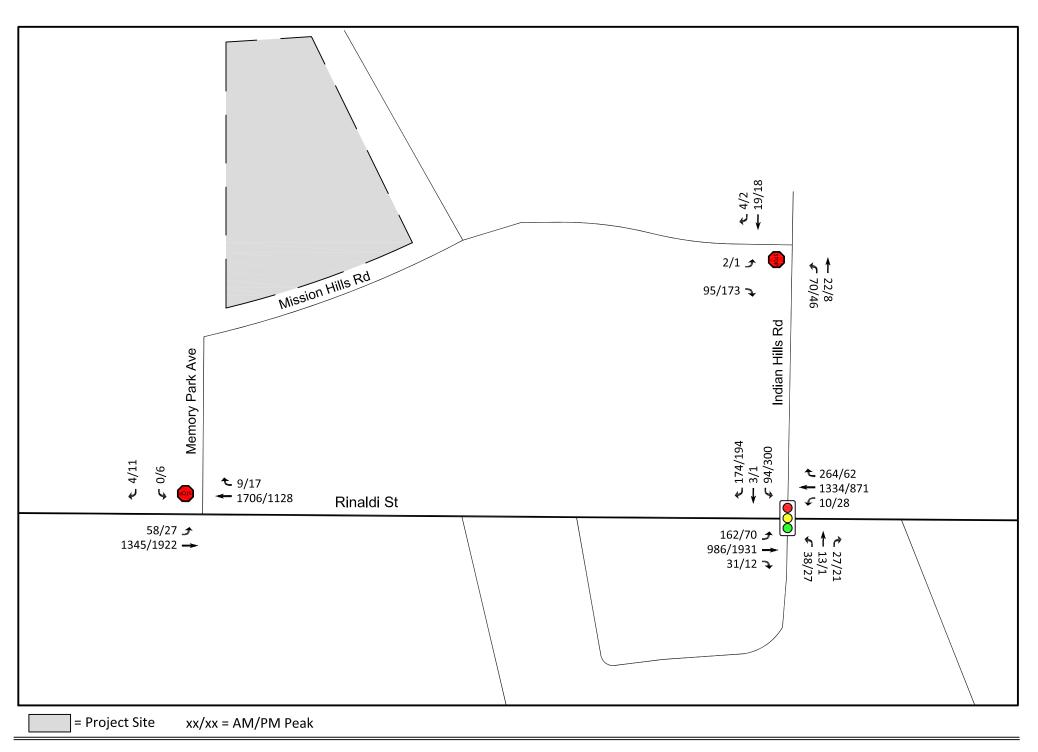


Figure 12: Future (2023) without Project (AM/PM Peak) Traffic Volumes

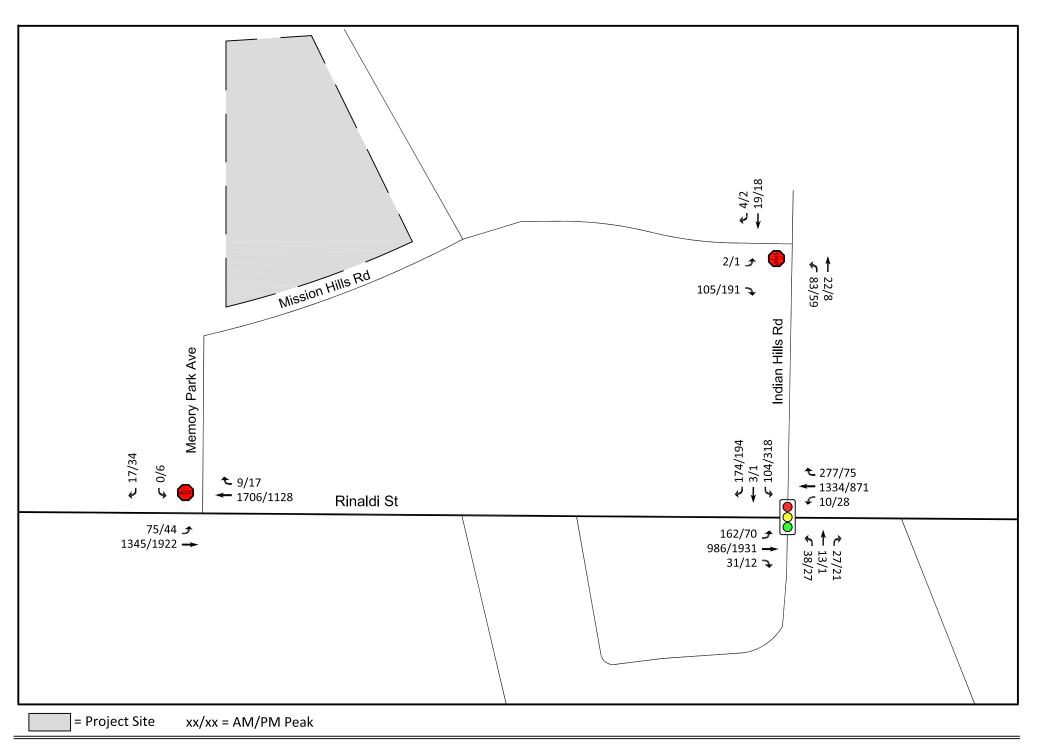


Figure 13: Future (2023) Plus Project (AM/PM Peak) Traffic Volumes

5.6.1 Level of Service/Queueing Methodology

5.6.1.1 Signalized Intersections

For signalized intersections, the City of Los Angeles utilizes the Highway Capacity Manual (HCM) operations methodology for performing signalized intersection capacity analysis. This method relies on the determination of a Level of Service (LOS) at each of the study intersection by first determining their corresponding average control delay per vehicle. Control delay includes initial deceleration delay, queue move-up time, stopped delay and final acceleration delay. It is a measure of driver discomfort, frustration, fuel consumption and lost travel time.

Level of Service varies from at best LOS A (free flow/excellent) to at worst LOS of F (stop-and-go/failure). Shown below are the LOS categories and their corresponding HCM average control delay ranges for signalized intersections.

Table 8: Level of Service Criteria for Signalized Intersections

Level of Service	Average Control Delay per Vehicle (Sec/Veh)			
А	0 to 10.00			
В	10.01 to 20.00			
С	20.01 to 35.00			
D	35.01 to 55.00			
E	55.01 to 80.00			
F	Over 80.00			

In this study, the intersection of Rinaldi Street and Indian Hills Road is controlled by a 2-phase traffic signal.

5.6.1.2 Unsignalized Intersections

For unsignalized intersections, the City utilizes the Highway Capacity Manual (HCM) methodologies for performing two-way stop-controlled (TWSC) and all-way Stop-controlled (AWTC) intersection capacity analyses. For TWSC intersection analysis, LOS is calculated for each movement of the intersection and the most critical LOS is the one that represents the effectiveness of that intersection. For AWSC intersection analysis, LOS is defined by the control delay of the entire intersection. The LOS thresholds for TWSC and AWSC intersections differ from those for signalized intersections to reflect different driver expectations. **Table 9** shows the LOS categories and their corresponding HCM average control delay ranges for TWSC and AWSC intersections.

Table 9: Level of Service Criteria for Unsignalized Intersections

Level of Service	Average Control Delay per Vehicle (Sec/Veh)
А	0 to 10.00
В	Over 10 to 15
С	Over 15 to 25
D	Over 25 to 35
E	Over 35 to 50
F	Over 50

In this study, the southbound approach of Memory Park Avenue at Rinaldi Street and the eastbound approach of Mission Hills Road at Indian Hills Road are STOP controlled.

5.6.2 Level of Service/Queueing Analysis

The results of the operational analyses are summarized in the following tables:

- **Table 10**: Levels of Service, Delays and Queue Lengths for Existing and Existing Plus Project Conditions.
- **Table 11**: Levels of Service, Delays and Queue Lengths for Future with and without Project Conditions.

Please refer to **Appendix C** for the (HCM) analysis worksheets for the analyzed intersections.

As shown in **Tables 10** and **11**, with the addition of the Project traffic, most traffic movements at the 3 study intersections will operate at levels of service D or better and with the 95th percentile queue lengths of 200 feet (approximately 10 vehicles) or shorter. However, there are two particular exceptions and are addressed on the next page.

Rinaldi Street and Memory Park Avenue

Shown below is a comparison of the afternoon peak hour estimated delays for the southbound left-turn movement under the 4 study scenarios:

<u>Existing</u>	Existing with Project	Change in Delay
137.17 sec/veh	145.74 sec/veh	8.57 sec/veh
Future without Project	Future with Project	Change in Delay
192.11 sec/veh	203.08 sec/veh	10.97 sec/veh

The southbound left-turn movement at this intersection is currently opening at LOS F with an average delay of 137.17 seconds per vehicle during the afternoon peak hour. With the addition of the Project traffic, it is estimated to remain at LOS F with an average delay of 145.74 seconds per vehicle under the Existing with Project condition and 203.08 seconds per vehicle under the Future with Project condition.

This three-legged intersection currently is STOP-controlled for the southbound direction and uncontrolled for the east and westbound directions. The long delay for the southbound approach is caused by the lack of available gaps in east-west traffic to allow vehicles from Memory Park Avenue to enter Rinaldi Street. Consideration should be given to signalizing this intersection to allocate adequate timing to accommodate the southbound approach.

Rinaldi Street and Indian Hills Road

Shown below is a comparison of the afternoon peak hour estimated 95th percentile queue lengths for the eastbound through movement under the 4 study scenarios:

<u>Existing</u>	Existing with Project	Change in Queue Length
541.93 ft (or 21.7 veh)	576.72 ft (or 23.1 veh)	34.79 ft (or 1.4 veh)
	·	
Future without Project	Future with Project	Change in Queue Length
744.25 ft (or 29.7 veh)	818.01 ft (or 32.7 veh)	73.76 ft (or 2.95 veh)

The eastbound through movement at this intersection currently has a 95th percentile queue length of 541.93 feet (or approximately 21.7 vehicles) during the afternoon peak hour. With the addition of the Project traffic, the queue length is estimated to extend to 576.72 feet (or approximately 23.1 vehicles) under the Existing with Project condition and 818.01 feet (or approximately 32.7 vehicles) under the Future with Project condition. Even though the queuing for the eastbound through movement is

substantial and is estimated to extend beyond the adjacent intersection of Rinaldi Street and Lev Avenue under future conditions, the Project will only increase the queue length by approximately 3 vehicles.

The City may consider adjusting the signal timing at this intersection to reduce the queuing on Rinaldi Street. In addition, signalizing the Rinaldi Street and Memory Park Avenue could potentially reduce the traffic demand at the Rinaldi Street and Indian Hills Road intersection and improve traffic operations.

<u>Table 10: Levels of Service, Delays & Queue Lengths for Existing & Existing Plus Project Conditions</u>

								AM F	Peak					PM I	Peak												
	Study Intersections Intersection Control		Interception			ı	Existin	g	Existing + Project			E	Existing	9	Existing + Pro		roject										
			Approach	Movement	Delay (sec)	LOS	Queue (ft)	Delay (sec)	LOS	Queue (ft)	Delay (sec)	LOS	Queue (ft)	Delay (sec)	LOS	Queue (ft)											
				SB	Left	-	-	-	-	-	-	137.17	F	19.00	145.74	F	27.88										
				28	Right	17.24	С	1.02	17.90	С	5.08	25.81	D	19.00	22.70	С	27.88										
	1 F	Rinaldi St & Memory	Stop	EB	Left	16.86	С	14.37	17.60	С	19.80	10.94	В	3.21	11.12	В	5.60										
	_	Park Ave	Controlled	EB	Through	0.00	А	0.00	0.00	А	0.00	0.00	А	0.00	0.00	Α	0.00										
				WB	Right	0.00	Α	0.00	0.00	Α	0.00	0.00	Α	0.00	0.00	Α	0.00										
				5	Through	0.00	А	0.00	0.00	А	0.00	0.00	А	0.00	0.00	А	0.00										
				NB	Thru/Left/Right	40.81	D	73.95	43.15	D	76.10	28.32	С	34.44	27.69	С	34.15										
				SB	SB Thru/Left	36.31	D	89.37	36.93	D	100.13	34.41	С	258.91	34.56	С	274.65										
				36	SB Right	40.46	D	170.32	40.39	D	170.13	24.43	С	137.83	23.17	С	133.54										
					Left	17.48	В	107.22	17.51	В	107.41	15.48	В	40.28	16.87	В	42.48										
:	2	Rinaldi St & Indian Hills Rd	Signalized	Signalized	Signalized	Signalized	Signalized	Signalized	Signalized	Signalized	Signalized	Signalized	Signalized	EB	Through	3.88	Α	97.91	3.89	Α	98.23	20.23	С	541.93	22.89	С	576.72
					Right	2.65	А	4.98	2.66	А	5.00	7.39	Α	3.88	8.04	Α	4.10										
					Left	6.02	Α	3.31	6.04	А	3.32	39.72	D	31.06	43.85	D	33.04										
				WB	Through	4.66	Α	153.87	4.68	Α	154.37	10.06	В	181.76	10.94	В	191.72										
					Right	3.48	Α	49.33	3.55	А	52.49	7.73	Α	21.46	8.51	Α	27.94										
				NB	Thru/Left	7.37	Α	3.50	7.39	А	4.23	7.35	А	2.69	7.37	А	3.50										
;	3	Indian Hills Rd & Mission Hills Rd	Stop Controlled	SB	Thru/Right	0.00	А	0.00	0.00	А	0.00	0.00	Α	0.00	0.00	Α	0.00										
				EB	Left/Right	10.05	В	7.72	10.31	В	8.69	10.21	В	17.93	10.56	В	20.49										

<u>Table 11: Levels of Service, Delays & Queue Lengths for Future without & with Project Conditions</u>

						AM Peak				PM Peak													
	Study Intersections Intersection Control		Intersection	on.						Future without Project			Future + Project			Future without Project			Future + Project				
			Approach	Movement	Delay (sec)	LOS	Queue (ft)	Delay (sec)	LOS	Queue (ft)	Delay (sec)	LOS	Queue (ft)	Delay (sec)	LOS	Queue (ft)							
				SB	Left	-	-	-	-	-	-	192.11	F	24.35	203.08	F	38.43						
				28	Right	18.70	С	1.14	19.52	С	5.71	37.47	E	24.35	31.84	D	38.43						
	Rinaldi St &	Memory	Stop Controlled	EB	Left	19.09	С	18.11	20.20	С	25.19	11.50	В	3.71	11.72	В	6.41						
	Park A	ive	Stop Controlled	ЕВ	Through	0.00	А	0.00	0.00	А	0.00	0.00	А	0.00	0.00	Α	0.00						
				WB	Right	0.00	Α	0.00	0.00	Α	0.00	0.00	Α	0.00	0.00	Α	0.00						
				***5	Through	0.00	Α	0.00	0.00	Α	0.00	0.00	Α	0.00	0.00	Α	0.00						
				NB	Thru/Left/Right	41.90	D	80.30	45.49	D	83.84	29.46	С	37.97	29.31	С	38.14						
				SB	SB Thru/Left	35.97	D	95.71	36.67	D	107.70	34.59	С	278.16	34.87	С	294.35						
				36	SB Right	40.11	D	183.76	40.03	D	183.54	23.04	С	144.37	21.86	С	139.84						
				EB	Left	25.08	С	148.69	25.14	С	148.96	18.87	В	49.19	20.43	С	51.66						
	Rinaldi St 8 Hills F		Signalized		Through	4.34	Α	117.81	4.35	Α	118.22	33.96	С	744.25	41.96	D	818.01						
					Right	2.86	Α	5.66	2.87	Α	5.68	8.27	Α	4.56	8.94	Α	4.79						
											Left	6.87	Α	4.00	6.88	Α	4.01	55.26	E	40.04	56.78	E	40.85
						WB	Through	5.33	Α	188.06	5.35	Α	188.71	11.65	В	213.13	12.59	В	222.07				
					Right	3.82	А	57.80	3.90	А	61.57	8.69	А	25.16	9.51	Α	32.58						
		ndian Hills Rd & Mission Hills Rd					NB	Thru/Left	7.38	А	3.76	7.40	А	4.55	7.35	А	2.84	7.38	А	3.71			
	₹ I		Stop Controlled	SB	Thru/Right	0.00	Α	0.00	0.00	А	0.00	0.00	А	0.00	0.00	А	0.00						
				EB	Left/Right	10.18	В	8.43	10.47	В	9.42	10.33	В	19.83	10.73	В	22.38						

Appendix A

VMT Calculator Analysis

CITY OF LOS ANGELES VMT CALCULATOR Version 1.3



Project Screening Criteria: Is this project required to conduct a vehicle miles traveled analysis?

Project: Ararat Home Scenario: Address: 15105 W MISSION HILLS ROAD, 91345

Is the project replacing an existing number of residential units with a smaller number of residential units AND is located within one-half mile of a fixed-rail or fixed-guideway transit

• Yes • No

Existing Land Use



Project Screening Summary

Existing Land Use	Propos	ed
0	978	
Daily Vehicle Trips	Daily Vehicle	e Trips
0 Daily VMT	9,16 Daily VN	
Tier 1 Screen	ning Criteria	
Project will have less reside to existing residential units mile of a fixed-rail station. Tier 2 Screen	& is within one-h	
The net increase in daily tri		978 Net Daily Trips
The net increase in daily VM	M T ≤ 0	9,161 Net Daily VMT
The proposed project consi land uses ≤ 50,000 square for		0.000 ksf
The proposed project in VMT ar		perform



CITY OF LOS ANGELES VMT CALCULATOR Version 1.3



Project Information

Project: Ararat Home

Scenario: 15105 W MISSION HILLS ROAD, 91345



Proposed Project Land Use Type	Value	Unit
Housing Multi-Family	87	DU
(custom) Assisted Living & Nursing Home Daily	566	Trips
(custom) Assisted Living & Nursing Home HBW-	13	Percent
(custom) Assisted Living & Nursing Home HBO-/	70	Percent
(custom) Assisted Living & Nursing Home NHB-A	9	Percent
(custom) Assisted Living & Nursing Home HBW-	0	Percent
(custom) Assisted Living & Nursing Home HBO-F	0	Percent
(custom) Assisted Living & Nursing Home NHB-F	8	Percent
(custom) Assisted Living & Nursing Home Daily	0	Residents
(custom) Assisted Living & Nursing Home Daily	70	Employees
(custom) Assisted Living & Nursing Home Daily	Non-Retail	Retail/Non-Re

TDM Strategies

Select each section to show individual strategies Use **✓** to denote if the TDM strategy is part of the proposed project or is a mitigation strategy **Proposed Project** With Mitigation Max Home Based TDM Achieved? No No Max Work Based TDM Achieved? No No A **Parking** Reduce Parking Supply 100 city code parking provision for the project site actual parking provision for the project site Proposed Prj Mitigation Unbundle Parking monthly parking cost (dollar) for the project Proposed Prj Mitigation Parking Cash-Out 50 percent of employees eligible Proposed Prj Mitigation Price Workplace Parking daily parking charge (dollar) percent of employees subject to priced 50 Proposed Pri Mitigation Residential Area Parking cost (dollar) of annual permit Proposed Prj Mitigation B **Transit C Education & Encouragement** O **Commute Trip Reductions** E **Shared Mobility** F **Bicycle Infrastructure** G **Neighborhood Enhancement**

Analysis Results

Proposed Project	With
978	895
Daily Vehicle Trips	Daily Vehicle Trips
9,161	8,397
Daily VMT	Daily VMT
10.5	9.2
Houseshold VMT per Capita	Houseshold VMT
13.1	13.1
Work VMT per Employee	Work VMT per Employee
Significant \	/MT Impact?
Household: Yes	Household: No
Threshold = 9.2 15% Below APC	Threshold = 9.2 15% Below APC
Work: No	Work: No
Threshold = 15.0 15% Below APC	Threshold = 15.0 15% Below APC



Date: August 17, 2020 Project Name: Ararat Home Project Scenario: Project Address: 15105 W MISSION HILLS ROAD, 91345

Version 1.3

Report 1: Project & Analysis Overview

	Project Informati	on	
Land	l Use Type	Value	Units
	Single Family	0	DU
	Multi Family	87	DU
Housing	Townhouse	0	DU
	Hotel	0	Rooms
	Motel	0	Rooms
	Family	0	DU
Affordable Housing	Senior	0	DU
Affordable Housing	Special Needs	0	DU
	Permanent Supportive	0	DU
	General Retail	0.000	ksf
	Furniture Store	0.000	ksf
Retail	Pharmacy/Drugstore	0.000	ksf
	Supermarket	0.000	ksf
	Bank	0.000	ksf
	Health Club	0.000	ksf
	High-Turnover Sit-Down	0.000	1 6
	Restaurant	0.000	ksf
	Fast-Food Restaurant	0.000	ksf
	Quality Restaurant	0.000	ksf
	Auto Repair	0.000	ksf
	Home Improvement	0.000	ksf
	Free-Standing Discount	0.000	ksf
	Movie Theater	0	Seats
Office	General Office	0.000	ksf
Office	Medical Office	0.000	ksf
	Light Industrial	0.000	ksf
Industrial	Manufacturing	0.000	ksf
	Warehousing/Self-Storage	0.000	ksf
	University	0	Students
	High School	0	Students
School	Middle School	0	Students
	Elementary	0	Students
	Private School (K-12)	0	Students
Other	Assisted Living & Nursing Home	566	Trips

	Analysis Res	sults								
	Total Employees: 70									
	Total Population: 196									
Propos	Proposed Project With Mitigation									
978	Daily Vehicle Trips	895	Daily Vehicle Trips							
9,161	Daily VMT	8,397	Daily VMT							
10.5	Household VMT	0.3	Household VMT per							
10.5	per Capita	9.2	Capita							
13.1	Work VMT	13.1	Work VMT per							
13.1	per Employee	13.1	Employee							
	Significant VMT	Impact?								
	APC: North V	alley								
	Impact Threshold: 15% Bel	ow APC Average								
	Household = 9	9.2								
	Work = 15.0									
Propos	red Project	With M	itigation							
VMT Threshold	Impact	VMT Threshold	Impact							
Household > 9.2	Yes	Household > 9.2	No							
Work > 15.0	No	Work > 15.0	No							



	1	TDM Strategy Inp	uts	
Stra	tegy Type	Description	Proposed Project	Mitigations
	Reduce parking	City code parking provision (spaces)	0	0
	supply	Actual parking provision (spaces)	0	0
	Unbundle parking	Monthly cost for parking (\$)	\$0	\$50
Parking	Parking cash-out	Employees eligible (%)	0%	0%
	Price workplace	Daily parking charge (\$)	\$0.00	\$0.00
	parking	Employees subject to priced parking (%)	0%	0%
	Residential area parking permits	Cost of annual permit (\$)	\$0	\$0

(cont. on following page)

	TDM	Strategy Inputs,	Cont.	
Strate	gy Type	Description	Proposed Project	Mitigations
		Reduction in headways (increase in frequency) (%)	0%	0%
	Reduce transit headways	Existing transit mode share (as a percent of total daily trips) (%)	0%	0%
		Lines within project site improved (<50%, >=50%)	0	0
Transit	Implement neighborhood shuttle	Degree of implementation (low, medium, high)	0	0
		Employees and residents eligible (%)	0%	0%
		Employees and residents eligible (%)	0%	0%
	Transit subsidies	Amount of transit subsidy per passenger (daily equivalent) (\$)	\$0.00	\$0.00
Education &	Voluntary travel behavior change program	Employees and residents participating (%)	0%	100%
Encouragement	Promotions and marketing	Employees and residents participating (%)	0%	0%

(cont. on following page)

		Strategy Inputs,		
Strate	gy Type	Description	Proposed Project	Mitigations
	Required commute trip reduction program	Employees participating (%)	0%	0%
	Alternative Work Schedules and	Employees participating (%)	0%	0%
	Telecommute	Type of program	0	
Commute Trip Reductions	5 maleure version of	Degree of implementation (low, medium, high)	0	0
	Employer sponsored vanpool or shuttle	Employees eligible (%)	0%	0%
		Employer size (small, medium, large)	0	0
	Ride-share program	Employees eligible (%)	0%	0%
	Car share	Car share project setting (Urban, Suburban, All Other)	0	0
Shared Mobility	f Bike share	Within 600 feet of existing bike share station - OR- implementing new bike share station (Yes/No)	0	o
	School carpool program	Level of implementation (Low, Medium, Hiah)	0	0

(cont. on following page)

	TDM	Strategy Inputs,	Cont.		
Strate	gy Type	Description	Proposed Project	Mitigations	
	Implement/Improve on-street bicycle facility	Provide bicycle facility along site (Yes/No)	0	0	
Bicycle	Include Bike parking per LAMC	Meets City Bike Parking Code (Yes/No)	0	Yes	
Infrastructure	Include secure bike parking and showers	Includes indoor bike parking/lockers, showers, & repair station (Yes/No)	0	Yes	
	Traffic calming	Streets with traffic calming improvements (%)	0%	0%	
Neighborhood Enhancement	improvements	Intersections with traffic calming improvements (%)	0%	0%	
Emancement	Pedestrian network improvements	Included (within project and connecting off- site/within project only)	0	within project and connecting off-site	





						Place type	: Suburban	1						
		Ноте Во	ased Work	Home Bo	sed Work		sed Other		sed Other	Non-Home	Based Other	Non-Home	Based Other	
		Prod	luction	Attro	action	Prod	uction	Attr	action	Prod	luction	Attr	action	Source
		Proposed	Mitigated	Proposed	Mitigated	Proposed	Mitigated	Proposed	Mitigated	Proposed	Mitigated	Proposed	Mitigated	
	Reduce parking supply	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	
	Unbundle parking	0%	6%	0%	0%	0%	6%	0%	0%	0%	0%	0%	0%	TDM Strategy
Parking	Parking cash-out	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	Appendix, Parking
_	Price workplace	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	sections 1 - 5
	Residential area parking permits	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	
Transit	Reduce transit headways	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	TDM Strategy
	Implement neighborhood shuttle	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	Appendix, Transit sections 1 - 3
	Transit subsidies	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	
Education &	Voluntary travel behavior change program	0%	4%	0%	4%	0%	4%	0%	4%	0%	4%	0%	4%	TDM Strategy Appendix, Education &
Encouragement	Promotions and marketing	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	Encouragement sections 1 - 2
	Required commute trip reduction program	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	T0145
Commute Trip Reductions	Alternative Work Schedules and Telecommute Program	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	TDM Strategy Appendix, Commute Trip Reductions
	Employer sponsored vanpool or shuttle	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	sections 1 - 4
	Ride-share program	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	
	Car-share	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	TDM Strategy
hared Mobility	Bike share	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	Appendix, Shared Mobility sections 1 - 3
nared Mobility	School carpool program	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	

TDM Adjustments by Trip Purpose & Strategy, Cont.														
Place type: Suburban														
		Home Bo	ased Work	Home Bo	ased Work	Home Bo	sed Other	Home Bo	sed Other	Non-Home	Based Other	Non-Home	Based Other	
		Prod	luction	Attro	action	Prod	uction	Attro	Attraction		luction	Attr	action	Source
		Proposed	Mitigated	Proposed	Mitigated	Proposed	Mitigated	Proposed	Mitigated	Proposed	Mitigated	Proposed	Mitigated	-
Bicycle	Implement/ Improve on-street bicycle facility	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	TDM Strategy
Infrastructure	Include Bike parking per LAMC	0.0%	0.6%	0.0%	0.6%	0.0%	0.6%	0.0%	0.6%	0.0%	0.6%	0.0%	0.6%	Infrastructure sections 1 - 3
	Include secure bike parking and showers	0.0%	0.6%	0.0%	0.6%	0.0%	0.6%	0.0%	0.6%	0.0%	0.6%	0.0%	0.6%	Sections 1 - 3
Neighborhood	Traffic calming improvements	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	TDM Strategy Appendix,
Enhancement	Pedestrian network improvements	0.0%	2.0%	0.0%	2.0%	0.0%	2.0%	0.0%	2.0%	0.0%	2.0%	0.0%	2.0%	Neighborhood Enhancement

Final Combined & Maximum TDM Effect													
	Home Based Work Production		Home Based Work H Attraction			Home Based Other H Production		Home Based Other Attraction		Non-Home Based Other Production		Non-Home Based Other Attraction	
	Proposed	Mitigated	Proposed	Mitigated	Proposed	Mitigated	Proposed	Mitigated	Proposed	Mitigated	Proposed	Mitigated	
COMBINED TOTAL	0%	13%	0%	7%	0%	13%	0%	7%	0%	7%	0%	7%	
MAX. TDM EFFECT	0%	13%	0%	7%	0%	13%	0%	7%	0%	7%	0%	7%	

= Mini	= Minimum (X%, 1-[(1-A)*(1-B)])							
where X%=								
PLACE	urban	75%						
TYPE	compact infill	40%						
MAX:	suburban center	20%						
	suburban	15%						

Note: (1-[(1-A)*(1-B)...]) reflects the dampened combined effectiveness of TDM Strategies (e.g., A, B,...). See the TDM Strategy Appendix (*Transportation Assessment Guidelines Attachment G*) for further discussion of dampening.

CITY OF LOS ANGELES VMT CALCULATOR

Report 4: MXD Methodology

Date: August 17, 2020 Project Name: Ararat Home

Project Scenario:

Project Address: 15105 W MISSION HILLS ROAD, 91345



Version 1.3

MXD Methodology - Project Without TDM											
	Unadjusted Trips	MXD Adjustment	MXD Trips	Average Trip Length	Unadjusted VMT	MXD VMT					
Home Based Work Production	78	-12.8%	68	10.9	850	741					
Home Based Other Production	216	-16.7%	180	7.3	1,577	1,314					
Non-Home Based Other Production	146	-0.7%	145	9.2	1,343	1,334					
Home-Based Work Attraction	74	-9.5%	67	13.7	1,014	918					
Home-Based Other Attraction	499	-11.0%	444	9.0	4,491	3,996					
Non-Home Based Other Attraction	75	-1.3%	74	11.6	870	858					

	MXD Methodology with TDM Measures											
		Proposed Project		Project with Mitigation Measures								
	TDM Adjustment	Project Trips	Project VMT	TDM Adjustment	Mitigated Trips	Mitigated VMT						
Home Based Work Production	0.0%	68	741	-12.7%	59	647						
Home Based Other Production	0.0%	180	1,314	-12.7%	157	1,148						
Non-Home Based Other Production	0.0%	145	1,334	-7.1%	135	1,239						
Home-Based Work Attraction	0.0%	67	918	-7.1%	62	853						
Home-Based Other Attraction	0.0%	444	3,996	-7.1%	413	3,713						
Non-Home Based Other Attraction	0.0%	74	858	-7.1%	69	797						

	MXD VMT Methodology Per Capita & Per E	mployee
	Total Population:	196
	Total Employees:	70
	APC:	North Valley
	Proposed Project	Project with Mitigation Measures
Total Home Based Production VMT	2,055	1,795
Total Home Based Work Attraction VMT	918	853
Total Home Based VMT Per Capita	10.5	9.2
Total Work Based VMT Per Employee	13.1	13.1

Appendix B

Manual Traffic Counts

File Name : MemoryPark_Rinaldi Site Code : 00000000

Start Date : 11/6/2019

Page No : 1

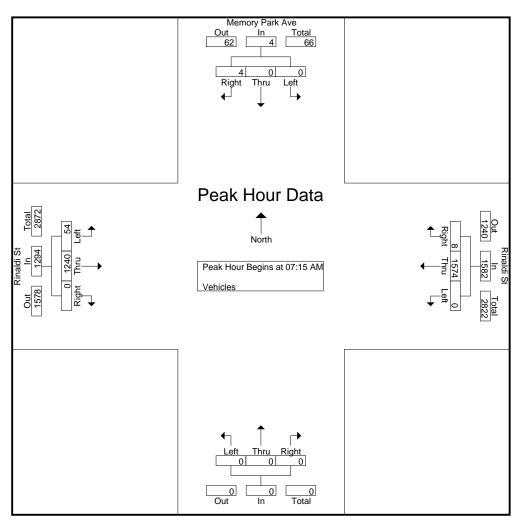
Groups Printed- Vehicles

						Printed- \	Vehicles						
	Memo	ry Park A	ve		inaldi St					R	inaldi St		
	Sou	uthbound		We	estbound		Nor	rthbound		Ea	astbound		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	0	0	0	0	332	1	0	0	0	11	193	0	537
07:15 AM	0	0	2	0	346	4	0	0	0	11	269	0	632
07:30 AM	0	0	1	0	472	0	0	0	0	14	298	0	785
07:45 AM	0	0	0	0	410	2	0	0	0	11	348	0	771
Total	0	0	3	0	1560	7	0	0	0	47	1108	0	2725
08:00 AM	0	0	1	0	346	2	0	0	0	18	325	0	692
08:15 AM	0	0	0	0	225	3	0	0	0	23	347	0	598
08:30 AM	0	0	1	0	253	1	0	0	0	7	227	0	489
08:45 AM	0	0	0	0	177	0	0	0	0	7	226	0	410
Total	0	0	2	0	1001	6	0	0	0	55	1125	0	2189
09:00 AM	1	0	0	0	208	3	0	0	0	2	177	0	391
09:15 AM	0	0	3	0	211	1	0	0	0	3	174	0	392
09:30 AM	1	0	2	0	156	4	0	0	0	3	189	0	355
09:45 AM	0	0	2	0	214	1	0	0	0	1	187	0	405
Total	2	0	7	0	789	9	0	0	0	9	727	0	1543
			. 1			- 1						- 1	
03:00 PM	0	0	4	0	301	3	0	0	0	4	428	0	740
03:15 PM	0	0	0	0	233	2	0	0	0	2	345	0	582
03:30 PM	1	0	1	0	250	2	0	0	0	2	389	0	645
03:45 PM	0	0	1	0	245	3	0	0	0	7	389	0	645
Total	1	0	6	0	1029	10	0	0	0	15	1551	0	2612
04:00 PM	0	0	2	0	248	1	0	0	0	3	383	0	637
04:15 PM	2	0	4	0	247	6	0	0	0	12	363	0	634
04:30 PM	3	0	17	0	252	4	0	0	0	5	412	0	693
04:45 PM	2	0	3	0	233	4	0	0	0	11	445	0	698
Total	7	0	26	0	980	15	0	0	0	31	1603	0	2662
05:00 PM	3	0	1	0	266	3	0	0	0	7	439	0	719
05:15 PM	1	0	4	0	309	6	0	0	0	5	421	0	746
05:30 PM	0	0	2	0	229	3	0	0	0	2	467	4	707
05:45 PM	1	0	5	0	200	8	0	0	0	5	477	0	696
Total	5	0	12	0	1004	20	0	0	0	19	1804	4	2868
Grand Total	15	0	56	0	6363	67	0	0	0	176	7918	4	14599
Apprch %	21.1	0	78.9	0	99	1	0	0	0	2.2	97.8	0	
Total %	0.1	0	0.4	0	43.6	0.5	0	0	0	1.2	54.2	0	

File Name : MemoryPark_Rinaldi Site Code : 00000000

Start Date : 11/6/2019

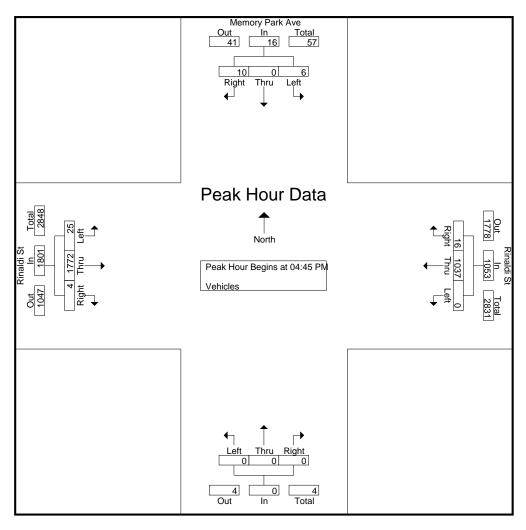
	М	emory	Park A	ve		Rina	ldi St							Rina	aldi St		
		South	bound			West	bound			North	bound			East	bound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	ysis From	า 07:00	AM to 1	1:45 AM -	Peak 1	of 1	_				_				_		
Peak Hour for E	ntire Inte	rsection	Begins	at 07:15	AM												
07:15 AM	0	0	2	2	0	346	4	350	0	0	0	0	11	269	0	280	632
07:30 AM	0	0	1	1	0	472	0	472	0	0	0	0	14	298	0	312	785
07:45 AM	0	0	0	0	0	410	2	412	0	0	0	0	11	348	0	359	771
08:00 AM	0	0	1	1	0	346	2	348	0	0	0	0	18	325	0	343	692
Total Volume	0	0	4	4	0	1574	8	1582	0	0	0	0	54	1240	0	1294	2880
% App. Total	0	0	100		0	99.5	0.5		0	0	0		4.2	95.8	0		
PHF	.000	.000	.500	.500	.000	.834	.500	.838	.000	.000	.000	.000	.750	.891	.000	.901	.917



File Name : MemoryPark_Rinaldi Site Code : 00000000

Site Code : 00000000 Start Date : 11/6/2019

	М	emory	Park A	ve		Rina	ldi St							Rina	aldi St		
		South	bound			West	bound			North	bound			East	bound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	ysis From	12:00	PM to 0	5:45 PM -	Peak 1	of 1											
Peak Hour for E	ntire Inte	rsection	Begins	at 04:45	PM												
04:45 PM	2	0	3	5	0	233	4	237	0	0	0	0	11	445	0	456	698
05:00 PM	3	0	1	4	0	266	3	269	0	0	0	0	7	439	0	446	719
05:15 PM	1	0	4	5	0	309	6	315	0	0	0	0	5	421	0	426	746
05:30 PM	0	0	2	2	0	229	3	232	0	0	0	0	2	467	4	473	707
Total Volume	6	0	10	16	0	1037	16	1053	0	0	0	0	25	1772	4	1801	2870
% App. Total	37.5	0	62.5		0	98.5	1.5		0	0	0		1.4	98.4	0.2		
PHF	.500	.000	.625	.800	.000	.839	.667	.836	.000	.000	.000	.000	.568	.949	.250	.952	.962



File Name : MemoryPark_Rinaldi_BP Site Code : 00000000

Site Code : 00000000 Start Date : 11/6/2019

Page No : 1

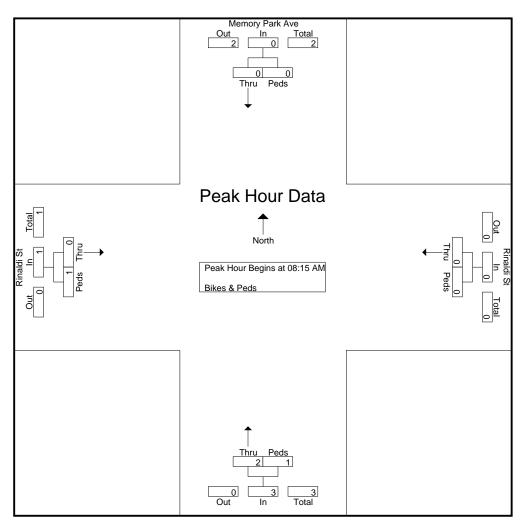
Groups Printed- Bikes & Peds

				rinted- Bikes	& Peds				
	Memory Par	k Ave	Rinaldi S	St			Rinaldi S	St	
	Southbou		Westbou		Northbou		Eastbour		
Start Time	Thru	Peds	Bikes	Peds	Bikes	Peds	Bikes	Peds	Int. Total
07:15 AM	0	0	0	0	1	2	0	0	3
Total	0	0	0	0	1	2	0	0	3
08:15 AM	0	0	0	0	2	1	0	0	3
Total	0	0	0	0	2	1	0	0	3
09:00 AM	0	0	0	0	0	0	0	1	1
Total	0	0	0	0	0	0	0	1	1
03:00 PM 03:15 PM 03:30 PM 03:45 PM Total	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0	1 0 0 0	0 25 3 6	0 0 0 0	0 0 0 0	1 25 3 6 35
04:00 PM 04:15 PM	0 0	0 0	0 0	0	1 0	23	0 0	0	24 1
Total	0	0	0	0	1	24	0	0	25
05:00 PM 05:15 PM	0 0	0 0	0 0	0	1 0	0	0 0	0	1 1
Total	0	0	0	0	1	1	0	0	2
Grand Total Apprch % Total %	0 0 0	0 0 0	0 0 0	0 0 0	6 8.8 8.7	62 91.2 89.9	0 0 0	1 100 1.4	69

File Name : MemoryPark_Rinaldi_BP Site Code : 00000000

Site Code : 00000000 Start Date : 11/6/2019

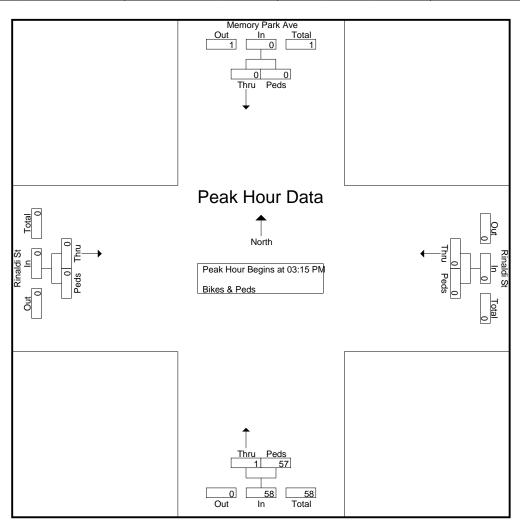
		nory Par			Rinaldi S	St					Rinaldi S	St	
	S	outhbou	ınd	V	Vestbou	nd	N	lorthbou	nd		Eastbour	nd	
Start Time	Thru	Peds	App. Total	Bikes	Peds	App. Total	Bikes	Peds	App. Total	Bikes	Peds	App. Total	Int. Total
Peak Hour Analysis	From 07:0	0 AM to 1	1:45 AM - P	eak 1 of 1									
Peak Hour for Entire	Intersection	on Begins	at 08:15 AN	Λ									
08:15 AM	0	0	0	0	0	0	2	1	3	0	0	0	3
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00 AM	0	0	0	0	0	0	0	0	0	0	1	1	1_
Total Volume	0	0	0	0	0	0	2	1	3	0	1	1	4
% App. Total	0	0		0	0		66.7	33.3		0	100		
PHF	.000	.000	.000	.000	.000	.000	.250	.250	.250	.000	.250	.250	.333



File Name: MemoryPark_Rinaldi_BP

Site Code : 00000000 Start Date : 11/6/2019

		mory Parl Southbou		,	Rinaldi S Westbou		ļ	Northbou	nd		Rinaldi S Eastboun	-	
Start Time	Thru	Peds	App. Total	Bikes	Peds	App. Total	Bikes	Peds	App. Total	Bikes	Peds	App. Total	Int. Total
Peak Hour Analysis	From 12:	:00 PM to	05:45 PM -	Peak 1 of	1								
Peak Hour for Entire	e Intersec	tion Begir	ns at 03:15 F	PM									
03:15 PM	0	Ō	0	0	0	0	0	25	25	0	0	0	25
03:30 PM	0	0	0	0	0	0	0	3	3	0	0	0	3
03:45 PM	0	0	0	0	0	0	0	6	6	0	0	0	6
04:00 PM	0	0	0	0	0	0	1	23	24	0	0	0	24
Total Volume	0	0	0	0	0	0	1	57	58	0	0	0	58
% App. Total	0	0		0	0		1.7	98.3		0	0		
PHF	.000	.000	.000	.000	.000	.000	.250	.570	.580	.000	.000	.000	.580



File Name : IndianHills_Rinaldi_BP Site Code : 00000000

Start Date : 11/6/2019

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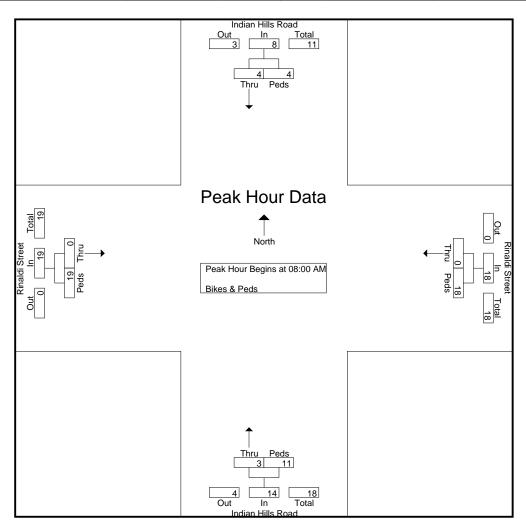
Groups Printed- Rikes & Peds

				<u>rinted- Biķes</u>					
	Indian Hills	Road	Rinaldi St		Indian Hills	Road	Rinaldi St	reet	
	Southbou	ınd	Westbou	nd	Northbou	ınd	Eastbou	nd	
Start Time	Bikes	Peds	Bikes	Peds	Bikes	Peds	Bikes	Peds	Int. Total
07:00 AM	0	5	0	3	0	1	0	0	9
07:15 AM	0	1	0	2	1	2	0	2	8
07:30 AM	0	3	0	1	0	2	0	4	10
07:45 AM	0	3	0	4	0	1	0	2	10
Total	0	12	0	10	1	6	0	8	37
08:00 AM	0	0	0	3	1	5	0	6	15
08:15 AM	4	2	0	6	2	3	0	5	22
08:30 AM	0	0	0	2	0	1	0	2	5
08:45 AM	0	2	0	7	0	2	0	6	17
Total	4	4	0	18	3	11	0	19	59
09:00 AM	0	3	0	3	0	0	1	6	13
09:15 AM	1	0	0	1	0	2	0	0	4
09:30 AM	0	3	0	6	0	3	1	7	20
09:45 AM	0	2	0	4	0	5	0	5	16_
Total	1	8	0	14	0	10	2	18	53
		- I		- 1		- 1		1	
03:00 PM	0	0	0	6	0	6	0	10	22
03:15 PM	0	5	0	3	0	6	0	14	28
03:30 PM	1	5	0	4	1	6	0	8	25
03:45 PM	1	3	0	7	0	5	0	4	20
Total	2	13	0	20	1	23	0	36	95
04:00 PM	0	6	0	15	0	11	0	9	41
04:15 PM	0	3	0	4	0	6	0	4	17
04:30 PM	0	1	0	4	0	8	0	3	16
04:45 PM	3	6	0	6	2	4	0	4	25
Total	3	16	0	29	2	29	0	20	99
05:00 PM	0	2	0	6	2	10	1	1	22
05:15 PM	0	1	0	2	0	4	0	2	9
05:30 PM	0	7	0	2	0	2	0	2	13
05:45 PM	0	2	1	2	1	4	0	1	11
Total	0	12	1	12	3	20	1	6	55
Grand Total	10	65	1	103	10	99	3	107	398
Apprch %	13.3	86.7	1	99	9.2	90.8	2.7	97.3	
Total %	2.5	16.3	0.3	25.9	2.5	24.9	8.0	26.9	

File Name: IndianHills_Rinaldi_BP

Site Code : 00000000 Start Date : 11/6/2019

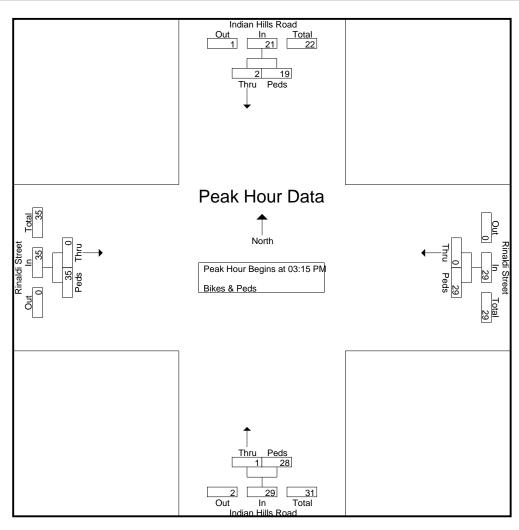
		ian Hills Southbou			inaldi Stı Vestbou			an Hills Iorthbou			inaldi Str Eastbour		
Start Time	Bikes	Peds	App. Total	Bikes	Peds	App. Total	Bikes	Peds	App. Total	Bikes	Peds	App. Total	Int. Total
Peak Hour Analysis	From 07:0	00 AM to 1	1:45 AM - P	eak 1 of 1									
Peak Hour for Entire	Intersecti	on Begins	at 08:00 AN	/									
08:00 AM	0	0	0	0	3	3	1	5	6	0	6	6	15
08:15 AM	4	2	6	0	6	6	2	3	5	0	5	5	22
08:30 AM	0	0	0	0	2	2	0	1	1	0	2	2	5
08:45 AM	0	2	2	0	7	7	0	2	2	0	6	6	17
Total Volume	4	4	8	0	18	18	3	11	14	0	19	19	59
% App. Total	50	50		0	100		21.4	78.6		0	100		
PHF	.250	.500	.333	.000	.643	.643	.375	.550	.583	.000	.792	.792	.670



File Name: IndianHills_Rinaldi_BP

Site Code : 00000000 Start Date : 11/6/2019

		ian Hills I Southbou			inaldi St Vestbou			lian Hills Northbou		-	Rinaldi Str Eastboun		
Start Time	Bikes	Peds	App. Total	Bikes	Peds	App. Total	Bikes	Peds	App. Total	Bikes	Peds	App. Total	Int. Total
Peak Hour Analysis	From 12	:00 PM to	05:45 PM -	Peak 1 of	1								
Peak Hour for Entire	e Intersec	tion Begin	s at 03:15 F	PM									
03:15 PM	0	5	5	0	3	3	0	6	6	0	14	14	28
03:30 PM	1	5	6	0	4	4	1	6	7	0	8	8	25
03:45 PM	1	3	4	0	7	7	0	5	5	0	4	4	20
04:00 PM	0	6	6	0	15	15	0	11	11	0	9	9	41
Total Volume	2	19	21	0	29	29	1	28	29	0	35	35	114
% App. Total	9.5	90.5		0	100		3.4	96.6		0	100		
PHF	.500	.792	.875	.000	.483	.483	.250	.636	.659	.000	.625	.625	.695



File Name: IndianHills_Rinaldi Site Code: 00000000

Start Date: 11/6/2019

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Groups Printed- Vehicles

	Indian	Hills Road		Rin	aldi Street		Indian	Hills Road		Rin	aldi Street		
	Sou	ıthbound		W	estbound		Nor	thbound		Ea	stbound		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
7:00 AM	17	0	23	3	297	31	12	1	9	36	128	1	558
7:15 AM	15	0	27	0	316	35	7	0	6	34	176	3	619
7:30 AM	39	2	67	4	349	39	12	2	6	30	213	6	769
7:45 AM	16	1	37	2	307	88	11	6	7	43	238	7	763
Total	87	3	154	9	1269	193	42	9	28	143	755	17	2709
8:00 AM	17	0	30	3	258	82	5	4	6	43	280	13	741
8:15 AM	10	2	21	3	193	53	11	1	3	54	258	2	611
8:30 AM	26	2	19	6	223	56	2	4	4	60	182	3	587
8:45 AM	23	2	21	4	164	48	4	1	4	52	163	1	487
Total	76	6	91	16	838	239	22	10	17	209	883	19	2426
9:00 AM	19	0	19	1	177	30	8	1	1	38	116	4	414
9:15 AM	19	0	29	8	173	36	6	2	2	39	116	0	430
9:30 AM	19	0	16	5	139	38	5	0	6	47	135	0	410
9:45 AM	23	1	41	3	178	34	11	0	5	55	127	0	478
Total	80	1	105	17	667	138	30	3	14	179	494	4	1732
•													
3:00 PM 3:15 PM 3:30 PM	69 61 54	0 1 1	39 50 41	6 8 4	215 181 179	20 24 19	3 13 4	1 0 0	3 6 5	28 31 24	345 300 354	0 6 4	729 681 689
3:15 PM 3:30 PM 3:45 PM	61 54 46	1 1 0	50 41 48	8 4 4	181 179 149	24 19 21	13 4 10	0 0 0	6 5 6	31 24 26	300 354 367	6 4 2	681 689 679
3:15 PM 3:30 PM	61 54	1 1	50 41	8 4	181 179	24 19	13 4	0 0	6 5	31 24	300 354	6	681 689 679
3:15 PM 3:30 PM 3:45 PM Total	61 54 46 230	1 1 0 2	50 41 48 178	8 4 4 22	181 179 149 724	24 19 21 84	13 4 10 30	0 0 0 1	6 5 6 20	31 24 26 109	300 354 367 1366	6 4 2	681 689 679 2778
3:15 PM 3:30 PM 3:45 PM Total 4:00 PM 4:15 PM	61 54 46 230	1 1 0 2	50 41 48 178 49 40	8 4 4 22 4 6	181 179 149 724 154 188	24 19 21 84 14 7	13 4 10 30 6 6	0 0 0 1	6 5 6 20	31 24 26 109 27 19	300 354 367 1366 386 384	6 4 2 12 4 4	681 689 679 2778 702 698
3:15 PM 3:30 PM 3:45 PM Total 4:00 PM 4:15 PM 4:30 PM	61 54 46 230 44 41 61	1 1 0 2	50 41 48 178 49 40 33	8 4 4 22 4 6 5	181 179 149 724 154 188 227	24 19 21 84 14 7	13 4 10 30 6 6 7	0 0 0 1	6 5 6 20	31 24 26 109 27 19 22	300 354 367 1366 386 384 425	6 4 2 12 4 4 2	681 689 679 2778 702 698 803
3:15 PM 3:30 PM 3:45 PM Total 4:00 PM 4:15 PM	61 54 46 230	1 1 0 2	50 41 48 178 49 40	8 4 4 22 4 6	181 179 149 724 154 188	24 19 21 84 14 7	13 4 10 30 6 6	0 0 0 1	6 5 6 20	31 24 26 109 27 19	300 354 367 1366 386 384	6 4 2 12 4 4	681 689
3:15 PM 3:30 PM 3:45 PM Total 4:00 PM 4:15 PM 4:30 PM 4:45 PM Total	61 54 46 230 44 41 61 65	1 1 0 2 0 0 0 0	50 41 48 178 49 40 33 41	8 4 4 22 4 6 5 6	181 179 149 724 154 188 227 185 754	24 19 21 84 14 7 15 12	13 4 10 30 6 6 7 5	0 0 0 1	6 5 6 20 13 3 5 4	31 24 26 109 27 19 22 11	300 354 367 1366 386 384 425 460	4 4 4 2 3 13	681 689 679 2778 702 698 803 792 2995
3:15 PM 3:30 PM 3:45 PM Total 4:00 PM 4:15 PM 4:30 PM 4:45 PM Total	61 54 46 230 44 41 61 65 211	1 1 0 2 0 0 0 0	50 41 48 178 49 40 33 41 163	8 4 4 22 4 6 5 6 21	181 179 149 724 154 188 227 185 754	24 19 21 84 14 7 15 12 48	13 4 10 30 6 6 6 7 5 24	0 0 0 1 1 1 0 1 0 2	6 5 6 20 13 3 5 5 4 25 7	31 24 26 109 27 19 22 11 79	300 354 367 1366 386 384 425 460 1655	4 4 4 2 3 13	681 689 679 2778 702 698 803 792 2995
3:15 PM 3:30 PM 3:45 PM Total 4:00 PM 4:15 PM 4:30 PM 4:45 PM Total 5:00 PM 5:15 PM	61 54 46 230 44 41 61 65 211	1 1 0 2 0 0 0 0	50 41 48 178 49 40 33 41 163	8 4 4 22 4 6 5 6 21	181 179 149 724 154 188 227 185 754	24 19 21 84 14 7 15 12 48	13 4 10 30 6 6 7 5 24	0 0 0 1 1 1 0 1 0 2	13 3 5 4 25 7 3	31 24 26 109 27 19 22 11 79	300 354 367 1366 386 384 425 460 1655	4 4 4 2 3 13	681 689 679 2778 702 698 803 792 2995
3:15 PM 3:30 PM 3:45 PM Total 4:00 PM 4:15 PM 4:30 PM 4:45 PM Total 5:00 PM 5:15 PM 5:30 PM	61 54 46 230 44 41 61 65 211	1 1 0 2 0 0 0 0 0	50 41 48 178 49 40 33 41 163	8 4 4 22 4 6 5 6 21	181 179 149 724 154 188 227 185 754	24 19 21 84 14 7 15 12 48	13 4 10 30 6 6 6 7 5 24	0 0 0 1 1 1 0 1 0 2	6 5 6 20 13 3 5 5 4 25 7	31 24 26 109 27 19 22 11 79	300 354 367 1366 386 384 425 460 1655	4 4 4 2 3 13	681 689 679 2778 702 698 803 792 2995
3:15 PM 3:30 PM 3:45 PM Total 4:00 PM 4:15 PM 4:30 PM 4:45 PM Total 5:00 PM 5:15 PM 5:30 PM 5:45 PM	61 54 46 230 44 41 61 65 211 89 63 42 30	1 1 0 2 0 0 0 0 0	50 41 48 178 49 40 33 41 163	8 4 4 22 4 6 5 6 21	181 179 149 724 154 188 227 185 754	24 19 21 84 14 7 15 12 48	13 4 10 30 6 6 7 5 24	0 0 0 1 1 1 0 1 0 2	13 3 5 4 25 7 3 3 4	31 24 26 109 27 19 22 111 79	300 354 367 1366 386 384 425 460 1655	6 4 2 12 4 4 2 3 13	681 685 675 2776 702 696 803 792 2996 836 811 786
3:15 PM 3:30 PM 3:45 PM Total 4:00 PM 4:15 PM 4:30 PM 4:45 PM Total 5:00 PM 5:15 PM 5:30 PM	61 54 46 230 44 41 61 65 211	1 1 0 2 0 0 0 0 0	50 41 48 178 49 40 33 41 163	8 4 4 22 4 6 5 6 21	181 179 149 724 154 188 227 185 754	24 19 21 84 14 7 15 12 48	13 4 10 30 6 6 6 7 5 24	0 0 0 1 1 1 0 1 0 2	13 3 5 4 25 7 3	31 24 26 109 27 19 22 11 79	300 354 367 1366 386 384 425 460 1655	4 4 4 2 3 13	681 685 675 2776 702 696 803 792 2996 836 811 786
3:15 PM 3:30 PM 3:45 PM Total 4:00 PM 4:15 PM 4:30 PM 4:45 PM Total 5:00 PM 5:15 PM 5:30 PM 5:45 PM	61 54 46 230 44 41 61 65 211 89 63 42 30	1 1 0 2 0 0 0 0 0	50 41 48 178 49 40 33 41 163	8 4 4 22 4 6 5 6 21	181 179 149 724 154 188 227 185 754	24 19 21 84 14 7 15 12 48	13 4 10 30 6 6 7 5 24	0 0 0 1 1 1 0 1 0 2	13 3 5 4 25 7 3 3 4	31 24 26 109 27 19 22 111 79	300 354 367 1366 386 384 425 460 1655	6 4 2 12 4 4 2 3 13	681 689 679 2778 702 698 803 792 2995 838 810 786 735 3169
3:15 PM 3:30 PM 3:45 PM Total 4:00 PM 4:15 PM 4:30 PM 4:45 PM Total 5:00 PM 5:15 PM 5:30 PM 5:35 PM 5:45 PM	61 54 46 230 44 41 61 65 211 89 63 42 30 224	1 1 0 2 0 0 0 0 0	50 41 48 178 49 40 33 41 163 51 55 27 21	8 4 4 22 4 6 5 6 21 10 5 5 5 25	181 179 149 724 154 188 227 185 754 184 203 216 189 792	24 19 21 84 14 7 15 12 48	13 4 10 30 6 6 6 7 5 24	0 0 0 1 1 1 0 1 0 2	13 3 5 4 25 7 3 3 3 4 4 17	31 24 26 109 27 19 22 11 79 17 15 19 21 72	300 354 367 1366 386 384 425 460 1655 449 447 447 447	4 4 4 2 3 13	681 689 679 2778 702 698 803 792

File Name: IndianHills_Rinaldi Site Code: 00000000

Start Date: 11/6/2019 Page No: 2

		Indian Hi	lls Road			Rinaldi S	Street			Indian Hil	ls Road			Rinald	Street		
		South	ound			Westbo	und			Northb	ound			Eastb	ound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
	nalysis From 0																
Peak Hour fo	r Entire Interse	ction Begin	s at 07:15 AM														
7:15 AM	15	0	27	42	0	316	35	351	7	0	6	13	34	176	3	213	619
7:30 AM	39	2	67	108	4	349	39	392	12	2	6	20	30	213	6	249	769
7:45 AM	16	1	37	54	2	307	88	397	11	6	7	24	43	238	7	288	763
8:00 AM	17	0	30	47	3	258	82	343	5	4	6	15	43	280	13	336	741
Total Volume	87	3	161	251	9	1230	244	1483	35	12	25	72	150	907	29	1086	2892
% App. Total		1.2	64.1		0.6	82.9	16.5		48.6	16.7	34.7		13.8	83.5	2.7		
PHF	0.558	0.375	0.601	0.581	0.563	0.881	0.693	0.934	0.729	0.5	0.893	0.75	0.872	0.81	0.558	0.808	0.94

File Name: IndianHills_Rinaldi Site Code: 00000000 Start Date: 11/6/2019

		Indian Hills	Indian Hills Road Rinaldi Street					Indian Hills Road									
	Southbound					Westbound				Northbound				Eastbound			
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
	nalysis From 1			(1 of 1													
Peak Hour fo	r Entire Interse	ction Begins	at 04:30 PM														
4:30 PM	61	0	33	94	5	227	15	247	7	1	5	13	22	425	2	449	803
4:45 PM	65	0	41	106	6	185	12	203	5	0	4	9	11	460	3	474	792
5:00 PM	89	0	51	140	10	184	22	216	6	0	7	13	17	449	3	469	838
5:15 PM	63	1	55	119	5	203	8	216	7	0	3	10	15	447	3	465	810
Total Volume		1	180	459	26	799	57	882	25	1	19	45	65	1781	11	1857	3243
% App. Total	60.6	0.2	39.2		2.9	90.6	6.5		55.6	2.2	42.2		3.5	95.9	0.6		
PHF	0.781	0.25	0.818	0.82	0.65	0.88	0.648	0.893	0.893	0.25	0.679	0.865	0.739	0.968	0.917	0.979	0.967

File Name : IndianHills_MissionHills Site Code : 00000000

Start Date : 11/6/2019

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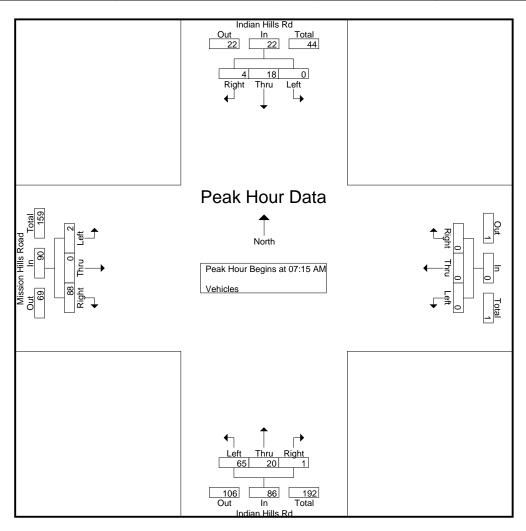
Groups Printed- Vehicles

	Groups Printed- Venicles Indian Hills Rd												
	Indian Hills Rd											oad	
		uthbound			stbound			rthbound			stbound		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	0	3	1	0	0	0	19	3	0	2	0	19	47
07:15 AM	0	5	1	0	0	0	22	3	1	0	0	18	50
07:30 AM	0	3	0	0	0	0	10	2	0	0	0	31	46
07:45 AM	0	3	2	0	0	0	15	9	0	1	0	24	54
Total	0	14	4	0	0	0	66	17	1	3	0	92	197
08:00 AM	0	7	1	0	0	0	18	6	0	1	0	15	48
08:15 AM	0	2	0	0	0	0	17	4	0	0	0	14	37
08:30 AM	0	3	1	0	0	0	23	2	0	0	0	18	47
08:45 AM	0	2	0	0	0	0	12	6	0	1_	0	7	28
Total	0	14	2	0	0	0	70	18	0	2	0	54	160
09:00 AM	0	5	0	0	0	0	3	1	0	0	0	5	14
09:15 AM	0	1	0	0	0	0	11	5	0	0	0	5	22
09:30 AM	0	1	0	0	0	0	5	3	0	0	0	9	18
09:45 AM	0	9	0	0	0	0	12	4	9	0	0	11	45
Total	0	16	0	0	0	0	31	13	9	0	0	30	99
03:00 PM	0	4	0	0	0	0	14	0	0	0	0	55	73
03:15 PM	0	3	0	0	0	0	15	2	0	0	0	42	62
03:30 PM	0	5	1	0	0	0	9	2	0	0	0	34	51
03:45 PM	0	5	1	0	0	0	5	3	0	1	0	29	44
Total	0	17	2	0	0	0	43	7	0	1	0	160	230
04:00 PM	0	6	0	0	0	0	3	3	0	0	0	23	35
04:15 PM	0	5	0	0	0	0	3	3	0	0	0	23	34
04:30 PM	0	3	0	0	0	0	6	6	0	1	0	27	43
04:45 PM	0	9	0	0	0	0	2	2	0	0	1	20	34
Total	0	23	0	0	0	0	14	14	0	1	1	93	146
05:00 PM	0	4	0	0	0	0	5	2	0	0	0	28	39
05:15 PM	0	2	0	0	0	0	3	2	0	0	0	32	39
05:30 PM	0	4	0	0	0	0	7	4	0	0	0	22	37
05:45 PM	0	3	0	0	0	0	3	4	0	0	0	11	21_
Total	0	13	0	0	0	0	18	12	0	0	0	93	136
Grand Total	0	97	8	0	0	0	242	81	10	7	1	522	968
Apprch %	0	92.4	7.6	0	0	0	72.7	24.3	3	1.3	0.2	98.5	
Total %	0	10	0.8	0	0	0	25	8.4	1	0.7	0.1	53.9	

File Name: IndianHills_MissionHills

Site Code : 00000000 Start Date : 11/6/2019

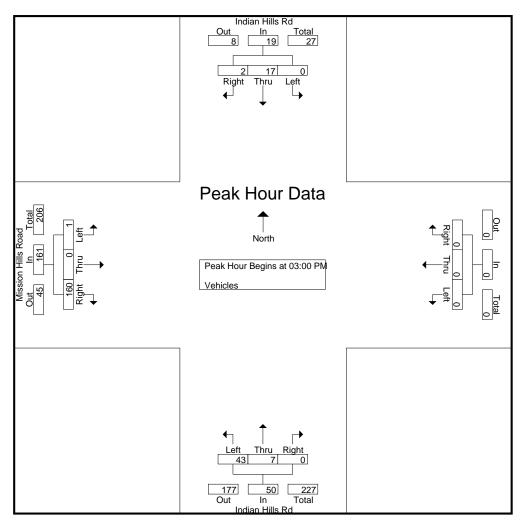
	Indian Hills Rd Southbound				Westbound				Indian Hills Rd Northbound				Mission Hills Road Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Anal	ysis Fron	n 07:00	AM to 1	1:45 AM -	Peak 1	of 1									_		
Peak Hour for E	ntire Inte	rsection	Begins	at 07:15	AM												
07:15 AM	0	5	1	6	0	0	0	0	22	3	1	26	0	0	18	18	50
07:30 AM	0	3	0	3	0	0	0	0	10	2	0	12	0	0	31	31	46
07:45 AM	0	3	2	5	0	0	0	0	15	9	0	24	1	0	24	25	54
08:00 AM	0	7	1	8	0	0	0	0	18	6	0	24	1	0	15	16	48
Total Volume	0	18	4	22	0	0	0	0	65	20	1	86	2	0	88	90	198
_ % App. Total	0	81.8	18.2		0	0	0		75.6	23.3	1.2		2.2	0	97.8		
PHF	.000	.643	.500	.688	.000	.000	.000	.000	.739	.556	.250	.827	.500	.000	.710	.726	.917



File Name: IndianHills_MissionHills

Site Code : 00000000 Start Date : 11/6/2019

	Indian Hills Rd Southbound			Weethound				Indian Hills Rd				М					
			Westbound				Northbound				Eastbound						
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 12:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for E	ntire Inte	rsection	Begins	at 03:00	PM												
03:00 PM	0	4	0	4	0	0	0	0	14	0	0	14	0	0	55	55	73
03:15 PM	0	3	0	3	0	0	0	0	15	2	0	17	0	0	42	42	62
03:30 PM	0	5	1	6	0	0	0	0	9	2	0	11	0	0	34	34	51
03:45 PM	0	5	1	6	0	0	0	0	5	3	0	8	1	0	29	30	44
Total Volume	0	17	2	19	0	0	0	0	43	7	0	50	1	0	160	161	230
% App. Total	0	89.5	10.5		0	0	0		86	14	0		0.6	0	99.4		
PHF	.000	.850	.500	.792	.000	.000	.000	.000	.717	.583	.000	.735	.250	.000	.727	.732	.788



File Name : IndianHills_MissionHills_BP Site Code : 00000000

Start Date : 11/6/2019

Page No : 1

Groups Printed- Rikes & Peds

Groups Printed- Bikes & Peds Indian Hills Rd Indian Hills Rd Mission Hills Road													
	Indian Hills	Rd			Indian Hills	s Rd	Mission Hills	Road					
	Southbound		Westbou	nd	Northbou	nd	Eastbour	nd					
Start Time	Bikes	Peds	Bikes	Peds	Bikes	Peds	Bikes	Peds	Int. Total				
07:00 AM	0	0	0	4	0	0	0	0	4				
07:15 AM	0	5	0	7	0	0	0	0	12				
07:30 AM	0	2	0	5	0	0	0	0	7				
07:45 AM	0	2	0	11	0	0	0	0	13_				
Total	0	9	0	27	0	0	0	0	36				
	-	- '	-			- ,							
08:15 AM	0	2 3	0	2 6	0	0	0	0	4				
08:30 AM	0	3	0	6	0	0	0	0	9				
08:45 AM	0	3	0	2	0	0	0	0	9 5				
Total	0	8	0	10	0	0	0	0	18				
		- 1		- 1		- 1		- 1					
09:00 AM	0	3	0	2	0	0	0	0	5 5 2				
09:15 AM	0	1	0	4	0	0	0	0	5				
09:30 AM	0	0	0	2	0	0	0	0	2				
09:45 AM	0	2	0	7	0	0	0	0	9				
Total	0	6	0	15	0	0	0	0	21				
03:00 PM	0	٥١	0	2	0	0	0	0	2				
		0	0	3	-				3 6				
03:15 PM	0	2	0	4	0	0	0	0					
03:30 PM	0	8	0	3	0	0	0	0	11				
03:45 PM	0	7 17	0	13	0	0	0	0	10 30				
Total	U	17	U	13	U	0	U	0	30				
04:00 PM	0	1	0	0	0	0	0	0	1				
04:15 PM	0	Ö	0	1	0	0	0	0	1				
04:30 PM	0	2	0	3	0	0	0	0	5				
04:45 PM	0	4	0	3	0	0	0	0	5 7				
Total	0	7	0	7	0	0	0	0	14				
i otai	O	, ,	O	, ,	O	0	U	0	14				
05:15 PM	0	0	0	1	0	0	0	0	1				
05:30 PM	Ö	1	1	1	Ö	0	Ö	o	3				
00.00 T III	Ü	• 1	•	• •	ŭ	0	Ü	0	· ·				
Total	0	1	1	2	0	0	0	0	4				
		1		1		1		1					
Grand Total	0	48	1	74	0	0	0	0	123				
Apprch %	0	100	1.3	98.7	0	0	0	0					
Total %	0	39	0.8	60.2	0	0	0	0					

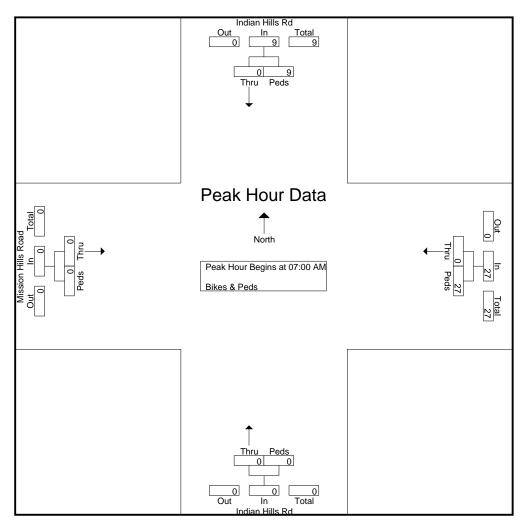
CITY TRAFFIC COUNTERS WWW.CTCOUNTERS.COM

File Name: IndianHills_MissionHills_BP

Site Code : 00000000 Start Date : 11/6/2019

Page No : 2

		dian Hills Southbou		,	Westbound			lian Hills orthbou		Mis			
Start Time	Bikes	Peds	App. Total	Bikes	Peds	App. Total	Bikes	Peds	App. Total	Bikes	Peds	App. Total	Int. Total
Peak Hour Analysis	From 07:0	0 AM to 1	1:45 AM - P	eak 1 of 1									
Peak Hour for Entire	Intersection	on Begins	at 07:00 AN	/									
07:00 AM	0	0	0	0	4	4	0	0	0	0	0	0	4
07:15 AM	0	5	5	0	7	7	0	0	0	0	0	0	12
07:30 AM	0	2	2	0	5	5	0	0	0	0	0	0	7
07:45 AM	0	2	2	0	11	11	0	0	0	0	0	0	13
Total Volume	0	9	9	0	27	27	0	0	0	0	0	0	36
% App. Total	0	100		0	100		0	0		0	0		
PHF	.000	.450	.450	.000	.614	.614	.000	.000	.000	.000	.000	.000	.692



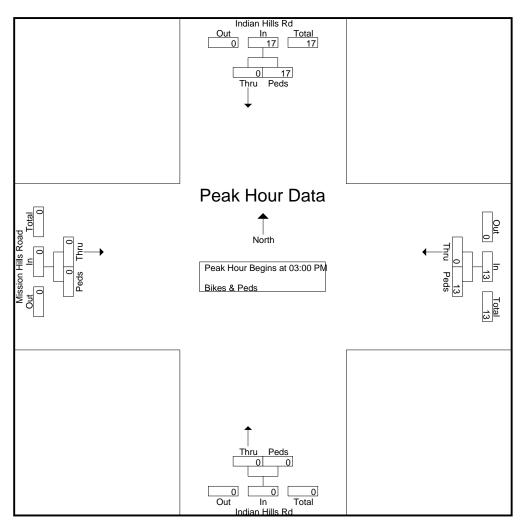
CITY TRAFFIC COUNTERS WWW.CTCOUNTERS.COM

File Name: IndianHills_MissionHills_BP

Site Code : 00000000 Start Date : 11/6/2019

Page No : 3

		dian Hills Southbou		Westbound			Indian Hills Rd Northbound			Mis			
Start Time	Bikes	Peds	App. Total	Bikes	Peds	App. Total	Bikes	Peds	App. Total	Bikes	Peds	App. Total	Int. Total
Peak Hour Analysis	From 12:	:00 PM to	05:45 PM -	Peak 1 of	1								
Peak Hour for Entire	e Intersec	tion Begin	s at 03:00 F	PM									
03:00 PM	0	Ō	0	0	3	3	0	0	0	0	0	0	3
03:15 PM	0	2	2	0	4	4	0	0	0	0	0	0	6
03:30 PM	0	8	8	0	3	3	0	0	0	0	0	0	11
03:45 PM	0	7	7	0	3	3	0	0	0	0	0	0	10
Total Volume	0	17	17	0	13	13	0	0	0	0	0	0	30
% App. Total	0	100		0	100		0	0		0	0		
PHF	.000	.531	.531	.000	.813	.813	.000	.000	.000	.000	.000	.000	.682



Appendix C

HCM Analysis

Intersection Level Of Service Report Intersection 1: Rinaldi & Indian Hills

Control Type:SignalizedDelay (sec / veh):8.8Analysis Method:HCM 6th EditionLevel Of Service:AAnalysis Period:15 minutesVolume to Capacity (v/c):0.489

Intersection Setup

Name	ı	ndian Hills	3	I	ndian Hills	S		Rinaldi			Rinaldi		
Approach	١	orthboun	d	S	Southbound			Eastbound	d t	V	Westbound		
Lane Configuration		+			46			1 r		ıllı			
Turning Movement	Left	eft Thru Right L			Thru	Right	Left	Thru	Right	Left	Thru	Right	
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	
No. of Lanes in Entry Pocket	0	0 0 0			0	1	1	0	1	1	0	1	
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	1	0	0	0	
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	49.21	0.00	0.00	0.00	
Speed [mph]		30.00			30.00			30.00			30.00		
Grade [%]		0.00			0.00			0.00			0.00		
Curb Present		No			No			No		No			
Crosswalk		Yes			Yes			Yes			Yes		

Name	I	ndian Hills	3	I	ndian Hills	3		Rinaldi			Rinaldi	
Base Volume Input [veh/h]	35	12	25	87	3	161	150	907	29	9	1230	244
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	35	12	25	87	3	161	150	907	29	9	1230	244
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	9	3	7	23	1	43	40	241	8	2	327	65
Total Analysis Volume [veh/h]	37	13	27	93	3	171	160	965	31	10	1309	260
Presence of On-Street Parking	No		No	No		No	No		No	No		No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing)	11			4			19			18	
v_di, Inbound Pedestrian Volume crossing r	n	19			18			11			4	
v_co, Outbound Pedestrian Volume crossing	3	0			0			0				
v_ci, Inbound Pedestrian Volume crossing n	ni	0			0			0				
v_ab, Corner Pedestrian Volume [ped/h]		0			0			0				
Bicycle Volume [bicycles/h]		0			0			0			0	

Version 2020 (SP 0-0)

Intersection Settings

Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permiss											
Signal Group	0	8	0	0	4	0	0	6	0	0	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	-	-	-	-	-	_
Minimum Green [s]	0	5	0	0	8	0	0	10	0	0	10	0
Maximum Green [s]	0	30	0	0	30	0	0	30	0	0	30	0
Amber [s]	0.0	3.0	0.0	0.0	3.6	0.0	0.0	4.4	0.0	0.0	4.4	0.0
All red [s]	0.0	1.0	0.0	0.0	2.0	0.0	0.0	0.5	0.0	0.0	0.5	0.0
Split [s]	0	40	0	0	40	0	0	50	0	0	50	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
Walk [s]	0	5	0	0	7	0	0	7	0	0	7	0
Pedestrian Clearance [s]	0	10	0	0	19	0	0	8	0	0	8	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No			No			Yes			Yes	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
l2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
Minimum Recall		No			No			Yes			Yes	
Maximum Recall		No			No			No			No	
Pedestrian Recall		No			No			Yes			Yes	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Exclusive Pedestrian Phase

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0

Lane Group Calculations

Lane Group	С	С	R	L	С	R	L	С	R
C, Cycle Length [s]	90	90	90	90	90	90	90	90	90
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
I1_p, Permitted Start-Up Lost Time [s]	2.00	2.00	0.00	2.00	0.00	0.00	2.00	0.00	0.00
l2, Clearance Lost Time [s]	0.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	13	13	13	68	68	68	68	68	68
g / C, Green / Cycle	0.15	0.15	0.15	0.76	0.76	0.76	0.76	0.76	0.76
(v / s)_i Volume / Saturation Flow Rate	0.08	0.08	0.11	0.38	0.27	0.02	0.02	0.37	0.17
s, saturation flow rate [veh/h]	1002	1220	1589	419	3560	1558	580	3560	1567
c, Capacity [veh/h]	126	261	238	327	2711	1186	451	2711	1193
d1, Uniform Delay [s]	36.15	35.45	36.40	12.31	3.51	2.61	5.93	4.05	3.06
k, delay calibration	0.11	0.11	0.11	0.50	0.50	0.50	0.50	0.50	0.50
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	4.67	0.86	4.06	5.16	0.37	0.04	0.09	0.62	0.42
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

·									
X, volume / capacity	0.61	0.37	0.72	0.49	0.36	0.03	0.02	0.48	0.22
d, Delay for Lane Group [s/veh]	40.81	36.31	40.46	17.48	3.88	2.65	6.02	4.66	3.48
Lane Group LOS	D	D	D	В	Α	Α	Α	А	Α
Critical Lane Group	No	No	Yes	Yes	No	No	No	No	No
50th-Percentile Queue Length [veh/ln]	1.64	1.99	3.78	2.38	2.18	0.11	0.07	3.42	1.10
50th-Percentile Queue Length [ft/In]	41.08	49.65	94.62	59.57	54.39	2.77	1.84	85.48	27.40
95th-Percentile Queue Length [veh/ln]	2.96	3.57	6.81	4.29	3.92	0.20	0.13	6.15	1.97
95th-Percentile Queue Length [ft/In]	73.95	89.37	170.32	107.22	97.91	4.98	3.31	153.87	49.33

Version 2020 (SP 0-0) AM Existing

Movement, Approach, & Intersection Results

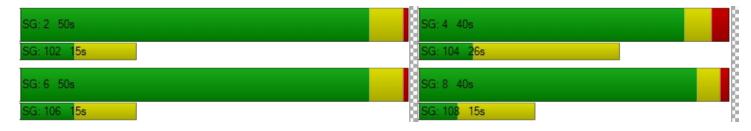
d_M, Delay for Movement [s/veh]	40.81	40.81	40.81	36.31	36.31	40.46	17.48	3.88	2.65	6.02	4.66	3.48
Movement LOS	D D D D D B A		D D D B A A			D D D			Α	Α	Α	Α
d_A, Approach Delay [s/veh]		40.81			38.97			5.73				
Approach LOS		D			D			Α			Α	
d_I, Intersection Delay [s/veh]					8.85							
Intersection LOS						F	4					
Intersection V/C	0.489											

Other Modes

-				
g_Walk,mi, Effective Walk Time [s]	26.0	26.0	11.0	9.0
M_corner, Corner Circulation Area [ft²/ped]	0.00	0.00	0.00	0.00
M_CW, Crosswalk Circulation Area [ft²/ped	0.00	0.00	0.00	0.00
d_p, Pedestrian Delay [s]	22.76	22.76	34.67	36.45
I_p,int, Pedestrian LOS Score for Intersection	n 1.771	2.378	3.071	3.020
Crosswalk LOS	Α	В	С	С
s_b, Saturation Flow Rate of the bicycle lane	2000	2000	2000	2000
c_b, Capacity of the bicycle lane [bicycles/h] 800	764	1002	1002
d_b, Bicycle Delay [s]	16.20	17.17	11.20	11.20
I_b,int, Bicycle LOS Score for Intersection	1.687	2.000	2.513	2.862
Bicycle LOS	А	В	В	С

Sequence

Ring 1	2	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	6	8	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	_
Ring 4	_	-	-	-	_	_	_	_	_	_	_	_	_	_	_	_



17.2

С

Intersection Level Of Service Report Intersection 2: Rinaldi & Memory Park

Control Type: Two-way stop Delay (sec / veh): Analysis Method: HCM 6th Edition Level Of Service: Analysis Period: 15 minutes Volume to Capacity (v/c): 0.013

Intersection Setup

Name										Rinaldi			
Approach	Northbound			s	outhboun	d	E	Eastbound	d	Westbound			
Lane Configuration	+					пII		11-					
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	
No. of Lanes in Entry Pocket	0	0	0	0	0	0	1	0	0	0	0	0	
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0	
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Speed [mph]		30.00	-		30.00	-		30.00	-		30.00		
Grade [%]		0.00			0.00			0.00			0.00		
Crosswalk		Yes			Yes		Yes			Yes			

Name											Rinaldi	
Base Volume Input [veh/h]	0	0	0	0	0	4	54	1240	0	0	1574	8
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	0	0	0	0	0	4	54	1240	0	0	1574	8
Peak Hour Factor	1.0000	1.0000	1.0000	0.9170	0.9170	0.9170	0.9170	0.9170	1.0000	1.0000	0.9170	0.9170
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	0	0	0	0	1	15	338	0	0	429	2
Total Analysis Volume [veh/h]	0	0	0	0	0	4	59	1352	0	0	1716	9
Pedestrian Volume [ped/h]		0			0			0			0	

Version 2020 (SP 0-0) AM Existing

Intersection Settings

Priority Scheme	Stop	Stop	Free	Free
Flared Lane		No		
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance		No		
Number of Storage Spaces in Median	0	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.00	0.00	0.01	0.16	0.01	0.00	0.00	0.02	0.00
d_M, Delay for Movement [s/veh]	0.00	0.00	0.00	308.17	436.08	17.24	16.86	0.00	0.00	0.00	0.00	0.00
Movement LOS				F	F	С	С	Α			Α	Α
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.00	0.04	0.04	0.04	0.57	0.00	0.00	0.00	0.00	0.00
95th-Percentile Queue Length [ft/ln]	0.00	0.00	0.00	1.02	1.02	1.02	14.37	0.00	0.00	0.00	0.00	0.00
d_A, Approach Delay [s/veh]		0.00			17.24			0.70			0.00	
Approach LOS		А			С			Α			Α	
d_I, Intersection Delay [s/veh]		0.34										
Intersection LOS		С										

Intersection Level Of Service Report Intersection 3: Mission Hills & Indian Hills

Control Type:Two-way stopDelay (sec / veh):10.0Analysis Method:HCM 6th EditionLevel Of Service:BAnalysis Period:15 minutesVolume to Capacity (v/c):0.003

Intersection Setup

Name	India	n Hills	India	n Hills	Mission Hills		
Approach	North	bound	South	bound	Eastbound		
Lane Configuration	•	1	Τ'				
Turning Movement	Left	Thru	Thru	Right	Left	Right	
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	
No. of Lanes in Entry Pocket	0 0		0	0	0	0	
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	
No. of Lanes in Exit Pocket	0	0	0	0	0	0	
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	
Speed [mph]	30	0.00	30	0.00	30.00		
Grade [%]	0.	.00	0.	.00	0.00		
Crosswalk	Y	es es	Y	es	Yes		

Name	Indiar	n Hills	Indiar	n Hills	Missio	n Hills
Base Volume Input [veh/h]	65	20	18	4	2	88
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	65	20	18	4	2	88
Peak Hour Factor	0.9170	0.9170	0.9170	0.9170	0.9170	0.9170
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	18	5	5	1	1	24
Total Analysis Volume [veh/h]	71	22	20	4	2	96
Pedestrian Volume [ped/h]	()	()	0	

Version 2020 (SP 0-0)

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			No
Number of Storage Spaces in Median	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.04	0.00	0.00	0.00	0.00	0.09			
d_M, Delay for Movement [s/veh]	7.37	0.00	0.00	0.00	10.05	8.77			
Movement LOS	Α	A	А	А	В	A			
95th-Percentile Queue Length [veh/ln]	0.14	0.14	0.00	0.00	0.31	0.31			
95th-Percentile Queue Length [ft/ln]	3.50	3.50	0.00	0.00	7.72	7.72			
d_A, Approach Delay [s/veh]	5.	63	0	.00	8.	79			
Approach LOS	,	A		A	A	4			
d_I, Intersection Delay [s/veh]		6.44							
Intersection LOS				В					

Intersection Level Of Service Report Intersection 1: Rinaldi & Indian Hills

Control Type:SignalizedDelay (sec / veh):9.0Analysis Method:HCM 6th EditionLevel Of Service:AAnalysis Period:15 minutesVolume to Capacity (v/c):0.489

Intersection Setup

Name	I	ndian Hills	3	ı	ndian Hills	3		Rinaldi		Rinaldi		
Approach	١	Northbound			outhboun	d	E	Eastbound	d	Westbound		
Lane Configuration		+			4		•	1 ۲		•		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	1	1	0	1	1	0	1
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	1	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	49.21	0.00	0.00	0.00
Speed [mph]		30.00			30.00			30.00			30.00	
Grade [%]		0.00			0.00			0.00			0.00	
Curb Present		No			No		No			No		
Crosswalk		Yes			Yes			Yes		Yes		

Name	ı	ndian Hills	3	I	ndian Hills	3		Rinaldi			Rinaldi	
Base Volume Input [veh/h]	35	12	25	87	3	161	150	907	29	9	1230	244
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	10	0	0	0	0	0	0	0	13
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	35	12	25	97	3	161	150	907	29	9	1230	257
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	9	3	7	26	1	43	40	241	8	2	327	68
Total Analysis Volume [veh/h]	37	13	27	103	3	171	160	965	31	10	1309	273
Presence of On-Street Parking	No		No	No		No	No		No	No		No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing	9	11			4			19			18	
v_di, Inbound Pedestrian Volume crossing r	n	19			18			11			4	
v_co, Outbound Pedestrian Volume crossing	9	0			0			0			0	
v_ci, Inbound Pedestrian Volume crossing n	ni	0			0			0			0	
v_ab, Corner Pedestrian Volume [ped/h]		0			0			0			0	
Bicycle Volume [bicycles/h]		0			0			0			0	

Intersection Settings

Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permiss											
Signal Group	0	8	0	0	4	0	0	6	0	0	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	-	-	-	-	-	-
Minimum Green [s]	0	5	0	0	8	0	0	10	0	0	10	0
Maximum Green [s]	0	30	0	0	30	0	0	30	0	0	30	0
Amber [s]	0.0	3.0	0.0	0.0	3.6	0.0	0.0	4.4	0.0	0.0	4.4	0.0
All red [s]	0.0	1.0	0.0	0.0	2.0	0.0	0.0	0.5	0.0	0.0	0.5	0.0
Split [s]	0	40	0	0	40	0	0	50	0	0	50	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
Walk [s]	0	5	0	0	7	0	0	7	0	0	7	0
Pedestrian Clearance [s]	0	10	0	0	19	0	0	8	0	0	8	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No			No			Yes			Yes	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
Minimum Recall		No			No			Yes			Yes	
Maximum Recall		No			No			No			No	
Pedestrian Recall		No			No			Yes			Yes	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Exclusive Pedestrian Phase

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0



Lane Group Calculations

Lane Group	С	С	R	L	С	R	L	С	R
C, Cycle Length [s]	90	90	90	90	90	90	90	90	90
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
I1_p, Permitted Start-Up Lost Time [s]	2.00	2.00	0.00	2.00	0.00	0.00	2.00	0.00	0.00
I2, Clearance Lost Time [s]	0.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	14	14	14	68	68	68	68	68	68
g / C, Green / Cycle	0.15	0.15	0.15	0.76	0.76	0.76	0.76	0.76	0.76
(v / s)_i Volume / Saturation Flow Rate	0.08	0.09	0.11	0.38	0.27	0.02	0.02	0.37	0.17
s, saturation flow rate [veh/h]	906	1186	1589	419	3560	1558	580	3560	1567
c, Capacity [veh/h]	114	257	238	327	2710	1186	451	2710	1192
d1, Uniform Delay [s]	36.46	35.87	36.37	12.34	3.52	2.62	5.95	4.06	3.10
k, delay calibration	0.11	0.11	0.11	0.50	0.50	0.50	0.50	0.50	0.50
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	6.69	1.06	4.02	5.17	0.37	0.04	0.09	0.62	0.45
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.67	0.41	0.72	0.49	0.36	0.03	0.02	0.48	0.23
d, Delay for Lane Group [s/veh]	43.15	36.93	40.39	17.51	3.89	2.66	6.04	4.68	3.55
Lane Group LOS	D	D	D	В	Α	Α	Α	А	Α
Critical Lane Group	No	No	Yes	Yes	No	No	No	No	No
50th-Percentile Queue Length [veh/ln]	1.69	2.23	3.78	2.39	2.18	0.11	0.07	3.43	1.17
50th-Percentile Queue Length [ft/In]	42.28	55.63	94.52	59.67	54.57	2.78	1.84	85.76	29.16
95th-Percentile Queue Length [veh/ln]	3.04	4.01	6.81	4.30	3.93	0.20	0.13	6.17	2.10
95th-Percentile Queue Length [ft/ln]	76.10	100.13	170.13	107.41	98.23	5.00	3.32	154.37	52.49

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Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	43.15	43.15	43.15	36.93	36.93	40.39	17.51	3.89	2.66	6.04	4.68	3.55
Movement LOS	D	D	D	D	D	D	В	Α	Α	Α	Α	Α
d_A, Approach Delay [s/veh]	43.15			39.07				5.74		4.49		
Approach LOS	D			D				Α			Α	
d_I, Intersection Delay [s/veh]						9.	00					
Intersection LOS						F	4					
Intersection V/C		0.489										

Other Modes

g_Walk,mi, Effective Walk Time [s]	26.0	26.0	11.0	9.0
M_corner, Corner Circulation Area [ft²/ped]	0.00	0.00	0.00	0.00
M_CW, Crosswalk Circulation Area [ft²/ped	0.00	0.00	0.00	0.00
d_p, Pedestrian Delay [s]	22.76	22.76	34.67	36.45
I_p,int, Pedestrian LOS Score for Intersection	n 1.771	2.385	3.071	3.038
Crosswalk LOS	Α	В	С	С
s_b, Saturation Flow Rate of the bicycle lane	2000	2000	2000	2000
c_b, Capacity of the bicycle lane [bicycles/h	800	764	1002	1002
d_b, Bicycle Delay [s]	16.20	17.17	11.20	11.20
I_b,int, Bicycle LOS Score for Intersection	1.687	2.017	2.513	2.873
Bicycle LOS	Α	В	В	С

Sequence

_		_														
Ring 1	2	4	-	1	-	-	-	-	-	-	-	1	1	-	-	-
Ring 2	6	8	-	-	-	-	-	-	-	-	-	-	1	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	_





Intersection Level Of Service Report Intersection 2: Rinaldi & Memory Park

Control Type: Two-way stop
Analysis Method: HCM 6th Edition
Analysis Period: 15 minutes

Delay (sec / veh): 17.9
Level Of Service: C
Volume to Capacity (v/c): 0.064

Intersection Setup

Name											Rinaldi		
Approach	١	lorthboun	d	s	Southbound			Eastbound			Westbound		
Lane Configuration					+			пII		11-			
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	
No. of Lanes in Entry Pocket	0	0	0	0	0	0	1	0	0	0	0	0	
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0	
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Speed [mph]	30.00				30.00	-		30.00	-	30.00			
Grade [%]	0.00			0.00				0.00		0.00			
Crosswalk	Yes			Yes				Yes		Yes			

Name											Rinaldi	
Base Volume Input [veh/h]	0	0	0	0	0	4	54	1240	0	0	1574	8
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	13	17	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	0	0	0	0	0	17	71	1240	0	0	1574	8
Peak Hour Factor	1.0000	1.0000	1.0000	0.9170	0.9170	0.9170	0.9170	0.9170	1.0000	1.0000	0.9170	0.9170
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	0	0	0	0	5	19	338	0	0	429	2
Total Analysis Volume [veh/h]	0	0	0	0	0	19	77	1352	0	0	1716	9
Pedestrian Volume [ped/h]	0			0				0		0		

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Intersection Settings

Priority Scheme	Stop	Stop	Free	Free
Flared Lane		No		
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance		No		
Number of Storage Spaces in Median	0	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.00	0.00	0.06	0.21	0.01	0.00	0.00	0.02	0.00
d_M, Delay for Movement [s/veh]	0.00	0.00	0.00	349.39	490.01	17.90	17.60	0.00	0.00	0.00	0.00	0.00
Movement LOS				F	F	С	С	Α			Α	Α
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.00	0.20	0.20	0.20	0.79	0.00	0.00	0.00	0.00	0.00
95th-Percentile Queue Length [ft/ln]	0.00	0.00	0.00	5.08	5.08	5.08	19.80	0.00	0.00	0.00	0.00	0.00
d_A, Approach Delay [s/veh]		0.00		17.90				0.95			0.00	
Approach LOS		А			С			Α			Α	
d_I, Intersection Delay [s/veh]		0.53										
Intersection LOS	С											

Intersection Level Of Service Report Intersection 3: Mission Hills & Indian Hills

Control Type:Two-way stopDelay (sec / veh):10.3Analysis Method:HCM 6th EditionLevel Of Service:BAnalysis Period:15 minutesVolume to Capacity (v/c):0.003

Intersection Setup

Name	India	n Hills	India	n Hills	Missi	on Hills	
Approach	North	bound	South	bound	Eastbound		
Lane Configuration	+	1	ŀ	→	т -		
Turning Movement	Left	Thru	Thru	Right	Left	Right	
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	
No. of Lanes in Entry Pocket	0	0	0	0	0	0	
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	
No. of Lanes in Exit Pocket	0	0	0	0	0	0	
Exit Pocket Length [ft]	0.00	0.00 0.00 0.00		0.00	0.00		
Speed [mph]	30	.00	30	.00	30	0.00	
Grade [%]	0.	00	0.	00	0.00		
Crosswalk	Y	es	Y	es	Yes		

Name	Indiar	n Hills	India	n Hills	Missio	n Hills
Base Volume Input [veh/h]	65	20	18	4	2	88
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	13	0	0	0	0	10
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	78	20	18	4	2	98
Peak Hour Factor	0.9170	0.9170	0.9170	0.9170	0.9170	0.9170
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	21	5	5	1	1	27
Total Analysis Volume [veh/h]	85	22	20	4	2	107
Pedestrian Volume [ped/h]	()	()	()

<u>version 2020 (er 0 0)</u>

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			No
Number of Storage Spaces in Median	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.05	0.00	0.00	0.00	0.00	0.10				
d_M, Delay for Movement [s/veh]	7.39	0.00	0.00	0.00	10.31	8.81				
Movement LOS	Α	Α	Α	A	В	А				
95th-Percentile Queue Length [veh/ln]	0.17	0.17	0.00	0.00	0.35	0.35				
95th-Percentile Queue Length [ft/In]	4.23	4.23	0.00	0.00	8.69	8.69				
d_A, Approach Delay [s/veh]	5.3	87	0.	00	8.8	34				
Approach LOS	A A				Α					
d_I, Intersection Delay [s/veh]		6.63								
Intersection LOS	В									

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Intersection Level Of Service Report Intersection 1: Rinaldi & Indian Hills

Control Type: Analysis Method: Signalized HCM 6th Edition Delay (sec / veh): Level Of Service: 9.7 Α Analysis Period: 0.572 15 minutes Volume to Capacity (v/c):

Intersection Setup

Name	lı	ndian Hill	s	lı	ndian Hill	s		Rinaldi			Rinaldi	
Approach	Northbound			S	Southbound		Eastbound			Westbound		
Lane Configuration	+			46			+	1 r	•	חוור		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	1	1	0	1	1	0	1
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	1	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	49.21	0.00	0.00	0.00
Speed [mph]		30.00			30.00			30.00			30.00	
Grade [%]	0.00				0.00			0.00		0.00		
Curb Present	No				No		No			No		
Crosswalk	Yes			Yes				Yes			Yes	



Name	lı	ndian Hill	s	lı	ndian Hill	s		Rinaldi			Rinaldi	
Base Volume Input [veh/h]	35	12	25	87	3	161	150	907	29	9	1230	244
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800
In-Process Volume [veh/h]	0	0	0	0	0	0	0	6	0	0	6	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	38	13	27	94	3	174	162	986	31	10	1334	264
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	10	3	7	25	1	46	43	262	8	3	355	70
Total Analysis Volume [veh/h]	40	14	29	100	3	185	172	1049	33	11	1419	281
Presence of On-Street Parking	No		No	No		No	No		No	No		No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing major stre	е	11			4			19			18	
v_di, Inbound Pedestrian Volume crossing major street	[19			18			11			4	
v_co, Outbound Pedestrian Volume crossing minor stre	е	0			0			0		0		
v_ci, Inbound Pedestrian Volume crossing minor street	t [0			0		0			0			
v_ab, Corner Pedestrian Volume [ped/h]	0			0			0		0			
Bicycle Volume [bicycles/h]		0			0			0			0	

Intersection Settings

Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permis	Permis	Permis	Permis	Permis	Permis	Permis	Permis	Permis	Permis	Permis	Permis
Signal Group	0	8	0	0	4	0	0	6	0	0	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	-	-	-	-	-	-
Minimum Green [s]	0	5	0	0	8	0	0	10	0	0	10	0
Maximum Green [s]	0	30	0	0	30	0	0	30	0	0	30	0
Amber [s]	0.0	3.0	0.0	0.0	3.6	0.0	0.0	4.4	0.0	0.0	4.4	0.0
All red [s]	0.0	1.0	0.0	0.0	2.0	0.0	0.0	0.5	0.0	0.0	0.5	0.0
Split [s]	0	40	0	0	40	0	0	50	0	0	50	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
Walk [s]	0	5	0	0	7	0	0	7	0	0	7	0
Pedestrian Clearance [s]	0	10	0	0	19	0	0	8	0	0	8	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No			No			Yes			Yes	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
l2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
Minimum Recall		No			No			Yes			Yes	
Maximum Recall		No			No			No			No	
Pedestrian Recall		No			No			Yes			Yes	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	n Filtering Factor 1.00 1.00 1.00 1.00 1.00 1.00		1.00	1.00	1.00	1.00	1.00	1.00				

Exclusive Pedestrian Phase

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0

Lane Group Calculations

Lane Group	С	С	R	L	С	R	L	С	R
C, Cycle Length [s]	90	90	90	90	90	90	90	90	90
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
I1_p, Permitted Start-Up Lost Time [s]	2.00	2.00	0.00	2.00	0.00	0.00	2.00	0.00	0.00
I2, Clearance Lost Time [s]	0.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	14	14	14	68	68	68	68	68	68
g / C, Green / Cycle	0.16	0.16	0.16	0.75	0.75	0.75	0.75	0.75	0.75
(v / s)_i Volume / Saturation Flow Rate	0.09	0.09	0.12	0.46	0.29	0.02	0.02	0.40	0.18
s, saturation flow rate [veh/h]	958	1187	1589	378	3560	1558	536	3560	1566
c, Capacity [veh/h]	125	267	252	289	2679	1172	411	2679	1178
d1, Uniform Delay [s]	36.05	35.06	36.00	16.39	3.90	2.81	6.75	4.58	3.35
k, delay calibration	0.11	0.11	0.11	0.50	0.50	0.50	0.50	0.50	0.50
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	5.85	0.91	4.12	8.69	0.43	0.04	0.12	0.75	0.48
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.66	0.39	0.73	0.59	0.39	0.03	0.03	0.53	0.24
d, Delay for Lane Group [s/veh]	41.90	35.97	40.11	25.08	4.34	2.86	6.87	5.33	3.82
Lane Group LOS	D	D	D	С	Α	Α	Α	Α	Α
Critical Lane Group	No	No	Yes	Yes	No	No	No	No	No
50th-Percentile Queue Length [veh/ln]	1.78	2.13	4.08	3.30	2.62	0.13	0.09	4.18	1.28
50th-Percentile Queue Length [ft/ln]	44.61	53.17	102.09	82.61	65.45	3.15	2.22	104.48	32.11
95th-Percentile Queue Length [veh/ln]	3.21	3.83	7.35	5.95	4.71	0.23	0.16	7.52	2.31
95th-Percentile Queue Length [ft/ln]	80.30	95.71	183.76	148.69	117.81	5.66	4.00	188.06	57.80

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Movement, Approach, & Intersection Results

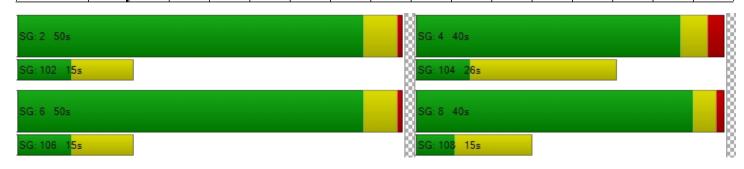
d_M, Delay for Movement [s/veh]	41.90	41.90	41.90	35.97	35.97	40.11	25.08	4.34	2.86	6.87	5.33	3.82
Movement LOS	D	D	D	D	D	D	С	Α	Α	Α	Α	Α
d_A, Approach Delay [s/veh]		41.90			38.63			7.14			5.09	
Approach LOS		D			D			Α				
d_I, Intersection Delay [s/veh]						9.	68					
Intersection LOS	A											
Intersection V/C	0.572											

Other Modes

g_Walk,mi, Effective Walk Time [s]	26.0	26.0	11.0	9.0
M_corner, Corner Circulation Area [ft²/ped]	0.00	0.00	0.00	0.00
M_CW, Crosswalk Circulation Area [ft²/ped]	0.00	0.00	0.00	0.00
d_p, Pedestrian Delay [s]	22.76	22.76	34.67	36.45
I_p,int, Pedestrian LOS Score for Intersection	1.776	2.413	3.110	3.066
Crosswalk LOS	A	В	С	С
s_b, Saturation Flow Rate of the bicycle lane [bicycles/l	1] 2000	2000	2000	2000
c_b, Capacity of the bicycle lane [bicycles/h]	800	764	1002	1002
d_b, Bicycle Delay [s]	16.20	17.17	11.20	11.20
I_b,int, Bicycle LOS Score for Intersection	1.697	2.035	2.594	2.971
Bicycle LOS	A	В	В	С

Sequence

Ring 1	2	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	6	8	-	-	-	-	-	-	-	-	-	-	-	1	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	_	-	-	-



Intersection Level Of Service Report Intersection 2: Rinaldi & Memory Park

Control Type: Two-way stop Analysis Method: HCM 6th Edition Analysis Period: 15 minutes

Delay (sec / veh): 19.1 Level Of Service: С 0.198 Volume to Capacity (v/c):

Intersection Setup

Name										Rinaldi		
Approach	Northbound			S	outhbour	nd	Е	astboun	d	Westbound		d
Lane Configuration	+				пII		11-					
Turning Movement	Left	Left Thru Right Left Thru Right L			Left	Thru	Right	Left	Thru	Right		
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0	1	0	0	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]		30.00			30.00			30.00		30.00		
Grade [%]	0.00			0.00				0.00		0.00		
Crosswalk	Yes				Yes		Yes			Yes		

Name											Rinaldi	
Base Volume Input [veh/h]	0	0	0	0	0	4	54	1240	0	0	1574	8
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800
In-Process Volume [veh/h]	0	0	0	0	0	0	0	6	0	0	6	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	0	0	0	0	0	4	58	1345	0	0	1706	9
Peak Hour Factor	1.0000	1.0000	1.0000	0.9170	0.9170	0.9170	0.9170	0.9170	1.0000	1.0000	0.9170	0.9170
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	0	0	0	0	1	16	367	0	0	465	2
Total Analysis Volume [veh/h]	0	0	0	0	0	4	63	1467	0	0	1860	10
Pedestrian Volume [ped/h]	0				0		0			0		

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Intersection Settings

Priority Scheme	Stop	Stop	Free	Free
Flared Lane		No		
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance		No		
Number of Storage Spaces in Median	0	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.00	0.00	0.01	0.20	0.01	0.00	0.00	0.02	0.00
d_M, Delay for Movement [s/veh]	0.00	0.00	0.00	466.36	685.14	18.70	19.09	0.00	0.00	0.00	0.00	0.00
Movement LOS				F	F	С	С	Α			Α	Α
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.00	0.05	0.05	0.05	0.72	0.00	0.00	0.00	0.00	0.00
95th-Percentile Queue Length [ft/ln]	0.00	0.00	0.00	1.14	1.14	1.14	18.11	0.00	0.00	0.00	0.00	0.00
d_A, Approach Delay [s/veh]		0.00			18.70			0.79				
Approach LOS		Α			С		A A					
d_I, Intersection Delay [s/veh]	0.38											
Intersection LOS	С											



Intersection Level Of Service Report Intersection 3: Mission Hills & Indian Hills

Control Type: Two-way stop
Analysis Method: HCM 6th Edition
Analysis Period: 15 minutes

Delay (sec / veh): 10.2
Level Of Service: B
Volume to Capacity (v/c): 0.003

Intersection Setup

Name	India	n Hills	India	n Hills	Missio	n Hills	
Approach	North	bound	South	bound	Eastbound		
Lane Configuration	•	1	ŀ	+	Ψ.		
Turning Movement	Left	Thru	Thru	Right	Left	Right	
Lane Width [ft]	12.00 12.00		12.00	12.00	12.00	12.00	
No. of Lanes in Entry Pocket	0 0		0	0	0	0	
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	
No. of Lanes in Exit Pocket	0	0	0	0	0	0	
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	
Speed [mph]	30	.00	30	.00	30.00		
Grade [%]	0.	.00	0.	00	0.00		
Crosswalk	Y	es	Y	es	Yes		

Name	Indiar	n Hills	Indiar	n Hills	Missio	n Hills
Base Volume Input [veh/h]	65 20		18	4	2	88
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	70	22	19	4	2	95
Peak Hour Factor	0.9170	0.9170	0.9170	0.9170	0.9170	0.9170
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	19	6	5	1	1	26
Total Analysis Volume [veh/h]	76	24	21	4	2	104
Pedestrian Volume [ped/h]	0 0			0		



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Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			No
Number of Storage Spaces in Median	0	0	0

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.05	0.00	0.00	0.00	0.00	0.10			
d_M, Delay for Movement [s/veh]	7.38	0.00	0.00	0.00	10.18	8.80			
Movement LOS	Α	Α	Α	A	В	Α			
95th-Percentile Queue Length [veh/ln]	0.15	0.15	0.00	0.00	0.34	0.34			
95th-Percentile Queue Length [ft/ln]	3.76	3.76	0.00	0.00	8.43	8.43			
d_A, Approach Delay [s/veh]	5.0	61	0.	00	8.83				
Approach LOS	A	4	,	4	P	\			
d_I, Intersection Delay [s/veh]	6.48								
Intersection LOS	В								

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AM Future + Project

Intersection Level Of Service Report Intersection 1: Rinaldi & Indian Hills

Control Type:SignalizedDelay (sec / veh):9.9Analysis Method:HCM 6th EditionLevel Of Service:AAnalysis Period:15 minutesVolume to Capacity (v/c):0.572

Intersection Setup

Name	lı	Indian Hills			Indian Hills			Rinaldi			Rinaldi		
Approach	Northbound			S	outhbour	uthbound			d	Westbound			
Lane Configuration	+				4r			1116	•	ıllı			
Turning Movement	Left Thru Right			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	
No. of Lanes in Entry Pocket	0	0	0	0	0	1	1	0	1	1	0	1	
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	1	0	0	0	
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	49.21	0.00	0.00	0.00	
Speed [mph]		30.00			30.00			30.00			30.00		
Grade [%]		0.00			0.00			0.00			0.00		
Curb Present	No				No		No			No			
Crosswalk		Yes			Yes		Yes			Yes			

Name	lı	ndian Hill	s	lı	ndian Hill	s		Rinaldi		Rinaldi		
Base Volume Input [veh/h]	35	12	25	87	3	161	150	907	29	9	1230	244
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800
In-Process Volume [veh/h]	0	0	0	0	0	0	0	6	0	0	6	0
Site-Generated Trips [veh/h]	0	0	0	10	0	0	0	0	0	0	0	13
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	38	13	27	104	3	174	162	986	31	10	1334	277
Peak Hour Factor	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400	0.9400
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	10	3	7	28	1	46	43	262	8	3	355	74
Total Analysis Volume [veh/h]	40	14	29	111	3	185	172	1049	33	11	1419	295
Presence of On-Street Parking	No		No	No		No	No		No	No		No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing major stre	е	11			4			19			18	
v_di, Inbound Pedestrian Volume crossing major street	[19			18			11			4	
v_co, Outbound Pedestrian Volume crossing minor stre	е	0			0			0			0	
v_ci, Inbound Pedestrian Volume crossing minor street	t [0			0			0			0		
v_ab, Corner Pedestrian Volume [ped/h]	0				0			0				
Bicycle Volume [bicycles/h]		0			0			0			0	

Intersection Settings

Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permis											
Signal Group	0	8	0	0	4	0	0	6	0	0	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	-	-	-	-	-	-
Minimum Green [s]	0	5	0	0	8	0	0	10	0	0	10	0
Maximum Green [s]	0	30	0	0	30	0	0	30	0	0	30	0
Amber [s]	0.0	3.0	0.0	0.0	3.6	0.0	0.0	4.4	0.0	0.0	4.4	0.0
All red [s]	0.0	1.0	0.0	0.0	2.0	0.0	0.0	0.5	0.0	0.0	0.5	0.0
Split [s]	0	40	0	0	40	0	0	50	0	0	50	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
Walk [s]	0	5	0	0	7	0	0	7	0	0	7	0
Pedestrian Clearance [s]	0	10	0	0	19	0	0	8	0	0	8	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No			No			Yes			Yes	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
l2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
Minimum Recall		No			No			Yes			Yes	
Maximum Recall		No			No			No			No	
Pedestrian Recall		No			No			Yes			Yes	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Exclusive Pedestrian Phase

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0

Lane Group Calculations

Lane Group	С	С	R	L	С	R	L	С	R
C, Cycle Length [s]	90	90	90	90	90	90	90	90	90
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
I1_p, Permitted Start-Up Lost Time [s]	2.00	2.00	0.00	2.00	0.00	0.00	2.00	0.00	0.00
I2, Clearance Lost Time [s]	0.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	14	14	14	68	68	68	68	68	68
g / C, Green / Cycle	0.16	0.16	0.16	0.75	0.75	0.75	0.75	0.75	0.75
(v / s)_i Volume / Saturation Flow Rate	0.10	0.10	0.12	0.46	0.29	0.02	0.02	0.40	0.19
s, saturation flow rate [veh/h]	853	1151	1589	378	3560	1558	536	3560	1566
c, Capacity [veh/h]	112	262	253	289	2677	1171	410	2677	1178
d1, Uniform Delay [s]	36.42	35.53	35.96	16.43	3.92	2.82	6.76	4.59	3.39
k, delay calibration	0.11	0.11	0.11	0.50	0.50	0.50	0.50	0.50	0.50
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	9.08	1.14	4.07	8.71	0.43	0.04	0.12	0.76	0.51
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.74	0.44	0.73	0.59	0.39	0.03	0.03	0.53	0.25
d, Delay for Lane Group [s/veh]	45.49	36.67	40.03	25.14	4.35	2.87	6.88	5.35	3.90
Lane Group LOS	D	D	D	С	Α	Α	Α	Α	Α
Critical Lane Group	No	No	Yes	Yes	No	No	No	No	No
50th-Percentile Queue Length [veh/ln]	1.86	2.39	4.08	3.31	2.63	0.13	0.09	4.19	1.37
50th-Percentile Queue Length [ft/ln]	46.58	59.83	101.97	82.76	65.68	3.16	2.23	104.84	34.21
95th-Percentile Queue Length [veh/ln]	3.35	4.31	7.34	5.96	4.73	0.23	0.16	7.55	2.46
95th-Percentile Queue Length [ft/ln]	83.84	107.70	183.54	148.96	118.22	5.68	4.01	188.71	61.57

Version 2020 (SP 0-5) AM Future + Project

Movement, Approach, & Intersection Results

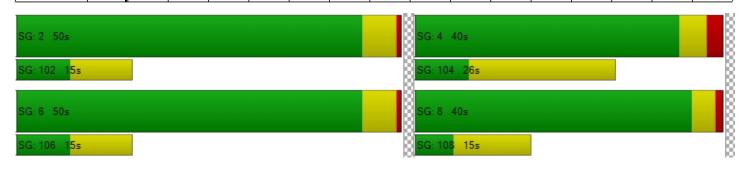
d_M, Delay for Movement [s/veh]	45.49	45.49	45.49	36.67	36.67	40.03	25.14	4.35	2.87	6.88	5.35	3.90
Movement LOS	D	D	D	D	D	D	С	Α	Α	Α	Α	Α
d_A, Approach Delay [s/veh]	45.49				38.75 7.			7.16		5.11		
Approach LOS	D			D			A			A		
d_I, Intersection Delay [s/veh]						9.8	87					
Intersection LOS	A											
Intersection V/C	0.572											

Other Modes

g_Walk,mi, Effective Walk Time [s]	26.0	26.0	11.0	9.0
M_corner, Corner Circulation Area [ft²/ped]	0.00	0.00	0.00	0.00
M_CW, Crosswalk Circulation Area [ft²/ped]	0.00	0.00	0.00	0.00
d_p, Pedestrian Delay [s]	22.76	22.76	34.67	36.45
I_p,int, Pedestrian LOS Score for Intersection	1.776	2.421	3.110	3.086
Crosswalk LOS	A	В	С	С
s_b, Saturation Flow Rate of the bicycle lane [bicycles/l	1] 2000	2000	2000	2000
c_b, Capacity of the bicycle lane [bicycles/h]	800	764	1002	1002
d_b, Bicycle Delay [s]	16.20	17.17	11.20	11.20
I_b,int, Bicycle LOS Score for Intersection	1.697	2.053	2.594	2.983
Bicycle LOS	A	В	В	С

Sequence

-		_														
Ring 1	2	4	-	-	-	-	-	-	-	-	-	-	ı	•	-	-
Ring 2	6	8	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Version 2020 (SP 0-5)

AM Future + Project

Intersection Level Of Service Report Intersection 2: Rinaldi & Memory Park

Control Type: Two-way stop
Analysis Method: HCM 6th Edition
Analysis Period: 15 minutes

Delay (sec / veh): 20.2
Level Of Service: C
Volume to Capacity (v/c): 0.258

Intersection Setup

Name											Rinaldi		
Approach	Northbound			Southbound			Eastbound			Westbound			
Lane Configuration	+						пII		11-				
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	
No. of Lanes in Entry Pocket	0	0	0	0	0	0	1	0	0	0	0	0	
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0	
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Speed [mph]		30.00			30.00			30.00		30.00			
Grade [%]	0.00			0.00			0.00			0.00			
Crosswalk	Yes		Yes				Yes		Yes				

Name											Rinaldi	
Base Volume Input [veh/h]	0	0	0	0	0	4	54	1240	0	0	1574	8
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800
In-Process Volume [veh/h]	0	0	0	0	0	0	0	6	0	0	6	0
Site-Generated Trips [veh/h]	0	0	0	0	0	13	17	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	0	0	0	0	0	17	75	1345	0	0	1706	9
Peak Hour Factor	1.0000	1.0000	1.0000	0.9170	0.9170	0.9170	0.9170	0.9170	1.0000	1.0000	0.9170	0.9170
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	0	0	0	0	5	20	367	0	0	465	2
Total Analysis Volume [veh/h]	0	0	0	0	0	19	82	1467	0	0	1860	10
Pedestrian Volume [ped/h]		0		0 0					0			

Priority Scheme	Stop	Stop	Free	Free
Flared Lane		No		
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance		No		
Number of Storage Spaces in Median	0	0	0	0

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.00	0.00	0.07	0.26	0.01	0.00	0.00	0.02	0.00
d_M, Delay for Movement [s/veh]	0.00	0.00	0.00	539.66	785.41	19.52	20.20	0.00	0.00	0.00	0.00	0.00
Movement LOS				F	F	С	С	Α			Α	А
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.00	0.23	0.23	0.23	1.01	0.00	0.00	0.00	0.00	0.00
95th-Percentile Queue Length [ft/ln]	0.00	0.00	0.00	5.71	5.71	5.71	25.19	0.00	0.00	0.00	0.00	0.00
d_A, Approach Delay [s/veh]		0.00		19.52				1.07			0.00	
Approach LOS		Α			С		A A					
d_I, Intersection Delay [s/veh]	0.59											
Intersection LOS		С										

Intersection Level Of Service Report Intersection 3: Mission Hills & Indian Hills

Control Type:Two-way stopDelay (sec / veh):10.5Analysis Method:HCM 6th EditionLevel Of Service:BAnalysis Period:15 minutesVolume to Capacity (v/c):0.003

Intersection Setup

Name	India	n Hills	India	n Hills	Missio	n Hills	
Approach	North	bound	South	bound	East	oound	
Lane Configuration	+	1	ŀ	•	Τ'		
Turning Movement	Left	Thru	Thru	Right	Left	Right	
Lane Width [ft]	12.00 12.00		12.00 12.00		12.00	12.00	
No. of Lanes in Entry Pocket	0 0		0	0 0		0	
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00 100.00		100.00	
No. of Lanes in Exit Pocket	0	0	0	0	0	0	
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	
Speed [mph]	30.00		30	.00	30	.00	
Grade [%]	0.	00	0.	00	0.00		
Crosswalk	Y	es	Y	es	Yes		

Name	Indiar	n Hills	Indiar	n Hills	Missio	n Hills
Base Volume Input [veh/h]	65	20	18	4	2	88
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	13	0	0	0	0	10
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	83	22	19	4	2	105
Peak Hour Factor	0.9170	0.9170	0.9170	0.9170	0.9170	0.9170
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	23	6	5	1	1	29
Total Analysis Volume [veh/h]	91	24	21	4	2	115
Pedestrian Volume [ped/h]	()	()	()

Version 2020 (SP 0-5)

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			No
Number of Storage Spaces in Median	0	0	0

V/C, Movement V/C Ratio	0.06	0.00	0.00	0.00	0.00	0.11	
d_M, Delay for Movement [s/veh]	7.40	0.00	0.00	0.00	10.47	8.85	
Movement LOS	Α	Α	Α	А	В	Α	
95th-Percentile Queue Length [veh/ln]	0.18	0.18	0.00	0.00	0.38	0.38	
95th-Percentile Queue Length [ft/ln]	4.55	4.55	0.00	0.00	9.42	9.42	
d_A, Approach Delay [s/veh]	5.8	86	0.	00	8.8	38	
Approach LOS	A	4	,	4	P	١	
d_I, Intersection Delay [s/veh]	6.66						
Intersection LOS			E	3			

Intersection Level Of Service Report Intersection 1: Rinaldi & Indian Hills

Control Type:SignalizedDelay (sec / veh):19.1Analysis Method:HCM 6th EditionLevel Of Service:BAnalysis Period:15 minutesVolume to Capacity (v/c):0.793

Intersection Setup

Name	ı	ndian Hills	S	l	ndian Hills	S		Rinaldi			Rinaldi		
Approach	١	Northbound			outhboun	d	Eastbound			Westbound			
Lane Configuration		+			1r			Hir			Hir		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	
No. of Lanes in Entry Pocket	0	0	0	0	0	1	1	0	1	1	0	1	
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	1	0	0	0	
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	49.21	0.00	0.00	0.00	
Speed [mph]		30.00			30.00			30.00			30.00		
Grade [%]		0.00			0.00		0.00			0.00			
Curb Present		No			No		No			No			
Crosswalk		Yes			Yes		Yes			Yes			

Name	I	ndian Hills	3	I	ndian Hills	3		Rinaldi			Rinaldi	
Base Volume Input [veh/h]	25	1	19	278	1	180	65	1781	11	26	799	57
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	25	1	19	278	1	180	65	1781	11	26	799	57
Peak Hour Factor	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	6	0	5	72	0	47	17	460	3	7	207	15
Total Analysis Volume [veh/h]	26	1	20	287	1	186	67	1842	11	27	826	59
Presence of On-Street Parking	No		No	No		No	No		No	No		No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing	9	28			19			35			29	
v_di, Inbound Pedestrian Volume crossing r	n	35			29			28			19	
v_co, Outbound Pedestrian Volume crossing	9	0			0			0			0	
v_ci, Inbound Pedestrian Volume crossing n	ni	0			0			0			0	
v_ab, Corner Pedestrian Volume [ped/h]		0			0			0			0	
Bicycle Volume [bicycles/h]		0			0			0			0	



Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permiss											
Signal Group	0	8	0	0	4	0	0	6	0	0	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	_	_	-	-	_	-	-	_
Minimum Green [s]	0	8	0	0	8	0	0	10	0	0	10	0
Maximum Green [s]	0	30	0	0	30	0	0	44	0	0	44	0
Amber [s]	0.0	3.6	0.0	0.0	3.6	0.0	0.0	4.4	0.0	0.0	4.4	0.0
All red [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	0.5	0.0	0.0	0.5	0.0
Split [s]	0	40	0	0	40	0	0	50	0	0	50	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
Walk [s]	0	7	0	0	7	0	0	7	0	0	7	0
Pedestrian Clearance [s]	0	19	0	0	19	0	0	8	0	0	8	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No			No			Yes			Yes	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
l2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
Minimum Recall		No			No			Yes			Yes	
Maximum Recall		No			No			Yes			Yes	
Pedestrian Recall		No			No			Yes			Yes	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Exclusive Pedestrian Phase

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0

Lane Group Calculations

Lane Group	С	С	R	L	С	R	L	С	R
C, Cycle Length [s]	90	90	90	90	90	90	90	90	90
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
I1_p, Permitted Start-Up Lost Time [s]	2.00	2.00	0.00	2.00	0.00	0.00	2.00	0.00	0.00
I2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	28	28	28	54	54	54	54	54	54
g / C, Green / Cycle	0.32	0.32	0.32	0.60	0.60	0.60	0.60	0.60	0.60
(v / s)_i Volume / Saturation Flow Rate	0.22	0.28	0.12	0.10	0.52	0.01	0.11	0.23	0.04
s, saturation flow rate [veh/h]	216	1044	1589	657	3560	1506	251	3560	1526
c, Capacity [veh/h]	130	408	499	384	2125	899	121	2125	911
d1, Uniform Delay [s]	26.18	29.29	23.97	14.49	15.15	7.36	35.46	9.52	7.59
k, delay calibration	0.14	0.25	0.11	0.50	0.50	0.50	0.50	0.50	0.50
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	2.15	5.12	0.46	0.99	5.08	0.02	4.26	0.54	0.14
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.36	0.71	0.37	0.17	0.87	0.01	0.22	0.39	0.06
d, Delay for Lane Group [s/veh]	28.32	34.41	24.43	15.48	20.23	7.39	39.72	10.06	7.73
Lane Group LOS	С	С	С	В	С	Α	D	В	Α
Critical Lane Group	No	Yes	No	No	Yes	No	No	No	No
50th-Percentile Queue Length [veh/ln]	0.77	6.25	3.06	0.90	15.27	0.09	0.69	4.04	0.48
50th-Percentile Queue Length [ft/In]	19.13	156.37	76.57	22.38	381.72	2.15	17.26	100.98	11.92
95th-Percentile Queue Length [veh/ln]	1.38	10.36	5.51	1.61	21.68	0.16	1.24	7.27	0.86
95th-Percentile Queue Length [ft/ln]	34.44	258.91	137.83	40.28	541.93	3.88	31.06	181.76	21.46

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	28.32	28.32	28.32	34.41	34.41	24.43	15.48	20.23	7.39	39.72	10.06	7.73
Movement LOS	С	С	С	С	С	С	В	С	Α	D	В	Α
d_A, Approach Delay [s/veh]	28.32			30.49			19.99			10.78		
Approach LOS		С			С			В			В	
d_I, Intersection Delay [s/veh]						19	.09					
Intersection LOS						E	3					
Intersection V/C		0.793										

Other Modes

g_Walk,mi, Effective Walk Time [s]	26.0	26.0	11.0	11.0
M_corner, Corner Circulation Area [ft²/ped]	0.00	0.00	0.00	0.00
M_CW, Crosswalk Circulation Area [ft²/ped	430.39	516.43	0.00	0.00
d_p, Pedestrian Delay [s]	22.76	22.76	34.67	34.67
I_p,int, Pedestrian LOS Score for Intersection	n 1.778	2.213	3.042	3.358
Crosswalk LOS	Α	В	С	С
s_b, Saturation Flow Rate of the bicycle lane	2000	2000	2000	2000
c_b, Capacity of the bicycle lane [bicycles/h] 764	764	1002	1002
d_b, Bicycle Delay [s]	17.17	17.17	11.20	11.20
I_b,int, Bicycle LOS Score for Intersection	1.637	2.342	3.144	2.312
Bicycle LOS	А	В	С	В

Sequence

Ring 1	2	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	6	8	-	-	_	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Intersection Level Of Service Report Intersection 2: Rinaldi & Memory Park

Control Type:Two-way stopDelay (sec / veh):137.2Analysis Method:HCM 6th EditionLevel Of Service:FAnalysis Period:15 minutesVolume to Capacity (v/c):0.198

Intersection Setup

Name										Rinaldi		
Approach	١	lorthboun	d	s	Southbound		Eastbound			Westbound		
Lane Configuration				+			nii n			11-		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0	1	0	0	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]		30.00			30.00	-	30.00			30.00		
Grade [%]	0.00		0.00		0.00			0.00				
Crosswalk	Yes		Yes		Yes			Yes				

Name											Rinaldi	
Base Volume Input [veh/h]	0	0	0	6	0	10	25	1772	0	0	1037	16
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	0	0	0	6	0	10	25	1772	0	0	1037	16
Peak Hour Factor	1.0000	1.0000	1.0000	0.9600	0.9600	0.9600	0.9600	0.9600	1.0000	1.0000	0.9600	0.9600
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	0	0	2	0	3	7	461	0	0	270	4
Total Analysis Volume [veh/h]	0	0	0	6	0	10	26	1846	0	0	1080	17
Pedestrian Volume [ped/h]		0			0			0			0	

Priority Scheme	Stop	Stop	Free	Free
Flared Lane		No		
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance		No		
Number of Storage Spaces in Median	0	0	0	0

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.20	0.00	0.02	0.04	0.02	0.00	0.00	0.01	0.00
d_M, Delay for Movement [s/veh]	0.00	0.00	0.00	137.17	293.73	25.81	10.94	0.00	0.00	0.00	0.00	0.00
Movement LOS				F	F	D	В	Α			Α	Α
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.00	0.76	0.76	0.76	0.13	0.00	0.00	0.00	0.00	0.00
95th-Percentile Queue Length [ft/ln]	0.00	0.00	0.00	19.00	19.00	19.00	3.21	0.00	0.00	0.00	0.00	0.00
d_A, Approach Delay [s/veh]		0.00		67.57				0.15			0.00	
Approach LOS		А			F A					Α		
d_I, Intersection Delay [s/veh]	0.46											
Intersection LOS	F											

Intersection Level Of Service Report Intersection 3: Mission Hills & Indian Hills

Control Type:Two-way stopDelay (sec / veh):10.2Analysis Method:HCM 6th EditionLevel Of Service:BAnalysis Period:15 minutesVolume to Capacity (v/c):0.001

Intersection Setup

Name	India	n Hills	India	n Hills	Missi	on Hills	
Approach	North	bound	South	bound	Eastbound		
Lane Configuration	4		1	→	₩.		
Turning Movement	Left Thru		Thru	Right	Left	Right	
Lane Width [ft]	12.00 12.00		12.00	12.00	12.00	12.00	
No. of Lanes in Entry Pocket	0 0		0	0	0	0	
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	
No. of Lanes in Exit Pocket	0	0	0	0	0	0	
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	
Speed [mph]	30.00		30	.00	30.00		
Grade [%]	0.00		0.	00	0.00		
Crosswalk	Y	es	Y	es	Yes		

Name	India	n Hills	India	n Hills	Missio	n Hills
Base Volume Input [veh/h]	43	7	17	2	1	160
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	43	7	17	2	1	160
Peak Hour Factor	0.7880	0.7880	0.7880	0.7880	0.7880	0.7880
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	14	2	5	1	0	51
Total Analysis Volume [veh/h]	55	9	22	3	1	203
Pedestrian Volume [ped/h]	(0		0	()

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			No
Number of Storage Spaces in Median	0	0	0

V/C, Movement V/C Ratio	0.03	0.00	0.00	0.00	0.00	0.19	
d_M, Delay for Movement [s/veh]	7.35	0.00	0.00	0.00	10.21	9.24	
Movement LOS	Α	A	А	А	В	A	
95th-Percentile Queue Length [veh/ln]	0.11	0.11	0.00	0.00	0.72	0.72	
95th-Percentile Queue Length [ft/ln]	2.69	2.69	0.00	0.00	17.93	17.93	
d_A, Approach Delay [s/veh]	6.	31	0	.00	9.:	25	
Approach LOS	,	A		A	,	4	
d_I, Intersection Delay [s/veh]	7.82						
Intersection LOS	В						

Intersection Level Of Service Report Intersection 1: Rinaldi & Indian Hills

Control Type:SignalizedDelay (sec / veh):20.8Analysis Method:HCM 6th EditionLevel Of Service:CAnalysis Period:15 minutesVolume to Capacity (v/c):0.812

Intersection Setup

Name	ı	ndian Hills	3	I	ndian Hills	S		Rinaldi		Rinaldi		
Approach	١	orthboun	d	S	outhboun	d	E	Eastbound	d t	V	Vestbound	d
Lane Configuration		+			4		•	1 r		•	1 r	
Turning Movement							Thru	Right	Left	Thru	Right	
Lane Width [ft]	12.00	2.00 12.00 12.00 12			12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0 0 0			0	1	1	0	1	1	0	1
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	1	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	49.21	0.00	0.00	0.00
Speed [mph]		30.00			30.00			30.00			30.00	
Grade [%]		0.00			0.00			0.00			0.00	
Curb Present	No		No				No		No			
Crosswalk		Yes		Yes				Yes		Yes		

Name	I	ndian Hills	3	ı	ndian Hills	6		Rinaldi			Rinaldi		
Base Volume Input [veh/h]	25	1	19	278	1	180	65	1781	11	26	799	57	
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	
Site-Generated Trips [veh/h]	0	0	0	18	0	0	0	0	0	0	0	13	
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	
Total Hourly Volume [veh/h]	25	1	19	296	1	180	65	1781	11	26	799	70	
Peak Hour Factor	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	
Total 15-Minute Volume [veh/h]	6	0	5	77	0	47	17	460	3	7	207	18	
Total Analysis Volume [veh/h]	26	1	20	306	1	186	67	1842	11	27	826	72	
Presence of On-Street Parking	No		No	No		No	No		No	No		No	
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	
v_do, Outbound Pedestrian Volume crossing	9	28			19			35			29		
v_di, Inbound Pedestrian Volume crossing r	n	35			29			28			19		
v_co, Outbound Pedestrian Volume crossing	3	0			0	_		0			0	_	
v_ci, Inbound Pedestrian Volume crossing n	ni	0			0			0			0		
v_ab, Corner Pedestrian Volume [ped/h]		0		0			0						
Bicycle Volume [bicycles/h]	0			0				0		0			

Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permiss											
Signal Group	0	8	0	0	4	0	0	6	0	0	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	_	_	-	-	_	-	-	_
Minimum Green [s]	0	8	0	0	8	0	0	10	0	0	10	0
Maximum Green [s]	0	30	0	0	30	0	0	44	0	0	44	0
Amber [s]	0.0	3.6	0.0	0.0	3.6	0.0	0.0	4.4	0.0	0.0	4.4	0.0
All red [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	0.5	0.0	0.0	0.5	0.0
Split [s]	0	40	0	0	40	0	0	50	0	0	50	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
Walk [s]	0	7	0	0	7	0	0	7	0	0	7	0
Pedestrian Clearance [s]	0	19	0	0	19	0	0	8	0	0	8	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No			No			Yes			Yes	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
l2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
Minimum Recall		No			No			Yes			Yes	
Maximum Recall		No			No			Yes			Yes	
Pedestrian Recall		No			No			Yes			Yes	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Exclusive Pedestrian Phase

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0

Lane Group Calculations

Lane Group	С	С	R	L	С	R	L	С	R
C, Cycle Length [s]	90	90	90	90	90	90	90	90	90
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
I1_p, Permitted Start-Up Lost Time [s]	2.00	2.00	0.00	2.00	0.00	0.00	2.00	0.00	0.00
I2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	30	30	30	52	52	52	52	52	52
g / C, Green / Cycle	0.33	0.33	0.33	0.58	0.58	0.58	0.58	0.58	0.58
(v / s)_i Volume / Saturation Flow Rate	0.23	0.29	0.12	0.10	0.52	0.01	0.11	0.23	0.05
s, saturation flow rate [veh/h]	204	1042	1589	656	3560	1503	251	3560	1524
c, Capacity [veh/h]	130	425	527	368	2063	871	111	2063	883
d1, Uniform Delay [s]	25.14	28.55	22.77	15.78	16.48	8.01	38.72	10.36	8.33
k, delay calibration	0.16	0.29	0.11	0.50	0.50	0.50	0.50	0.50	0.50
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	2.55	6.01	0.40	1.09	6.41	0.03	5.13	0.58	0.18
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.36	0.72	0.35	0.18	0.89	0.01	0.24	0.40	0.08
d, Delay for Lane Group [s/veh]	27.69	34.56	23.17	16.87	22.89	8.04	43.85	10.94	8.51
Lane Group LOS	С	С	С	В	С	Α	D	В	Α
Critical Lane Group	No	Yes	No	No	Yes	No	No	No	No
50th-Percentile Queue Length [veh/ln]	0.76	6.73	2.97	0.94	16.42	0.09	0.73	4.28	0.62
50th-Percentile Queue Length [ft/In]	18.97	168.28	74.19	23.60	410.57	2.28	18.36	106.93	15.52
95th-Percentile Queue Length [veh/ln]	1.37	10.99	5.34	1.70	23.07	0.16	1.32	7.67	1.12
95th-Percentile Queue Length [ft/In]	34.15	274.65	133.54	42.48	576.72	4.10	33.04	191.72	27.94

PM Existing + Project Version 2020 (SP 0-0)

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	27.69	27.69	27.69	34.56	34.56	23.17	16.87	22.89	8.04	43.85	10.94	8.51	
Movement LOS	С	С	С	С	С	С	В	С	Α	D	В	Α	
d_A, Approach Delay [s/veh]		27.69			30.27			22.59		11.71			
Approach LOS		С			С			С			В		
d_I, Intersection Delay [s/veh]						20	.81						
Intersection LOS						()						
Intersection V/C	0.812												

Other Modes

g_Walk,mi, Effective Walk Time [s]	26.0	26.0	11.0	11.0
M_corner, Corner Circulation Area [ft²/ped]	0.00	0.00	0.00	0.00
M_CW, Crosswalk Circulation Area [ft²/ped	430.39	509.08	0.00	0.00
d_p, Pedestrian Delay [s]	22.76	22.76	34.67	34.67
I_p,int, Pedestrian LOS Score for Intersection	n 1.778	2.224	3.042	3.390
Crosswalk LOS	А	В	С	С
s_b, Saturation Flow Rate of the bicycle lane	2000	2000	2000	2000
c_b, Capacity of the bicycle lane [bicycles/h	764	764	1002	1002
d_b, Bicycle Delay [s]	17.17	17.17	11.20	11.20
I_b,int, Bicycle LOS Score for Intersection	1.637	2.373	3.144	2.323
Bicycle LOS	А	В	С	В

Sequence

_		_														
Ring 1	2	4	-	-	-	-	-	-	-	-	-	1	1	-	-	-
Ring 2	6	8	-	-	_	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Intersection Level Of Service Report Intersection 2: Rinaldi & Memory Park

Control Type: Two-way stop
Analysis Method: HCM 6th Edition
Analysis Period: 15 minutes

Delay (sec / veh): 145.7 Level Of Service: F Volume to Capacity (v/c): 0.218

Intersection Setup

Name											Rinaldi		
Approach	١	Northboun	d	S	Southboun	d	E	Eastbound	t	٧	Westbound		
Lane Configuration					+			пП			1H		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
Lane Width [ft]	12.00	12.00 12.00 12.00 1			12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	
No. of Lanes in Entry Pocket	0	0	0	0	0	0	1	0	0	0	0	0	
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00 100.00 100.00		100.00	100.00	100.00	100.00	100.00	100.00	
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0	
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Speed [mph]		30.00			30.00	-		30.00	-	30.00			
Grade [%]	0.00			0.00			0.00		0.00				
Crosswalk	Yes		Yes				Yes		Yes				

Name											Rinaldi	
Base Volume Input [veh/h]	0	0	0	6	0	10	25	1772	0	0	1037	16
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	23	17	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	0	0	0	6	0	33	42	1772	0	0	1037	16
Peak Hour Factor	1.0000	1.0000	1.0000	0.9600	0.9600	0.9600	0.9600	0.9600	1.0000	1.0000	0.9600	0.9600
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	0	0	2	0	9	11	461	0	0	270	4
Total Analysis Volume [veh/h]	0	0	0	6	0	34	44	1846	0	0	1080	17
Pedestrian Volume [ped/h]		0			0			0		0		

Priority Scheme	Stop	Stop	Free	Free
Flared Lane		No		
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance		No		
Number of Storage Spaces in Median	0	0	0	0

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.22	0.00	0.07	0.07	0.02	0.00	0.00	0.01	0.00
d_M, Delay for Movement [s/veh]	0.00	0.00	0.00	145.74	315.11	22.70	11.12	0.00	0.00	0.00	0.00	0.00
Movement LOS				F	F	С	В	Α			Α	Α
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.00	1.12	1.12	1.12	0.22	0.00	0.00	0.00	0.00	0.00
95th-Percentile Queue Length [ft/ln]	0.00	0.00	0.00	27.88	27.88	27.88	5.60	0.00	0.00	0.00	0.00	0.00
d_A, Approach Delay [s/veh]		0.00		41.15				0.26			0.00	
Approach LOS		А			E				Α			
d_I, Intersection Delay [s/veh]		0.71										
Intersection LOS						ı	F					

Intersection Level Of Service Report Intersection 3: Mission Hills & Indian Hills

Control Type:Two-way stopDelay (sec / veh):10.6Analysis Method:HCM 6th EditionLevel Of Service:BAnalysis Period:15 minutesVolume to Capacity (v/c):0.001

Intersection Setup

Name	India	n Hills	India	n Hills	Missi	on Hills	
Approach	North	bound	South	bound	East	bound	
Lane Configuration	4		1	→	-	Γ	
Turning Movement	Left Thru		Thru	Right	Left	Right	
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	
No. of Lanes in Entry Pocket	0	0 0		0	0	0	
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	
No. of Lanes in Exit Pocket	0	0	0	0	0	0	
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	
Speed [mph]	30	30.00		30.00		0.00	
Grade [%]	0.00		0.	.00	0.00		
Crosswalk	Y	'es	Y	es	Yes		

Name	Indiar	n Hills	India	n Hills	Missio	n Hills
Base Volume Input [veh/h]	43	7	17	2	1	160
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	13	0	0	0	0	18
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	56	7	17	2	1	178
Peak Hour Factor	0.7880	0.7880	0.7880	0.7880	0.7880	0.7880
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	18	2	5	1	0	56
Total Analysis Volume [veh/h]	71	9	22	3	1	226
Pedestrian Volume [ped/h]	()	()	()

Version 2020 (SP 0-0)

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			No
Number of Storage Spaces in Median	0	0	0

V/C, Movement V/C Ratio	0.04	0.00	0.00	0.00	0.00	0.21			
d_M, Delay for Movement [s/veh]	7.37	0.00	0.00	0.00	10.56	9.36			
Movement LOS	Α	А	Α	А	В	A			
95th-Percentile Queue Length [veh/ln]	0.14	0.14	0.00	0.00	0.82	0.82			
95th-Percentile Queue Length [ft/ln]	3.50	3.50	0.00	0.00	20.49	20.49			
d_A, Approach Delay [s/veh]	6.	54	0.	.00	9.	36			
Approach LOS	,	4		A	A				
d_I, Intersection Delay [s/veh]		7.98							
Intersection LOS				В					



Intersection Level Of Service Report Intersection 1: Rinaldi & Indian Hills

Control Type: Signalized
Analysis Method: HCM 6th Edition
Analysis Period: 15 minutes

Delay (sec / veh): 27.2
Level Of Service: C
Volume to Capacity (v/c): 0.862

Intersection Setup

Name	lı	ndian Hill	s	lı	ndian Hill	s					Rinaldi	
Approach	Northbound			S	outhbour	ıd	Eastbound			Westbound		
Lane Configuration	+		46			ПIF			ПIL			
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	1	1	0	1	1	0	1
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	1	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	49.21	0.00	0.00	0.00
Speed [mph]		30.00			30.00			30.00			30.00	
Grade [%]		0.00			0.00			0.00			0.00	
Curb Present	No			No		No			No			
Crosswalk		Yes			Yes		Yes			Yes		

Name	lı	ndian Hill	s	lı	ndian Hill	s					Rinaldi	
Base Volume Input [veh/h]	25	1	19	278	1	180	65	1781	11	26	799	57
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800
In-Process Volume [veh/h]	0	0	0	0	0	0	0	8	0	0	8	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	27	1	21	300	1	194	70	1931	12	28	871	62
Peak Hour Factor	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	7	0	5	78	0	50	18	499	3	7	225	16
Total Analysis Volume [veh/h]	28	1	22	310	1	201	72	1997	12	29	901	64
Presence of On-Street Parking	No		No	No		No	No		No	No		No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing major stre	е	28			19			35			29	
v_di, Inbound Pedestrian Volume crossing major street	[35			29			28			19	
v_co, Outbound Pedestrian Volume crossing minor stre	е	0			0			0			0	
v_ci, Inbound Pedestrian Volume crossing minor street	[0			0			0			0	
v_ab, Corner Pedestrian Volume [ped/h]		0			0			0			0	
Bicycle Volume [bicycles/h]		0			0			0			0	



Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permis											
Signal Group	0	8	0	0	4	0	0	6	0	0	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	-	-	-	-	-	-
Minimum Green [s]	0	8	0	0	8	0	0	10	0	0	10	0
Maximum Green [s]	0	34	0	0	34	0	0	44	0	0	44	0
Amber [s]	0.0	3.6	0.0	0.0	3.6	0.0	0.0	4.4	0.0	0.0	4.4	0.0
All red [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	0.5	0.0	0.0	0.5	0.0
Split [s]	0	40	0	0	40	0	0	50	0	0	50	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
Walk [s]	0	7	0	0	7	0	0	7	0	0	7	0
Pedestrian Clearance [s]	0	19	0	0	19	0	0	8	0	0	8	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No			No			No			No	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
Minimum Recall		No			No			Yes			Yes	
Maximum Recall		No			No			Yes			Yes	
Pedestrian Recall		No			No			Yes			Yes	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Exclusive Pedestrian Phase

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0

PM Future w/o Project Version 2020 (SP 0-5)

Lane Group Calculations

Lane Group	С	С	R	L	С	R	L	С	R
C, Cycle Length [s]	90	90	90	90	90	90	90	90	90
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
I1_p, Permitted Start-Up Lost Time [s]	2.00	2.00	0.00	2.00	0.00	0.00	2.00	0.00	0.00
I2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	30	30	30	52	52	52	52	52	52
g / C, Green / Cycle	0.34	0.34	0.34	0.57	0.57	0.57	0.57	0.57	0.57
(v / s)_i Volume / Saturation Flow Rate	0.25	0.30	0.13	0.12	0.56	0.01	0.13	0.25	0.04
s, saturation flow rate [veh/h]	204	1034	1589	618	3560	1502	216	3560	1523
c, Capacity [veh/h]	131	429	536	337	2042	862	85	2042	874
d1, Uniform Delay [s]	25.99	28.30	22.61	17.42	18.63	8.24	44.79	10.95	8.53
k, delay calibration	0.20	0.30	0.11	0.50	0.50	0.50	0.50	0.50	0.50
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	3.47	6.29	0.43	1.45	15.33	0.03	10.47	0.69	0.16
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.39	0.73	0.37	0.21	0.98	0.01	0.34	0.44	0.07
d, Delay for Lane Group [s/veh]	29.46	34.59	23.04	18.87	33.96	8.27	55.26	11.65	8.69
Lane Group LOS	С	С	С	В	С	Α	Е	В	Α
Critical Lane Group	No	Yes	No	No	Yes	No	No	No	No
50th-Percentile Queue Length [veh/ln]	0.84	6.84	3.21	1.09	22.07	0.10	0.89	4.90	0.56
50th-Percentile Queue Length [ft/ln]	21.10	170.95	80.20	27.33	551.65	2.53	22.24	122.41	13.98
95th-Percentile Queue Length [veh/ln]	1.52	11.13	5.77	1.97	29.77	0.18	1.60	8.53	1.01
95th-Percentile Queue Length [ft/ln]	37.97	278.16	144.37	49.19	744.25	4.56	40.04	213.13	25.16

Version 2020 (SP 0-5)

PM Future w/o Project

Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	29.46	29.46	29.46	34.59	34.59	23.04	18.87	33.96	8.27	55.26	11.65	8.69
Movement LOS	С	С	С	С	С	С	В	С	Α	Е	В	Α
d_A, Approach Delay [s/veh]	29.46			30.06			33.29					
Approach LOS	С			С			С					
d_I, Intersection Delay [s/veh]						27	.17					
Intersection LOS						()					
Intersection V/C	0.862											

Other Modes

g_Walk,mi, Effective Walk Time [s]	11.0	11.0	11.0	11.0
M_corner, Corner Circulation Area [ft²/ped]	0.00	0.00	0.00	0.00
M_CW, Crosswalk Circulation Area [ft²/ped]	170.97	171.44	0.00	0.00
d_p, Pedestrian Delay [s]	34.67	34.67	34.67	34.67
I_p,int, Pedestrian LOS Score for Intersection	1.801	2.253	3.080	3.433
Crosswalk LOS	A	В	С	С
s_b, Saturation Flow Rate of the bicycle lane [bicycles/l	1] 2000	2000	2000	2000
c_b, Capacity of the bicycle lane [bicycles/h]	764	764	1002	1002
d_b, Bicycle Delay [s]	17.17	17.17	11.20	11.20
I_b,int, Bicycle LOS Score for Intersection	1.644	2.404	3.276	2.380
Bicycle LOS	A	В	С	В

Sequence

_		_														
Ring 1	2	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	6	8	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-





Intersection Level Of Service Report Intersection 2: Rinaldi & Memory Park

Control Type: Two-way stop
Analysis Method: HCM 6th Edition
Analysis Period: 15 minutes

Delay (sec / veh): 192.1 Level Of Service: F Volume to Capacity (v/c): 0.271

Intersection Setup

Name												
Approach	N	orthbour	ıd	Southbound		Eastbound			Westbound		d	
Lane Configuration				+		пΠ			11-			
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0	1	0	0	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]		30.00			30.00			30.00			30.00	
Grade [%]	0.00		0.00			0.00			0.00			
Crosswalk		Yes			Yes			Yes			Yes	

Name												
Base Volume Input [veh/h]	0	0	0	6	0	10	25	1772	0	0	1037	16
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800
In-Process Volume [veh/h]	0	0	0	0	0	0	0	8	0	0	8	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	0	0	0	6	0	11	27	1922	0	0	1128	17
Peak Hour Factor	1.0000	1.0000	1.0000	0.9620	0.9620	0.9620	0.9620	0.9620	1.0000	1.0000	0.9620	0.9620
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	0	0	2	0	3	7	499	0	0	293	4
Total Analysis Volume [veh/h]	0	0	0	6	0	11	28	1998	0	0	1173	18
Pedestrian Volume [ped/h]	0		0			0			0			

Version 2020 (SP 0-5)

Intersection Settings

Priority Scheme	Stop	Stop	Free	Free
Flared Lane		No		
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance		No		
Number of Storage Spaces in Median	0	0	0	0

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.27	0.00	0.02	0.05	0.02	0.00	0.00	0.01	0.00
d_M, Delay for Movement [s/veh]	0.00	0.00	0.00	192.11	436.02	37.47	11.50	0.00	0.00	0.00	0.00	0.00
Movement LOS				F	F	Е	В	Α			Α	Α
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.00	1.04	1.04	1.04	0.15	0.00	0.00	0.00	0.00	0.00
95th-Percentile Queue Length [ft/ln]	0.00	0.00	0.00	26.11	26.11	26.11	3.78	0.00	0.00	0.00	0.00	0.00
d_A, Approach Delay [s/veh]		0.00			92.05			0.16			0.00	
Approach LOS		Α			F			Α	A			
d_I, Intersection Delay [s/veh]	0.58											
Intersection LOS	F											

Intersection Level Of Service Report

Intersection 3: Mission Hills & Indian Hills

Control Type:Two-way stopDelay (sec / veh):10.4Analysis Method:HCM 6th EditionLevel Of Service:BAnalysis Period:15 minutesVolume to Capacity (v/c):0.001

Intersection Setup

Name	India	n Hills	India	n Hills	Missio	on Hills	
Approach	North	bound	South	bound	Eastbound		
Lane Configuration	4		ŀ	•	₩.		
Turning Movement	Left	Thru	Thru	Right	Left	Right	
Lane Width [ft]	12.00 12.00		12.00	12.00 12.00		12.00	
No. of Lanes in Entry Pocket	0 0		0	0	0	0	
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	
No. of Lanes in Exit Pocket	0	0	0	0	0	0	
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	
Speed [mph]	30	.00	30	30.00		.00	
Grade [%]	0.00		0.	00	0.00		
Crosswalk	Yes		Y	es	Yes		

Name	Indiar	n Hills	Indiar	n Hills	Missio	n Hills	
Base Volume Input [veh/h]	43	7	17	2	1	160	
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	
Growth Factor	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	
In-Process Volume [veh/h]	0	0	0	0	0	0	
Site-Generated Trips [veh/h]	0	0	0	0	0	0	
Diverted Trips [veh/h]	0	0	0	0	0	0	
Pass-by Trips [veh/h]	0	0	0	0	0	0	
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	
Other Volume [veh/h]	0	0	0	0	0	0	
Total Hourly Volume [veh/h]	46	8	18	2	1	173	
Peak Hour Factor	0.7880	0.7880	0.7880	0.7880	0.7880	0.7880	
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	
Total 15-Minute Volume [veh/h]	15	3	6	1	0	55	
Total Analysis Volume [veh/h]	58	10	23	3	1	220	
Pedestrian Volume [ped/h]	(0)	0		

Version 2020 (SP 0-5)

Intersection Settings

Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			No
Number of Storage Spaces in Median	0	0	0

V/C, Movement V/C Ratio	0.04	0.00	0.00	0.00	0.00	0.21		
d_M, Delay for Movement [s/veh]	7.35	0.00	0.00	0.00	10.35	9.33		
Movement LOS	Α	A	Α	A	В	Α		
95th-Percentile Queue Length [veh/ln]	0.11	0.11	0.00	0.00	0.79	0.79		
95th-Percentile Queue Length [ft/ln]	2.84	2.84	0.00	0.00	19.83	19.83		
d_A, Approach Delay [s/veh]	6.:	27	0.	00	9.3	34		
Approach LOS	,	4	,	4	J.	١		
d_I, Intersection Delay [s/veh]	7.91							
Intersection LOS	В							

PM Future + Project

31.7

С

0.880

Intersection Level Of Service Report Intersection 1: Rinaldi & Indian Hills

Control Type:SignalizedDelay (sec / veh):Analysis Method:HCM 6th EditionLevel Of Service:Analysis Period:15 minutesVolume to Capacity (v/c):

Intersection Setup

Name	lı	ndian Hill	s	li	ndian Hill	s						
Approach	N	orthboun	ıd	S	outhbour	ıd	Е	astboun	d	Westbound		
Lane Configuration		+			1 r		+	1116	•	HILL		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0 0 0			0	1	1	0	1	1	0	1
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	1	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	49.21	0.00	0.00	0.00
Speed [mph]		30.00			30.00			30.00			30.00	
Grade [%]	0.00				0.00			0.00			0.00	
Curb Present	No		No			No			No			
Crosswalk	Yes			Yes			Yes			Yes		

Name	Ir	Indian Hills		Indian Hills								
Base Volume Input [veh/h]	25	1	19	278	1	180	65	1781	11	26	799	57
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800
In-Process Volume [veh/h]	0	0	0	0	0	0	0	8	0	0	8	0
Site-Generated Trips [veh/h]	0	0	0	18	0	0	0	0	0	0	0	13
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	27	1	21	318	1	194	70	1931	12	28	871	75
Peak Hour Factor	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670	0.9670
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	7	0	5	82	0	50	18	499	3	7	225	19
Total Analysis Volume [veh/h]	28	1	22	329	1	201	72	1997	12	29	901	78
Presence of On-Street Parking	No		No	No		No	No		No	No		No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing major stre	е	28			19			35			29	
v_di, Inbound Pedestrian Volume crossing major street	[35			29			28			19	
v_co, Outbound Pedestrian Volume crossing minor stre	е	0			0			0			0	
v_ci, Inbound Pedestrian Volume crossing minor street	[0			0			0			0	
v_ab, Corner Pedestrian Volume [ped/h]		0			0			0			0	
Bicycle Volume [bicycles/h]		0			0			0			0	

Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

Phasing & Timing

Control Type	Permis											
Signal Group	0	8	0	0	4	0	0	6	0	0	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	-	-	-	-	-	-
Minimum Green [s]	0	8	0	0	8	0	0	10	0	0	10	0
Maximum Green [s]	0	34	0	0	34	0	0	44	0	0	44	0
Amber [s]	0.0	3.6	0.0	0.0	3.6	0.0	0.0	4.4	0.0	0.0	4.4	0.0
All red [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	0.5	0.0	0.0	0.5	0.0
Split [s]	0	40	0	0	40	0	0	50	0	0	50	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0
Walk [s]	0	7	0	0	7	0	0	7	0	0	7	0
Pedestrian Clearance [s]	0	19	0	0	19	0	0	8	0	0	8	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No			No			No			No	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
l2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0
Minimum Recall		No			No			Yes			Yes	
Maximum Recall		No			No			Yes			Yes	
Pedestrian Recall		No			No			Yes			Yes	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Exclusive Pedestrian Phase

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0



Lane Group Calculations

Lane Group	С	С	R	L	С	R	L	С	R
C, Cycle Length [s]	90	90	90	90	90	90	90	90	90
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
I1_p, Permitted Start-Up Lost Time [s]	2.00	2.00	0.00	2.00	0.00	0.00	2.00	0.00	0.00
I2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
g_i, Effective Green Time [s]	32	32	32	50	50	50	50	50	50
g / C, Green / Cycle	0.35	0.35	0.35	0.56	0.56	0.56	0.56	0.56	0.56
(v / s)_i Volume / Saturation Flow Rate	0.27	0.32	0.13	0.12	0.56	0.01	0.13	0.25	0.05
s, saturation flow rate [veh/h]	190	1033	1589	618	3560	1500	216	3560	1521
c, Capacity [veh/h]	129	446	563	322	1982	835	81	1982	847
d1, Uniform Delay [s]	25.09	27.62	21.48	18.83	19.94	8.91	44.99	11.83	9.30
k, delay calibration	0.24	0.34	0.11	0.50	0.50	0.50	0.50	0.50	0.50
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	4.22	7.25	0.38	1.60	22.02	0.03	11.80	0.75	0.22
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Lane Group Results

X, volume / capacity	0.39	0.74	0.36	0.22	1.01	0.01	0.36	0.45	0.09
d, Delay for Lane Group [s/veh]	29.31	34.87	21.86	20.43	41.96	8.94	56.78	12.59	9.51
Lane Group LOS	С	С	С	С	F	Α	Е	В	Α
Critical Lane Group	No	Yes	No	No	Yes	No	No	No	No
50th-Percentile Queue Length [veh/ln]	0.85	7.33	3.11	1.15	24.42	0.11	0.91	5.16	0.72
50th-Percentile Queue Length [ft/ln]	21.19	183.32	77.69	28.70	610.44	2.66	22.69	128.95	18.10
95th-Percentile Queue Length [veh/ln]	1.53	11.77	5.59	2.07	32.72	0.19	1.63	8.88	1.30
95th-Percentile Queue Length [ft/ln]	38.14	294.35	139.84	51.66	818.01	4.79	40.85	222.07	32.58

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Movement, Approach, & Intersection Results

d_M, Delay for Movement [s/veh]	29.31	29.31	29.31	34.87	34.87	21.86	20.43	41.96	8.94	56.78	12.59	9.51
Movement LOS	С	С	С	С	С	С	С	F	Α	Е	В	Α
d_A, Approach Delay [s/veh]		29.31 29.94						41.03	13.62			
Approach LOS	C C D						В					
d_I, Intersection Delay [s/veh]						31	.74					
Intersection LOS						()					
Intersection V/C	0.880											

Other Modes

g_Walk,mi, Effective Walk Time [s]	11.0	11.0	11.0	11.0
M_corner, Corner Circulation Area [ft²/ped]	0.00	0.00	0.00	0.00
M_CW, Crosswalk Circulation Area [ft²/ped]	170.97	163.54	0.00	0.00
d_p, Pedestrian Delay [s]	34.67	34.67	34.67	34.67
I_p,int, Pedestrian LOS Score for Intersection	1.801	2.264	3.080	3.466
Crosswalk LOS	A	В	С	С
s_b, Saturation Flow Rate of the bicycle lane [bicycles/l	1] 2000	2000	2000	2000
c_b, Capacity of the bicycle lane [bicycles/h]	764	764	1002	1002
d_b, Bicycle Delay [s]	17.17	17.17	11.20	11.20
I_b,int, Bicycle LOS Score for Intersection	1.644	2.436	3.276	2.391
Bicycle LOS	A	В	С	В

Sequence

_		_														
Ring 1	2	4	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	6	8	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Intersection Level Of Service Report Intersection 2: Rinaldi & Memory Park

Control Type: Two-way stop
Analysis Method: HCM 6th Edition
Analysis Period: 15 minutes

Delay (sec / veh): 203.1 Level Of Service: F Volume to Capacity (v/c): 0.299

Intersection Setup

Name												
Approach	N	orthboun	ıd	S	outhbour	ıd	Е	astboun	d	Westbound		
Lane Configuration					+			пII			1H	
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0	1	0	0	0	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]		30.00			30.00			30.00			30.00	
Grade [%]		0.00			0.00			0.00			0.00	
Crosswalk	Yes		Yes			Yes						

Name												
Base Volume Input [veh/h]	0	0	0	6	0	10	25	1772	0	0	1037	16
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800
In-Process Volume [veh/h]	0	0	0	0	0	0	0	8	0	0	8	0
Site-Generated Trips [veh/h]	0	0	0	0	0	23	17	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	0	0	0	6	0	34	44	1922	0	0	1128	17
Peak Hour Factor	1.0000	1.0000	1.0000	0.9620	0.9620	0.9620	0.9620	0.9620	1.0000	1.0000	0.9620	0.9620
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	0	0	0	2	0	9	11	499	0	0	293	4
Total Analysis Volume [veh/h]	0	0	0	6	0	35	46	1998	0	0	1173	18
Pedestrian Volume [ped/h]		0			0			0			0	

-

Intersection Settings

Priority Scheme	Stop	Stop	Free	Free
Flared Lane		No		
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance		No		
Number of Storage Spaces in Median	0	0	0	0

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.30	0.00	0.08	0.08	0.02	0.00	0.00	0.01	0.00
d_M, Delay for Movement [s/veh]	0.00	0.00	0.00	203.08	468.15	31.84	11.72	0.00	0.00	0.00	0.00	0.00
Movement LOS				F	F	D	В	Α			Α	Α
95th-Percentile Queue Length [veh/ln]	0.00	0.00	0.00	1.54	1.54	1.54	0.26	0.00	0.00	0.00	0.00	0.00
95th-Percentile Queue Length [ft/ln]	0.00	0.00	0.00	38.43	38.43	38.43	6.41	0.00	0.00	0.00	0.00	0.00
d_A, Approach Delay [s/veh]		0.00		56.90				0.26		0.00		
Approach LOS		Α			F			Α		А		
d_I, Intersection Delay [s/veh]	0.88											
Intersection LOS	F											

Intersection Level Of Service Report Intersection 3: Mission Hills & Indian Hills

Control Type:Two-way stopDelay (sec / veh):10.7Analysis Method:HCM 6th EditionLevel Of Service:BAnalysis Period:15 minutesVolume to Capacity (v/c):0.001

Intersection Setup

Name	India	n Hills	India	n Hills	Missio	n Hills		
Approach	North	bound	South	bound	Eastl	oound		
Lane Configuration	+	1	ŀ	•	Τ			
Turning Movement	Left	Thru	Thru	Right	Left	Right		
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00		
No. of Lanes in Entry Pocket	0	0	0	0	0	0		
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00		
No. of Lanes in Exit Pocket	0	0	0	0	0	0		
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00		
Speed [mph]	30	30.00		.00	30.00			
Grade [%]	0.	00	0.	00	0.00 Yes			
Crosswalk	Y	es	Y	es				

Name	Indiar	n Hills	Indiar	n Hills	Missio	n Hills
Base Volume Input [veh/h]	43	7	17	2	1	160
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0800	1.0800	1.0800	1.0800	1.0800	1.0800
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	13	0	0	0	0	18
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	59	8	18	2	1	191
Peak Hour Factor	0.7880	0.7880	0.7880	0.7880	0.7880	0.7880
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	19	3	6	1	0	61
Total Analysis Volume [veh/h]	75	10	23	3	1	242
Pedestrian Volume [ped/h]	()	()	()



Priority Scheme	Free	Free	Stop
Flared Lane			No
Storage Area [veh]	0	0	0
Two-Stage Gap Acceptance			No
Number of Storage Spaces in Median	0	0	0

V/C, Movement V/C Ratio	0.05	0.00	0.00	0.00	0.00	0.23		
d_M, Delay for Movement [s/veh]	7.38	7.38 0.00		0.00	10.73	9.45		
Movement LOS	Α	Α	Α	A	В	Α		
95th-Percentile Queue Length [veh/ln]	0.15	0.15	0.00	0.00	0.90	0.90		
95th-Percentile Queue Length [ft/ln]	3.71	3.71	0.00	0.00	22.38	22.38		
d_A, Approach Delay [s/veh]	6.	51	0.	00	9.46			
Approach LOS	A	4	,	4	A			
d_I, Intersection Delay [s/veh]	8.05							
Intersection LOS	В							

Appendix D

Circular 212 Worksheet



Level of Service Workheet



(Circular 212 Method)

I/S #:	North-South Street:	Indian H	ills Road			Yea	r of Count	2019	Amb	ient Grov	vth: (%):	2	Condu	cted by:	Т	`A	Date:		7/17/2020)
2	East-West Street:	Rinaldi S	Street			Proje	ction Year	2023		Pea	ak Hour:	AM	Revie	wed by:	J	В	Project:	Ar	arat Hom	es
Орр	No. of osed Ø'ing: N/S-1, E/W-2 or	Phases Both-3?			2	2 0					2				2				2	
Right ⁻	Turns: FREE-1, NRTOR-2 or	OLA-3?	NB 0 EB 0	SB WB	0	NB EB	0 SE 0 W		NB EB	0	SB WB	0	NB EB	0 0	SB WB	0	NB EB	0	SB WB	0
	ATSAC-1 or ATSAC+ATCS-2? Override Capacity		LB	WB	2		0 1/1	2 0	EB	U	WB	2	EB	U	WB	2 0		0	WB	2 0
			EXISTI	NG CONDI	TION	EXISTI	NG PLUS PI	ROJECT	FUTUR	E CONDITION	ON W/O PR	OJECT	FUTURE CONDITION W/ PR			OJECT	FUTURE	W/ PROJE	ECT W/ MITIGATION	
	MOVEMENT		Volume	No. of Lanes	Lane Volume	Project Traffic	Total Volume	Lane Volume	Added Volume	Total Volume	No. of Lanes	Lane Volume	Added Volume	Total Volume	No. of Lanes	Lane Volume	Added Volume	Total Volume	No. of Lanes	Lane Volume
9	Left		35	0	35	0	35	35	0	38	0	38	0	38	0	38	0	38	0	38
IBOUN	← Left-Through ↑ Through ↑ Through-Right		12	0 0 0	72	0	12	72	0	13	0 0 0	78	0	13	0 0 0	78	0	13	0 0 0	78
NORTHBOUND	Right Left-Through-Right		25	0	0	0	25	0	0	27	0	0	0	27	0	0	0	27	0	0
	Left-Right			0							0				0				0	
S S	∟ Left		87	0 1	87	10	97	97	0	94	0 1	94	10	104	0	104	0	104	0	104
) g	Through		3	0	90	0	3	100	0	3	0	97	0	3	0	107	0	3	0	107
SOUTHBOUND			161	0 1 0	86	0	161	86	0	174	0 1 0	93	0	174	0 1 0	93	0	174	0 1 0	93
, J	Left-Right			0							0				0				0	
Q.	✓ Left✓ Left-Through		150	1 0	150	0	150	150	0	162	1 0	162	0	162	1 0	162	0	162	1 0	162
EASTBOUND	→ Through → Through-Right		907	2	454	0	907	454	6	988	2	494	0	988	2	494	0	988	2	494
EAS	Right Left-Through-Right Left-Right		29	0 0	29	0	29	29	0	31	1 0 0	31	0	31	0	31	0	31	0	31
	* -			-							-									
QND	✓ Left✓ Left-Through← Through		9 1230	1 0 2	9 615	0	9 1230	9 615	0 6	10 1337	1 0 2	10	0	10 1337	1 0 2	10 669	0	10 1337	1 0	10 669
<u>ğ</u>	Through-Right		1230	0	915		1230	010	O	1331	0	669		1331	0	909		1337	0	669
WESTBOUND	Right Left-Through-Right Left-Right		244	1 0 0	244	13	257	257	0	264	1 0 0	264	13	277	1 0 0	277	0	277	1 0 0	277
 	, zon ragin		Nor	th-South:	159	No	rth-South:	169		Nor	th-South:	172		Nort	th-South:	182		Nor	th-South:	182
	CRITICAL VO	DLUMES		ast-West: SUM:	765 924		ast-West: SUM:	765 934			ast-West: SUM:	831 1003			st-West: SUM:	831 1013			ast-West: SUM:	831 1013
	VOLUME/CAPACITY (V/C)	RATIO:			0.616			0.623				0.669				0.675				0.675
V/C	LESS ATSAC/ATCS ADJUS	TMENT:			0.516			0.523				0.569				0.575				0.575
	LEVEL OF SERVICE	E (LOS):			Α			Α				Α				Α				Α

REMARKS:

Version: 1i Beta; 8/4/2011

PROJECT IMPACT

Change in v/c due to project: 0.006
Significant impacted? NO

 $\Delta v/c$ after mitigation: 0.006 Fully mitigated? N/A

7/26/2020-4:59 PM 1 Rinaldi_Indian Hills CMA



Level of Service Workheet



(Circular 212 Method)

I/S #:	North-South Street: Indian H	ills Road		Year of Count: 2019			Ambient Growth: (%):			2	Condu	cted by:	Т	`A	Date: 7/17/2020		7/17/2020)	
2	East-West Street: Rinaldi S	Street		Proje	ction Year	2023		Pea	ak Hour:	PM	Revie	wed by:	J	В	Project:	Ar	arat Hom	es	
	No. of Phases		NB EB			NB EB	0	SB WB	2 0 0 0	NB EB	0	SB WB	2 0 0 0	NB EB	0	SB WB	2 0 0 0		
	ATSAC-1 or ATSAC+ATCS-2? Override Capacity		2			2 0				2				2 0				2	
	EXISTING CONDITION			EXISTI	EXISTING PLUS PROJECT			FUTURE CONDITION W/O PROJECT			FUTUF	RE CONDIT	ION W/ PR	OJECT	FUTURE W/ PROJECT W/ MITIGATI				
	MOVEMENT	No. o Volume Lane		Project Traffic	Total Volume	Lane Volume	Added Volume	Total Volume	No. of Lanes	Lane Volume	Added Volume	Total Volume	No. of Lanes	Lane Volume	Added Volume	Total Volume	No. of Lanes	Lane Volume	
BOUND	 ← Left ← Left-Through ← Through ← Through-Right 	25 0 0 1 0	25 45	0	25 1	25 45	0	27 1	0 0 0	27 49	0	27 1	0 0 0	27 49	0	27 1	0 0 0	27 49	
NORTHBOUND	Right Left-Through-Right Left-Right	19 0 1 0	0	0	19	0	0	21	0 1 0	0	0	21	0 1 0	0	0	21	0 1 0	0	
SOUTHBOUND	↓ Left ↓ Left-Through ↓ Through ↓ Through-Right	278 0 1 1 0 0	278 279	18	296	296 297	0	301	0 1 0 0	301 302	18	319	0 1 0 0	319 320	0	319	0 1 0 0	319 320	
SOUT	→ Right → Left-Through-Right Left-Right	180 1 0 0	148	0	180	148	0	195	1 0 0	160	0	195	1 0 0	160	0	195	1 0 0	160	
DUND	<i>Ĵ</i> Left <i>Ĵ</i> Left-Through → Through	65 1 0 1781 2	65 891	0	65 1781	65 891	0	70 1936	1 0 2	70 968	0	70 1936	1 0 2	70 968	0	70 1936	1 0 2	70 968	
EASTBOUND	→ Through-Right → Right → Left-Through-Right → Left-Right	11 1 0 0	11	0	11	11	0	12	0 1 0 0	12	0	12	0 1 0 0	12	0	12	0 1 0 0	12	
QNC	← Left ← Left-Through	26 1 0	26	0	26	26	0	28	1 0	28	0	28	1 0	28	0	28	1 0	28	
WESTBOUND	← Through ← Through-Right ← Right ← Left-Through-Right	799 2 0 57 1 0	400 57	13	799 70	400 70	0	873 62	2 0 1 0	437 62	13	873 75	2 0 1 0	437 75	0	873 75	2 0 1 0	437 75	
	├ Left-Right CRITICAL VOLUMES	0 North-South: 323 East-West: 917 SUM: 1240			rth-South: East-West: SUM:	341 917 1258	0 North-South: East-West: SUM:		350 996 1346	996 Ea		0 th-South: ast-West: SUM:	368 996 1364	0 North-South: East-West: SUM:		368 996 1364			
V/C	VOLUME/CAPACITY (V/C) RATIO: LESS ATSAC/ATCS ADJUSTMENT:		0.827 0.727			0.839 0.739				0.897 0.797				0.909 0.809				0.909 0.809	
	LEVEL OF SERVICE (LOS):		C			C				C C				D				D.003	

2

REMARKS:

Version: 1i Beta; 8/4/2011

PROJECT IMPACT

Change in v/c due to project: 0.012
Significant impacted? NO

 $\Delta v/c$ after mitigation: 0.012 Fully mitigated? N/A