Appendices

Appendix N Traffic Study

Appendices

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TRAFFIC IMPACT ANALYSIS

NOHL RANCH CONDOMINIUMS ANAHEIM, ORANGE COUNTY, CALIFORNIA



June 2019

TRAFFIC IMPACT ANALYSIS

NOHL RANCH CONDOMINIUMS ANAHEIM, ORANGE COUNTY, CALIFORNIA

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Project No. SRR1801



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- A: Traffic Volume Data
- B: ICU Level of Service Worksheets
- C: HCM Level of Service Worksheets



LIST OF ABBREVIATIONS AND ACRONYMS

ATAM Anaheim Traffic Analysis Model

CalEEMod California Emission Estimator Model
CEQA California Environmental Quality Act

City City of Anaheim

CMP Congestion Management Program

ft foot/feet

ICU intersection capacity utilization

ITE Institute of Transportation Engineers

LOS level of service mph miles per hour

SEIR Supplemental Environmental Impact Report
SWITRS Statewide Integrated Traffic Records System

TIMS Transportation Injury Mapping System

v/c volume-to-capacity [ratio]

VMT vehicle miles traveled



INTRODUCTION

LSA has prepared the following analysis to identify the potential traffic impacts resulting from the development of 60 townhome dwelling units on approximately 3 acres in an area currently occupied by the Serrano Center retail center in the Anaheim Hills neighborhood of Anaheim. LSA has prepared this analysis consistent with the requirements of the City of Anaheim (City) *Criteria for Preparation of Traffic Impact Studies* and applicable provisions of the California Environmental Quality Act (CEQA).

Figure 1 shows the location of the project site. The Serrano Center is currently leasing space to a variety of commercial uses, including a dry cleaner, professional services, professional offices, retail, a children's swim school, a children's dance school, after-school tutoring, and a children's day care. Conversion of the site usage from commercial to residential would require a change to the site's General Plan land use designation. The traffic analysis for the proposed project examines six scenarios:

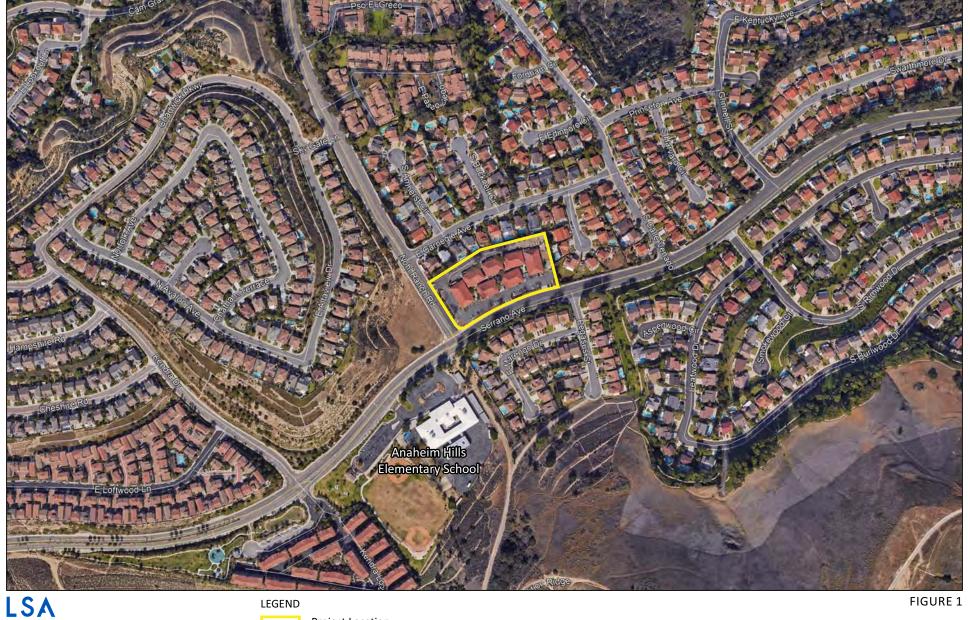
- 1. Existing (2018) Conditions
- 2. Existing (2018) Plus Project Conditions
- 3. Project Opening Year (2022) Baseline Conditions
- 4. Project Opening Year (2022) Plus Project Conditions
- 5. General Plan Buildout (2035) Baseline Conditions
- 6. General Plan Buildout (2035) Plus Project Conditions

This report addresses special considerations, including the potential for cut-through traffic within residential neighborhoods, driveway access, and school proximity.

Project Description

As shown on Figure 1, the existing site is bordered by Nohl Ranch Road to the west, Serrano Avenue to the south, and residential uses to the north and east. The site is accessed by one driveway on Nohl Ranch Road and two driveways on Serrano Avenue.

The proposed project will demolish the existing uses and construct up to 60 townhome dwelling units. The driveway on Nohl Ranch Road will be moved south of its existing location. The eastern driveway on Serrano Avenue will remain in its current location and the western driveway on Serrano Avenue will be closed as part of the project. These driveways are further analyzed in the Access Analysis section of this report. A site plan of the proposed project is illustrated on Figure 2.



Project Location

Nohl Ranch Condominiums **Project Location**

SOURCE: Google Earth, 2018



Nohl Ranch Condominiums Site Plan

N-8

SOURCE: MVE Partners

METHODOLOGY

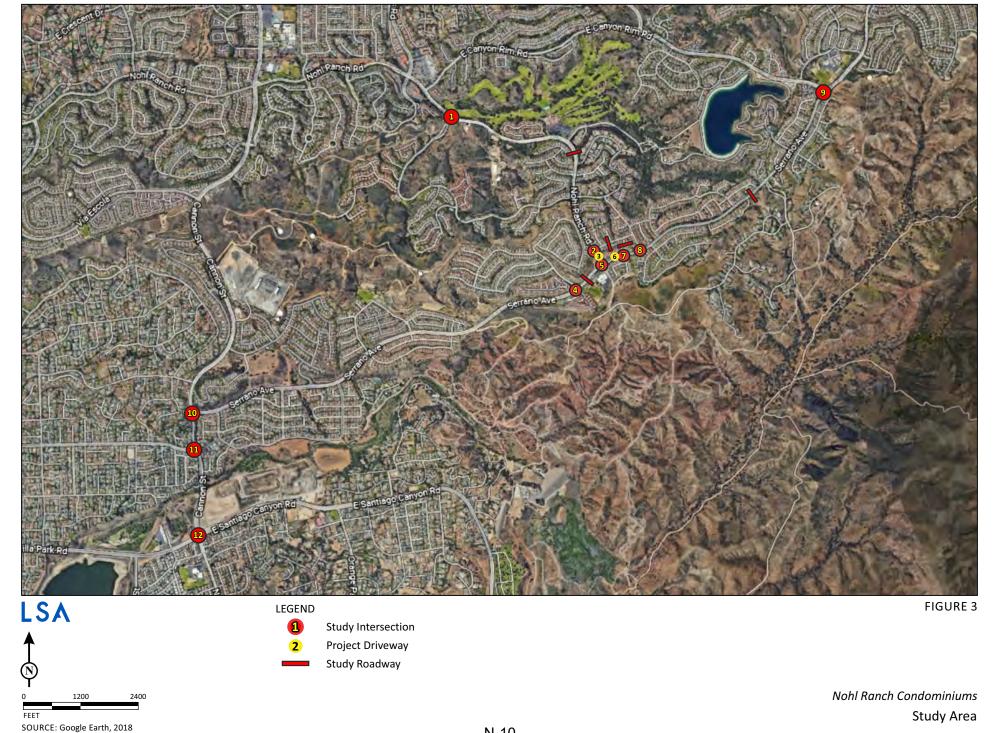
The City's *Criteria for Preparation of Traffic Impact Studies* requires a capacity analysis at intersections and roadway segments where the project contributes at least 51 trips. As demonstrated later in this report, the project is not anticipated to contribute 51 or more trips to any intersection or roadway segment. However, as requested by the City's Traffic Engineering staff, the study area analyzed in this report includes the following intersections and roadway segments. Figure 3 illustrates the locations of the intersections and roadway segments included in the study area. Figure 4 provides the existing geometrics and traffic control devices at each study area intersection.

	Study Area Intersections		Roadway Segments
1.	Nohl Ranch Road/Stage Coach Road (traffic signal)	1.	Nohl Ranch Road (Stage Coach Road to
2.	Nohl Ranch Road/Carnegie Avenue (side-street stop)		Serrano Avenue)
3.	Nohl Ranch Road/Project Driveway (side-street stop)	2.	Serrano Avenue (Kendra Drive to Nohl Ranch
4.	Kendra Drive/Serrano Avenue (Orange) (traffic signal)		Road)
5.	Nohl Ranch Road/Serrano Avenue (traffic signal)	3.	Serrano Avenue (Nohl Ranch Road to Canyon
6.	Project Driveway/Serrano Avenue (side-street stop)		Rim Road)
7.	Pegasus Street/Serrano Avenue (side-street stop)	4.	Carnegie Avenue (Nohl Ranch Road to Calle
8.	Calle Venado/Serrano Avenue (side-street stop)		Venado)
9.	Canyon Rim Road/Serrano Avenue (traffic signal)	5.	Calle Venado (Carnegie Avenue to Serrano
10.	. Cannon Street/Serrano Avenue (Orange) (traffic signal)		Avenue)
11.	. Cannon Street/Taft Avenue (Orange) (traffic signal)		
12.	. Cannon Street/Santiago Canyon Road (Orange) (traffic signal)		

Intersection Level of Service Methodology

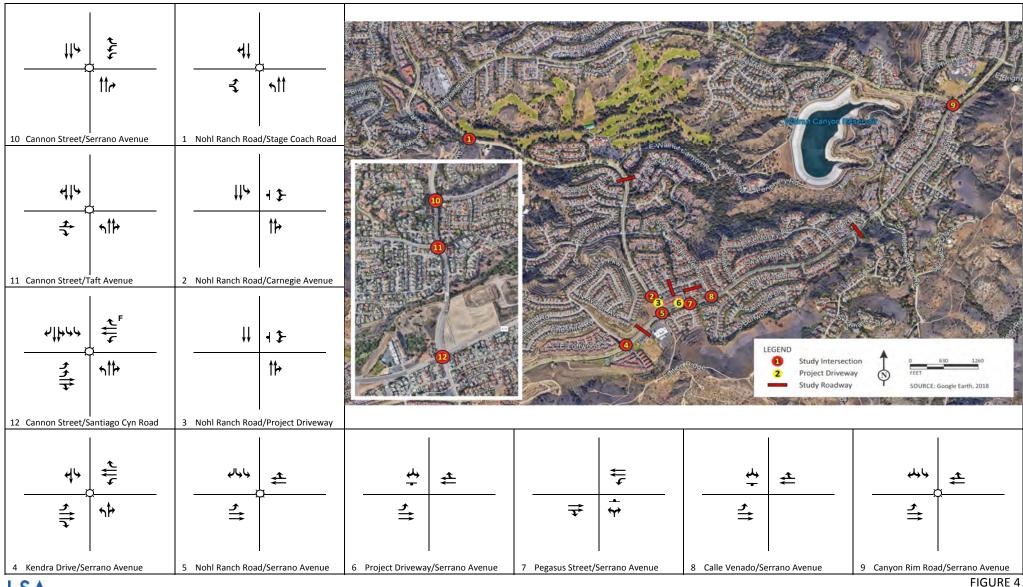
In accordance with the City's *Criteria for Preparation of Traffic Impact Studies* and the City of Orange *Traffic Impact Analysis Guidelines*, the study area intersections were analyzed using intersection capacity utilization (ICU) methodology for signalized intersections (i.e., study area intersections) and *Highway Capacity Manual* (HCM), 6th Edition methodology for unsignalized intersections (i.e., project driveways). Traffix (Version 8.0) and Synchro 10 are the software applications utilized to determine the levels of service (LOS) for signalized and unsignalized intersections, respectively. These programs calculate LOS based on traffic volume and intersection geometry inputs.

The ICU methodology compares the amount of traffic an intersection is able to process (capacity) to the level of traffic during peak hours (volume). The resulting volume-to-capacity (v/c) ratio is expressed in terms of LOS. The HCM methodology calculates the delay experienced by all movements through an intersection. At a two-way, stop-controlled intersection (i.e., an unsignalized intersection where the main street is uncontrolled and traffic on the minor street has to stop before finding a gap to enter the main street), delay is reported for the most delayed approach. LOS criteria for intersections are presented as follows.



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N-10



LSA

LEGEND

- $\mathbf{Z}_{\mathsf{Signal}}$
- Stop Sign
- F Free Right Turn

Nohl Ranch Condominiums
Existing Geometrics

Level of Service Descriptions

LOS is a qualitative assessment of the quantitative effects of such factors as traffic volume, roadway geometrics, speed, delay, and maneuverability on roadway and intersection operations. LOS is assigned along the following letter gradient where LOS A represents free-flow activity, and LOS F represents overcapacity operation:

- LOS A: No approach phase is fully utilized by traffic, and no vehicle waits longer than one red indication. Typically, the approach appears quite open, turns are made easily, and nearly all drivers find freedom of operation.
- LOS B: This service level represents stable operation, where an occasional approach phase is fully utilized, and a substantial number are nearing full use. Many drivers begin to feel restricted within platoons of vehicles.
- LOS C: This level still represents stable operating conditions. Occasionally, drivers may have to wait through more than one red signal indication, and backups may develop behind turning vehicles. Most drivers feel somewhat restricted, but not objectionably so.
- LOS D: This level encompasses a zone of increasing restriction approaching instability at the
 intersection. Delays to approaching vehicles may be substantial during short peaks within the
 peak period; however, enough cycles with lower demand occur to permit periodic clearance of
 developing queues, thus preventing excessive backups.
- LOS E: Capacity occurs at the upper end of this service level. It represents the most vehicles that any particular intersection approach can accommodate. Full utilization of every signal cycle is attained no matter how great the demand.
- LOS F: This level describes forced-flow operations at low speeds, where volumes exceed capacity. These conditions usually result from queues of vehicles backing up from a restriction downstream. Speeds are reduced substantially, and stoppages may occur for short or long periods of time due to the congestion. In the extreme case, speed can drop to zero.

The relationship between LOS and the delay (in seconds) or v/c ratio at unsignalized and signalized intersections is as follows:

Level of Service	Delay (seconds) (HCM Methodology)	Volume-to-Capacity Ratio (ICU Methodology)
Α	≤10.0	< 0.60
В	>10.0 and ≤15.0	0.61-0.70
С	>15.0 and ≤25.0	0.71-0.80
D	>25.0 and ≤35.0	0.81-0.90
E	>35.0 and ≤50.0	0.91–1.00
F	>50.0	> 1.00

HCM = Highway Capacity Manual ICU = intersection capacity utilization

Both the City's guidelines and City of Orange guidelines specify the use of a saturation flow rate of 1,700 vehicles per lane per hour and a clearance interval factor of 5 percent. These guidelines have been applied in the analysis of all signalized study area intersections.

Roadway Segment Level of Service Methodology

Using the same v/c methodology discussed above, daily roadway link v/c ratios were determined using roadway volume data and the theoretical daily capacities provided by the City of Anaheim. The theoretical daily capacity of a roadway is dependent on roadway classification, as shown in the table below.

Type of Arterial	Daily Capacity
Eight Lanes Divided	75,000
Six Lanes Divided	56,300
Four Lanes Divided	37,500
Four Lanes (Undivided)	25,000
Two Lanes (Undivided)	12,500

Source: City of Anaheim.

For roadway segments, the City's General Plan establishes a target of LOS C. If a segment is found to operate at LOS D, E, or F under daily conditions, its operation is also analyzed under peak-hour conditions. If the roadway segment also operates at LOS D, E, or F under peak-hour conditions and project traffic increases the daily v/c ratio by 0.01 or greater, then the project is determined to have a significant impact. The relationship between LOS and the v/c ratio for roadways is shown in the following table.

Level of Service	V/C Ratio
А	<u><</u> 0.60
В	0.61-0.70
С	0.71-0.80
D	0.81-0.90
E	0.91-1.00
F	> 1.00

V/C = volume-to-capacity

Significance Criteria

A transportation impact on an intersection is considered significant in accordance with the following table. The "Final V/C Ratio" includes the future v/c ratio at an intersection, considering traffic from existing conditions, ambient growth, approved/related projects, and the proposed project but without any proposed mitigation. Mitigation is required for any intersection where project traffic is considered to have a significant impact.

Level of Service	Final V/C Ratio	Project-Related Increase in V/C Ratio
С	> 0.701–0.800	≥ 0.050
D	> 0.801–0.900	≥ 0.030
E, F	> 0.901	≥ 0.010

Source: City of Anaheim, *Criteria for Preparation of Traffic Impact Studies*. V/C = volume-to-capacity

For intersections within the jurisdiction of the City of Orange, a v/c ratio of 0.90 (LOS D) is considered the upper limit of satisfactory operations. The City of Orange *Traffic Impact Analysis Guidelines* state that a transportation impact on an intersection shall be deemed significant and require mitigation if the final v/c ratio exceeds 0.90 and the project-related increase in v/c is equal to or greater than 0.01.

The Orange County Congestion Management Program (CMP) stipulates the requirements for maintaining LOS E at CMP intersections. However, no CMP intersections are located near the project.

Project Fair Share

Based on the City's *Criteria for Preparation of Traffic Impact Studies*, for cumulative long-range analysis (i.e., General Plan Build Out), the project participates in future improvements on a fair-share basis. A project's equitable share is to be calculated using the following equation:

$$P = \frac{T}{T_R - T_E}$$

Where:

P = the equitable share for the proposed project's traffic impact

T = the vehicle trips generated by the project during the peak hour of the adjacent street, vehicles per hour at the impacted location

T_B = the General Plan Build Out forecast traffic volume (i.e., the 20-year model or the furthest future model date feasible), vehicles per hour at the impacted location

T_E = the existing traffic volumes plus approved projects that have not been constructed or occupied, vehicles per hour at the impacted location

EXISTING (2018) CONDITIONS

Existing Circulation System

Key roadways in the vicinity of the proposed project are as follows:

• Nohl Ranch Road: Nohl Ranch Road, a north-south roadway located west of and adjacent to the project site, is classified as a Hillside Secondary Arterial by the City's General Plan Circulation Element. Nohl Ranch Road, which is adjacent to the project site, is a four-lane roadway divided by a two-way left-turn lane that acts as a median. Nohl Ranch Road provides direct access to the project site at an unsignalized driveway. The posted speed limit on Nohl Ranch Road is 45 miles



per hour (mph). There are sidewalks provided on both sides of the street. There are no bike lanes, and on-street parking is prohibited.

- **Serrano Avenue:** Serrano Avenue is an east-west roadway located south of the project that provides access to the project site at unsignalized driveways. The route is designated as a Hillside Secondary Arterial by the City's General Plan Circulation Element. The posted speed limit on Serrano Avenue is 45 mph. In the vicinity of the project site, the roadway has four lanes and a two-way left-turn lane that acts as a median. There are sidewalks provided on both sides of the street. There are Class II bike lanes, and on-street parking is prohibited.
- Carnegie Avenue: Carnegie Avenue is a local road that is not included in the City's General Plan Circulation Element. Carnegie Avenue has two undivided lanes and provides direct access to residences within the neighborhood. Sidewalks are provided on both sides of the street, and on-street parking is permitted.
- Calle Venado: Calle Venado is a local road that is not included in the City's General Plan Circulation Element. Calle Venado has two undivided lanes and provides direct access to residences within the neighborhood. Sidewalks are provided on both sides of the street, and on-street parking is permitted.
- Cannon Street: Between the northern city limits and Santiago Canyon Road, the City of Orange
 Master Plan of Streets and Highways classifies Cannon Street as a Major Arterial. Cannon Street
 has a posted speed limit of 45 mph. The roadway has four lanes and a striped median becoming
 a left-turn lane at intersections. A continuous sidewalk is provided on the east side of the
 roadway, while portions of the west side of the roadway have an interrupted sidewalk. Narrow
 Class II bike lanes are provided on both sides of the street. The City of Orange Master Plan of
 Streets and Highways indicates this roadway will be six lanes at General Plan buildout.
- Santiago Canyon Road: The City of Orange Master Plan of Streets and Highways classifies
 Santiago Canyon Road as a Major Arterial. Santiago Canyon Road has a posted speed limit of
 50 mph. The roadway has four lanes and a two-way left-turn lane that acts as a median.
 A continuous sidewalk is provided on the south side of the roadway, while portions of the north
 side of the roadway have an interrupted sidewalk. Class II bike lanes are provided on both sides
 of the street. The City of Orange Master Plan of Streets and Highways indicates this roadway will
 be six lanes at General Plan buildout.

Existing (2018) Intersection Level of Service Analysis

Vehicle turning volumes were collected for the study area intersections during the peak morning (7:00 a.m.–9:00 a.m.) and evening (4:00 p.m.–6:00 p.m.) commute periods. Peak-hour intersection turn volumes were surveyed at the study area intersections in Anaheim on a typical weekday (Wednesday, May 16, 2018) while schools were in session. After a request from the City of Orange, additional turn volumes were collected at intersections in Orange on a typical weekday (Thursday, February 7, 2019) while schools were in session. These volumes were taken in 15-minute increments and then totaled as hourly volumes, which is the standard procedure for volume data collection.

Figure 5 presents the existing a.m. and p.m. peak-hour turn movement volumes for the study area intersections. The traffic volume data sheets are provided in Appendix A.

Table A summarizes the results of the existing a.m. and p.m. peak-hour LOS analysis for the study area intersections. The intersection of Nohl Ranch Road/Serrano Avenue is located adjacent to Anaheim Hills Elementary School and experiences a surge in traffic immediately prior to school start time during the a.m. peak hour. At this intersection, the peak-hour factor (i.e., the concentration of peak-hour traffic volume during the busiest 15 minutes) identified in existing conditions was applied for all a.m. peak-period analyses. School departure occurs outside of the p.m. peak hour, and no peak-hour factors were applied during the p.m. peak hour. All ICU analysis worksheets are provided in Appendix B. All HCM analysis worksheets are provided in Appendix C. As Table A indicates, all study area intersections operate at an acceptable LOS (i.e., LOS D or better) in the a.m. and p.m. peak hours, except for the intersections of Cannon Street/Serrano Avenue (Orange) and Cannon Street/Taft Avenue (Orange).

Table A: Existing (2018) Intersection LOS Summary

Study	Study Intersections AM Peak Hour		AM Peak Hour PM Peak Ho		our	
Area No.	intersections	V/C or Delay	LOS	V/C or Delay	LOS	
1	Nohl Ranch Road/Stage Coach Road	0.319	Α	0.274	Α	
2	Nohl Ranch Road/Carnegie Avenue (u)	11.3 sec	В	10.4 sec	В	
3	Nohl Ranch Road/Project Driveway (u)	9.2 sec	Α	9.2 sec	Α	
4	Kendra Drive/Serrano Avenue	0.411	Α	0.440	Α	
5	Nohl Ranch Road/Serrano Avenue	0.593	Α	0.427	Α	
6	Project Driveway/Serrano Avenue (u)	11.9 sec	В	9.9 sec	Α	
7	Pegasus Street/Serrano Avenue (u)	12.3 sec	В	23.0 sec	С	
8	Calle Venado/Serrano Avenue (u)	11.7 sec	В	17.6 sec	С	
9	Canyon Rim Road/Serrano Avenue	0.488	Α	0.420	Α	
10	Cannon Street/Serrano Avenue (o)	0.816	D	0.991	E	
11	Cannon Street/Taft Avenue (o)	0.946	E	0.957	E	
12	Cannon Street/Santiago Canyon Road (o)	0.761	С	0.774	С	

Unsatisfactory LOS

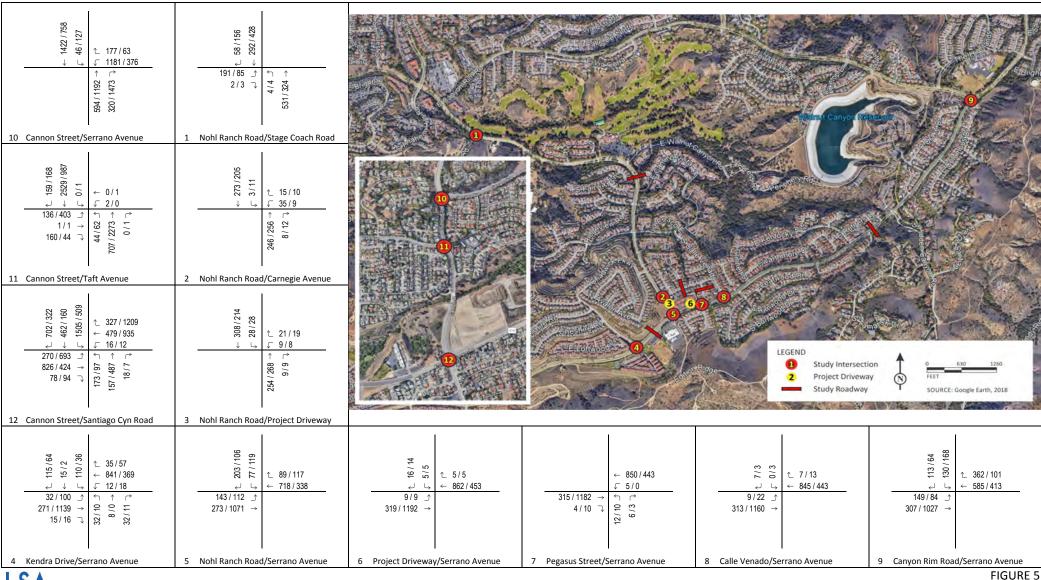
LOS = level of service (u) = unsignalized, worst approach delay shown

sec = seconds V/C = volume-to-capacity

(o) = City of Orange jurisdiction

Existing (2018) Roadway Segment Level of Service Analysis

Roadway LOS is calculated by comparing the daily traffic volume to the theoretical daily capacity of that roadway. Existing daily traffic volumes were collected on a typical weekday (Wednesday, May 16, 2018). Table B summarizes the daily traffic volumes and v/c ratios for the five study area roadway segments in the existing condition. As Table B illustrates, all study area roadway segments operate at an acceptable LOS (i.e., LOS C or better).



LSA

LEGEND

xxx/yyy AM / PM Volume

Nohl Ranch Condominiums
Existing Volume

Table B: Existing (2018) Roadway LOS Comparison

Roadway Segment	Mid-Block Lanes	Capacity	Existing	V/C	LOS
Nohl Ranch Road (Stage Coach Road to Serrano Avenue)	4D	37,500	5,599	0.15	Α
Serrano Avenue (Kendra Drive to Nohl Ranch Road)	4D	37,500	14,121	0.38	Α
Serrano Avenue (Nohl Ranch Road to Canyon Rim Road)	4D	37,500	14,013	0.37	Α
Carnegie Avenue (Nohl Ranch Road to Calle Venado)	2U	12,500	695	0.06	Α
Calle Venado (Carnegie Avenue to Serrano Avenue)	2U	12,500	424	0.03	Α

LOS = level of service V/C = volume-to-capacity

PROJECT OPENING YEAR (2022) BASELINE CONDITIONS

The proposed project is anticipated to be completed by 2022. LSA queried the City's development records and planning staff and identified no nearby (i.e., within 2 miles) approved or pending projects that could be completed and thereby contribute traffic to the study area by 2022. LSA also reviewed the pending land use applications for the City of Orange. Although the Santiago Hills II project is listed as delayed by the City of Orange, LSA added traffic volumes at the study intersections consistent with the project volumes reported in the *Santiago Hills II Traffic Study* (Stantec 2016). In addition to this specific development project, LSA escalated existing roadway and intersection volumes by 1 percent per year, for a total of 4 percent over the next 4 years, in order to account for ambient traffic growth from existing traffic volumes collected in 2018 and early 2019.

Project Opening Year (2022) Baseline Intersection Level of Service Analysis

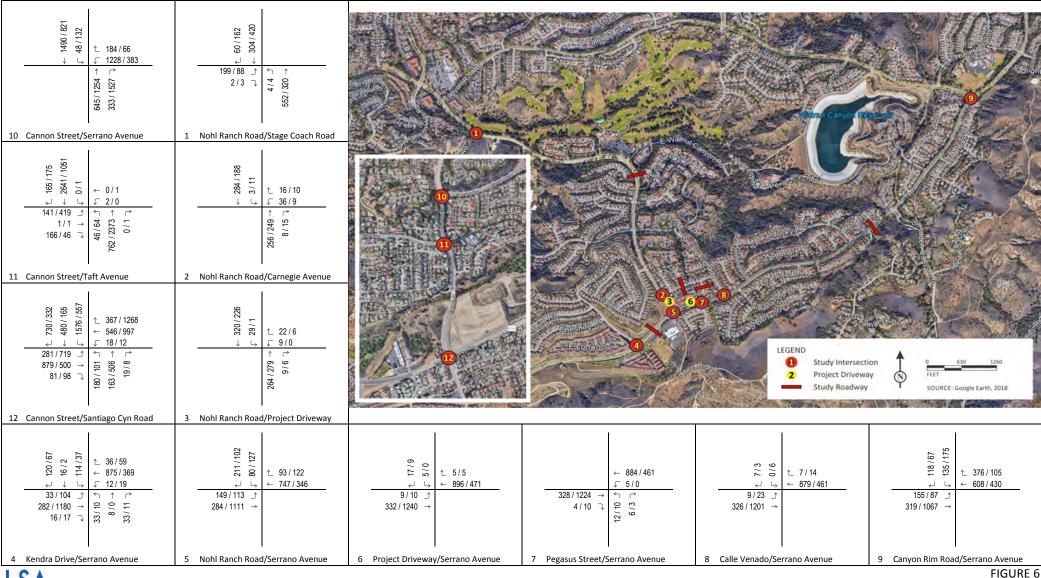
Intersection geometrics at the study area intersections are not anticipated to change by the project opening year. The future a.m. and p.m. peak-hour traffic volumes are shown on Figure 6. Table C summarizes the results of the future a.m. and p.m. peak-hour LOS analysis for the study area intersections. As indicated in Table C, all study area intersections operate at an acceptable LOS in the a.m. and p.m. peak hours in the Project Opening Year (2022) Baseline condition except for the intersections of Cannon Street/Serrano Avenue (Orange) and Cannon Street/Taft Avenue (Orange), which operate at unacceptable LOS in existing conditions.

Project Opening Year (2022) Baseline Roadway Segment Level of Service Analysis

Similar to the intersection analysis, an ambient traffic growth rate of 4 percent (1 percent per year) was applied to daily roadway traffic volumes. Table D summarizes the daily traffic volumes and v/c ratios for the five study area roadway segments in the Future (2022) condition. As Table D illustrates, all study area roadway segments operate at an acceptable LOS (i.e., LOS C or better).

GENERAL PLAN BUILDOUT (2035)

The City performed a citywide analysis of intersections and roadway segments included in the General Plan Circulation Element using the Anaheim Traffic Analysis Model (ATAM) with the certified Housing Opportunities Rezoning Project (Supplemental Environmental Impact Report [SEIR] 346). The City of Anaheim *Housing Opportunities Rezoning Project SEIR 346 Technical Traffic Study* (Iteris 2013) included three of the intersections included in the project's study area but none of the roadway segments.



LSA

LEGEND

xxx/yyy AM/PM Volume

Nohl Ranch Condominiums
Project Opening Year (2022) Volume

Table C: Project Opening Year (2022) Intersection LOS Summary

Study	Intovocations	AM Peak Ho	ur	PM Peak Hour		
Area No.	Intersections	V/C or Delay	LOS	V/C or Delay	LOS	
1	Nohl Ranch Road/Stage Coach Road	0.329	Α	0.276	Α	
2	Nohl Ranch Road/Carnegie Avenue (u)	11.4 sec	В	13.8 sec	В	
3	Nohl Ranch Road/Project Driveway (u)	9.2 sec	Α	9.3 sec	Α	
4	Kendra Drive/Serrano Avenue	0.426	Α	0.454	Α	
5	Nohl Ranch Road/Serrano Avenue	0.615	В	0.437	Α	
6	Project Driveway/Serrano Avenue (u)	12.1 sec	В	10.0 sec	В	
7	Pegasus Street/Serrano Avenue (u)	12.5 sec	В	24.2 sec	С	
8	Calle Venado/Serrano Avenue (u)	11.9 sec	В	18.3 sec	С	
9	Canyon Rim Road/Serrano Avenue	0.505	Α	0.435	Α	
10	Cannon Street/Serrano Avenue (o)	0.849	D	1.026	F	
11	Cannon Street/Taft Avenue (o)	0.986	E	0.996	Е	
12	Cannon Street/Santiago Canyon Road (o)	0.795	С	0.815	D	

Unsatisfactory LOS

LOS = level of service

(u) = unsignalized, worst approach delay shown

sec = seconds V/C = volume-to-capacity

(o) = City of Orange jurisdiction

Table D: Project Opening Year (2022) Roadway LOS Comparison

Roadway Segment	Mid-Block Lanes	Capacity	Opening Year	V/C	LOS
Nohl Ranch Road (Stage Coach Road to Serrano Avenue)	4D	37,500	5,823	0.16	Α
Serrano Avenue (Kendra Drive to Nohl Ranch Road)	4D	37,500	14,686	0.39	Α
Serrano Avenue (Nohl Ranch Road to Canyon Rim Road)	4D	37,500	14,574	0.39	Α
Carnegie Avenue (Nohl Ranch Road to Calle Venado)	2U	12,500	723	0.06	Α
Calle Venado (Carnegie Avenue to Serrano Avenue)	2U	12,500	441	0.04	Α

LOS = level of service

V/C = volume-to-capacity

General Plan Buildout (2035) Intersection Level of Service Analysis

The City applied the ATAM traffic growth rates to the existing turn volume data collected for this analysis and provided General Plan horizon traffic volumes for the three study intersections included in the traffic model (Nohl Ranch Road/Stage Coach Road, Nohl Ranch Road/Serrano Avenue, and Canyon Rim Road/Serrano Avenue). The Santiago Hills II Traffic Study (Stantec 2016) included one additional study intersection and provided City of Orange General Plan horizon traffic volumes for that intersection (Canon Street/Santiago Canyon Road). LSA compared the General Plan horizon traffic volumes at these intersections to the existing traffic volumes and identified the growth in traffic volume passing through the intersections. This same growth in traffic was then applied to study intersections adjacent to the intersections for which General Plan horizon traffic volumes were provided.



As noted previously, the City of Orange Master Plan of Streets and Highways indicates that the roadways of Cannon Street and Santiago Canyon Road will be six lanes at General Plan buildout. Through traffic lanes at the intersections of Cannon Street/Serrano Avenue (Orange), Cannon Street/Taft Avenue (Orange), and Cannon Street/Santiago Canyon Road (Orange) were adjusted in the General Plan Buildout (2035) analysis to account for the wider roadways. The City of Orange also reported that a project to add a second northbound right-turn lane at Cannon Street/Serrano Avenue (Orange) is in process but may not be completed by the 2022 opening year for the proposed project. This improvement was also included in the General Plan (2035) analysis.

Figure 7 displays the resulting General Plan Buildout (2035) traffic volumes at all study intersections. Table E summarizes the results of the a.m. and p.m. peak-hour LOS analysis for the study area intersections. As indicated in Table E, all study area intersections operate at an acceptable LOS in the a.m. and p.m. peak hours in the General Plan Buildout (2035) condition with planned improvements.

Table E: General Plan Buildout (2035) Intersection LOS Summary

Study	Interceptions	AM Peak Ho	ur	PM Peak Hour		
Area No.	Intersections	V/C or Delay	LOS	V/C or Delay	LOS	
1	Nohl Ranch Road/Stage Coach Road	0.316	Α	0.261	Α	
2	Nohl Ranch Road/Carnegie Avenue (u)	11.9 sec	В	10.2 sec	В	
3	Nohl Ranch Road/Project Driveway (u)	9.4 sec	Α	9.2 sec	Α	
4	Kendra Drive/Serrano Avenue	0.478	Α	0.463	Α	
5	Nohl Ranch Road/Serrano Avenue	0.714	С	0.439	Α	
6	Project Driveway/Serrano Avenue (u)	13.3 sec	В	10.1 sec	В	
7	Pegasus Street/Serrano Avenue (u)	13.2 sec	В	25.8 sec	D	
8	Calle Venado/Serrano Avenue (u)	13.0 sec	В	19.7 sec	С	
9	Canyon Rim Road/Serrano Avenue	0.535	Α	0.487	Α	
10	Cannon Street/Serrano Avenue (o)	0.746	С	0.558	Α	
11	Cannon Street/Taft Avenue (o)	0.753	С	0.801	D	
12	Cannon Street/Santiago Canyon Road (o)	0.795	С	0.818	D	

[☐] Unsatisfactory LOS

General Plan Buildout (2035) Roadway Segment Level of Service Analysis

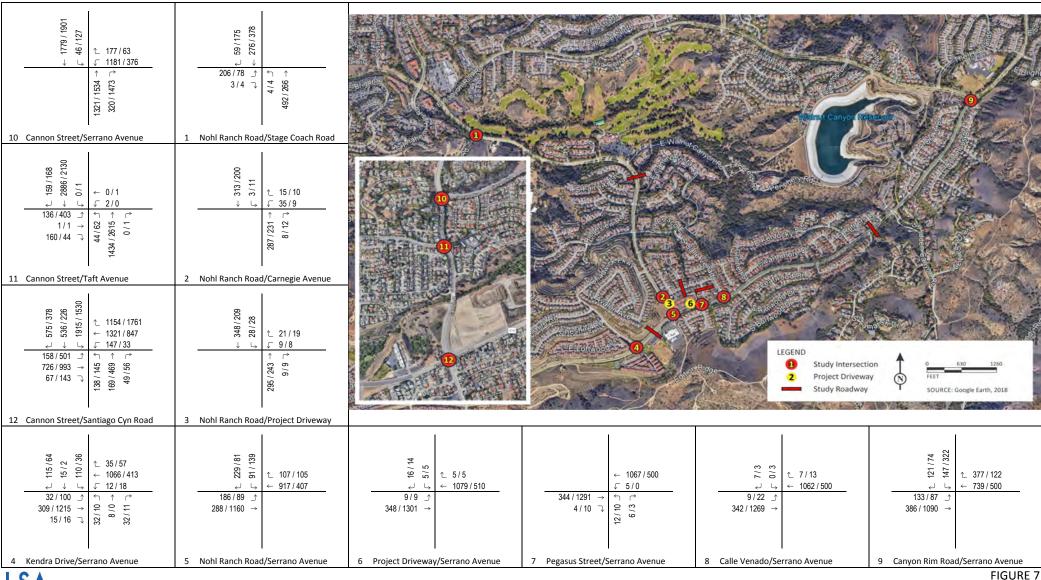
To calculate daily roadway volumes for segments not included in the previous General Plan forecasting, LSA determined the ratio between daily and peak-hour roadway volumes in the existing condition and applied that ratio to peak-hour roadway volumes evident from intersection turn volumes. Table F summarizes the daily traffic volumes and v/c ratios for the five study area roadway segments in the General Plan Buildout (2035) condition. As Table F illustrates, all study area roadway segments operate at an acceptable LOS (i.e., LOS C or better).

LOS = level of service sec = seconds

⁽u) = unsignalized, worst approach delay shown

V/C = volume-to-capacity

⁽o) = City of Orange jurisdiction



LSA

LEGEND

xxx/yyy AM/PM Volume

Nohl Ranch Condominiums General Plan (2035) Volume

Table F: General Plan Buildout (2035) Roadway LOS Comparison

Roadway Segment	Mid-Block Lanes	Capacity	General Plan Buildout	V/C	LOS
Nohl Ranch Road (Stage Coach Road to Serrano Avenue)	4D	37,500	4,954	0.13	Α
Serrano Avenue (Kendra Drive to Nohl Ranch Road)	4D	37,500	17,891	0.48	Α
Serrano Avenue (Nohl Ranch Road to Canyon Rim Road)	4D	37,500	17,407	0.46	Α
Carnegie Avenue (Nohl Ranch Road to Calle Venado)	2U	12,500	693	0.06	Α
Calle Venado (Carnegie Avenue to Serrano Avenue)	2U	12,500	422	0.03	Α

LOS = level of service V/C = volume-to-capacity

EFFECTS OF THE PROJECT

Trip Generation

The Nohl Ranch Condominiums project considers the demolition of the Serrano Center and construction of up to 60 residential dwelling units on the approximately 3-acre site. The daily and peak-hour trips for the project were generated using trip rates contained in the Institute of Transportation Engineers (ITE) *Trip Generation Manual*, Tenth Edition (2017). The project trip generation being added to the roadway network is presented in Table G. As Table G shows, the 60 dwelling units are anticipated to generate 439 trips per day, of which 28 would occur in the a.m. peak hour and 34 would occur in the p.m. peak hour.

Table G: Existing Trips and Trip Generation

Land Use	Size	Unit	ADT	AN	Л Peak Н	lour	PM Peak Hour		
Land Ose	Size	Onit	ADI	In	Out	Total	In	Out	Total
Trip Rates (Land Use Code)									
Multifamily Housing (220) ¹		DU	7.32	0.11	0.35	0.46	0.35	0.21	0.56
Project Trip Generation									
Nohl Ranch Condominiums (220)	60 DU		439	7	21	28	21	13	34
Existing Trip Generation to be Replaced ²									
Serrano Center	Surve	yed	(1,003)	(51)	(51)	(102)	(51)	(46)	(97)
School Traffic Diverted to Adjacent Intersect	ions³								
Drop-off/Pick-up	Obser	ved	126	20	20	40	0	0	0
Net New Trip Generation			(438)	(24)	(10)	(34)	(30)	(33)	(63)

 $^{^{1}}$ Trip rates referenced from the ITE *Trip Generation Manual,* 10th Edition (2017)

DU = dwelling unit TSF = thousand square feet

Table G also shows the existing trips generated by the Serrano Center that will be removed from the roadway network. The Serrano Center is currently leasing space to a variety of commercial uses, including a grocery store, dry cleaner, professional services, professional offices, children's swim school, children's dance school, after-school tutoring, and children's day care. Rather than using ITE trip generation rates to estimate the trip generation for these various uses, LSA determined trip generation by counting vehicles entering and exiting the Serrano Center driveways. Driveway

² Total trips observed at shopping center driveways

³ Trips observed at shopping center driveways that would remain on the roadways with closure of the shopping center

surveys were conducted on a typical weekday (Wednesday, May 16, 2018) for a 24-hour period. The results of these surveys showed that the Serrano Center currently generates 1,003 trips per day, of which 102 occur in the a.m. peak hour and 97 occur in the p.m. peak hour, which is about 75 percent of the trips (i.e., compared to the trips estimated by ITE trip rates) that would be generated if the shopping center were fully occupied. It should be noted that the surveys revealed that the western driveway on Serrano Avenue (which would be closed by the proposed project) is used infrequently during the a.m. and p.m. peak hours.

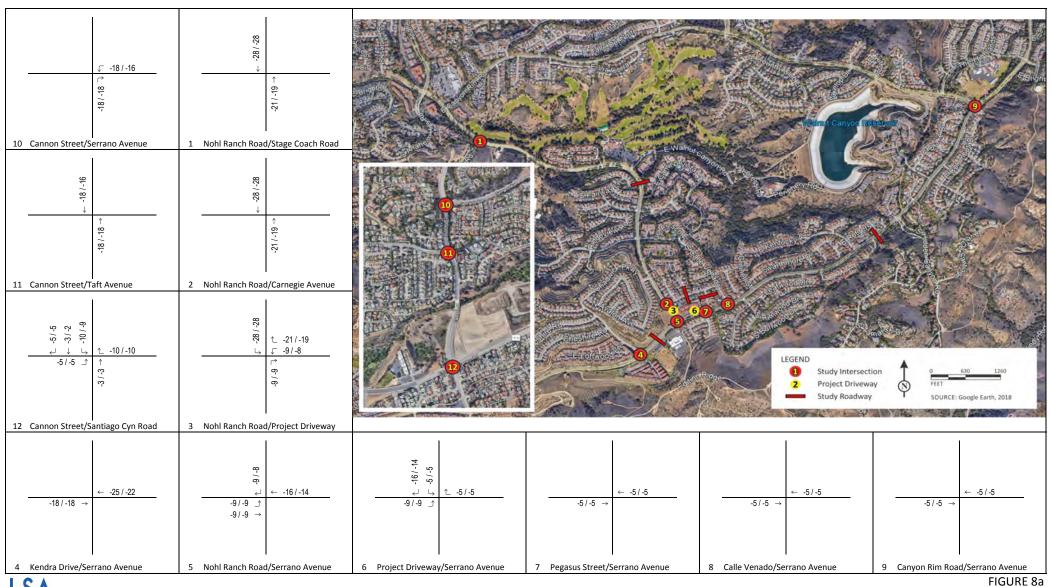
As noted previously, some of the trips into and out of the Serrano Center driveways in the existing condition are the result of parents dropping off and picking up students of Anaheim Hills Elementary School. These trips will not be eliminated from the roadway network with the closure of the Serrano Center. In order to determine how many of the trips into the Serrano Center driveways will be redirected rather than eliminated, LSA conducted site visits and observed the Serrano Center at times of school drop-off and pick-up. Based on these observations, LSA estimated that approximately 20 vehicles were using the Serrano Center parking lot to drop off students (which occurs during the a.m. peak hour) and approximately 43 vehicles were using the Serrano Center parking lot to pick up students (which occurs outside of the p.m. peak hour). Because each vehicle generates both an inbound and outbound trip, school trips account for 126 trips per day, of which 40 occur in the a.m. peak hour, 86 occur outside of the peak commute hours (7:00 a.m. to 9:00 a.m. and 4:00 p.m. to 6:00 p.m.), and none occur in the p.m. peak hour.

As Table G shows, the townhome project trip generation would be less than the observed trip generation of the existing Serrano Center, even when taking into account school drop-off and pick-up trips into the shopping center. As such, the daily and peak-hour trip generation for the project is negative.

Trip Distribution and Assignment

Trip distribution defines the regional percentage origins/destinations for a project. To determine trip distribution for the proposed project, LSA considered the existing traffic patterns adjacent to the project site. The land uses surrounding the project site are largely residential, and residents of the proposed project are likely to have similar traffic patterns. Traffic from the project site was distributed 15 percent north, 60 percent west (and then south on Cannon Street), and 25 percent east.

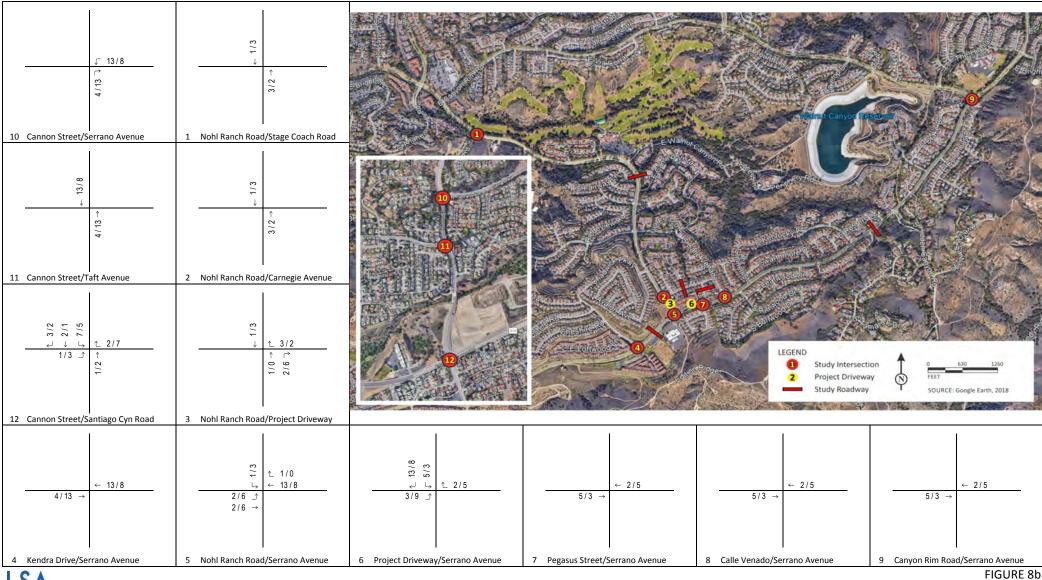
Trips were assigned to travel paths based on accessibility of the site. As discussed in the Access Analysis section later in this report, left-turn movements will be prohibited in or out of the project driveway on Nohl Ranch Road with implementation of the project. As also discussed in the Access Analysis section, left-turn movements will be permitted at the project driveway on Serrano Avenue. Figure 8a illustrates the project trip assignment resulting from the subtraction of existing traffic generated by the Serrano Center and redirecting school trips currently terminating at the Serrano Center to terminate at the school instead. Figure 8b illustrates the project trip assignment for residential project trips accounting for future turn restrictions. Figure 8c illustrates the net trips resulting from the project. As Figure 8c shows, eliminating Serrano Center traffic and adding project traffic results in lower traffic volumes for most turn movements.



LEGEND

xxx / yyy AM / PM Volume

Nohl Ranch Condominiums
Serrano Center Trip Assignment

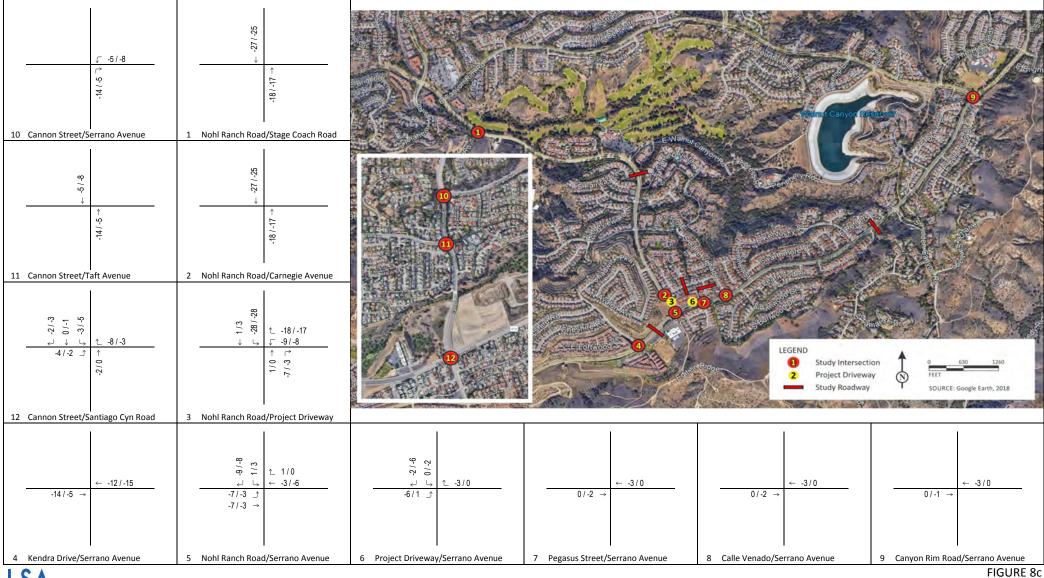


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LEGEND

xxx / yyy AM / PM Volume

Nohl Ranch Condominiums
Residential Project Trip Assignment



LEGEND

AM / PM Volume XXX / YYY

> **Nohl Ranch Condominiums** Net Project Trip Assignment

EXISTING PLUS PROJECT CONDITION

The project trips were added to the existing traffic volumes at the study intersections and roadway segments. Figure 9 shows the resulting Existing Plus Project a.m. and p.m. peak-hour traffic volumes.

Existing Plus Project Intersection Level of Service Analysis

Table H summarizes the results of the Existing Plus Project a.m. and p.m. peak-hour LOS analysis for all study area intersections. As Table H indicates, all study area intersections are anticipated to operate at an acceptable LOS (i.e., LOS D or better) in the a.m. and p.m. peak hours except for the intersections of Cannon Street/Serrano Avenue (Orange) and Cannon Street/Taft Avenue (Orange), which operate at unacceptable LOS in existing conditions.

Table H: Existing Plus Project Intersection LOS Summary

			Exis	ting		Plus P	oject		Change		
Study Area	Intersections	AM Peak Hour		PM Peal	PM Peak Hour		AM Peak Hour		ak r	V/C or Delay	
No.		V/C or Delay	LOS	V/C or Delay	LOS	V/C or Delay	LOS	V/C or Delay	LOS	AM	PM
1	Nohl Ranch Rd./Stage Coach Rd.	0.319	Α	0.274	Α	0.313	Α	0.267	Α	(0.006)	(0.007)
2	Nohl Ranch Rd./Carnegie Ave. (u)	11.3 sec	В	10.4 sec	В	11.0 sec	В	10.2 sec	В	(0.3) sec	(0.2) sec
3	Nohl Ranch Rd./Project Dwy. (u)	9.2 sec	Α	9.2 sec	Α	9.1 sec	Α	9.1 sec	Α	(0.1) sec	(0.1) sec
4	Kendra Dr./Serrano Ave.	0.411	Α	0.440	Α	0.408	Α	0.439	Α	(0.003)	(0.001)
5	Nohl Ranch Rd./Serrano Ave.	0.593	Α	0.427	Α	0.581	Α	0.422	Α	(0.012)	(0.005)
6	Project Dwy./Serrano Ave. (u)	11.9 sec	В	9.9 sec	Α	11.9 sec	В	9.9 sec	Α	0.0 sec	0.0 sec
7	Pegasus St./Serrano Ave. (u)	12.3 sec	В	23.0 sec	С	12.3 sec	В	22.9 sec	С	0.0 sec	(0.1) sec
8	Calle Venado/Serrano Ave. (u)	11.7 sec	В	17.6 sec	С	11.7 sec	В	17.5 sec	С	0.0 sec	(0.1)sec
9	Canyon Rim Rd./Serrano Ave.	0.488	Α	0.420	Α	0.487	Α	0.420	Α	(0.001)	0.000
10	Cannon St./Serrano Ave. (o)	0.816	D	0.991	E	0.814	D	0.988	E	(0.002)	(0.003)
11	Cannon St./Taft Ave. (o)	0.946	E	0.957	Е	0.945	E	0.956	Е	(0.001)	(0.001)
12	Cannon St./Santiago Cyn Rd. (o)	0.761	С	0.774	С	0.761	С	0.772	С	0.000	(0.002)

LOS = level of service sec = seconds (u) = unsignalized, worst approach delay shown

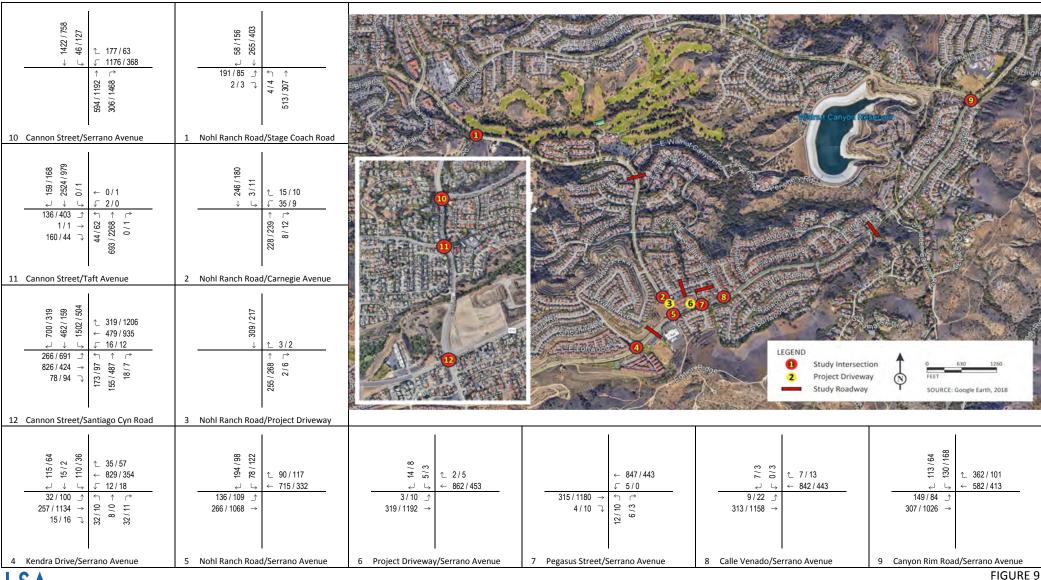
V/C = volume-to-capacity

(o) = City of Orange jurisdiction

Based on empirical data collected at the driveways of the existing retail center, the project is forecast to generate less traffic. As a result of generating less traffic, the project is forecast to reduce v/c ratios and delay at the intersections included in the study area. Based on the City's criteria for determining significant traffic impacts (i.e., City of Anaheim criteria for intersections in Anaheim and City of Orange criteria for intersections in Orange as described in the Methodology section of this report), the proposed project is not expected to result in a significant impact at any of the study area intersections.

Existing Plus Project Roadway Segment Level of Service Analysis

Table I summarizes the daily traffic volumes and v/c ratios for the five study area roadway segments with the addition of project traffic. As Table I illustrates, all study area roadway segments operate at an acceptable LOS (i.e., LOS C or better).



LSA

LEGEND

xxx/yyy AM/PM Volume

Nohl Ranch Condominiums
Existing Plus Project Volume

Table I: Existing Plus Project Roadway LOS Comparison

Roadway Segment	Mid-Block Lanes	Capacity	Existing	V/C	LOS	Plus Project	v/c	LOS	Change
Nohl Ranch Road (Stage Coach Road to Serrano Avenue)	4D	37,500	5,599	0.15	Α	5,182	0.14	Α	(0.01)
Serrano Avenue (Kendra Drive to Nohl Ranch Road)	4D	37,500	14,121	0.38	Α	14,196	0.38	Α	0.00
Serrano Avenue (Nohl Ranch Road to Canyon Rim Road)	4D	37,500	14,013	0.37	Α	14,023	0.37	Α	0.00
Carnegie Avenue (Nohl Ranch Road to Calle Venado)	2U	12,500	695	0.06	Α	695	0.06	Α	0.00
Calle Venado (Carnegie Avenue to Serrano Avenue)	2U	12,500	424	0.03	A	424	0.03	Α	0.00

LOS = level of service

PROJECT OPENING YEAR (2022) PLUS PROJECT CONDITION

Traffic generated by the project was added to the Project Opening Year (2022) traffic volumes at each study area intersection and roadway segment. Figure 10 illustrates the resulting Project Opening Year (2022) plus project a.m. and p.m. peak-hour traffic volumes.

Project Opening Year (2022) Plus Project Intersection Level of Service Summary Analysis

Table J summarizes the results of the Project Opening Year (2022) plus project a.m. and p.m. peak-hour LOS analysis for all study area intersections. As Table J indicates, all study area intersections are anticipated to operate at an acceptable LOS (i.e., LOS D or better) in the a.m. and p.m. peak hours except for the intersections of Cannon Street/Serrano Avenue (Orange) and Cannon Street/Taft Avenue (Orange), which operate at unacceptable LOS in existing conditions.

Table J: Project Opening Year (2022) Plus Project Intersection LOS Summary

			Base	eline			Plus P	oject		Change	
Study Area	Intersections	AM Peak Hour		PM Peal	PM Peak Hour		AM Peak Hour		ak r	V/C or Delay	
No.		V/C or Delay	LOS	V/C or Delay	LOS	V/C or Delay	LOS	V/C or Delay	LOS	AM	PM
1	Nohl Ranch Rd./Stage Coach Rd.	0.329	Α	0.276	Α	0.324	Α	0.276	Α	(0.005)	0.000
2	Nohl Ranch Rd./Carnegie Ave. (u)	11.4 sec	В	13.8 sec	В	11.2 sec	В	10.3 sec	В	(0.2) sec	(3.5) sec
3	Nohl Ranch Rd./Project Dwy. (u)	9.2 sec	Α	9.3 sec	Α	9.1 sec	Α	9.2 sec	Α	(0.1) sec	(0.1) sec
4	Kendra Dr./Serrano Ave.	0.426	Α	0.454	Α	0.422	Α	0.454	Α	(0.004)	0.000
5	Nohl Ranch Rd./Serrano Ave.	0.615	В	0.437	Α	0.602	В	0.437	Α	(0.013)	0.000
6	Project Dwy./Serrano Ave. (u)	12.1 sec	В	10.0 sec	В	12.1 sec	В	9.9 sec	Α	0.0 sec	(0.1) sec
7	Pegasus St./Serrano Ave. (u)	12.5 sec	В	24.2 sec	С	12.5 sec	В	24.1 sec	С	0.0 sec	(0.1) sec
8	Calle Venado/Serrano Ave. (u)	11.9 sec	В	18.3 sec	С	11.9 sec	В	18.3 sec	С	0.0 sec	0.0 sec
9	Canyon Rim Rd./Serrano Ave.	0.505	Α	0.435	Α	0.504	Α	0.435	Α	(0.001)	0.000
10	Cannon St./Serrano Ave. (o)	0.849	D	1.026	F	0.848	D	1.026	F	(0.001)	0.000
11	Cannon St./Taft Ave. (o)	0.986	Е	0.996	Е	0.984	Е	0.996	E	(0.002)	0.000
12	Cannon St./Santiago Cyn Rd. (o)	0.795	C	0.815	D	0.795	С	0.815	D	0.000	0.000

LOS = level of service

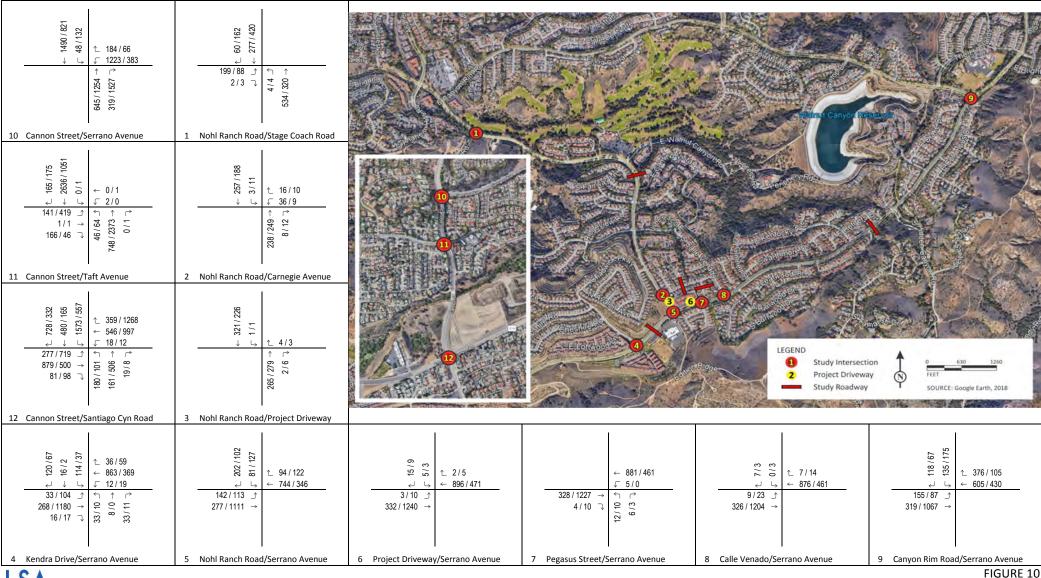
sec = seconds

(o) = City of Orange jurisdiction

(u) = unsignalized, worst approach delay shown

V/C = volume-to-capacity

V/C = volume-to-capacity ratio



LSA

LEGEND

xxx / yyy AM / PM Volume

Nohl Ranch Condominiums
Project Opening Year (2022) Plus Project Volume

Based on empirical data collected at the driveways of the existing retail center, the project is forecast to generate less traffic. As a result of generating less traffic, the project is forecast to reduce v/c ratios and delay at the intersections included in the study area. Based on the City's criteria for determining significant traffic impacts (i.e., City of Anaheim criteria for intersections in Anaheim and City of Orange criteria for intersections in Orange), the proposed project is not expected to result in a significant impact at any of the study area intersections.

Project Opening Year (2022) Plus Project Roadway Segment Level of Service Analysis

Table K summarizes the daily traffic volumes and v/c ratios for the five study area roadway segments with the addition of project traffic. As Table K illustrates, all study area roadway segments operate at an acceptable LOS (i.e., LOS C or better).

Table K: Project Opening Year (2022) Plus Project Roadway LOS Comparison

Roadway Segment	Mid-Block Lanes	Capacity	Baseline	v/c	LOS	Plus Project	v/c	LOS	Change
Nohl Ranch Road (Stage Coach Road to Serrano Avenue)	4D	37,500	5,823	0.16	Α	5,406	0.14	Α	(0.02)
Serrano Avenue (Kendra Drive to Nohl Ranch Road)	4D	37,500	14,686	0.39	Α	14,761	0.39	Α	0.00
Serrano Avenue (Nohl Ranch Road to Canyon Rim Road)	4D	37,500	14,574	0.39	Α	14,584	0.39	Α	0.00
Carnegie Avenue (Nohl Ranch Road to Calle Venado)	2U	12,500	723	0.06	Α	723	0.06	Α	0.00
Calle Venado (Carnegie Avenue to Serrano Avenue)	2U	12,500	441	0.04	Α	441	0.04	Α	0.00

LOS = level of service

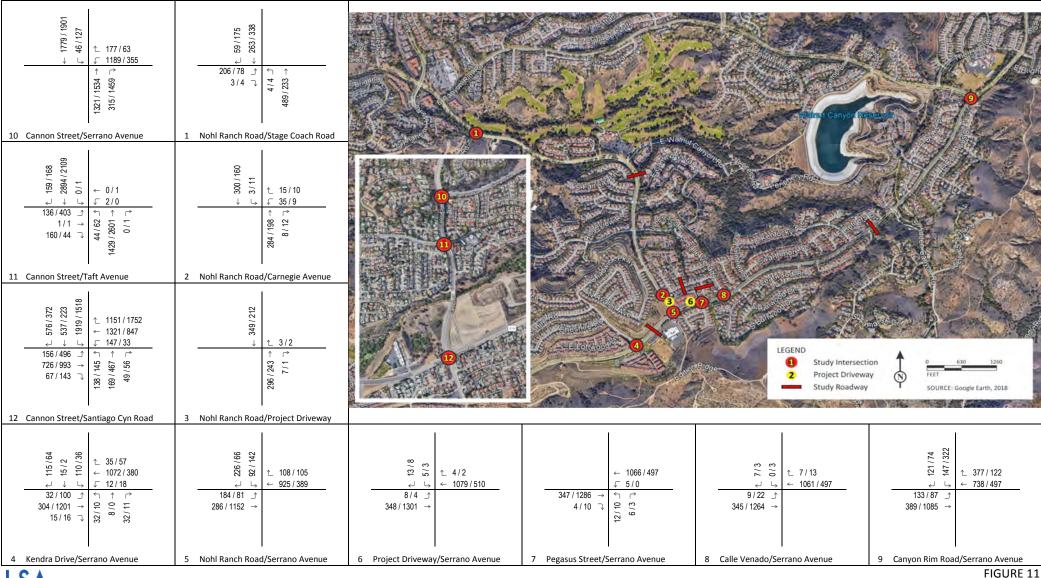
V/C = volume-to-capacity ratio

GENERAL PLAN BUILDOUT (2035) PLUS PROJECT CONDITION

This study found that that the Serrano Center experiences additional a.m. peak-hour trips due to its proximity to a school and that the Serrano Center generates approximately 75 percent of the p.m. peak-hour trip generation of a fully occupied center (i.e., when empirical trip generation data are compared to the trip generation predicted by ITE trip rates) in the existing condition. However, in ATAM and General Plan conditions, the Serrano Center would have the potential to generate traffic as a fully occupied shopping center. Table L presents the trip generation comparison under these conditions. Net project trips were added to the study area resulting in the General Plan (2035) Plus Project traffic volumes illustrated on Figure 11.

General Plan Buildout (2035) Plus Project Intersection Level of Service Summary Analysis

Table M summarizes the results of the General Plan (2035) plus project a.m. and p.m. peak-hour LOS analysis for all study area intersections. As Table M indicates, all study area intersections are anticipated to operate at an acceptable LOS (i.e., LOS D or better) in the a.m. and p.m. peak hours with roadway improvements planned by General Plan Buildout.



LSA

LEGEND

xxx/yyy AM / PM Volume

Nohl Ranch Condominiums General Plan (2035) Plus Project Volume

Table L: General Plan Buildout Trip Generation Comparison

Land Use	Size	Unit	ADT	A۱	/I Peak H	lour	PM Peak Hour			
Land Ose	Size		ADI	In	Out	Total	In	Out	Total	
Trip Rates (Land Use Code)										
Multifamily Housing (220) ¹		DU	7.32	0.11	0.35	0.46	0.35	0.21	0.56	
Shopping Center (820) ¹		TSF	37.75	0.58	0.36	0.94	1.83	1.98	3.81	
Project Trip Generation										
Nohl Ranch Condominiums (220)	60	DU	439	7	21	28	21	13	34	
Existing Trip Generation to Be Replaced										
Serrano Center (820)	42.526	TSF	1,605	25	15	40	78	84	162	
Net New Trip Generation			(1,166)	(18)	6	(12)	(57)	(71)	(128)	

Trip rates referenced from the ITE *Trip Generation Manual*, 10th Edition (2017).

ADT = average daily trips ITE = Institute of Transportation Engineers

DU = dwelling unit TSF = thousand square feet

Table M: General Plan (2035) Plus Project Intersection LOS Summary

			Base	eline			Plus P	oject		Change	
Study Area	Intersections	AM Peak Hour		PM Peal	PM Peak Hour		AM Peak Hour		ak r	V/C or Delay	
No.		V/C or Delay	LOS	V/C or Delay	LOS	V/C or Delay	LOS	V/C or Delay	LOS	AM	PM
1	Nohl Ranch Rd./Stage Coach Rd.	0.316	Α	0.261	Α	0.315	Α	0.249	Α	(0.001)	(0.012)
2	Nohl Ranch Rd./Carnegie Ave. (u)	11.9 sec	В	10.2 sec	В	11.8 sec	В	9.9 sec	Α	(0.1) sec	(1.6) sec
3	Nohl Ranch Rd./Project Dwy. (u)	9.4 sec	Α	9.2 sec	Α	9.3 sec	Α	9.0 sec	Α	(0.1) sec	(0.2) sec
4	Kendra Dr./Serrano Ave.	0.478	Α	0.463	Α	0.479	Α	0.459	Α	0.001	(0.004)
5	Nohl Ranch Rd./Serrano Ave.	0.714	C	0.439	Α	0.714	С	0.431	Α	0.000	(0.008)
6	Project Dwy./Serrano Ave. (u)	13.3 sec	В	10.1 sec	В	13.2 sec	В	10.1 sec	В	(0.1) sec	0.0 sec
7	Pegasus St./Serrano Ave. (u)	13.2 sec	В	25.8 sec	D	13.2 sec	В	25.7 sec	D	0.0 sec	(0.1) sec
8	Calle Venado/Serrano Ave. (u)	13.0 sec	В	19.7 sec	С	11.1 sec	В	19.7 sec	С	(1.9) sec	0.0 sec
9	Canyon Rim Rd./Serrano Ave.	0.535	Α	0.487	Α	0.535	Α	0.486	Α	0.000	(0.001)
10	Cannon St./Serrano Ave. (o)	0.746	С	0.558	Α	0.749	С	0.554	Α	0.003	(0.004)
11	Cannon St./Taft Ave. (o)	0.753	С	0.801	D	0.755	С	0.798	С	0.002	(0.003)
12	Cannon St./Santiago Cyn Rd. (o)	0.795	С	0.818	D	0.795	С	0.813	D	0.000	(0.005)

LOS = level of service (u) = unsignalized, worst approach delay shown

sec = seconds V/C = volume-to-capacity

(o) = City of Orange jurisdiction

Based on empirical data collected at the driveways of the existing retail center, the project is forecast to generate less traffic. As a result of generating less traffic, the project is forecast to reduce v/c ratios and delay at the intersections included in the study area. Based on the City's criteria for determining significant traffic impacts (i.e., City of Anaheim criteria for intersections in Anaheim and City of Orange criteria for intersections in Orange), the proposed project is not expected to result in a significant impact at any of the study area intersections.

General Plan (2035) Plus Project Roadway Segment Level of Service Analysis

Daily roadway traffic volumes were developed from intersection turn movements using the same methodology discussed for General Plan (2035) roadway volumes. Table N summarizes the daily traffic volumes and v/c ratios for the five study area roadway segments, with the addition of project traffic. As Table N illustrates, all study area roadway segments operate at an acceptable LOS (i.e., LOS C or better).

Table N: General Plan (2035) Plus Project Roadway LOS Comparison

Roadway Segment	Mid-Block Lanes	Capacity	Baseline	V/C	LOS	Plus Project	V/C	LOS	Change
Nohl Ranch Road (Stage Coach Road to Serrano Avenue)	4D	37,500	4,954	0.13	Α	4,202	0.11	Α	(0.02)
Serrano Avenue (Kendra Drive to Nohl Ranch Road)	4D	37,500	17,891	0.48	Α	17,386	0.46	Α	(0.02)
Serrano Avenue (Nohl Ranch Road to Canyon Rim Road)	4D	37,500	17,407	0.46	Α	17,325	0.46	Α	0.00
Carnegie Avenue (Nohl Ranch Road to Calle Venado)	2U	12,500	693	0.06	Α	693	0.06	Α	0.00
Calle Venado (Carnegie Avenue to Serrano Avenue)	2U	12,500	422	0.03	Α	422	0.03	Α	0.00

LOS = level of service

V/C = volume-to-capacity ratio

SPECIAL CONSIDERATIONS

Access Analysis

Serrano Avenue Driveway

Figure 2 shows that the project would close the western driveway on Serrano Avenue and would not alter the location of the eastern driveway. Left-turn movements into and out of the driveways on Serrano Avenue are supported by a two-way left-turn median on Serrano Avenue. The two-way left-turn median provides a space for vehicles turning into a driveway to wait out of the flow of traffic before turning into the driveway. Similarly, vehicles turning left out of a driveway can complete their turn in two parts: (1) entering the two-way left-turn median during an appropriate gap in westbound traffic, and (2) exiting the two-way left-turn median into travel lanes during an appropriate gap in eastbound traffic.

At the eastern driveway, however, the two-way left-turn median is also used by vehicles turning left out of Pegasus Street. The distance between the eastern driveway and Pegasus Street is approximately 75 ft. This is sufficient distance to accommodate two vehicles, but it is also close enough to warrant considering the likelihood that two vehicles would enter at the same time from opposite directions. Adequate sight distance is provided at this location, and vehicles exiting Pegasus Street and vehicles exiting the project driveway can see each other.

Pegasus Street serves a neighborhood of 33 homes. The traffic volume generated by 33 homes is relatively low as evidenced from the existing traffic volume data. The existing traffic data showed a maximum of 12 left-turns from this neighborhood in the busiest hour (i.e., the a.m. peak hour). This equates to one vehicle every 5 minutes entering the two-way left-turn median during the highest 1-hour period of the day. The analysis presented above estimated that during the a.m. peak hour, the proposed project would generate five outbound trips headed eastbound on Serrano Avenue.

This equates to one vehicle every 12 minutes entering the two-way left-turn median. The infrequency of use by vehicles in either direction makes it less likely that two vehicles would use the lane simultaneously.

The use of the two-way left-turn median by vehicles coming from Pegasus Street would not change as a result of the project. The estimated volume of vehicles that would enter the two-way left-turn median from the project driveway is similar to the existing volume. LSA queried collision statistics to determine whether the spacing between the two points accessing the two-way left-turn median presents an issue. LSA examined the Statewide Integrated Traffic Records System (SWITRS) data within the Transportation Injury Mapping System (TIMS) for 2011–2017. During this period, there were no reported collisions at Pegasus Street or the Serrano Center driveway. Given the low frequency of use of the two-way left-turn median and the lack of collisions historically from similar traffic volumes, LSA recommends that full access at the eastern driveway remain as it is in the existing condition and as analyzed in this report.

Neighborhood Traffic

Access to the driveway on Nohl Ranch Road is to be limited to right-in/right-out with implementation of the project. Left turns out of the project driveway on Serrano Avenue could continue as they do in the existing condition. Figure 12 illustrates the paths of travel possible to and from the project site. As Figure 12 shows, travel to and from the site would not require travel through residential neighborhoods except in one scenario. The analysis above demonstrates that the project driveway on Serrano Avenue is anticipated to operate at a satisfactory level of service. Because excess delay for exiting vehicles is not anticipated at this driveway, it is anticipated that most drivers traveling east on Serrano Avenue would choose the shorter path and turn left out of the driveway. However, in a worst case where residents choose to not turn left out of the project driveway onto Serrano Avenue, then travel along Carnegie Avenue and Calle Venado would be necessary to travel eastbound on Serrano Avenue because no westbound left-turn lane is provided at the intersection of Nohl Ranch Road/Serrano Avenue, and U-turns are explicitly prohibited at this location. All other movements to or from the project site would occur along arterial roadways.

Table O compares intersection performance in the project opening year under a theoretical condition where traffic exiting the project and traveling east uses Carnegie Avenue and Calle Venado. Table P presents roadway v/c ratios under the same theoretical condition.

Table O: Project Opening Year (2022) With Worst-Case Neighborhood

Traffic Intersection LOS Summary

Carrelia	Study		Base	eline			Plus Pr	oject		Cha	inge
•	Area Intersections		AM Peak Hour PM Peak Hour		AM Peal	ak Hour PM Peak Hour			V/C or Delay		
No.	intersections	V/C or	LOS	V/C or	100	V/C or	100	V/C or	100	A B 4	DNA
NO.		Delay	LUS	Delay	LOS	Delay	LOS	Delay	LOS	AM	PM
2	Nohl Ranch Road/Carnegie Avenue (u)	11.4 sec	В	13.8 sec	В	11.2 sec	В	10.3 sec	В	(0.2) sec	(3.5) sec
8	Calle Venado/Serrano Avenue (u)	11.9 sec	В	18.3 sec	С	17.5 sec	С	21.3 sec	С	5.6 sec	3.0 sec

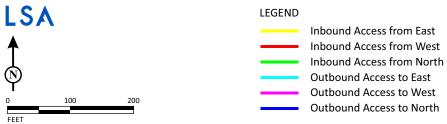
LOS = level of service

(u) = unsignalized, worst approach delay shown

sec = seconds

V/C = volume-to-capacity





Nohl Ranch Condominiums
Travel Paths To and From
the Project Site

SOURCE: Google Earth, 2018

Table P: Project Opening Year (2022) With Worst-Case Neighborhood Traffic Roadway LOS Comparison

Roadway Segment	Mid-Block Lanes	Capacity	Baseline	v/c	LOS	Plus Project	v/c	LOS	Change
Carnegie Avenue (Nohl Ranch Road to Calle Venado)	2U	12,500	723	0.06	Α	778	0.06	Α	0.00
Calle Venado (Carnegie Avenue to Serrano Avenue)	2U	12,500	441	0.04	Α	496	0.04	Α	0.00

LOS = level of service

V/C = volume-to-capacity ratio

The analysis above demonstrates that with the addition of project trips using Carnegie Avenue and Calle Venado to travel eastbound on Serrano Avenue, the intersections of Nohl Ranch Road/ Carnegie Avenue and Calle Venado/Serrano Avenue would operate at an acceptable LOS. The analysis also showed that daily traffic volume on Carnegie Avenue and Calle Venado would be well below the capacity of a two-lane undivided roadway. However, these would still represent trips through a neighborhood that neither originate from nor are destined to the neighborhood.

The City's *Criteria for Preparation of Traffic Impact Studies* do not provide thresholds for determining when traffic added to neighborhood streets represents an impact. In the worst-case scenario where all outbound trips traveling eastbound on Serrano Avenue use Carnegie Avenue and Calle Venado, Table P shows that the project would result in potentially up to 55 additional daily trips. This represents less 0.5 percent of the capacity of the roadway. During the peak hour, the project would result in five additional trips, again representing less than 0.5 percent of the capacity of a travel lane. Given the low capacity of use by the project, it appears that the potential to impact the roadways is low.

School Parking

As Figure 1 shows, the project site is located near Anaheim Hills Elementary School. The Serrano Center parking lot is currently used by some parents to park while dropping off or picking up students at Anaheim Hills Elementary School. While the Serrano Center is not obligated to provide parking for off-site uses and signs are posted at the parking lot entrances indicating that parking is for the Serrano Center only, the public parks within this lot under the assumption they are visiting businesses within the Serrano Center as well. As the public will not be permitted to park within the Nohl Ranch Condominiums, the school drop-off/pick-up activity occurring today will not continue in the future. This report analyzes the special consideration of the elimination of school parking on the site.

Similar to many areas adjacent to schools, Serrano Avenue experiences brief periods of high traffic volume and queueing around the beginning and end of the school day. LSA performed site visits at the beginning and end of the school day to identify how many school trips would need to be accounted for in the analysis of project traffic conditions. During those site visits, LSA also made notes on school conditions. This section presents those observations for informational purposes because the school is an adjacent land use.



Anaheim Hills Elementary School has a driveway on Serrano Avenue approximately 310 ft west of the intersection of Nohl Ranch Road/Serrano Avenue. The school provides approximately 900 ft of queuing space (holding approximately 36 vehicles) inside the property for vehicles dropping off or picking up students. When queues exceed the internal storage, vehicles begin to queue on Serrano Avenue. Eastbound Serrano Avenue has a right-turn pocket leading to the school driveway that has approximately 180 ft of storage, which is enough room for seven cars. Westbound Serrano Avenue has a two-way left-turn median with sufficient room for six vehicles. When queues exceed this storage outside of travel lanes, vehicles begin to impact other movements. Eastbound traffic on Serrano Avenue has approximately 485 ft from the school driveway to Kendra Drive. This provides space for an additional 12 vehicles that would block one of the two through lanes. On westbound Serrano Avenue, an additional 2 vehicles cross the double yellow lines of the two-way left-turn median (an area normally functioning as the bay taper for eastbound left turns onto Nohl Ranch Road), then 4 more vehicles could queue in one of the two through lanes before reaching the intersection of Nohl Ranch Road/Serrano Avenue.

Some vehicles were observed traveling westbound on Serrano Avenue, then making a U-turn at Kendra Drive to join the eastbound right-turn queue into the school. This may have been because the eastbound direction has more space for vehicles to queue. It may also be because right-turning vehicles are not delayed by through traffic and the eastbound right-turn queue moves with greater frequency than the westbound left-turn queue.

Anaheim Hills Elementary School begins instruction for all K-6 students at 8:00 a.m. At 7:40 a.m., a maximum of three vehicles were waiting in the westbound left-turn queue on Serrano Avenue. Between 7:45 a.m. and 7:55 a.m., the westbound left-turn queue filled all available space in the two-way left-turn median with one vehicle extending into the through travel lane. By 8:00 a.m., however, this queue had receded to a single vehicle waiting to turn left. A queue for eastbound right-turning traffic was observed but not quantified. Some vehicles arrived and parked at the Serrano Center before school started. About half of the vehicles using the Serrano Center parking lot arrived between 7:50 a.m. and 7:55 a.m. The remainder of the arrivals were evenly split between the 5-minute period before and the 5-minute period after this.

Instruction ends at 2:20 p.m. for all K-6 students. By 2:20 p.m., the queues on Serrano Avenue were 4 vehicles in the westbound direction and 6 vehicles in the eastbound direction. By 2:25 p.m., the queues reached their peak at 9 vehicles in the westbound direction (i.e., 1 vehicle extending into the through travel lane) and 10 vehicles in the eastbound direction (i.e., at least 3 vehicles extending into the through travel lane). Queues were contained within the turn lanes by 2:30 p.m., and no vehicles were queued onto Serrano Avenue by 2:35 p.m. More vehicles parked at the Serrano Center after school than before school. Some vehicles had arrived by 2:05 p.m., and vehicles continued to arrive until 2:30 p.m. It should be noted that some of the parents who parked at the Serrano Center may have picked up students from the school and walked them to the Serrano Center to one of the children-oriented businesses located in the shopping center.

The traffic impact analysis for the project (i.e., the construction of 60 residential dwelling units) took into account the redirection of school traffic volume from the Serrano Center to the school. The LOS analysis concluded that the study intersections would function at acceptable levels with project traffic and the redirection of school traffic. It is difficult, however, to quantify the effects on

queueing because drop-off and pick-up activity can redistribute itself temporally. Drivers typically arriving at a particular time may choose to arrive earlier or later in response to queueing and time spent waiting. Queues may rebalance to existing levels.

Vehicle Miles Traveled

On December 28, 2018, the California Office of Administrative Law cleared the revised CEQA Guidelines for use. Among the changes to the guidelines was the removal of vehicle delay and LOS from consideration under CEQA. With the adopted guidelines, transportation impacts are to be evaluated based on a project's effect on vehicle miles traveled (VMT). Lead agencies are allowed to opt-in to the revised transportation guidelines, but the new guidelines must be used starting July 1, 2020. The City has not adopted revised traffic impact analysis guidelines, and analysis of vehicle LOS, as provided above, remains the appropriate method for determining a project's transportation impact. However, a disclosure of the project's effect on VMT is provided here for informational purposes.

The California Emissions Estimator Model (CalEEMod) is a sketch model used statewide to estimate pollutant and greenhouse gas emissions for various aspects of construction and operation of a proposed project. The *Technical Advisory on Evaluating Transportation Impacts in CEQA* (Governor's Office of Planning and Research 2018) identifies sketch models (and CalEEMod specifically) as potential tools for analyzing a project's VMT. The air quality analysis used the trip generation identified in Table G to analyze the emissions produced by vehicles traveling to/from the Serrano Center and the emissions produced by vehicles traveling to/from the project. These calculations include estimates of VMT in existing conditions and with the project. Table Q provides a comparison of VMT.

Table Q: Project Effect on Vehicle Miles Traveled

	Annual VMT
Existing Serrano Center	3,130,943
Proposed Nohl Ranch Condominiums	1,325,859
Net Change	(1,805,084)

VMT = vehicle miles traveled

As Table Q shows, the project is anticipated to produce lower VMT than the existing land use. California Public Resources Code Section 15064.3(b)(1) states that projects that decrease VMT compared to existing conditions would be presumed to have a less than significant transportation impact. While significance criteria for impacts related to VMT are not yet adopted by the City, when significance criteria are adopted, and if those significance criteria are consistent with State law, then the proposed project would likely be determined to have a less than significant impact.

CONCLUSION

This traffic analysis considered the effects of closing the Serrano Center and constructing in its place 60 townhome dwelling units on nine intersections and five roadway segments near the project site. The analysis also considered the effect of redirecting school drop-off and pick-up parking that occurs



on the site. Serrano Center trip generation was calculated from traffic volume observed at the center's driveways. School-related activity at the Serrano Center was observed. Even accounting for the number of school trips, which would be redirected along and not eliminated from the roadway network, the proposed project would generate fewer trips than the existing uses.

The impact analysis concluded that the project is not anticipated to result in a significant traffic impact at any study intersection or roadway segment. No CMP facilities are located in the vicinity of the project.

The traffic analysis further examined access to the site. Full access is currently provided onto Serrano Avenue. Some vehicles exiting the eastern driveway on Serrano Avenue would likely be destined for eastbound Serrano Avenue and would turn left if full access is preserved. Traffic volume turning left from the adjacent Pegasus Street is low, and left-turning traffic from the project driveway is anticipated to be similar to existing conditions. The opportunity for conflict from these two low-volume movements is low. This is supported by the lack of reported collision data at this location. Given the absence of reported collisions at this location, this traffic analysis recommended preserving full access on Serrano Avenue.

If left turns from the project driveway onto Serrano Avenue are prohibited, this could result in some traffic traveling through the neighborhood along Carnegie Avenue and Calle Venado to travel eastbound on Serrano Avenue. This traffic volume would use less than 0.5 percent of the capacity of the roadways. Intersection performance that was analyzed using this scenario determined that the intersections would continue to function at a satisfactory LOS.

The traffic analysis provided an informational item describing queuing on Serrano Avenue during school start and end times. School queues extend off site and can temporarily block one of the two through lanes in either direction. Existing queuing is limited to approximately 15 minutes before or after school. Redirecting school traffic from the Serrano Center to the school may extend the amount of time queueing occurs, or vehicles may spread out their arrival times to readjust queues.

The traffic analysis included an assessment of project-generated VMT. The analysis found that the proposed project is estimated to generate lower VMT than the Serrano Center it would replace. Because the project would result in decreased VMT, the CEQA Guidelines suggest that if VMT-based significance criteria were adopted by the City, the project would be found to have a less than significant impact.

REFERENCES

City of Anaheim. Criteria for Preparation of Traffic Impact Studies.

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State of California, Governor's Office of Planning and Research. 2018. *Technical Advisory on Evaluating Transportation Impacts in CEQA*. December.

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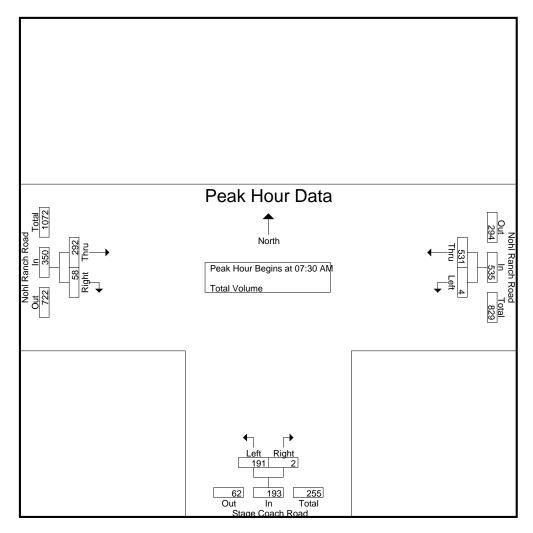
APPENDIX A

TRAFFIC VOLUME DATA

City of Anaheim N/S: Stage Coach Road E/W: Nohl Ranch Road Weather: Clear

File Name: 08_ANA_Stage Coach Road_Nohl Ranch AM Site Code: 00318390

Start Date : 5/16/2018 Page No : 2



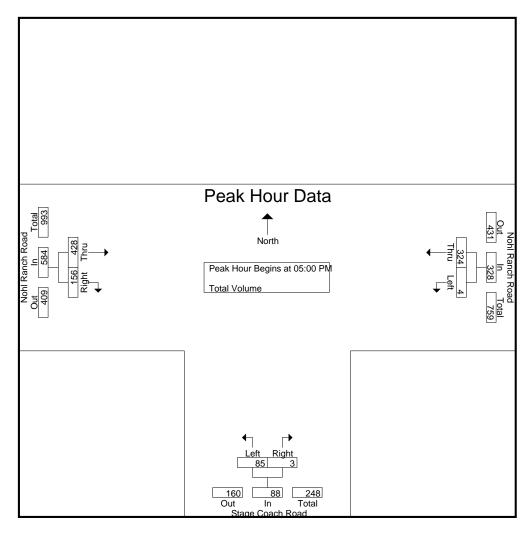
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

reak Hour for Lacif A	ppioacii beg	ii io at.							
	07:30 AM			07:00 AM			07:30 AM		
+0 mins.	1	150	151	39	1	40	67	10	77
+15 mins.	1	118	119	43	1	44	83	14	97
+30 mins.	2	149	151	64	1	65	66	16	82
+45 mins.	0	114	114	51	1	52	76	18	94
Total Volume	4	531	535	197	4	201	292	58	350
% App. Total	0.7	99.3		98	2		83.4	16.6	
PHF	.500	.885	.886	.770	1.000	.773	.880	.806	.902

City of Anaheim N/S: Stage Coach Road E/W: Nohl Ranch Road Weather: Clear

File Name: 08_ANA_Stage Coach Road_Nohl Ranch PM Site Code: 00318390

Start Date : 5/16/2018 Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

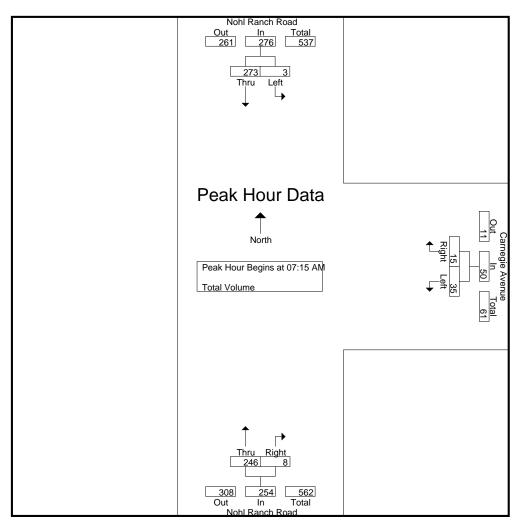
reak Hour for Lacif A	pproacri begi	nis at.							
	04:45 PM			04:15 PM			05:00 PM		
+0 mins.	2	101	103	18	1	19	89	30	119
+15 mins.	0	81	81	26	2	28	106	46	152
+30 mins.	2	81	83	14	0	14	110	42	152
+45 mins.	2	88	90	30	1	31	123	38	161
Total Volume	6	351	357	88	4	92	428	156	584
% App. Total	1.7	98.3		95.7	4.3		73.3	26.7	
PHF	.750	.869	.867	.733	.500	.742	.870	.848	.907

City of Anaheim N/S: Nohl Ranch Road E/W: Carnegie Avenue Weather: Clear

File Name : 02_ANA_Nohl Ranch_Carnegie AM Site Code : 00318390

Start Date : 5/16/2018

Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

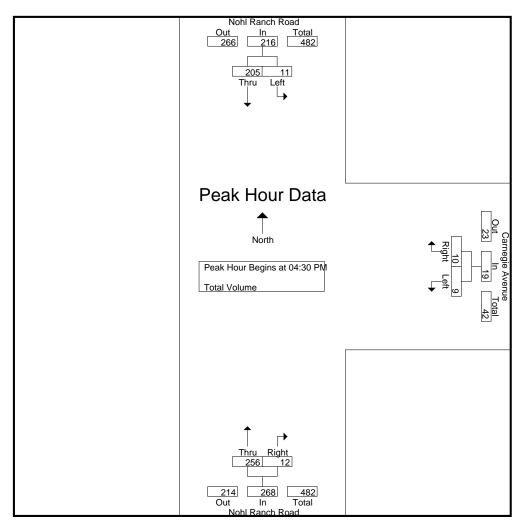
reak noul for Each A	prioacii begi	no at.							
	07:00 AM			07:15 AM			07:30 AM		
+0 mins.	2	44	46	4	4	8	45	0	45
+15 mins.	0	50	50	10	4	14	84	7	91
+30 mins.	2	77	79	16	4	20	75	1	76
+45 mins.	1	100	101	5	3	8	43	0	43
Total Volume	5	271	276	35	15	50	247	8	255
% App. Total	1.8	98.2		70	30		96.9	3.1	
PHF	.625	.678	.683	.547	.938	.625	.735	.286	.701

City of Anaheim N/S: Nohl Ranch Road E/W: Carnegie Avenue Weather: Clear

File Name : 02_ANA_Nohl Ranch_Carnegie PM Site Code : 00318390

Start Date : 5/16/2018

Page No : 2



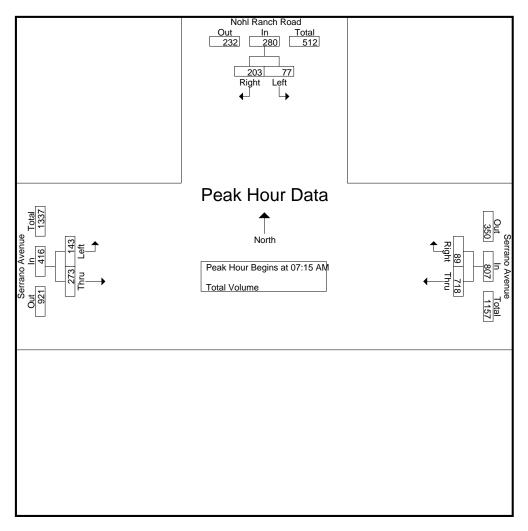
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

reak Hour for Lacif A	pproacri begi	ns at.							
	05:00 PM			04:00 PM			04:30 PM		
+0 mins.	3	48	51	2	5	7	60	2	62
+15 mins.	4	66	70	1	2	3	66	4	70
+30 mins.	4	60	64	1	2	3	63	5	68
+45 mins.	2	59	61	3	4	7	67	1	68
Total Volume	13	233	246	7	13	20	256	12	268
% App. Total	5.3	94.7		35	65		95.5	4.5	
PHF	.813	.883	.879	.583	.650	.714	.955	.600	.957

City of Anaheim N/S: Nohl Ranch Road E/W: Serrano Avenue Weather: Clear

File Name: 03_ANA_Nohl Ranch_Serrano AM Site Code: 00318390

Start Date : 5/16/2018 Page No : 2



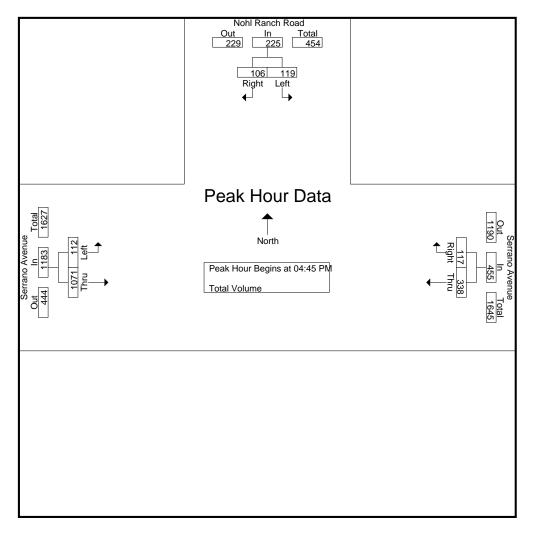
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

reak noul for Each A	privacii begii	is at.							
	07:15 AM			07:00 AM			07:15 AM		
+0 mins.	16	38	54	213	28	241	26	41	67
+15 mins.	23	56	79	166	22	188	25	70	95
+30 mins.	26	71	97	183	22	205	53	98	151
+45 mins.	12	38	50	193	22	215	39	64	103
Total Volume	77	203	280	755	94	849	143	273	416
% App. Total	27.5	72.5		88.9	11.1		34.4	65.6	
PHF	.740	.715	.722	.886	.839	.881	.675	.696	.689

City of Anaheim N/S: Nohl Ranch Road E/W: Serrano Avenue Weather: Clear

File Name: 03_ANA_Nohl Ranch_Serrano PM Site Code: 00318390

Start Date : 5/16/2018 Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

reak noul for Each Ap	privacii begi	iis ai.							
	04:45 PM			04:45 PM			04:45 PM		
+0 mins.	23	27	50	82	39	121	25	272	297
+15 mins.	34	20	54	71	31	102	27	255	282
+30 mins.	36	28	64	104	25	129	39	257	296
+45 mins.	26	31	57	81	22	103	21	287	308
Total Volume	119	106	225	338	117	455	112	1071	1183
% App. Total	52.9	47.1		74.3	25.7		9.5	90.5	
PHF	.826	.855	.879	.813	.750	.882	.718	.933	.960

Location: Anaheim
N/S: Nohl Ranch Road
E/W: Serrano Avenue



Date: 5/16/2018 Day: Wednesday

PEDESTRIANS

Γ	North Leg Nohl Ranch Road	East Leg Serrano Avenue	South Leg Dead End	West Leg Serrano Avenue]
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
7:00 AM	0	0	0	0	0
7:15 AM	0	0	0	0	0
7:30 AM	0	2	3	0	5
7:45 AM	0	75	59	0	134
8:00 AM	0	8	8	0	16
8:15 AM	0	4	2	0	6
8:30 AM	0	0	2	0	2
8:45 AM	0	0	0	0	0
TOTAL VOLUMES:	0	89	74	0	163

	North Leg Nohl Ranch Road	East Leg Serrano Avenue	South Leg Dead End	West Leg Serrano Avenue	
	Pedestrians	Pedestrians	Pedestrians	Pedestrians	
4:00 PM	0	2	2	0	4
4:15 PM	0	0	1	0	1
4:30 PM	0	0	0	0	0
4:45 PM	0	0	0	0	0
5:00 PM	0	0	0	0	0
5:15 PM	0	2	2	0	4
5:30 PM	0	0	0	0	0
5:45 PM	0	1	1	0	2
TOTAL VOLUMES:	0	5	6	0	11

Location: Anaheim
N/S: Nohl Ranch Road
E/W: Serrano Avenue



Date: 5/16/2018 Day: Wednesday

BICYCLES

		Southbound			Westbound			Northbound			Eastbound		
	No	ohl Ranch Ro	ad	Se	errano Aveni	ıe		Dead End		Se	errano Aveni	ıe	
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
7:00 AM	0	0	0	0	0	0	0	0	0	1	0	0	1
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0	0	0	0	0	1	0	0	1

		Southbound			Westbound			Northbound	i	_			
	No	ohl Ranch Ro	ad	Se	errano Aveni	ne er		Dead End		Si	errano Aveni	ue	
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	1	0	0	0	0	0	0	1
5:30 PM	0	0	0	0	0	1	0	0	0	0	0	0	1
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	0	0	0	0	0	2	0	0	0	0	0	0	2

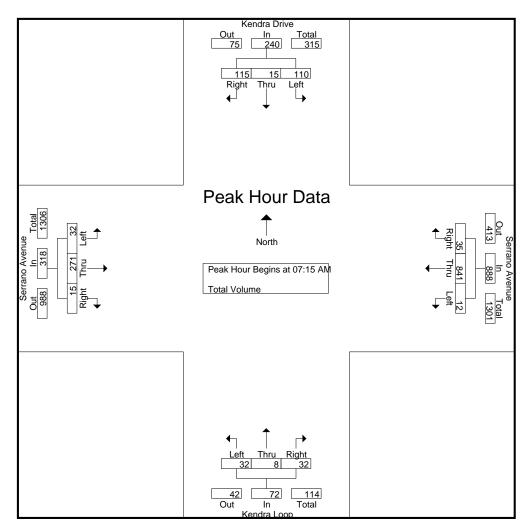
City of Orange N/S: Kendra Drive/Kendra Loop

E/W: Serrano Avenue Weather: Clear

File Name: 01_ORN_Kendra_Serrano AM Site Code: 00318390

Start Date : 5/16/2018

Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for	Each Ap	oproacl	n Begin	s at:												
	07:00 AM				07:00 AM	1			07:00 AM	1			07:30 AN	Л		
+0 mins.	15	0	32	47	1	238	3	242	7	0	12	19	10	80	0	90
+15 mins.	9	0	28	37	0	190	2	192	5	0	4	9	8	80	10	98
+30 mins.	30	0	37	67	3	199	5	207	5	0	6	11	11	52	4	67
+45 mins.	62	15	22	99	8	225	17	250	15	7	14	36	14	42	8	64
Total Volume	116	15	119	250	12	852	27	891	32	7	36	75	43	254	22	319
% App. Total	46.4	6	47.6		1.3	95.6	3		42.7	9.3	48		13.5	79.6	6.9	
PHF	.468	.250	.804	.631	.375	.895	.397	.891	.533	.250	.643	.521	.768	.794	.550	.814

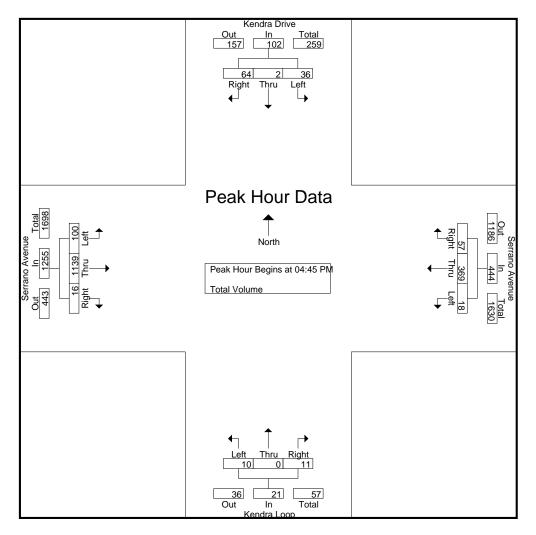
City of Orange N/S: Kendra Drive/Kendra Loop

E/W: Serrano Avenue Weather: Clear

File Name: 01_ORN_Kendra_Serrano PM Site Code: 00318390

Start Date : 5/16/2018

Page No : 2

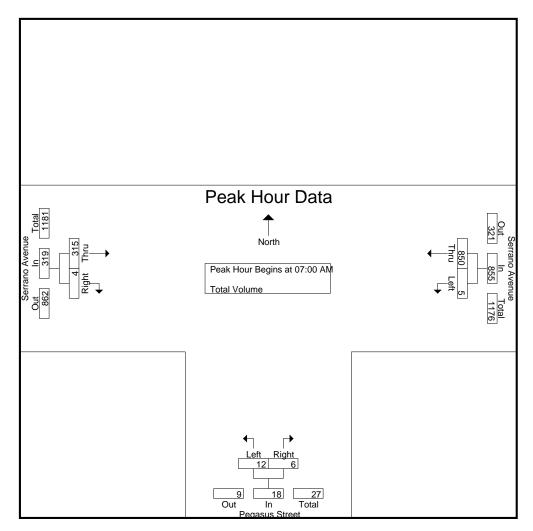


Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for	Each Ap	oproach	า Begin	s at:												
	04:15 PM				05:00 PM	1			04:45 PN	1			04:45 PN	Л		
+0 mins.	7	0	12	19	2	82	6	90	4	0	1	5	22	286	3	311
+15 mins.	14	0	15	29	5	114	16	135	3	0	3	6	24	257	1	282
+30 mins.	11	0	18	29	4	88	19	111	2	0	3	5	19	295	4	318
+45 mins.	15	2	20	37	4	95	14	113	1	0	4	5	35	301	8	344
Total Volume	47	2	65	114	15	379	55	449	10	0	11	21	100	1139	16	1255
% App. Total	41.2	1.8	57		3.3	84.4	12.2		47.6	0	52.4		8	90.8	1.3	
PHF	.783	.250	.813	.770	.750	.831	.724	.831	.625	.000	.688	.875	.714	.946	.500	.912

File Name: 04_ANA_Pegasus_Serrano AM Site Code: 00318390

Start Date : 5/16/2018 Page No : 2

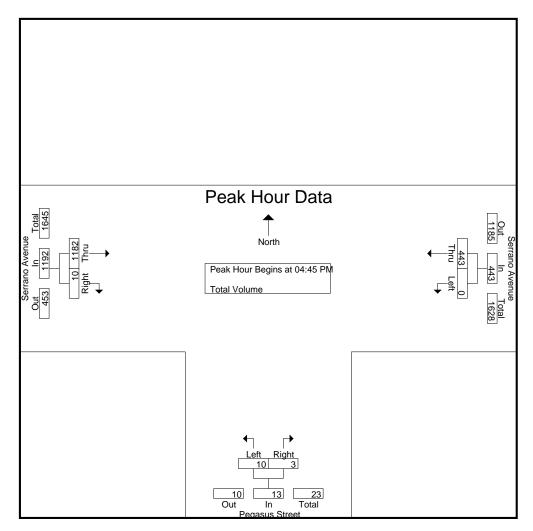


Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

Cak Hour for Lacif A	pproach begi	no at.							
	07:00 AM			07:30 AM			07:30 AM		
+0 mins.	2	239	241	5	3	8	99	2	101
+15 mins.	1	188	189	3	1	4	116	0	116
+30 mins.	1	204	205	2	0	2	81	1	82
+45 mins.	1	219	220	7	0	7	55	1	56
Total Volume	5	850	855	17	4	21	351	4	355
% App. Total	0.6	99.4		81	19		98.9	1.1	
PHF	.625	.889	.887	.607	.333	.656	.756	.500	.765

File Name: 04_ANA_Pegasus_Serrano PM Site Code: 00318390

Start Date : 5/16/2018 Page No : 2

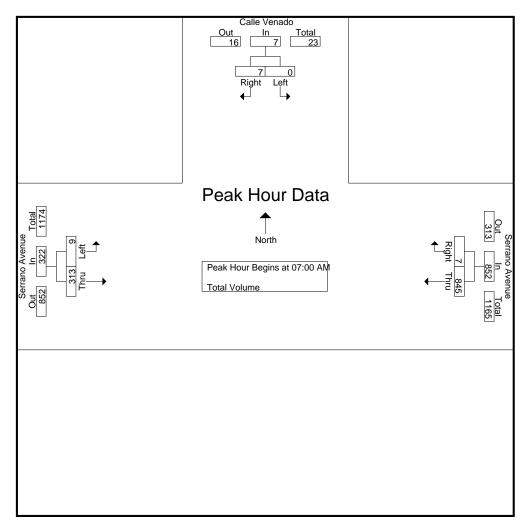


Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

reak noul for Each A	opioacii beg	iiis at.							
	04:30 PM			04:45 PM			04:45 PM		
+0 mins.	0	103	103	2	0	2	283	1	284
+15 mins.	0	116	116	2	1	3	296	2	298
+30 mins.	0	96	96	4	1	5	288	2	290
+45 mins.	0	133	133	2	1	3	315	5	320
Total Volume	0	448	448	10	3	13	1182	10	1192
% App. Total	0	100		76.9	23.1		99.2	0.8	
PHF	.000	.842	.842	.625	.750	.650	.938	.500	.931

File Name: 05_ANA_Calle Venado_Serrano AM Site Code: 00318390

Start Date : 5/16/2018 Page No : 2

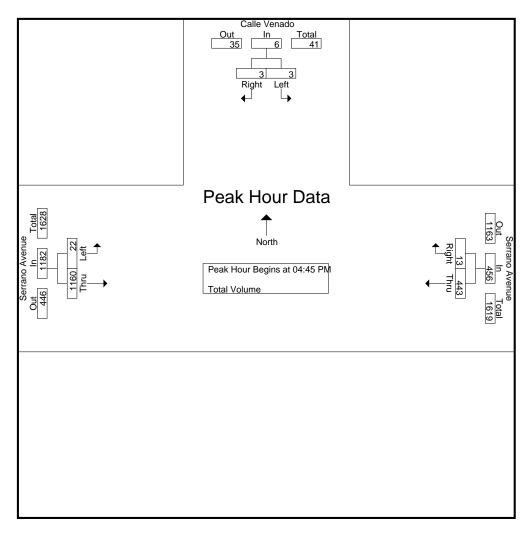


Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

reak noul for Each A	privacii begii	is al.							
	07:30 AM			07:00 AM			07:15 AM		
+0 mins.	0	3	3	238	0	238	1	55	56
+15 mins.	0	4	4	190	0	190	1	102	103
+30 mins.	0	5	5	199	1	200	6	109	115
+45 mins.	0	3	3	218	6	224	4	77	81
Total Volume	0	15	15	845	7	852	12	343	355
% App. Total	0	100		99.2	0.8		3.4	96.6	
PHF	.000	.750	.750	.888	.292	.895	.500	.787	.772

File Name: 05_ANA_Calle Venado_Serrano PM Site Code: 00318390

Start Date : 5/16/2018 Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

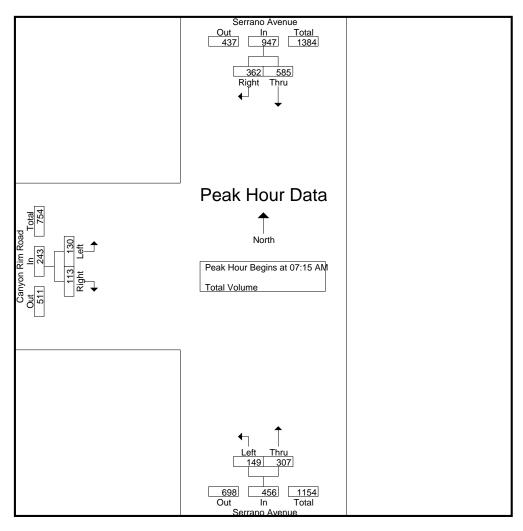
reak Hour for Lacif A	pproach begi	ns at.							
	04:00 PM			04:30 PM			04:45 PM		
+0 mins.	1	2	3	104	8	112	3	277	280
+15 mins.	0	1	1	114	1	115	8	290	298
+30 mins.	0	1	1	101	2	103	2	287	289
+45 mins.	1	2	3	129	2	131	9	306	315
Total Volume	2	6	8	448	13	461	22	1160	1182
% App. Total	25	75		97.2	2.8		1.9	98.1	
PHF	.500	.750	.667	.868	.406	.880	.611	.948	.938

City of Anaheim N/S: Serrano Avenue E/W: Canyon Rim Road

Weather: Clear

File Name : 09_ANA_Serrano_Canyon Rim AM Site Code : 00318390

Start Date : 5/16/2018 Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

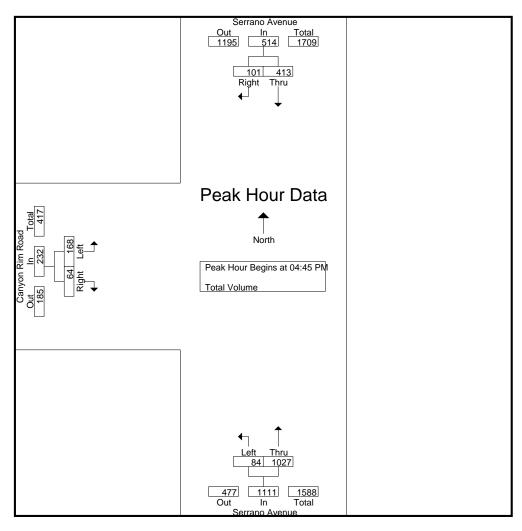
reak Hour for Lacif A	pproacri begi	no al.							
	07:15 AM			07:15 AM			07:30 AM		
+0 mins.	127	55	182	36	56	92	38	29	67
+15 mins.	145	103	248	68	75	143	56	42	98
+30 mins.	165	89	254	29	95	124	21	25	46
+45 mins.	148	115	263	16	81	97	32	18	50
Total Volume	585	362	947	149	307	456	147	114	261
% App. Total	61.8	38.2		32.7	67.3		56.3	43.7	
PHF	.886	.787	.900	.548	.808	.797	.656	.679	.666

City of Anaheim N/S: Serrano Avenue E/W: Canyon Rim Road

Weather: Clear

File Name : 09_ANA_Serrano_Canyon Rim PM Site Code : 00318390

Start Date : 5/16/2018 Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

reak noul for Each Ap	pproacri begi	iis al.							
	04:30 PM			04:45 PM			05:00 PM		
+0 mins.	102	29	131	20	257	277	42	19	61
+15 mins.	103	35	138	14	261	275	39	19	58
+30 mins.	107	18	125	15	244	259	49	15	64
+45 mins.	105	25	130	35	265	300	34	20	54
Total Volume	417	107	524	84	1027	1111	164	73	237
% App. Total	79.6	20.4		7.6	92.4		69.2	30.8	
PHF	.974	.764	.949	.600	.969	.926	.837	.913	.926

National Data & Surveying Services

Intersection Turning Movement Count

City: Orange Control: Signalized **Project ID:** 19-01025-001 **Date:** 2/7/2019

NS/EW Streets: Cannon St Cannon St Serrano Ave Serrano Ave Serrano Ave Serrano Ave AMM
AM O O O O O O O O O O O O O
NL NT NR NU SL ST SR SU EL ET ER EU WL WT WR 7:00 AM 0 67 37 0 5 317 0 0 0 0 0 0 0 221 0 26 7:15 AM 0 104 48 0 7 381 0 0 0 0 0 0 0 0 334 0 54 7:30 AM 0 156 63 0 13 378 0 0 0 0 0 0 0 340 0 47 7:45 AM 0 132 74 0 13 370 0 0 0 0 0 0 0 340 0 47 7:45 AM 0 145 89 0 9 310 0 0 0 0 0 0 0 294 0 40 8:00 AM 0 145 89 0 9 310 0 0 0 0 0 0 0 286 0 40 8:15 AM 0 161 94 0 11 364 0 0 0 0 0 0 0 261 0 50 8:30 AM 0 115 64 0 114 292 0 0 0 0 0 0 0 204 0 22 8:45 AM 0 118 58 0 8 274 0 0 0 0 0 0 0 0 236 0 13 **TOTAL VOLUMES: APPROACH %'s: 0.00% 65.44% 34.56% 0.00% 2.89% 97.11% 0.00% 0.00% 0.00% **PEAK HR : O7:30 AM - 08:30 A
7:00 AM
7:15 AM
7:30 AM
7:45 AM 0 132 74 0 13 370 0 0 0 0 0 0 294 0 40 8:00 AM 0 145 89 0 9 310 0 0 0 0 0 0 286 0 40 8:15 AM 0 161 94 0 11 364 0 0 0 0 0 0 0 261 0 50 8:30 AM 0 115 64 0 14 292 0 0 0 0 0 0 0 204 0 22 8:45 AM 0 118 58 0 8 274 0 0 0 0 0 0 0 236 0 13 TOTAL VOLUMES: 0 998 527 0 80 88 274 0 0 0 0 0 0 0 0 236 0 13 NL NT NR NU SL ST SR SU EL ET ER EU WL WT WR TOTAL VOLUMES: 0 998 527 0 80 2686 0 0 0 0 0 0 0 0 2176 0 292 APPROACH %'s: 0.00% 65.44% 34.56% 0.00% 2.89% 97.11% 0.00% 0.00% PEAK HR VOL: 0 594 320 0 46 1422 0 0 0 0 0 0 0 0 1181 0 177 PEAK HR FACTOR: 0.000 0.922 0.851 0.000 0.885 0.940 0.000 0.000 0.000 0.000 0.000 0.000 0.868 0.000 0.885 0.939 NORTHBOUND SOUTHBOUND EASTBOUND WESTBOUND
8:00 AM 0 145 89 0 9 310 0 0 0 0 0 0 286 0 40 8:15 AM 0 161 94 0 11 364 0 0 0 0 0 0 0 261 0 50 8:30 AM 0 115 64 0 14 292 0 0 0 0 0 0 0 204 0 22 8:45 AM 0 118 58 0 8 274 0 0 0 0 0 0 0 236 0 13 TOTAL VOLUMES: 0 998 57.1 NR NU SL ST SR SU EL ET ER EU WL WT WR APPROACH %'s: 0.00% 65.44% 34.56% 0.00% 2.89% 97.11% 0.00% 0.00% PEAK HR VOL: 0 594 320 0 46 1422 0 0 0 0 0 0 0 0 1181 0 177 PEAK HR FACTOR: 0.000 0.922 0.851 0.000 0.885 0.940 0.000 0.000 0.000 0.000 0.000 0.000 0.885 0.939 NORTHBOUND SOUTHBOUND EASTBOUND WESTBOUND
8:15 AM
8:30 AM
8:45 AM 0 118 58 0 8 274 0 0 0 0 0 0 0 236 0 13 TOTAL VOLUMES: 0 998 527 0 80 2686 0 0 0 0 0 0 0 0 0 2176 0 292 APPROACH %'s: 0.00% 65.44% 34.56% 0.00% 2.89% 97.11% 0.00% 0
NL
TOTAL VOLUMES: 0 998 527 0 80 2686 0 0 0 0 0 0 0 2176 0 292 APPROACH %'s: 0.00% 65.44% 34.56% 0.00% 2.89% 97.11% 0.00% 0.00% 0.00% PEAK HR: 07:30 AM - 08:30 AM PEAK HR VOL: 0 594 320 0 46 1422 0 0 0 0 0 0 0 0 1181 0 177 PEAK HR FACTOR: 0.000 0.922 0.851 0.000 0.885 0.940 0.000 0.000 0.000 0.000 0.000 0.000 0.868 0.000 0.885 0.897 NORTHBOUND SOUTHBOUND EASTBOUND WESTBOUND
APPROACH %'s: 0.00% 65.44% 34.56% 0.00% 2.89% 97.11% 0.00% 0.00%
PEAK HR: 07:30 AM - 08:30 AM PEAK HR VOL: 0 594 320 0 46 1422 0 0 0 0 0 0 0 1181 0 177 PEAK HR FACTOR: 0.000 0.922 0.851 0.000 0.885 0.940 0.000 0.000 0.000 0.000 0.000 0.868 0.000 0.885 0 0.896 0.939 0.939 0.877 0.877 0.877 0.877 0.877
PEAK HR VOL: PEAK HR FACTOR: 0 594 320 0 46 1422 0 0 0 0 0 0 1181 0 177 PEAK HR FACTOR: 0.000 0.922 0.851 0.000 0.885 0.940 0.000 0.000 0.000 0.000 0.000 0.868 0.000 0.885 0 0.877 0.
PEAK HR FACTOR: 0.000 0.922 0.851 0.000 0.885 0.940 0.000
0.896 0.939 0.877 NORTHBOUND SOUTHBOUND EASTBOUND WESTBOUND
NORTHBOUND SOUTHBOUND EASTBOUND WESTBOUND
PM 0 2 1 0 1 2 0 0 0 0 0 0 2 0 1
NL NT NR NU SL ST SR SU EL ET ER EU WL WT WR
4:00 PM 0 196 281 0 24 118 0 0 0 0 0 0 78 0 15
4:15 PM 0 278 316 0 23 159 0 0 0 0 0 113 0 20
4:30 PM 0 287 325 0 24 157 0 0 0 0 0 109 0 12
4:45 PM 0 260 362 0 20 194 0 0 0 0 0 0 114 0 13
5:00 PM 0 291 386 0 29 143 0 0 0 0 0 0 83 0 14
5:15 PM 0 311 368 0 27 191 0 0 0 0 0 0 85 0 10
5:15 PM 0 311 368 0 27 191 0 0 0 0 0 0 85 0 10 5:30 PM 0 307 369 0 37 224 0 0 0 0 0 97 0 14

National Data & Surveying Services

Intersection Turning Movement Count

Location: Cannon St & Taft Ave City: Orange Control: Signalized

Project ID: 19-01025-002 **Date:** 2/7/2019

_								To	tal						, ,		_
NS/EW Streets:		Canno	n St			Canno	n St			Taft .	Ave			Taft /	Ave		
		NORTH	BOUND			SOUTH	BOUND			EASTB	OUND			WESTE	BOUND		
AM	1 NL	2 NT	0 NR	0 NU	1 SL	2 ST	0 SR	0 SU	0.5 EL	0.5 ET	1 ER	0 EU	0 WL	1 WT	0 WR	<mark>0</mark> WU	TOTAL
7:00 AM	1	87	0	0	1	520	36	0	16	0	19	0	0	0	0	0	680
7:15 AM	5	145	0	0	0	667	36	0	26	1	26	0	0	0	0	0	906
7:30 AM	10	176	0	0	0	686	51	0	28	0	33	0	2	0	0	0	986
7:45 AM	12	180	0	0	0	607	38	0	42	0	51	0	0	0	0	0	930
8:00 AM	17	206	0	0	0	569	34	0	40	0	50	0	0	0	0	0	916
8:15 AM	12	204	0	0	0	572	31	0	34	0	47	0	0	0	0	0	900
8:30 AM	17	169	0	0	0	470	43	0	23	1	27	0	0	0	0	0	750
8:45 AM	14	145	0	0	0	449	37	0	23	0	20	0	0	0	0	0	688
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTA
TOTAL VOLUMES:	88	1312	0	0	1	4540	306	0	232	2	273	0	2	0	0	0	6756
APPROACH %'s:	6.29%	93.71%	0.00%	0.00%	0.02%	93.67%	6.31%	0.00%	45.76%	0.39%	53.85%	0.00%	100.00%	0.00%	0.00%	0.00%	
PEAK HR :)7:15 AM -		_	•	2520	450	•	126	_	160	•	2	•	•	•	TOTA
PEAK HR VOL :	44	707	0	0	0	2529	159	0	136	1	160	0	2	0	0	0	3738
PEAK HR FACTOR :	0.647	0.858 0.84	0.000 12	0.000	0.000	0.922 0.91	0.779 12	0.000	0.810	0.250 0.79	0.784 98	0.000	0.250	0.000 0.25	0.000 50	0.000	0.948
		NORTH	ROLIND			SOUTHI	ROLIND.			FASTR	SOUND			WESTE	ROLIND		
PM	1	2	0	0	1	2	0	0	0.5	0.5	1	0	0	1	0	0	l
FIVI	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ĒR	EU	WL	WΤ	WR	WU	TOTA
4:00 PM	28	445	0	0	0	169	42	0	49	0	12	0	0	0	0	0	745
4:15 PM	41	511	1	Ö	Ö	207	53	Ö	69	ī	9	Ö	Ō	1	Ō	Ö	893
4:30 PM	37	542	0	Ö	Ö	225	46	0	65	0	8	0	0	2	0	0	925
4:45 PM	31	543	Ō	Ö	Ō	248	40	Ö	89	Ō	12	Ō	Ö	0	Ō	Ō	963
5:00 PM	25	583	0	0	1	206	38	0	87	1	12	0	0	0	0	0	953
5:15 PM	17	588	0	0	0	259	37	0	111	0	9	0	0	1	0	0	1022
5:30 PM	7	541	Ō	Ö	Ō	251	42	Ö	114	Ō	11	Ō	0	0	0	Ō	966
5:45 PM	13	561	1	0	0	271	51	0	91	0	12	0	0	0	0	0	1000
																	4

National Data & Surveying Services

Location: Cannon St & Santiago Canyon Intersection Turning Movement Count
City: Orange
Control: Signalized

Project ID: 19-01025-003

-									tal								
NS/EW Streets:		Canno	n St			Canno	n St			Santiago Ca	anyon Rd		:	Santiago C	anyon Rd		
		NORTH	BOUND			SOUTH	BOUND			EASTB	OUND			WESTE	OUND		
AM	1	2	0	0	2.5	1.5	1	0	2	2	0	0	1	2	1	0	
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
7:00 AM	18	14	2	0	326	96	155	0	33	148	10	0	5	58	51	0	916
7:15 AM	31	33	3	0	346	128	190	0	63	186	13	0	3	100	52	0	1148
7:30 AM	40	44	5	0	407	119	196	0	59	210	18	0	4	117	88	0	1307
7:45 AM	46	36	2	0	355	105	188	0	63	189	34	0	8	149	83	0	1258
8:00 AM	56	44	8	0	397	110	128	0	85	241	13	0	1	113	104	0	1300
8:15 AM	25	20	3	0	380	92	141	0	61	163	8	0	2	129	115	0	1139
8:30 AM	29	23	2	0	295	103	106	0	51	173	15	0	2	113	87	0	999
8:45 AM	21	22	3	0	258	89	131	0	55	139	11	0	3	102	99	0	933
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES:	266	236	28	0	2764	842	1235	0	470	1449	122	0	28	881	679	0	9000
APPROACH %'s:	50.19%	44.53%	5.28%	0.00%	57.10%	17.39%	25.51%	0.00%	23.03%	70.99%	5.98%	0.00%	1.76%	55.48%	42.76%	0.00%	
PEAK HR :		07:15 AM -															TOTAL
PEAK HR VOL :	173	157	18	0	1505	462	702	0	270	826	78	0	16	479	327	0	5013
PEAK HR FACTOR :	0.772	0.892	0.563	0.000	0.924	0.902	0.895	0.000	0.794	0.857	0.574	0.000	0.500	0.804	0.786	0.000	0.959
		0.80	06			0.92	24			0.86	56			0.85	56		0.555
		NORTH	BOLIND			SOUTH	BOLIND			EASTB	OLIND			WESTE	NOLIND	I	
PM	1	2	0	0	2.5	1.5	1	0	2	2	0	0	1	2	1	0	
I IVI	NL	NT	NR	NU	SL	ST	SR	SU	ĒL	ĒT	ER	EU	ŴL	ΨT	WR	WU	TOTAL
4:00 PM	16	63	2	0	88	38	77	0	141	80	14	0	1	199	262	0	981
4:15 PM	16	63	3	Ō	106	39	98	0	148	96	15	Ō	1	201	359	1	1146
4:30 PM	21	90	1	0	106	27	85	0	136	88	9	0	1	215	340	0	1119
4:45 PM	15	100	3	0	117	36	97	0	210	88	24	0	1	233	296	0	1220
5:00 PM	27	447													222	0	1207
	21	117	1	0	106	36	66	0	158	106	16	0	5	247	322	U	1207
5:15 PM	28	133	1 2	0	137	45	72	0	154	106 119	22	0 0	5 3	236	340	0	1291
		133 137	_	0 0	137 149	45 43		-				_	-	236 219	340 251	-	-
5:15 PM	28	133	2	0	137	45	72	0	154	119	22	0	3	236	340	0	1291
5:15 PM 5:30 PM 5:45 PM	28 27 22 NL	133 137 125 NT	2 1 1	0 0 0	137 149 146	45 43 45 ST	72 87 82 SR	0 0 0	154 171 143 EL	119 111 114 ET	22 32 23 ER	0 0 0	3 2 3	236 219 227 WT	340 251 283 WR	0 1 0	1291 1231 1214 TOTAL
5:15 PM 5:30 PM 5:45 PM TOTAL VOLUMES :	28 27 22 NL 172	133 137 125 NT 828	2 1 1 NR 14	0 0 0 NU 0	137 149 146 SL 955	45 43 45 ST 309	72 87 82 SR 664	0 0 0 SU 0	154 171 143 EL 1261	119 111 114 ET 802	22 32 23 ER 155	0 0 0	3 2 3 WL 17	236 219 227 WT 1777	340 251 283 WR 2453	0 1 0 WU 2	1291 1231 1214
5:15 PM 5:30 PM 5:45 PM TOTAL VOLUMES : APPROACH %'s :	28 27 22 NL 172 16.96%	133 137 125 NT 828 81.66%	2 1 1 NR 14 1.38%	0 0 0	137 149 146	45 43 45 ST	72 87 82 SR	0 0 0	154 171 143 EL	119 111 114 ET	22 32 23 ER	0 0 0	3 2 3	236 219 227 WT	340 251 283 WR	0 1 0	1291 1231 1214 TOTAL 9409
5:15 PM 5:30 PM 5:45 PM TOTAL VOLUMES : APPROACH %'s : PEAK HR :	28 27 22 NL 172 16.96%	133 137 125 NT 828 81.66%	2 1 1 NR 14 1.38%	0 0 0 NU 0 0.00%	137 149 146 SL 955 49.53%	45 43 45 ST 309 16.03%	72 87 82 SR 664 34.44%	0 0 0 SU 0 0.00%	154 171 143 EL 1261 56.85%	119 111 114 ET 802 36.16%	22 32 23 ER 155 6.99%	0 0 0 EU 0 0.00%	3 2 3 WL 17 0.40%	236 219 227 WT 1777 41.82%	340 251 283 WR 2453 57.73%	0 1 0 WU 2 0.05%	1291 1231 1214 TOTAL 9409
5:15 PM 5:30 PM 5:45 PM TOTAL VOLUMES: APPROACH %'s: PEAK HR:	28 27 22 NL 172 16.96%	133 137 125 NT 828 81.66% 04:45 PM -	2 1 1 1 NR 14 1.38% 05:45 PM 7	0 0 0 NU 0 0.00%	137 149 146 SL 955 49.53%	45 43 45 ST 309 16.03%	72 87 82 SR 664 34.44%	0 0 0 SU 0 0.00%	154 171 143 EL 1261 56.85%	119 111 114 ET 802 36.16%	22 32 23 ER 155 6.99%	0 0 0 EU 0 0.00%	3 2 3 WL 17 0.40%	236 219 227 WT 1777 41.82%	340 251 283 WR 2453 57.73%	0 1 0 WU 2 0.05%	1291 1231 1214 TOTAL 9409
5:15 PM 5:30 PM 5:45 PM TOTAL VOLUMES : APPROACH %'s : PEAK HR :	28 27 22 NL 172 16.96%	133 137 125 NT 828 81.66%	2 1 1 1 NR 14 1.38% 05:45 PM 7 0.583	0 0 0 NU 0 0.00%	137 149 146 SL 955 49.53%	45 43 45 ST 309 16.03%	72 87 82 SR 664 34.44%	0 0 0 SU 0 0.00%	154 171 143 EL 1261 56.85%	119 111 114 ET 802 36.16%	22 32 23 ER 155 6.99%	0 0 0 EU 0 0.00%	3 2 3 WL 17 0.40%	236 219 227 WT 1777 41.82%	340 251 283 WR 2453 57.73%	0 1 0 WU 2 0.05%	1291 1231 1214 TOTAL 9409

City of Anaheim Nohl Ranch Road B/ Carnegie Avenue - Serrano Avenue 24 Hour Directional Volume Count

Counts Unlimited, Inc.
PO Box 1178
Corona, CA 92878
Phone: 951-268-6268 email: counts@countsunlimited.com

ANA001 Site Code: 003-18390

Start Time	16-May-18 Wed	Northbo Morning	ound Afternoon		Totals Afternoon	South Morning	bound Afternoon		Totals Afternoon	Combined	d Totals
12:00	wca	8	39	Worming	711101110011	8	40	woming	71101110011	worming	711101110011
12:15		8	29			5	33				
12:30		5	32			1	51				
12:45		2	36	23	136	2	39	16	163	39	299
01:00		6	44			2	41				
01:15		4	44			2	41				
01:30		0	36	4.4	400	0	32	_	400	40	200
01:45 02:00		4 2	42 42	14	166	1 1	48 55	5	162	19	328
02:00		2	42			5	59				
02:13		3	62			1	55				
02:45		4	43	11	187	3	98	10	267	21	454
03:00		3	49		107	1	104	10	207	21	707
03:15		3	34			3	68				
03:30		1	34			4	57				
03:45		4	33	11	150	5	56	13	285	24	435
04:00		3	33			5	68				
04:15		1	32			6	68				
04:30		11	44			15	54				
04:45		3	34	18	143	10	85	36	275	54	418
05:00		10	39			24	89				
05:15		8	36			23	62				
05:30		6	41			16	74				
05:45		22	43	46	159	29	62	92	287	138	446
06:00		11	43			29	80				
06:15 06:30		8 17	36 36			27 35	61 55				
06:45		18	30	54	145	46	38	137	234	191	379
07:00		31	33	34	143	44	41	137	254	191	379
07:15		42	36			57	33				
07:30		70	30			69	48				
07:45		48	33	191	132	118	25	288	147	479	279
08:00		56	17			48	27				2.0
08:15		30	15			64	24				
08:30		31	36			48	19				
08:45		23	20	140	88	43	34	203	104	343	192
09:00		16	30			39	14				
09:15		31	30			27	20				
09:30		14	27			38	18				
09:45		16	23	77	110	32	11	136	63	213	173
10:00		20	16			34	14				
10:15		29	18			35	11				
10:30		20	12	01	F7	34	15	124	47	225	101
10:45 11:00		22 26	11 23	91	57	31 32	7 8	134	47	225	104
11:15		23	14			38	5				
11:30		23 24	11			41	8				
11:45		32	16	105	64	41	4	152	25	257	89
Total		781	1537	781	1537	1222	2059	1222	2059	2003	3596
Combined											
Total		2318	5	23	18	328	31	32	81	559	9
AM Peak	-	07:15	-	-	-	07:30	-	-	-	-	-
Vol.	-	216	-	-	-	299	-	-	-	-	-
P.H.F.		0.771				0.633	_				
PM Peak	-	-	02:15	-	-	-	02:45	-	-	-	-
Vol.	-	-	194	-	-	-	327	-	-	-	-
P.H.F.			0.782				0.786				
Percentag		22 70/	66 20/			27.00/	60.00/				
e		33.7%	66.3%	ADT 5		37.2%	62.8%				
ADT/AADT	,	ADT 5,599	A	ADT 5,599							

City of Anaheim Serrano Avenue B/ Kendra Drive - Nohl Ranch Road 24 Hour Directional Volume Count

Counts Unlimited, Inc.
PO Box 1178
Corona, CA 92878
Phone: 951-268-6268 email: counts@countsunlimited.com

ANA002 Site Code: 003-18390

Start	16-May-18	Eastbo	ound		Totals	Westb	ound		Totals	Combine	
Time	Wed		Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon
12:00		7 5	63			3	73 107				
12:15 12:30		5 4	100 117			1 2	112				
12:30		6	64	22	344	3	83	9	375	31	719
01:00		2	48	22	344	2	75	9	3/3	31	719
01:00		3	84			0	79				
01:13		3	74			4	70				
01:45		2	81	10	287	1	59	7	283	17	570
02:00		2	88	. •	_0.	3	63	•			0.0
02:15		0	107			1	74				
02:30		5	106			0	76				
02:45		1	145	8	446	3	80	7	293	15	739
03:00		0	177			2	76				
03:15		1	201			1	82				
03:30		4	201			0	100				
03:45		1	266	6	845	0	81	3	339	9	1184
04:00		3	233			9	96				
04:15		4	277			14	92				
04:30		5	293			18	91				
04:45		6	297	18	1100	13	109	54	388	72	1488
05:00		6	282			21	91				
05:15		11	296			34	132				
05:30		8	308			62	112				
05:45		19	257	44	1143	72	122	189	457	233	1600
06:00		16	232			120	89				
06:15		14	248			148	68				
06:30		31	248	07	000	174	73	040	000	740	4005
06:45		36	205	97	933	177	62	619	292	716	1225
07:00		57	115			254	62				
07:15		67 05	141			204	58				
07:30		95 454	96	070	45.4	239	54	004	000	4004	000
07:45		151	102	370	454	264	52	961	226	1331	680
08:00		103	78			214	60				
08:15 08:30		51 65	84 65			124 147	48 41				
08:45		61	57	280	284	117	27	602	176	882	460
09:00		62	63	200	204	102	46	002	170	002	400
09:00		43	47			114	42				
09:30		49	43			88	27				
09:45		54	40	208	193	89	26	393	141	601	334
10:00		51	30		.00	77	13	000			00.
10:15		50	22			63	14				
10:30		47	2			79	6				
10:45		42	0	190	54	87	0	306	33	496	87
11:00		69	13			70	15				
11:15		53	16			79	11				
11:30		56	11			77	7				
11:45		61	5	239	45	86	3	312	36	551	81
Total		1492	6128	1492	6128	3462	3039	3462	3039	4954	9167
Combined		762	0	76	20	650)1	65	01	141	21
Total							•	00	•		- -
AM Peak	-	07:15	-	-	-	07:00	-	-	-	-	-
Vol.	-	416	-	-	-	961	-	-	-	-	-
P.H.F.		0.689	04.45			0.910	05.00				
PM Peak Vol.	-	-	04:45 1183	-	=	-	05:00 457	-	-	-	-
Vol. P.H.F.	-	-	0.960	-	-	-	457 0.866	-	-	-	-
г.п.г.			0.900				0.000				
Percentag											
e		19.6%	80.4%			53.3%	46.7%				
ADT/AADT	Α	DT 14,121	AA	DT 14,121							
•		•		,							

City of Anaheim Serrano Avenue B/ Nohl Ranch Road - Calle Venado 24 Hour Directional Volume Count

Counts Unlimited, Inc.
PO Box 1178
Corona, CA 92878
Phone: 951-268-6268 email: counts@countsunlimited.com

ANA003 Site Code: 003-18390

Start	16-May-18	Eastbo			Totals	Westl	oound		Totals	Combine	
Time	Wed	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon
12:00		9	63			2	74				
12:15		4	91			2	99				
12:30		7	95	07	045	2	127	0	070	0.5	000
12:45		7	66	27	315	2	78	8	378	35	693
01:00		2	50			3	77				
01:15		2 2	86			1	70				
01:30 01:45		3	78 92	9	306	5 1	71 57	10	275	19	581
02:00		2	88	9	300	1	76	10	213	19	301
02:15		0	118			1	77				
02:30		3	115			Ö	79				
02:45		Ö	160	5	481	3	79	5	311	10	792
03:00		1	188			2	85	_			
03:15		0	199			2 4	82				
03:30		3	197			0	95				
03:45		3	270	7	854	1	76	7	338	14	1192
04:00		4	229			7	85				
04:15		5	281			17	93				
04:30		3	283			18	96				
04:45		4	295	16	1088	11	121	53	395	69	1483
05:00		2	289			21	102				
05:15		8	293			34	129				
05:30		9	313			60	103				
05:45		15	254	34	1149	69	120	184	454	218	1603
06:00		13	220			116	97				
06:15		11	244			133	78				
06:30		26	232			164	79				
06:45		36	219	86	915	155	68	568	322	654	1237
07:00		<u>46</u>	120			241	61				
07:15		57	138			188	56				
07:30		93	107			205	57				
07:45		124	97	320	462	215	53	849	227	1169	689
08:00		76	86			199	55				
08:15		53	89			133	46				
08:30		62	64	074	007	147	42	500	470	0.07	470
08:45		80	58	271	297	117	30	596	173	867	470
09:00 09:15		63 47	69 52			103 121	42 36				
09:13		47 57	42			76	27				
09.30		53	43	220	206	76 89	25	389	130	609	336
10:00		53 51	27	220	200	76	20	309	130	009	330
10:15		56	23			71	16				
10:30		47	1			66	7				
10:45		52	0	206	51	84	0	297	43	503	94
11:00		70	15			81	17				
11:15		52	20			91	8				
11:30		67	9			82	7				
11:45		65	7	254	51	80	5	334	37	588	88
Total		1455	6175	1455	6175	3300	3083	3300	3083	4755	9258
Combined		763	kO.	76	30	63	83	63	83	140	13
Total				70.				00		140	
AM Peak	-	07:15	-	-	-	07:00	-	-	-	-	-
Vol.	-	350	-	-	-	849	-	-	-	-	-
P.H.F.		0.706	04.45			0.881	0.4.45				
PM Peak	-	-	04:45	-	-	-	04:45	-	-	-	-
Vol.	-	-	1190	-	-	-	455	-	-	-	-
P.H.F.			0.950				0.882				
Percentag											
e		19.1%	80.9%			51.7%	48.3%				
ADT/AADT	Α	DT 14,013	AA	DT 14,013							
				•							

City of Anaheim Carnegie Avenue B/ Nohl Ranch Road - Calle Venado 24 Hour Directional Volume Count

Counts Unlimited, Inc.
PO Box 1178
Corona, CA 92878
Phone: 951-268-6268 email: counts@countsunlimited.com

ANA004 Site Code: 003-18390

Start Time	16-May-18 Wed	Eastbo Morning	und Afternoon	Hour T Morning	Totals Afternoon	Westb Morning	ound Afternoon	Hour Morning	Totals Afternoon	Combine Morning	d Totals Afternoon
12:00	weu	1	6	Worming	Allemoon	1	2	Worning	Alternoon	Worning	Altemoon
12:15		0	1			Ö	6				
12:30		1	5			1	4				
12:45		1	1	3	13	0	2	2	14	5	27
01:00		0	6			0	1				
01:15		2	5			1	3				
01:30		1	5			2	5				
01:45		0	12	3	28	0	7	3	16	6	44
02:00		0	7	_		0	12	_		_	
02:15		0	10			1	13				
02:30		Õ	7			0	9				
02:45		Ö	9	0	33	1	6	2	40	2	73
03:00		1	8	ŭ	00	Ö	5	_	.0	_	
03:15		0	6			1	5				
03:30		Õ	4			0	7				
03:45		1	8	2	26	Ö	4	1	21	3	47
04:00		Ö	6	_	20	1	6	•	21	Ū	-77
04:15		Ő	6			Ö	4				
04:30		0	8			1	2				
04:45		1	9	1	29	1	2	3	14	4	43
05:00		2	8		25	7	4	3	17	7	40
05:00		0	3			1	8				
05:30		0	11			4	5				
05:45		1	6	3	28	1	6	13	23	16	51
06:00		0	9	3	20	6	10	13	25	10	31
06:00		1	6			7	4				
06:30		1	8			4	1				
06:45		5	4	7	27	9	6	26	21	33	48
07:00		1	8	,	21	6	6	20	21	33	40
07:00		9	7			3	5				
07:13		8				9	4				
			2	0.4	00	5		00	20	47	40
07:45		6 6	6	24	23		5	23	20	47	43
08:00			1			13	0				
08:15		9	6			7	2				
08:30		6	5	0.4	4.5	8	6	24	0		0.4
08:45		3	3	24	15	3	1	31	9	55	24
09:00		0	5			5 5	2				
09:15		3	5			5	3				
09:30		1	5	0	40	6	1	40	0	07	0.4
09:45		4	1	8	16	3	2	19	8	27	24
10:00 10:15		4 5	2			2	1				
			3			4	0				
10:30		0	0	0	11	2 2 3	0	10	2	10	4.4
10:45		0	6	9	11	2	2	10	3	19	14
11:00		2	4				1				
11:15		3	0			4	1				
11:30		7 2	3	4.4	40	4	2	40	4	00	4.4
11:45			3	14	10	1.45	102	12	4	26	14
Total		98	259	98	259	145	193	145	193	243	452
Combined		357		35	7	338	3	33	88	69	5
Total											
AM Peak	-	07:15	-	-	-	07:30	-	-	-	-	-
Vol. P.H.F.	-	29	-	-	-	34	-	-	-	-	-
		0.806	01.45			0.654 -	04.45				
PM Peak	-	-	01:45	-	-	-	01:45	-	-	-	-
Vol.	-	-	36	-	-	-	41	-	-	-	-
P.H.F.			0.750				0.788				
Percentag		27.5%	72.5%			42.9%	57.1%				
e				A A D T 227		42.970	31.170				
ADT/AADT		ADT 695		AADT 695							

City of Anaheim Calle Venado B/ Carnegie Avenue - Serrano Avenue 24 Hour Directional Volume Count

Counts Unlimited, Inc.
PO Box 1178
Corona, CA 92878
Phone: 951-268-6268 email: counts@countsunlimited.com

ANA005 Site Code: 003-18390

Start	16-May-18	Northb	ound	Hour	Totals	South	bound	Hour	Totals		ed Totals
Time 12:00	Wed	Morning 1	Afternoon 0	Morning	Afternoon	Morning 0	Afternoon 0	Morning	Afternoon	Morning	Afternoon
12:15		0	7			0	2				
12:30		Ő	7			0	0				
12:45		1	7	2	21	0	3	0	5	2	26
01:00		1	2			0	0	-			
01:15		0	5			0	1				
01:30		0	2			0	1				
01:45		1	5	2	14	0	0	0	2	2	16
02:00		0	3			0	2				
02:15		0	6			0	2				
02:30		0	5			0	2				
02:45		0	4	0	18	0	2	0	8	0	26
03:00		1	8			0	3				
03:15		0	11			0	1				
03:30		0	5	4	20	0	0	0	6	4	26
03:45 04:00		0 0	6 7	1	30	0 1	2 3	0	6	1	36
04:00		0	7			0	1				
04.13		0	15			0	1				
04:45		0	4	0	33	0	3	1	8	1	41
05:00		Ő	8	Ü	00	1	1		J		71
05:15		ő	4			0	Ö				
05:30		0	21			0	3				
05:45		0	7	0	40	1	2	2	6	2	46
06:00		1	12			1	2				
06:15		1	5			1	1				
06:30		1	10			1	1				
06:45		2	10	5	37	0	0	3	4	8	41
07:00		1	5			0	1				
07:15		0	6			0	1				
07:30		1	7			3	1				
07:45		13	6	15	24	4	1	7	4	22	28
08:00		6	9			5	1				
08:15		2	5			3	0				
08:30		3	2			2	2				
08:45		1	7	12	23	0	1	10	4	22	27
09:00		1	6			1	0				
09:15 09:30		0 4	1			4 3	1 0				
09.30		0	3	5	13	0	1	8	2	13	15
10:00		2	3	3	13	1	0	0	2	13	13
10:15		7	1			0	0				
10:30		2	2			2	1				
10:45		2	1	13	7	1	0	4	1	17	8
11:00		1	1			2	0				_
11:15		3	2			0	1				
11:30		3	1			2	0				
11:45		6	1	13	5	1	0	5	1	18	6
Total		68	265	68	265	40	51	40	51	108	316
Combined		333	3	33	33	9	1	9	1	42	24
Total											
AM Peak Vol.	-	07:45 24	-	-	-	07:30 15	-	-	-	-	-
voi. P.H.F.	-	0.462	-	-	-	0.750	-	-	-	-	-
PM Peak	_	0.402	05:30	_	_	0.750	02:15	_	_	_	_
Vol.	-	_	45	-	-	_	9	-	-	_	_
P.H.F.			0.536				0.750				
							2				
Percentag		20.4%	79.6%			44.0%	56.0%				
ADT/AADT		ADT 424		AADT 424			- 5.0 / 5				
ADI/AADI		ADT 424		AAD1 424							



 City:
 Anaheim

 Location:
 Nohl Ranch Road/Driveway

 Date:
 5/16/2018

 Count Type:
 Driveway Count

			1
	Entering	Exiting	Total
0:00	0	0	0
0:15	0	0	0
0:30	0	0	0
0:45	0	0	0
1:00	0	0	0
1:15	0	0	0
1:30	0	0	0
1:45	0	0	0
2:00	0	0	0
2:15	0	0	0
2:30 2:45	0	0	0
3:00	0	0	0
3:15	0	0	0
3:30	0	0	0
3:45	0	0	0
4:00	0	0	0
4:15	0	0	0
4:30	0	0	0
4:45	0	0	0
5:00	0	0	0
5:15	0	2	2
5:30	0	0	0
5:45	1	1	2
6:00	0	2	2
6:15	1	1	2
6:30 6:45	0	3	3
7:00	3	3	6
7:15	2	5	7
7:30	1	5	6
7:45	24	17	41
8:00	14	3	17
8:15	4	3	7
8:30	3	4	7
8:45	1	4	5
9:00	1	5	6
9:15	1	4	5
9:30	1	8	9
9:45	3	6	9
10:00	1	1	2
10:15 10:30	2	2	4 9
10:30	6	6	7
11:00	1	1	2
11:15	1	4	5
11:30	7	3	10
11:45	1	4	5
12:00	4	7	11
12:15	25	21	46
12:30	12	7	19
12:45	3	5	8
13:00	2	4	6
13:15	3	3	6
13:30	4	5	9
13:45	3	3 5	6 8
14:00 14:15	1	4	5
14:30	1	4	5
14:45	5	9	14
15:00	2	9	11
15:15	8	15	23
15:30	7	4	11
15:45	8	5	13
16:00	7	4	11
16:15	8	7	15
16:30	2	3	5
16:45	11	2	13
17:00 17:15	11 11	7	14 18
17:15	3	8	11
17:45	15	14	29
18:00	11	8	19
18:15	3	2	5
18:30	9	6	15
18:45	13	6	19
19:00	5	2	7
19:15	9	11	20
19:30	11 5	2	13 11
19:45		6	
20:00 20:15	8	8 10	16 19
20:15	20	2	22
20:45	4	1	5
21:00	7	1	8
21:15	1	0	1
21:30	0	0	0
21:45	0	0	0
22:00	0	1	1
22:15	1	1	2
22:30	1	0	1
22:45	0	0	0
23:00	0	0	0
23:15 23:30	0	0	0
23:30	0	0	0
TOTAL	361	324	685
			- 33

 City:
 Anaheim

 Location:
 West Driveway/Serrano Avenue

 Date:
 5/16/2018

 Count Type:
 Driveway Count

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 City:
 Anaheim

 Location:
 East Driveway / Serrano Avenue

 Date:
 5/16/2018

 Count Type:
 Driveway Count

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2:30	0	0	0
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22:45 23:00 23:15	0	0	0

APPENDIX B

ICU LEVEL OF SERVICE WORKSHEETS

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #1 Nohl Ranch Rd/Stage Coach Rd ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 18 Level Of Service: XXXXXX ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Control: Permitted Permitted Permitted Permitted Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #4 Kendra Dr/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 20 Level Of Service: XXXXXX ************************** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Control: Permitted Permitted Protected Protected Rights: Include Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #5 Nohl Ranch Rd/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 28 Level Of Service: XXXXXX ************************** Serrano Ave Street Name: Nohl Ranch Rd Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Protected Protected Include Include Include 0 0 0 0 0 0 0 0 0 0 Rights: Include 0 0 0 Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: $\begin{smallmatrix} 0 & 0 & 0 & 0 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 \\ \end{smallmatrix}$ Volume Module: 0 0 0 77 0 Base Vol: 203 143 273 0 0 718 89 Initial Bse: 0 0 0 77 0 203 143 273 0 0 718 0 0 0 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 0 0 0 PasserByVol: 0 0 Ω Ω 0 Ω 0 77 0 Initial Fut: 0 0 0 203 143 273 0 0 718 PHF Adj: 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 250 0 884 PHF Volume: 0 0 0 95 0 176 336 0 110 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: ()0 Reduced Vol: 0 0 0 9.5 Ω 250 176 336 0 884 110 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 0 0 0 95 0 250 176 336 0 0 884 Saturation Flow Module: 1700 Final Sat.: 0 0 0 3400 0 1700 1700 3400 0 0 3025 375 Capacity Analysis Module: Vol/Sat: 0.00 0.00 0.00 0.03 0.00 0.15 0.10 0.10 0.00 0.00 0.29 0.29 Crit Moves: ******************************

Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #9 Canyon Rim Rd/Serrano Ave **************************** 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 23 Level Of Service: XXXXXX ************************** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Control: Permitted Permitted Permitted Permitted Include Include Rights: Include Include 0 0 0 0 0 0 0 0 0 Min. Green:

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Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #10 Cannon Street/Serrano Avenue ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): 5 Average Delay (sec/veh): Level Of Service: Loss Time (sec): XXXXXX Optimal Cycle: 55 ******************************* Cannon Street Serrano Avenue Street Name: Approach: North Bound South Bound East Bound Movement: L-T-R L-T-RWest Bound Protected Protected Protected
Ovl Include Include Protected Include 0 0 0 Rights: $\bigcap \nabla Z \bigcap$ 0 0 0 0 0 0 0 0 Min. Green: 0 Y+R:0 0 2 0 1 1 0 2 0 0 0 0 0 0 0 Volume Module: 0 594 320 46 1422 Base Vol: 0 1181 0 0 0 177 1.00 1.00 1.00 Initial Bse: 0 594 320 46 1422 0 0 0 0 1181 177 0 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 PasserByVol: 0 Ω 0 0 0 0 \cap 0 0 \cap 0 0 0 0 594 Initial Fut: 320 46 1422 0 0 0 1181 1.00 1.00 User Adj: 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 46 1422 0 PHF Volume: 0 594 320 0 0 0 1181 177 0 0 0 0 0 0 0 0 0 0 Reduct Vol: ()Reduced Vol: 0 594 320 46 1422 0 0 0 1181 177 0 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 0 594 320 46 1422 0 0 0 0 1181 177 OvlAdjVol: 0 131 -----|----|-----|-----| Saturation Flow Module: 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 2.00 0.00 Lanes: 1.00 Final Sat.: 0 3400 1700 1700 3400 0 0 0 3400 0 1700 -----| Capacity Analysis Module: Vol/Sat: 0.00 0.17 0.19 0.03 0.42 0.00 0.00 0.00 0.00 0.35 0.00 0.10 OvlAdjV/S: 0.00 0.08 *** **** Crit Moves: ****

Wed May 22, 2019 16:56:05 Existing AM Page 13-1 Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #11 Cannon Street/ Taft Avenue ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): 5 Loss Time (sec): Average Delay (sec/veh): XXXXXX Loss Time (sec): 5
Optimal Cycle: 126 Level Of Service: ************************** Cannon Street Taft Avenue Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Permitted Include Include Ovl Permitted Rights: Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green: Y+R:Lanes: Volume Module: 0 0 2529 Base Vol: 44 707 159 136 1 160 2 \cap \cap Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0 0 2529 159 136 1 160 Initial Bse: 44 707 2 0 0 0 0 0 0 0 0 0 0 0 Added Vol: 0 0 Ω 0 0 \cap Ω Ω Ω 0 0 \cap 0 0 PasserByVol: Initial Fut: 44 707 0 0 2529 159 136 1 160 0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 PHF Volume: 44 707 0 0 2529 159 136 1 160 2 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 Reduced Vol: 44 707 Ω 0 2529 159 136 160 2. 1 ()PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Saturation Flow Module: 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 2.00 0.00 1.00 1.88 0.12 0.99 0.01 1.00 1.00 0.00 0.00 Lanes: 0 1700 3199 Final Sat.: 1700 3400 201 1688 12 1700 1700 0 -----| Capacity Analysis Module: Vol/Sat: 0.03 0.21 0.00 0.00 0.79 0.79 0.08 0.08 0.09 0.00 0.00 0.00 OvlAdjV/S: 0.07 **** **** **** Crit Moves: ****

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Crit Moves: ****

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #12 Cannon Street/E Santiago Canyon Road ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): 5 Loss Time (sec): Average Delay (sec/veh): XXXXXX 45 Level Of Service: Optimal Cycle: ******************************* Cannon Street E Santiago Canyon Road Street Name: East Bound Approach: North Bound South Bound East Bound L - T - R L - T - R North Bound South Bound West Bound L - T - R Movement: Split Phase Split Phase Protected Protected Ovl Include Include Rights: Ignore 0 0 0 0 0 0 0 0 0 0 Min. Green: 0 0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 2 0 1 1 0 Lanes: 1 0 1 1 0 2 1 1 0 1 Volume Module: Base Vol: 173 157 18 1505 462 702 270 826 78 16 479 327 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 18 1505 462 270 826 Initial Bse: 173 157 702 78 16 479 327 0 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 Ω 0 0 \cap Ω 0 0 Ω 0 0 0 0 PasserByVol: 18 Initial Fut: 173 157 1505 462 702 270 826 78 16 479 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 0.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.00 PHF Volume: 173 157 18 1505 462 702 270 826 78 16 479 0 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: Reduced Vol: 173 157 18 1505 462 702 270 826 78 16 479 ()1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PCE Adj: 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.00 MLF Adj: 1505 462 FinalVolume: 173 157 702 270 826 78 16 479 18 OvlAdjVol: 567 -----|-----| .-----||------| Saturation Flow Module: 1700 1700 1700 1700 1700 1700 Sat/Lane: 1700 1700 1700 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 3.00 1.00 1.00 1.79 0.21 1.00 2.00 1.83 0.17 1.00 2.00 Lanes: 1.00 Final Sat.: 1700 3050 350 5100 1700 1700 3400 3107 293 1700 3400 1700 -----||----||----||-----| Capacity Analysis Module: Vol/Sat: 0.10 0.05 0.05 0.30 0.27 0.41 0.08 0.27 0.27 0.01 0.14 0.00 OvlAdjV/S: 0.33

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #1 Nohl Ranch Rd/Stage Coach Rd ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 17 Level Of Service: XXXXXX ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Control: Permitted Permitted Permitted Permitted Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #4 Kendra Dr/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5
Optimal Cycle: 21 Average Delay (sec/veh): Level Of Service: XXXXXX ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----| Permitted Permitted Protected Protected Control: Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 1 0 0 1 0 1 0 0 1 0 1 0 2 0 1 4.0 4.0 4.0 Y+R: Lanes: 1 0 2 0 1 -----| Volume Module: 36 2 64 Base Vol: 10 0 11 100 1139 16 18 369 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 10 0 11 36 2 64 100 1139 18 369 16 57 0 0 0 0 Added Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 PasserByVol: 0 Initial Fut: 10 0 11 36 2 64 16 100 1139 18 369 57 1.00 1.00 PHF Volume: 10 0 36 2 64 100 1139 16 18 369 57 11 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 2 Reduced Vol: 10 0 11 36 100 1139 64 16 18 369 57 1.00 PCE Adj: MLF Adj: 1.00 11 FinalVolume: 10 0 36 2 64 100 1139 16 18 369 Saturation Flow Module: Sat/Lane: 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lanes: 1.00 0.00 1.00 1.00 0.03 0.97 1.00 2.00 1.00 1.00 2.00 1.00 Final Sat.: 1700 0 1700 1700 52 1648 1700 3400 1700 1700 3400 1700 -----|----|-----|------| Capacity Analysis Module: Vol/Sat: 0.01 0.00 0.01 0.02 0.04 0.04 0.06 0.34 0.01 0.01 0.11 0.03 Crit Moves: **** *** *************************

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #5 Nohl Ranch Rd/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 21 Level Of Service: XXXXXX ************************* Serrano Ave Street Name: Nohl Ranch Rd Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Protected Protected Include Include Include 0 0 0 0 0 0 0 0 0 0 Rights: Include 0 0 0 Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: $\begin{smallmatrix} 0 & 0 & 0 & 0 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 \\ \end{smallmatrix}$ Volume Module: 0 0 0 Base Vol: 119 0 106 112 1071 0 0 338 117 106 112 1071 Initial Bse: 0 0 0 119 0 0 0 338 117 0 0 0 0 0 0 0 0 0 0 0 Added Vol: ()0 0 0 0 \cap 0 0 0 0 0 Ω Ω PasserByVol: 119 0 Initial Fut: 0 0 0 106 112 1071 0 0 338 1.00 106 0 0 338 PHF Volume: 0 0 0 119 0 112 1071 117 0 0 0 0 0 0 0 0 0 0 Reduct Vol: ()Reduced Vol: 0 0 0 0 119 0 106 112 1071 0 338 117 1.00 1.00 0 0 338 FinalVolume: 0 0 0 119 0 106 112 1071 -----||-----|----||-----| Saturation Flow Module: 1700 Final Sat.: 0 0 0 3400 0 1700 1700 3400 0 0 2526 874 Capacity Analysis Module: Vol/Sat: 0.00 0.00 0.00 0.04 0.00 0.06 0.07 0.32 0.00 0.00 0.13 0.13 Crit Moves: ******************************

Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #9 Canyon Rim Rd/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh):
Optimal Cycle: 21 Level Of Service: XXXXXX ************************** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Control: Permitted Permitted Permitted Permitted Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #10 Cannon Street/Serrano Avenue ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): 5 Loss Time (sec): Average Delay (sec/veh): XXXXXX Optimal Cycle: 180 Level Of Service: ************************** Cannon Street Serrano Avenue Street Name: North Bound South Bound East Bound L - T - R L - T - R L - T - R Approach: West Bound Movement: Protected Protected Protected
Ovl Include Include Protected Rights: $\bigcap \nabla Z \bigcap$ 0 0 0 0 0 0 0 0 0 Min. Green: 0 0 0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: 0 0 2 0 1 1 0 2 0 0 0 0 0 0 0 Volume Module: Base Vol: 0 1192 1473 127 758 0 376 63 0 Ω \cap Ω 1.00 1.00 1473 127 758 376 Initial Bse: 0 1192 0 0 0 0 63 0 0 0 0 0 0 0 Added Vol: 0 0 0 0 0 0 0 0 Ω 0 0 \cap 0 0 0 0 0 PasserByVol: Initial Fut: 0 1192 1473 127 758 0 0 0 0 376 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 PHF Volume: 0 1192 1473 127 758 0 0 0 0 376 63 0 0 0 0 0 0 0 0 0 0 Reduct Vol: ()Reduced Vol: 0 1192 1473 127 758 0 0 Ω 376 0 6.3 1.00 1.00 1.00 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 0 1192 1473 127 758 0 0 0 0 376 63 OvlAdjVol: 1285 0 -----|----|-----|-----| Saturation Flow Module: Sat/Lane: 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 2.00 0.00 Lanes: 1.00 0 3400 1700 1700 3400 Final Sat.: 0 0 0 0 3400 0 1700 -----| Capacity Analysis Module: Vol/Sat: 0.00 0.35 0.87 0.07 0.22 0.00 0.00 0.00 0.00 0.11 0.00 0.04 OvlAdjV/S: 0.76 0.00

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Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #11 Cannon Street/ Taft Avenue ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): XXXXXX Optimal Cycle: 140 Level Of Service: ************************** Cannon Street Taft Avenue Street Name: North Bound South Bound East Bound West Bound L - T - R L - T - R Approach: Movement: Protected Protected Include Include Permitted Permitted Ovl Rights: Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green: 0 Y+R:1 0 1 1 0 Lanes: 1 0 1 1 0 0 1 0 0 1 Volume Module: 1 1 987 Base Vol: 62 2273 168 403 44 1 Ω \cap 1.00 1.00 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 403 1 Initial Bse: 62 2273 1 1 987 168 44 0 0 0 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 Ω 0 \cap Ω 0 Ω 0 0 0 0 PasserByVol: Initial Fut: 62 2273 1 1 987 168 403 1 44 0 0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1 PHF Volume: 62 2273 1 987 168 403 1 44 \cap 0 0 0 0 0 0 0 0 0 0 Reduct Vol: Reduced Vol: 62 2273 1 1 987 168 403 44 0 1 ()PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1 FinalVolume: 62 2273 1 987 168 403 1 44 0 OvlAdjVol: 0 -----|----| ----||-----| Saturation Flow Module: 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.99 0.01 1.00 1.71 0.29 0.99 0.01 1.00 0.00 1.00 0.00 Lanes: 1 1700 2905 Final Sat.: 1700 3399 495 1696 4 1700 0 1700 -----|----|-----|-----| Capacity Analysis Module: Vol/Sat: 0.04 0.67 0.67 0.00 0.34 0.34 0.24 0.24 0.03 0.00 0.00 0.00 OvlAdjV/S: 0.00 **** **** **** *** Crit Moves:

		 I	Level C	f Serv	 vice (Computa	tion I	Report	 t				
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)													

	<pre>Intersection #12 Cannon Street/E Santiago Canyon Road ************************************</pre>												
Cycle (sec): 100							<pre>cal Vol./Cap.(X):</pre>						
Cycle (sec): 100 Loss Time (sec): 5 Optimal Cycle: 47				Average Delay (sec/veh									
Optimal Cycle	Level Of Service: C												

	proach: North Bound			South Bound			E.a	ast Bo	ound	West Bound			
Movement:						- R							
Control: Rights:				Split Phase			Protected Include						
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			4.0									-	
Lanes:											2		
Volume Module													
	97		7		160	322						1209	
Growth Adj:			1.00			1.00		1.00			1.00	1.00	
Initial Bse: Added Vol:	97	487 0	7 0	509	160 0	322 0	693 0	424	94	12	935	1209 0	
PasserByVol:			0		0	0	0		0	0	0	0	
Initial Fut:			7	509	160	322	693	424	_	12	935	1209	
User Adj:			1.00	1.00	1.00	1.00		1.00			1.00	0.00	
PHF Adj:			1.00	1.00		1.00		1.00	1.00	1.00		0.00	
PHF Volume:	97	487	7	509	160	322	693	424	94	12	935	0	
	0	0	0	0	0	0	0	0	0	0	0	0	
Reduced Vol:			7	509	160	322	693	424		12	935	0	
_	1.00		1.00			1.00		1.00			1.00		
MLF Adj: FinalVolume:	1.00	487	1.00	1.00	160	1.00 322	693	1.00	1.00 94	1.00 12	935	0.00	
OvlAdjVol:	9 1	407	/	309	100	322	093	424	24	12	933	U	
Saturation Flow Module:													
Sat/Lane:	1700	1700	1700	1700	1700	1700	1700	1700	1700	1700	1700	1700	
Adjustment:	1.00		1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Lanes:	1.00		0.03		1.00	1.00		1.64		1.00		1.00	
Final Sat.:	1700		48	5100	1700	1700 l	3400	2783	617 l	1700	3400	1700	
Vol/Sat:	0.06		0.15	0.10	0.09	0.19	0.20	0.15	0.15	0.01	0.28	0.00	
OvlAdjV/S:	3.00	0.10	0.10	0.10	O • O J	0.00	0.20	0.10	0.10	0.01	0.20	0.00	
Crit Moves:		****		****			***				****		

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #1 Nohl Ranch Rd/Stage Coach Rd ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5 Average Delay (sec/veh):
Optimal Cycle: 18 Level Of Service: XXXXXX ************************** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Control: Permitted Permitted Permitted Permitted Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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 4 Y+R: Lanes: -----| Volume Module: Base Vol: 4 531 0 0 292 58 191 0 2 0 0 Initial Bse: 4 552 0 0 304 60 199 0 2 0 0 0 0 0 Added Vol: 0 0 0 0 0 0 0 PasserByVol: 0 0 Initial Fut: 4 552 0 0 0 0 0 0 0 0 0 0 0 0 0 304 60 2 199 0 0 0 ()PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Volume: 4 552 0 0 304 60 199 0 2 0 0 0 0 0 0 0 2 Reduct Vol: 0 0 0 0 0 0 0 Reduced Vol: 4 552 Ö 199 0 0 304 0 60 1.00 1.00 FinalVolume: 4 552 0 0 304 60 199 0 2 0 0 -----||-----||-----||-----| Saturation Flow Module: 1.00 Lanes: 1.00 2.00 0.00 0.00 1.67 0.33 1.00 0.00 1.00 0.00 0.00 0.00 Final Sat.: 1700 3400 0 0 2837 563 1700 0 1700 -----||-----| Capacity Analysis Module: Crit Moves: **** ****

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #4 Kendra Dr/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5
Optimal Cycle: 21 Average Delay (sec/veh):
Level Of Service: XXXXXX ************************** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----| Control: Permitted Permitted Protected Protected Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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 4 Y+R: Lanes: -----| Volume Module: Base Vol: 32 8 32 110 15 115 32 271 15 12 841 1.04 1.04 1.04 Initial Bse: 33 8 33 114 16 120 33 282 12 875 16 36 0 0 0 0 Added Vol: 0 0 0 0 0 0 PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 Initial Fut: 33 8 33 12 875 114 16 120 33 282 16 36 User Adj: 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Volume: 33 8 Reduct Vol: 0 0 33 114 16 120 33 282 16 12 875 36 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 33 8 33 33 282 12 875 114 16 120 16 36 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PCE Adj: 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 33 8 33 114 16 120 33 282 16 12 875 Saturation Flow Module: Sat/Lane: 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lanes: 1.00 0.20 0.80 1.00 0.12 0.88 1.00 2.00 1.00 1.00 2.00 1.00 Final Sat.: 1700 340 1360 1700 196 1504 1700 3400 1700 1700 3400 1700 Capacity Analysis Module: Vol/Sat: 0.02 0.02 0.02 0.07 0.08 0.08 0.02 0.08 0.01 0.01 0.26 Crit Moves: **** *** **************************

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #5 Nohl Ranch Rd/Serrano Ave ******************************** 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 30 Level Of Service: XXXXXX ************************* Serrano Ave Nohl Ranch Rd Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----| Protected Protected Protected Protected Include Include Include 0 0 0 0 0 0 0 0 0 0 Rights: Include 0 0 0 0 0 0 Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: $\begin{smallmatrix} 0 & 0 & 0 & 0 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 \\ \end{smallmatrix}$ Volume Module: 0 0 0 77 0 Base Vol: 203 143 273 0 0 718 89 Initial Bse: 0 0 0 80 0 211 149 284 0 0 747 0 0 0 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 0 0 0 0 0 0 0 \cap 0 0 PasserByVol: 0 80 0 Initial Fut: 0 0 211 149 284 0 0 747 93 1.00 PHF Adj: 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 99 260 183 350 PHF Volume: 0 0 0 0 0 0 920 114 0 0 0 0 0 0 0 0 0 0 Reduct Vol: ()Reduced Vol: 0 0 0 0 99 0 260 183 350 0 920 114 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 0 0 0 99 0 260 183 350 0 0 920 ----||------Saturation Flow Module: 1700 Final Sat.: 0 0 0 3400 0 1700 1700 3400 0 0 3025 375 Capacity Analysis Module: Vol/Sat: 0.00 0.00 0.00 0.03 0.00 0.15 0.11 0.10 0.00 0.00 0.30 0.30 Crit Moves: ******************************

Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #9 Canyon Rim Rd/Serrano Ave ******************************** 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 24 Level Of Service: XXXXXX ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Control: Permitted Permitted Permitted Permitted Include Include Rights: Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green: Y+R: Lanes: -----| Volume Module: Base Vol: 0 0 0 130 0 113 149 307 0 0 585 1.04 Initial Bse: 0 0 0 135 0 118 155 319 0 0 608 0 0 0 0 0 0 0 Added Vol: 0 0 0 PasserByVol: 0 0 Initial Fut: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 608 135 0 118 155 319 376 PHF Volume: 0 0 0 608 0 135 0 155 319 0 118 376 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 0 0 0 0 0 608 135 0 155 319 118 376 1.00 1.00 FinalVolume: 0 0 0 135 0 118 155 319 0 0 608 Saturation Flow Module: 1.00 Lanes: 0.00 0.00 0.00 1.07 xxxx 0.93 1.00 2.00 0.00 0.00 1.24 Final Sat.: 0 0 0 1819 0 1581 1700 3400 0 0 2100 1300 -----|----|-----|------| Capacity Analysis Module:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #10 Cannon Street/Serrano Avenue ******************************** 100 Critical Vol./Cap.(X): Cycle (sec): 5 Average Delay (sec/veh): Level Of Service: Loss Time (sec): XXXXXX Optimal Cycle: 65 ******************************* Cannon Street Serrano Avenue Street Name: Approach: North Bound South Bound East Bound Movement: L-T-R L-T-RWest Bound -----| Protected Protected Protected
Ovl Include Include Protected Include 0 0 Rights: $\bigcap \nabla Z \bigcap$ 0 0 0 0 0 0 0 Min. Green: 0 0 $4.0 \quad 4.0 \quad 4.0$ Y+R: 0 0 2 0 1 1 0 2 0 0 0 0 0 0 0 Volume Module: Base Vol: 0 594 320 46 1422 1181 0 0 0 0 177 1.04 1.04 1.04 Initial Bse: 0 618 333 48 1479 0 0 0 0 1228 184 0 0 0 0 0 0 Added Vol: 0 0 0 0 0 0 27 Ser. Hills2: () 0 0 11 0 0 0 0 \cap 0 0 Initial Fut: 0 645 333 48 1490 0 0 0 0 1228 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 0 PHF Volume: 0 645 333 48 1490 0 0 0 1228 184 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 0 Reduced Vol: 0 645 333 48 1490 Ω 0 0 1228 0 184 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 0 645 333 48 1490 0 0 0 0 1228 184 OvlAdjVol: 0 136 -----|----|-----|-----| Saturation Flow Module: 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 2.00 0.00 Lanes: 1.00 Final Sat.: 0 3400 1700 1700 3400 0 0 0 0 3400 0 1700 -----| Capacity Analysis Module: Vol/Sat: 0.00 0.19 0.20 0.03 0.44 0.00 0.00 0.00 0.00 0.36 0.00 0.11 OvlAdjV/S: 0.00 0.08 Crit Moves: **** *** ****

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #11 Cannon Street/ Taft Avenue ******************************** 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5
Optimal Cycle: 180 Average Delay (sec/veh): XXXXXX Level Of Service: ******************************* Cannon Street Taft Avenue Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Permitted Include Include Ovl Permitted Rights: Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green: Y+R:1 0 2 0 0 -----||-----|-----| Volume Module: 44 707 0 0 2529 Base Vol: 159 136 160 1 2 \cap \cap Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 Initial Bse: 46 735 141 1 0 0 2630 165 166 2 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 0 0 Ser. Hills2: Ω 27 0 \cap 11 0 0 0 0 \cap 0 0 Initial Fut: 46 762 0 0 2641 165 141 1 166 0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 PHF Volume: 46 762 0 0 2641 165 141 1 166 2 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 0 Reduced Vol: 46 762 0 0 2641 165 141 166 2 1 ()PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 46 762 0 0 2641 165 141 1 166 OvlAdjVol: 121 -----| -----Saturation Flow Module: 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 2.00 0.00 1.00 1.88 0.12 0.99 0.01 1.00 Lanes: 1.00 0.00 0.00 0 1700 3200 Final Sat.: 1700 3400 200 1688 12 1700 1700 0 -----| Capacity Analysis Module: Vol/Sat: 0.03 0.22 0.00 0.00 0.83 0.83 0.08 0.08 0.10 0.00 0.00 0.00 OvlAdjV/S: 0.07 Crit Moves: **** **** *** ****

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #12 Cannon Street/E Santiago Canyon Road ******************************** 100 Critical Vol./Cap.(X): Cycle (sec): 5 Average Delay (sec/veh): Loss Time (sec): XXXXXX 51 Level Of Service: Optimal Cycle: ******************************* Cannon Street E Santiago Canyon Road Street Name: East Bound Approach: North Bound South Bound East Bound L - T - R L - T - R South Bound West Bound L - T - R Movement: Split Phase Split Phase Protected Protected Λ Include Ovl Include Rights: Ignore 0 0 0 0 0 0 0 0 0 0 Min. Green: 0 0 4.0 4.0 4.0 Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Lanes: 1 0 1 1 0 2 1 1 0 1 2 0 1 1 0 Volume Module: Base Vol: 173 157 18 1505 462 702 270 826 78 16 479 327 Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 Initial Bse: 180 163 19 1565 480 730 281 859 81 17 498 340 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 0 11 Ser. Hills2: Ω Ω 0 0 Ω Ω 20 0 1 48 2.7 19 Initial Fut: 180 163 1576 480 730 281 879 81 18 546 367 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.00 PHF Volume: 180 163 19 1576 480 730 281 879 81 18 546 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 Reduced Vol: 180 163 19 1576 480 730 281 879 81 18 546 ()1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PCE Adj: 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 0.00 19 1576 480 730 281 879 18 546 FinalVolume: 180 163 81 OvlAdjVol: 590 _____| Saturation Flow Module: Sat/Lane: 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.79 0.21 3.00 1.00 2.00 1.83 0.17 1.00 2.00 Lanes: 1.00 1.00 Final Sat.: 1700 3050 350 5100 1700 1700 3400 3113 287 1700 3400 1700 -----| Capacity Analysis Module: Vol/Sat: 0.11 0.05 0.05 0.31 0.28 0.43 0.08 0.28 0.28 0.01 0.16 0.00 OvlAdjV/S: 0.35 Crit Moves: **** ****

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #1 Nohl Ranch Rd/Stage Coach Rd ******************************** 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5 Average Delay (sec/veh): Deptimal Cycle: 17 Level Of Service: XXXXXX ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Control: Permitted Permitted Permitted Permitted Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #4 Kendra Dr/Serrano Ave ******************************** 100 Cycle (sec): Critical Vol./Cap.(X): 5 5 22 Average Delay (sec/veh): Level Of Service: Loss Time (sec): XXXXXX Optimal Cycle: ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----| Permitted Permitted Protected Protected Control: Rights: Include Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 1 0 0 1 0 1 0 0 1 0 1 0 2 0 1 4.0 4.0 4.0 Y+R: Lanes: 1 0 2 0 1 -----| Volume Module: Base Vol: 10 0 11 36 2 64 100 1139 16 18 369 Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 Initial Bse: 10 0 11 37 2 67 104 1185 17 19 384 59 0 0 0 0 0 Added Vol: 0 0 0 -5 0 -150 0 0 0 0 0 0 0 0 0 0 0 PasserByVol: 0 11 Initial Fut: 10 0 37 2 67 104 1180 17 19 369 59 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Volume: 10 0 37 2 67 104 1180 17 19 369 59 11 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 37 2 0 17 19 369 Reduced Vol: 10 11 67 104 1180 59 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PCE Adj: 1.00 MLF Adj: 1.00 11 FinalVolume: 10 0 37 2 67 104 1180 17 19 369 Saturation Flow Module: Sat/Lane: 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lanes: 1.00 0.00 1.00 1.00 0.03 0.97 1.00 2.00 1.00 1.00 2.00 1.00 Final Sat.: 1700 0 1700 1700 52 1648 1700 3400 1700 1700 3400 Capacity Analysis Module: Vol/Sat: 0.01 0.00 0.01 0.02 0.04 0.04 0.06 0.35 0.01 0.01 0.11 0.03 Crit Moves: **** *** *************************

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #5 Nohl Ranch Rd/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5
Optimal Cycle: 21 Average Delay (sec/veh): Level Of Service: XXXXXX ************************* Serrano Ave Nohl Ranch Rd Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Protected Protected Include Include Include 0 0 0 0 0 0 0 0 0 0 Rights: Include 0 0 0 0 0 0 Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: $\begin{smallmatrix} 0 & 0 & 0 & 0 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 \\ \end{smallmatrix}$ Volume Module: 106 Base Vol: 0 0 0 119 0 112 1071 0 0 338 117 1.04 110 116 1114 Initial Bse: 0 0 0 124 0 0 0 352 122 0 0 0 3 0 -8 -3 -30 0 Added Vol: -6 ()0 0 0 0 0 0 \cap 0 0 0 Ω 0 PasserByVol: 127 0 Initial Fut: 0 0 0 102 113 1111 0 0 346 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 102 0 0 PHF Volume: 0 0 0 127 0 113 1111 346 122 0 0 0 0 0 0 0 0 0 0 Reduct Vol: ()Reduced Vol: 0 0 0 127 0 102 113 1111 0 346 122 0 1.00 1.00 FinalVolume: 0 0 0 127 0 102 113 1111 0 0 346 -----||-----||-----||-----| Saturation Flow Module: 1700 Final Sat.: 0 0 0 3400 0 1700 1700 3400 0 0 2514 886 Capacity Analysis Module: Vol/Sat: 0.00 0.00 0.00 0.04 0.00 0.06 0.07 0.33 0.00 0.00 0.14 0.14 Crit Moves: ******************************

Tue Feb 19, 2019 17:34:56 Opening PM Page 11-1 Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #9 Canyon Rim Rd/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 21 Level Of Service: XXXXXX ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Control: Permitted Permitted Permitted Permitted Include Include Rights: Include Include 0 0 0 0 0 0 0 0 0 0 Min. Green: Y+R: Lanes: -----| Volume Module: Base Vol: 0 0 0 168 0 64 84 1027 0 0 413 1.04 Initial Bse: 0 0 0 175 0 67 87 1068 0 0 430 105 0 0 0 0 0 Added Vol: 0 0 -1 0 0 PasserByVol: 0 0 Initial Fut: 0 0 0 0 0 175 0 67 0 0 0 0 0 0 0 0 0 0 430 87 1067 105 PHF Volume: 0 0 87 1067 0 430 0 175 0 67 0 105 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 67 Reduced Vol: 0 0 0 0 175 0 0 430 87 1067 105 1.00 1.00 FinalVolume: 0 0 0 175 0 67 87 1067 0 0 430 105 Saturation Flow Module: 1.00

Capacity Analysis Module:

Vol/Sat: 0.00 0.00 0.00 0.07 0.00 0.07 0.05 0.31 0.00 0.00 0.16 0.16 Crit Moves: ****

Crit Moves:

Opening PM Page 12-1 Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #10 Cannon Street/Serrano Avenue ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): 5 Loss Time (sec): Average Delay (sec/veh): XXXXXX Optimal Cycle: 180 Level Of Service: ************************* Cannon Street Serrano Avenue Street Name: North Bound South Bound East Bound L - T - R L - T - R L - T - R Approach: West Bound Movement: Protected Protected Protected
Ovl Include Include Protected Rights: $\bigcap \nabla Z \bigcap$ 0 0 0 0 0 0 0 0 0 Min. Green: 0 0 0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: 0 0 2 0 1 1 0 2 0 0 0 0 0 0 0 Volume Module: Base Vol: 0 1192 1473 127 758 0 376 63 0 0 \cap Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1532 132 788 Initial Bse: 0 1240 0 0 0 0 391 0 0 -5 0 0 0 0 -8 Added Vol: 0 0 0 0 Ser. Hills2: 0 14 0 Ω 33 0 \cap 0 0 Ω 0 0 Initial Fut: 0 1254 1527 132 821 0 0 0 0 383 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 PHF Volume: 0 1254 1527 132 821 0 0 0 0 383 66 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 Reduced Vol: 0 1254 1527 132 821 0 0 Ω 383 0 66 1.00 1.00 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 0 1254 1527 132 821 0 0 0 0 383 OvlAdjVol: 1335 0 -----|----|-----|-----| Saturation Flow Module: 1700 1700 Sat/Lane: 1700 1700 1700 1700 1700 1700 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 2.00 0.00 Lanes: 1.00 Final Sat.: 0 3400 1700 1700 3400 0 0 0 0 3400 0 1700 -----| Capacity Analysis Module: Vol/Sat: 0.00 0.37 0.90 0.08 0.24 0.00 0.00 0.00 0.00 0.11 0.00 0.04 OvlAdjV/S: 0.79 0.00

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Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #11 Cannon Street/ Taft Avenue ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): 5 Loss Time (sec): Average Delay (sec/veh): XXXXXX Optimal Cycle: 180 Level Of Service: ************************* Cannon Street Taft Avenue Street Name: North Bound South Bound East Bound West Bound L - T - R L - T - R Approach: Movement: Protected Protected Include Include Permitted Permitted Ovl Rights: Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green: 0 Y+R:1 0 1 1 0 Lanes: 1 0 1 1 0 0 1 0 0 1 -----||-----|-----| Volume Module: 1 1 987 Base Vol: 62 2273 168 403 44 1 0 \cap Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 419 1 Initial Bse: 64 2364 1 1 1026 175 46 0 0 -5 0 0 0 0 0 Added Vol: -8 0 0 0 Ser. Hills2: 0 14 0 \cap 33 Ω Ω 0 0 0 0 0 Initial Fut: 64 2373 1 1 1051 175 419 1 46 0 0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 PHF Volume: 64 2373 1 1 1051 175 419 1 46 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: Reduced Vol: 64 2373 1 1 1051 175 419 46 0 1 ()1.00 1.00 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1 1 1051 175 419 1 46 0 FinalVolume: 64 2373 OvlAdjVol: 0 -----| ----||-----| Saturation Flow Module: 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.99 0.01 1.00 1.72 0.28 0.99 0.01 1.00 0.00 1.00 Lanes: 0.00 1 1700 2916 484 1696 4 1700 Final Sat.: 1700 3399 0 1700 -----||-----||-----||------| Capacity Analysis Module: Vol/Sat: 0.04 0.70 0.70 0.00 0.36 0.36 0.25 0.25 0.03 0.00 0.00 0.00 OvlAdjV/S: 0.00 **** *** **** *** Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #12 Cannon Street/E Santiago Canyon Road ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): 5 Average Delay (sec/veh): Loss Time (sec): XXXXXX 55 Level Of Service: Optimal Cycle: ******************************* Cannon Street E Santiago Canyon Road Street Name: East Bound Approach: North Bound South Bound East Bound L - T - R L - T - R North Bound South Bound West Bound L - T - R Movement: Split Phase Split Phase Protected Protected Ovl Include Include Rights: Ignore 0 0 0 0 0 0 0 0 0 0 Min. Green: 0 0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 2 0 1 1 0 Lanes: 1 0 1 1 0 2 1 1 0 1 Volume Module: 97 487 7 Base Vol: 509 160 322 693 424 94 12 935 1209 Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 Initial Bse: 101 506 7 529 166 335 721 441 98 12 972 0 0 0 -5 -2 0 0 Added Vol: -1 -3 0 0 -3 Ser. Hills2: Ω Ω 1 33 0 Ω Ω 59 0 0 2.5 14 Initial Fut: 101 506 8 557 165 332 719 500 98 12 997 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 0.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 101 PHF Volume: 506 8 557 165 332 719 500 98 12 997 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 Reduced Vol: 101 506 8 557 165 332 719 500 98 12 997 ()1.00 1.00 1.00 1.00 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 0.00 8 557 165 FinalVolume: 101 506 332 719 500 98 12 997 OvlAdjVol: 0 _____| Saturation Flow Module: 1700 1700 1700 1700 1700 1700 Sat/Lane: 1700 1700 1700 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 3.00 1.00 1.00 1.97 0.03 1.00 2.00 1.67 0.33 1.00 2.00 Lanes: 1.00 55 5100 1700 1700 3400 2844 Final Sat.: 1700 3345 556 1700 3400 1700 -----||----||----||-----| Capacity Analysis Module: Vol/Sat: 0.06 0.15 0.15 0.11 0.10 0.20 0.21 0.18 0.18 0.01 0.29 0.00 OvlAdjV/S: 0.00 **** **** *** Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #1 Nohl Ranch Rd/Stage Coach Rd ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5 Average Delay (sec/veh):
Optimal Cycle: 18 Level Of Service: XXXXXX ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Control: Permitted Permitted Permitted Permitted Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #4 Kendra Dr/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5
Optimal Cycle: 23 Average Delay (sec/veh):
Level Of Service: XXXXXX ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----| Control: Permitted Permitted Protected Protected Rights: Include Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #5 Nohl Ranch Rd/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 39 Level Of Service: XXXXXX ************************* Serrano Ave Street Name: Nohl Ranch Rd Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Protected Protected Include Include Include 0 0 0 0 0 0 0 0 0 0 Rights: Include 0 0 0 Min. Green: Y+R: $\begin{smallmatrix} 0 & 0 & 0 & 0 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 \\ \end{smallmatrix}$ Volume Module: 0 0 0 91 0 Base Vol: 229 186 288 0 0 917 107 1.00 Initial Bse: 0 0 0 91 0 229 186 288 0 0 917 0 0 0 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 0 0 0 0 0 0 0 0 Ω PasserByVol: 0 91 0 Initial Fut: 0 0 0 229 186 288 0 0 917 1.00 0.82 279 227 351 0 1117 PHF Volume: 0 0 0 111 0 0 130 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: ()Reduced Vol: 0 0 0 0 111 0 279 227 351 0 1117 130 1.00 1.00 FinalVolume: 0 0 0 111 0 279 227 351 0 0 1117 Saturation Flow Module: 1700 Final Sat.: 0 0 0 3400 0 1700 1700 3400 0 0 3045 Capacity Analysis Module: Vol/Sat: 0.00 0.00 0.00 0.03 0.00 0.16 0.13 0.10 0.00 0.00 0.37 0.37 Crit Moves: ******************************

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #9 Canyon Rim Rd/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 25 Level Of Service: XXXXXX ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Control: Permitted Permitted Permitted Permitted Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #10 Cannon Street/Serrano Avenue ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5
Optimal Cycle: 43 Average Delay (sec/veh): Level Of Service: XXXXXX ******************************* Cannon Street Serrano Avenue Street Name: Approach: North Bound South Bound East Bound Movement: L-T-R L-T-RWest Bound Protected Protected Protected
Ovl Include Include Protected Include 0 0 Rights: $\bigcap \nabla Z \bigcap$ 0 0 0 0 0 0 0 0 Min. Green: 0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R:0 0 3 0 2 1 0 3 0 0 0 0 0 0 0 Volume Module: Base Vol: 0 1321 320 46 1779 1181 0 0 0 0 177 1.00 1.00 1.00 Initial Bse: 0 1321 320 46 1779 0 0 0 0 1181 177 0 0 0 0 0 0 0 Added Vol: 0 0 0 0 0 0 0 0 0 Ω 0 \cap 0 0 \cap 0 0 PasserByVol: Initial Fut: 0 1321 320 46 1779 0 0 0 0 1181 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 46 1779 0 PHF Volume: 0 1321 320 0 0 0 1181 177 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 ()Reduced Vol: 0 1321 320 46 1779 0 0 0 1181 177 0 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 0 1321 320 46 1779 0 0 0 0 1181 177 OvlAdjVol: 0 131 -----|----|-----|-----| Saturation Flow Module: 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.00 3.00 2.00 1.00 3.00 0.00 0.00 0.00 0.00 2.00 0.00 Lanes: 1.00 Final Sat.: 0 5100 3400 1700 5100 0 0 0 0 3400 0 1700 -----| Capacity Analysis Module: Vol/Sat: 0.00 0.26 0.09 0.03 0.35 0.00 0.00 0.00 0.00 0.35 0.00 0.10 OvlAdjV/S: 0.00 0.08 *** **** Crit Moves: ****

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #11 Cannon Street/ Taft Avenue ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5
Optimal Cycle: 44 Average Delay (sec/veh): XXXXXX Level Of Service: ************************* Cannon Street Taft Avenue Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Permitted Include Include Ovl Permitted Rights: Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green: Y+R:1 0 3 0 0 1 0 2 1 0 0 1 0 0 1 1 0 0 0 -----||-----|-----| Volume Module: 44 1434 0 0 2886 Base Vol: 159 136 1 160 2 \cap \cap 1.00 1.00 136 1 Initial Bse: 44 1434 0 0 2886 159 160 0 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 0 0 0 0 \cap 0 0 Ω 0 0 \cap 0 PasserByVol: Initial Fut: 44 1434 0 0 2886 159 136 1 160 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0 PHF Volume: 44 1434 0 2886 159 136 1 160 2 0 0 0 0 0 0 0 0 Reduct Vol: 0 0 Reduced Vol: 44 1434 Ω 0 2886 159 136 160 2. 1 ()PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0 0 2886 159 136 1 FinalVolume: 44 1434 160 OvlAdjVol: 116 -----| -----||-----| Saturation Flow Module: 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 3.00 0.00 1.00 2.84 0.16 0.99 0.01 1.00 Lanes: 1.00 0.00 0.00 0 1700 4834 266 1688 12 1700 Final Sat.: 1700 5100 1700 0 -----||-----| Capacity Analysis Module: Vol/Sat: 0.03 0.28 0.00 0.00 0.60 0.60 0.08 0.08 0.09 0.00 0.00 0.00 OvlAdjV/S: 0.07 **** **** **** Crit Moves: ****

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #12 Cannon Street/E Santiago Canyon Road ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): 5 Average Delay (sec/veh): Loss Time (sec): XXXXXX Optimal Cycle: 51 Level Of Service: ******************************* Cannon Street E Santiago Canyon Road Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RSplit Phase Split Phase Protected Include Ignore Include Protected Include 0 0 0 0 Rights: Ignore 0 0 0 0 0 0 0 0 Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R:2 0 1 1 0 3 0 3 0 1 2 0 3 0 1 Volume Module: Base Vol: 138 169 49 1915 536 575 158 726 67 147 1321 1154 1.00 1.00 Initial Bse: 138 169 49 1915 536 575 158 726 67 147 1321 0 0 0 0 0 0 0 Added Vol: 0 0 0 ()Ω 0 0 0 0 Ω Ω Ω 0 Ω 0 PasserByVol: ()49 1915 536 Initial Fut: 138 169 575 158 726 67 147 1321 1.00 1.00 1.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 User Adj: 0.00 0.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 49 PHF Volume: 138 169 1915 536 0 158 726 67 147 1321 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 Reduced Vol: 138 169 49 1915 536 0 158 726 67 147 1321 ()1.00 1.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 PCE Adj: 1.00 0.00 1.00 1.00 1.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 0.00 MLF Adj: 49 1915 536 0 158 726 FinalVolume: 138 169 67 -----Saturation Flow Module: Sat/Lane: 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1.00 1.00 1.00 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lanes: 2.00 1.55 0.45 3.00 3.00 1.00 2.00 3.00 1.00 2.00 3.00 1.00 764 5100 5100 1700 3400 5100 1700 3400 5100 Final Sat.: 3400 2636 1700 Capacity Analysis Module: Vol/Sat: 0.04 0.06 0.06 0.38 0.11 0.00 0.05 0.14 0.04 0.04 0.26 0.00 Crit Moves: **** **** ******************************

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #1 Nohl Ranch Rd/Stage Coach Rd ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5 Average Delay (sec/veh):
Optimal Cycle: 16 Level Of Service: XXXXXX ************************** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----|----|-----|------||-------| Control: Permitted Permitted Permitted Permitted Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #4 Kendra Dr/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5
Optimal Cycle: 22 Average Delay (sec/veh): Level Of Service: XXXXXX ************************** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----| Control: Permitted Permitted Protected Protected Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #5 Nohl Ranch Rd/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 21 Level Of Service: XXXXXX ************************** Serrano Ave Street Name: Nohl Ranch Rd Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Protected Protected Include Include Include 0 0 0 0 0 0 0 0 0 0 Rights: Include 0 0 0 Min. Green: Y+R: $\begin{smallmatrix} 0 & 0 & 0 & 0 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 \\ \end{smallmatrix}$ Volume Module: 0 0 0 Base Vol: 139 0 81 89 1160 0 0 407 105 Initial Bse: 0 0 0 139 0 81 89 1160 0 0 407 0 0 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 0 0 139 0 81 0 0 0 0 0 0 0 0 0 0 PasserByVol: Initial Fut: 89 1160 0 0 0 0 0 407 1.00 81 89 1160 0 0 407 PHF Volume: 0 0 0 139 0 105 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 Reduced Vol: 81 0 0 0 0 139 0 89 1160 0 407 105 1.00 1.00 0 0 407 FinalVolume: 0 0 0 139 0 81 89 1160 -----||-----||-----||-----| Saturation Flow Module: 1700 Final Sat.: 0 0 0 3400 0 1700 1700 3400 0 0 2703 697 Capacity Analysis Module: Vol/Sat: 0.00 0.00 0.00 0.04 0.00 0.05 0.05 0.34 0.00 0.00 0.15 0.15 Crit Moves: ******************************

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #9 Canyon Rim Rd/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 23 Level Of Service: XXXXXX ************************** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----|----|-----|------||-------| Control: Permitted Permitted Permitted Permitted Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Vol/Sat: 0.00 0.00 0.00 0.12 0.00 0.12 0.05 0.32 0.00 0.00 0.18 0.18

Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #10 Cannon Street/Serrano Avenue ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): 5 Average Delay (sec/veh): Level Of Service: Loss Time (sec): XXXXXX 26 Optimal Cycle: ******************************* Cannon Street Serrano Avenue Street Name: Approach: North Bound South Bound East Bound Movement: L-T-R L-T-RWest Bound Protected Protected Protected
Ovl Include Include Protected Include 0 0 Rights: $\bigcap \nabla Z \bigcap$ 0 0 0 0 0 0 0 Min. Green: 0 0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R:0 0 3 0 2 1 0 3 0 0 0 0 0 0 0 Volume Module: Base Vol: 0 1534 1473 127 1901 0 376 63 0 0 Ω 0 1534 1473 376 Initial Bse: 127 1901 0 0 0 0 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 0 0 0 0 0 Ω 0 0 \cap 0 0 0 0 PasserByVol: 0 Initial Fut: 0 1534 1473 127 1901 0 0 0 376 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 0 PHF Volume: 0 1534 1473 127 1901 0 0 0 376 63 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 Reduced Vol: 0 1534 1473 127 1901 Ω 0 0 376 0 6.3 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 0 1534 1473 127 1901 0 0 0 0 376 OvlAdjVol: 1097 0 _____| Saturation Flow Module: 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.00 3.00 2.00 1.00 3.00 0.00 0.00 0.00 0.00 2.00 0.00 Lanes: 1.00 Final Sat.: 0 5100 3400 1700 5100 0 0 0 3400 0 1700 -----| Capacity Analysis Module: Vol/Sat: 0.00 0.30 0.43 0.07 0.37 0.00 0.00 0.00 0.00 0.11 0.00 0.04 0.00 OvlAdjV/S: 0.32 **** **** *** Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #11 Cannon Street/ Taft Avenue ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5
Optimal Cycle: 52 Average Delay (sec/veh): Level Of Service: XXXXXX ************************** Cannon Street Taft Avenue Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Permitted Include Include Ovl Permitted Rights: Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green: Y+R: 1 0 2 1 0 1 0 2 1 0 0 1 0 0 1 Lanes: -----||-----||-----||------| Volume Module: 62 2615 1 1 2130 Base Vol: 168 403 1 44 Ω \cap Initial Bse: 62 2615 1 1 2130 403 1 168 44 0 0 0 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 0 0 0 0 0 Ω 0 0 0 PasserByVol: 1 Initial Fut: 62 2615 1 2130 168 403 1 44 0 0 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1 403 PHF Volume: 62 2615 1 2130 168 1 44 \cap 0 0 0 0 0 0 0 0 0 0 Reduct Vol: Reduced Vol: 62 2615 1 1 2130 168 403 44 Ω 1 0 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 62 2615 1 1 2130 168 403 1 44 0 1 OvlAdjVol: 0 -----|----| ----||-----| Saturation Flow Module: 1700 1700 1700 1.00 2.99 0.01 1.00 2.78 0.22 0.99 0.01 1.00 0.00 1.00 0.00 Lanes: 2 1700 4727 373 1696 4 1700 Final Sat.: 1700 5098 0 1700 -----|----|-----|-----| Capacity Analysis Module: Vol/Sat: 0.04 0.51 0.51 0.00 0.45 0.45 0.24 0.24 0.03 0.00 0.00 0.00 OvlAdjV/S: 0.00 **** **** **** **** Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #12 Cannon Street/E Santiago Canyon Road ******************************** 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5
Optimal Cycle: 56 Average Delay (sec/veh): Level Of Service: XXXXXX ******************************* Cannon Street E Santiago Canyon Road Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RSplit PhaseSplit PhaseProtectedProtectedIncludeIgnoreIncludeIgnore000000 Rights: 0 Min. Green: Y+R: $\begin{smallmatrix}2&0&1&1&0&&3&0&3&0&1&&2&0&3&0&1&&2&0&3&0&1\\\end{smallmatrix}$ Volume Module: 145 469 56 1530 226 Base Vol: 378 501 993 143 33 847 1761 Initial Bse: 145 469 56 1530 226 378 501 993 143 33 847 1761 0 0 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 Ω 0 0 0 Ω Ω 0 Ω Ω Ω 0 PasserByVol: 56 Initial Fut: 145 469 1530 226 378 501 993 143 33 847 1.00 1.00 1.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 User Adj: 0.00 PHF Adj: 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 56 0 PHF Volume: 145 469 1530 226 501 993 143 33 847 0 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: Reduced Vol: 145 469 56 1530 226 0 501 993 143 33 847 0 1.00 1.00 0.00 1.00 1.00 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 0.00 MLF Adj: 1.00 1.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 0.00 FinalVolume: 145 469 56 1530 226 0 501 993 143 33 847 -----| | ---------||----Saturation Flow Module: Sat/Lane: 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lanes: 2.00 1.79 0.21 3.00 3.00 1.00 2.00 3.00 1.00 2.00 3.00 1.00 Final Sat.: 3400 3037 363 5100 5100 1700 3400 5100 1700 3400 5100 1700 Capacity Analysis Module: Vol/Sat: 0.04 0.15 0.15 0.30 0.04 0.00 0.15 0.19 0.08 0.01 0.17 0.00 Crit Moves: **** **** ******************************

Crit Moves: ****

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #1 Nohl Ranch Rd/Stage Coach Rd ******************************** 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 18 Level Of Service: XXXXXX ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----|----|-----|------||-------| Control: Permitted Permitted Permitted Permitted Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #4 Kendra Dr/Serrano Ave ******************************** 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 20 Level Of Service: XXXXXX ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----|----|-----|------| Control: Permitted Permitted Protected Protected Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Capacity Analysis Module:

Vol/Sat: 0.02 0.02 0.02 0.06 0.08 0.08 0.02 0.08 0.01 0.01 0.24 Crit Moves: **** *** ***

Existing AM plus project Fri May 24, 2019 01:26:27 Page 7-1 Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #5 Nohl Ranch Rd/Serrano Ave ******************************** 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 28 Level Of Service: XXXXXX ************************* Serrano Ave Street Name: Nohl Ranch Rd Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Protected Protected Include Include Include 0 0 0 0 0 0 0 0 0 0 Rights: Include 0 0 0 Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: $\begin{smallmatrix} 0 & 0 & 0 & 0 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 \\ \end{smallmatrix}$ -----||-----||-----||------| Volume Module: 0 0 0 77 0 Base Vol: 203 143 273 0 0 718 89 203 143 273 Initial Bse: 0 0 0 77 0 0 0 718 0 0 0 1 0 -9 -7 -7 0 0 -3 Added Vol: 1 0 0 0 0 \cap Ω 0 0 0 \cap 0 PasserByVol: 0 78 0 Initial Fut: 0 0 0 194 136 266 0 0 715 1.00 PHF Adj: 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 239 167 328 0 881 PHF Volume: 0 0 0 96 0 0 111 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 Reduced Vol: 0 0 0 0 96 Ω 239 167 328 0 881 111 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 0 0 0 96 0 239 167 328 0 0 881 ----||----Saturation Flow Module: 1700

Capacity Analysis Module:

Vol/Sat: 0.00 0.00 0.00 0.03 0.00 0.14 0.10 0.10 0.00 0.00 0.29 0.29 Crit Moves: *** *** *** *** **** **** ****

Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #9 Canyon Rim Rd/Serrano Ave ******************************** 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 23 Level Of Service: XXXXXX ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----|----|-----|------| Control: Permitted Permitted Permitted Permitted Include Include Rights: Include Include 0 0 0 0 0 0 0 0 0 Min. Green:

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Existing AM plus project Fri May 24, 2019 01:26:27 Page 12-1 Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #10 Cannon Street/Serrano Avenue ******************************** 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5
Optimal Cycle: 55 Average Delay (sec/veh): Level Of Service: XXXXXX ******************************* Cannon Street Serrano Avenue Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Protected
Ovl Include Include Protected Include 0 0 0 Rights: $\bigcap \nabla Z \bigcap$ 0 0 0 0 0 0 0 Min. Green: 0 0 Y+R: 0 0 2 0 1 1 0 2 0 0 0 0 0 0 0 2 0 0 0 1 Volume Module: 320 46 1422 Base Vol: 0 594 0 1181 0 0 0 Ω 177 1.00 1.00 1.00 Initial Bse: 0 594 320 46 1422 0 0 0 0 1181 177 0 0 0 0 0 0 -5 Added Vol: -140 0 0 0 Ω 0 Ω 0 0 0 0 0 0 Ω 0 0 PasserByVol: 0 0 594 Initial Fut: 306 46 1422 0 0 0 1176 1.00 1.00 User Adj: 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 46 1422 0 PHF Volume: 0 594 306 0 0 0 1176 177 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 Reduced Vol: 0 594 306 46 1422 0 0 0 1176 177 0 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 0 594 306 46 1422 0 0 0 0 1176 177 OvlAdjVol: 0 131 -----|----|-----|-----||------| Saturation Flow Module: 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 2.00 0.00 1.00 Final Sat.: 0 3400 1700 1700 3400 0 0 0 3400 0 1700

Capacity Analysis Module:

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Existing AM plus project Fri May 24, 2019 01:26:27 Page 13-1 Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #11 Cannon Street/ Taft Avenue ******************************** 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5
Optimal Cycle: 124 Average Delay (sec/veh): XXXXXX Level Of Service: ************************** Cannon Street Taft Avenue Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Permitted Include Include Ovl Permitted Rights: Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green: Y+R:-----||-----||-----||------| Volume Module: 44 707 0 0 2529 Base Vol: 159 136 1 160 2 \cap \cap 1.00 1.00 136 1 160 Initial Bse: 44 707 0 0 2529 159 2 0 0 -140 0 -5 0 0 0 Added Vol: 0 0 0 0 0 Ω 0 0 \cap 0 Ω Ω 0 \cap 0 0 PasserByVol: Initial Fut: 44 693 0 0 2524 159 136 1 160 0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0 PHF Volume: 44 693 0 2524 159 136 1 160 2 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 0 Reduced Vol: 44 693 Ω 0 2524 159 136 160 2. 1 0 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0 0 2524 159 FinalVolume: 44 693 136 1 160 OvlAdjVol: 116 -----| Saturation Flow Module: 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 2.00 0.00 1.00 1.88 0.12 0.99 0.01 1.00 Lanes: 1.00 0.00 0.00

Capacity Analysis Module:
Vol/Sat: 0.03 0.20 0.00 0.00 0.79 0.79 0.08 0.08 0.09 0.00 0.00 0.00 0vlAdjV/S: 0.07

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1700 0

Final Sat.: 1700 3400

Existing AM plus project Fri May 24, 2019 01:26:27 Page 14-1 Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #12 Cannon Street/E Santiago Canyon Road ******************************** 100 Critical Vol./Cap.(X): Cycle (sec): 5 Average Delay (sec/veh): Loss Time (sec): XXXXXX 45 Level Of Service: Optimal Cycle: ******************************* Cannon Street E Santiago Canyon Road Street Name: East Bound North Bound South Bound East Bound L - T - R L - T - R Approach: West Bound L – T – R Movement: Split Phase Split Phase Protected Protected Include Ovl Include Rights: Ignore 0 0 0 0 0 0 0 0 0 0 Min. Green: 0 0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 Lanes: 1 0 1 1 0 2 1 1 0 1 2 0 1 1 0 Volume Module: Base Vol: 173 157 18 1505 462 702 270 826 78 16 479 327 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 18 1505 462 702 270 826 Initial Bse: 173 157 78 16 479 327 0 -2 0 -3 0 -2 0 0 Added Vol: -40 0 -8 Ω Ω 0 Ω Ω Ω Ω Ω 0 0 0 0 PasserByVol: 18 Initial Fut: 173 155 1502 462 700 266 826 78 16 479 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 0.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.00 PHF Volume: 173 155 18 1502 462 700 266 826 78 16 479 0 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: Reduced Vol: 173 155 18 1502 462 700 266 826 78 16 479 0 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 0.00 FinalVolume: 173 155 18 1502 462 700 266 826 78 16 479 OvlAdjVol: 567 -----| Saturation Flow Module: 1700 1700 1700 1700 1700 1700 Sat/Lane: 1700 1700 1700 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.79 0.21 3.00 1.00 2.00 1.83 0.17 1.00 2.00 Lanes: 1.00 1.00 354 5100 1700 1700 3400 3107 Final Sat.: 1700 3046 293 1700 3400 1700 -----| Capacity Analysis Module: Vol/Sat: 0.10 0.05 0.05 0.29 0.27 0.41 0.08 0.27 0.27 0.01 0.14 0.00

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OvlAdjV/S:

Crit Moves: ****

Capacity Analysis Module:

Crit Moves: ****

Vol/Sat: 0.00 0.09 0.00 0.00 0.16

Existing PM plus project Tue Feb 19, 2019 17:20:19 Page 3-1 Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #1 Nohl Ranch Rd/Stage Coach Rd ******************************** 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 17 Level Of Service: XXXXXX ************************** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----|----|-----|------||-------| Control: Permitted Permitted Permitted Permitted Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Existing PM plus project Tue Feb 19, 2019 17:20:19 Page 6-1 Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #4 Kendra Dr/Serrano Ave ******************************** 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5
Optimal Cycle: 21 Average Delay (sec/veh):
Level Of Service: XXXXXX ************************** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----|----|-----|------| Control: Permitted Permitted Protected Protected Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Capacity Analysis Module:

Vol/Sat: 0.01 0.00 0.01 0.02 0.04 0.04 0.06 0.33 0.01 0.01 0.03 Crit Moves: **** **** ****

Existing PM plus project Tue Feb 19, 2019 17:20:19 Page 7-1 Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #5 Nohl Ranch Rd/Serrano Ave ******************************** 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 21 Level Of Service: XXXXXX ************************* Serrano Ave Street Name: Nohl Ranch Rd Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Protected Protected Include Include Include 0 0 0 0 0 0 0 0 0 0 Rights: Include 0 0 0 Min. Green: Y+R: $\begin{smallmatrix} 0 & 0 & 0 & 0 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 \\ \end{smallmatrix}$ Volume Module: 0 0 0 Base Vol: 119 0 106 112 1071 0 0 338 117 Initial Bse: 0 0 0 119 0 106 112 1071 0 0 338 117 0 0 0 3 0 -8 -3 -30 0 Added Vol: -6 ()0 0 0 0 0 Ω 0 0 0 Ω 0 Ω PasserByVol: 0 122 0 98 109 1068 Initial Fut: 0 0 0 0 332 1.00 PHF Adj: 1.00 1.00 98 109 1068 0 0 332 PHF Volume: 0 0 0 122 0 117 0 0 0 0 0 0 0 0 0 0 Reduct Vol: ()Reduced Vol: 98 0 0 0 0 122 0 109 1068 0 332 117 1.00 1.00 FinalVolume: 0 0 0 122 0 98 109 1068 0 0 332 -----||-----||-----||-----| Saturation Flow Module: 1700 Final Sat.: 0 0 0 3400 0 1700 1700 3400 0 0 2514 886

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Vol/Sat: 0.00 0.00 0.00 0.04 0.00 0.06 0.06 0.31 0.00 0.00 0.13 0.13

Capacity Analysis Module:

Crit Moves:

Approach: Movement:		rth B	ound – R			ound		ast Bo		_	st Bo		
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Control:											Permitted		
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Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0	
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lanes:			0 0									1 0	
Base Vol:	0	0	0	168	0	64	84	1027	0	0	413	101	
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Initial Bse:	0	0	0	168	0	64	84	1027	0	0	413	101	
Added Vol:	0	0	0	0	0	0	0	-1	0	0	0	0	
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0	
Initial Fut:	0	0	0	168	0	64	84	1026	0	0	413	101	
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
PHF Volume:	0	0	0	168	0	64	84	1026	0	0	413	101	
	0	0	0	0	0	0	0	0	•	0	0	0	
Reduced Vol:	0	0	0	168	0	64		1026	0	0	413	101	
PCE Adj:	1.00		1.00	1.00	1.00	1.00		1.00		1.00		1.00	
MLF Adj:	1.00		1.00	1.00	1.00	1.00		1.00		1.00		1.00	
FinalVolume:	0	0	0	168	0	0 1	84	1026	0	0	413	101	
Saturation F. Sat/Lane:		1700	1700	1700	1700	1700	1700	1700	1700	1700	1700	1700	
•	1.00		1.00		1.00	1.00		1.00		1.00		1.00	
_	0.00		0.00		0.00	0.55		2.00		0.00		0.39	
Lanes: Final Sat.:			0.00	2462	0.00	938		3400	0.00		2732	668	
rillar Sat.:					-				I		2/32	000	
Capacity Anal				1			1						
Vol/Sat:			0.00		0.00	0.07	0.05		0.00		0.15	0.15	
Crit Moves: *******	****	****	*****	****	****	*****	****	**** ****	*****	****	****	*****	

Existing PM plus project Tue Feb 19, 2019 17:20:19 Page 12-1 Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #10 Cannon Street/Serrano Avenue ******************************** 100 Critical Vol./Cap.(X): Cycle (sec): 5 Loss Time (sec): Average Delay (sec/veh): XXXXXX Optimal Cycle: 180 Level Of Service: ******************************* Cannon Street Serrano Avenue Street Name: Approach: North Bound South Bound East Bound Movement: L-T-R L-T-RWest Bound Protected Protected Protected
Ovl Include Include Protected Include 0 0 Rights: $\bigcap \nabla Z \bigcap$ 0 0 0 0 0 0 0 0 0 0 Min. Green: 0 0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R:0 0 2 0 1 1 0 2 0 0 0 0 0 0 0 Volume Module: Base Vol: 0 1192 1473 127 758 0 376 63 0 Ω Ω Ω 1473 127 758 Initial Bse: 0 1192 0 0 0 0 376 0 0 -5 0 0 0 0 -8 Added Vol: 0 0 0 0 0 0 0 Ω 0 0 \cap 0 0 0 0 0 PasserByVol: Initial Fut: 0 1192 1468 127 758 0 0 0 0 368 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 PHF Volume: 0 1192 1468 127 758 0 0 0 0 368 63 0 0 0 0 0 0 0 0 0 0 Reduct Vol: ()Reduced Vol: 0 1192 1468 127 758 0 0 Ω 368 0 6.3 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 0 1192 127 758 0 0 0 0 368 1468 OvlAdjVol: 1284 0 -----|----|-----|-----||------| Saturation Flow Module: 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 2.00 0.00 Lanes: 1.00 Final Sat.: 0 3400 1700 1700 3400 0 0 0 0 3400 0 1700

OvlAdjV/S: 0.76 Crit Moves: **** ****

Capacity Analysis Module:

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Vol/Sat: 0.00 0.35 0.86 0.07 0.22 0.00 0.00 0.00 0.00 0.11 0.00 0.04

0.00

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Existing PM plus project Tue Feb 19, 2019 17:20:19 Page 13-1 Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #11 Cannon Street/ Taft Avenue ******************************** 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5
Optimal Cycle: 138 Average Delay (sec/veh): XXXXXX Level Of Service: ******************************* Cannon Street Taft Avenue Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Permitted Include Include Ovl Permitted Rights: Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green: Y+R:1 0 1 1 0 1 0 1 1 0 0 1 0 0 1 Lanes: -----||-----||-----||------| Volume Module: 62 2273 1 1 987 Base Vol: 168 403 44 1 Ω \cap Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 62 2273 1 1 987 403 1 168 44 0 0 -5 0 0 0 0 0 0 Added Vol: -8 0 0 0 0 0 0 \cap Ω 0 Ω 0 0 0 PasserByVol: Initial Fut: 62 2268 1 1 979 168 403 1 44 0 0 1.00 1.00 User Adj: PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1 PHF Volume: 62 2268 1 979 168 403 1 44 \cap 0 0 0 0 0 0 0 0 0 0 Reduct Vol: Reduced Vol: 62 2268 1 1 979 168 403 44 Ω 1 ()PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1 979 FinalVolume: 62 2268 1 168 403 1 44 0 1 OvlAdjVol: 0 -----|----| ----||-----| Saturation Flow Module: 1700 1700 1700 1.00 1.99 0.01 1.00 1.71 0.29 0.99 0.01 1.00 0.00 1.00 0.00 Lanes: 1 1700 2902 498 1696 4 1700 Final Sat.: 1700 3399 0 1700 -----|----|-----|-----| Capacity Analysis Module:

Vol/Sat: 0.04 0.67 0.67 0.00 0.34 0.34 0.24 0.24 0.03 0.00 0.00 0.00

0.00

OvlAdjV/S:

Crit Moves:

Existing PM plus project Tue Feb 19, 2019 17:20:19 Page 14-1 Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #12 Cannon Street/E Santiago Canyon Road ******************************** 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5
Optimal Cycle: 47 Average Delay (sec/veh): XXXXXX Level Of Service: ******************************* Cannon Street E Santiago Canyon Road Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RSplit Phase Split Phase Protected Protected Include Ovl Rights: Include Ignore 0 0 0 0 0 0 0 0 0 0 Min. Green: 0 0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R:1 0 1 1 0 2 1 1 0 1 2 0 1 1 0 1 0 2 0 1 Lanes: Volume Module: 97 487 7 Base Vol: 509 160 322 693 424 94 12 935 1209 1.00 Initial Bse: 97 487 7 509 160 322 693 424 94 12 935 1209 0 0 0 -5 -1 -3 -2 0 0 0 Added Vol: 0 -3 Ο Ω Ω ()Ω 0 0 0 0 0 0 0 PasserByVol: Initial Fut: 97 487 7 504 159 319 691 424 94 12 935 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 0.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.00 PHF Volume: 97 487 7 504 159 319 691 424 94 12 935 0 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: Reduced Vol: 97 487 7 504 159 319 691 424 94 12 935 ()PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.00 7 504 159 FinalVolume: 97 487 319 691 424 94 12 935 OvlAdjVol: 0 -----| Saturation Flow Module: 1700 1700 1700 1.00 1.00 1.97 0.03 3.00 1.00 1.00 2.00 1.64 0.36 1.00 2.00 Lanes: 1.00 48 5100 1700 1700 3400 2783 Final Sat.: 1700 3352 617 1700 3400 1700 -----|

Vol/Sat: 0.06 0.15 0.15 0.10 0.09 0.19 0.20 0.15 0.15 0.01 0.28 0.00

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Capacity Analysis Module:

OvlAdjV/S:

Crit Moves:

Opening AM plus project Fri May 24, 2019 01:28:48 Page 3-1 Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #1 Nohl Ranch Rd/Stage Coach Rd ******************************** 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 18 Level Of Service: XXXXXX ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----|----|-----|------||-------| Control: Permitted Permitted Permitted Permitted Include Include Rights: Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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PHF Volume: 4 534

Reduced Vol: 4 534

Reduct Vol:

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Opening AM plus project Fri May 24, 2019 01:28:48 Page 6-1 Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #4 Kendra Dr/Serrano Ave ******************************** 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 21 Level Of Service: XXXXXX ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----| Control: Permitted Permitted Protected Protected Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Final Sat.: 1700 340 1360 1700 196 1504 1700 3400 1700 1700 3400 1700

Capacity Analysis Module:

Vol/Sat: 0.02 0.02 0.02 0.07 0.08 0.08 0.02 0.08 0.01 0.01 0.25 Crit Moves: **** *** *** *************************

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Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #5 Nohl Ranch Rd/Serrano Ave ******************************** 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 29 Level Of Service: XXXXXX ******************************* Serrano Ave Street Name: Nohl Ranch Rd Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----| Protected Protected Protected Protected Include Include Include 0 0 0 0 0 0 0 0 0 0 Rights: Include 0 0 0 Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: $\begin{smallmatrix} 0 & 0 & 0 & 0 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 \\ \end{smallmatrix}$ -----||-----||-----||------| Volume Module: 0 0 0 77 0 Base Vol: 203 143 273 0 0 718 89 Initial Bse: 0 0 0 80 0 211 149 284 0 0 747 0 0 0 1 0 -9 -7 -7 0 0 -3 Added Vol: 1 0 0 0 0 () Ω Ω 0 0 \cap 0 PasserByVol: 0 81 0 Initial Fut: 0 0 0 202 142 277 0 0 744 1.00 PHF Adj: 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 0.81 249 175 341 0 916 PHF Volume: 0 0 0 100 0 0 115 0 0 0 0 0 0 0 0 0 0 Reduct Vol: ()Reduced Vol: 0 0 0 100 0 249 175 341 0 916 0 115 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 0 0 0 100 0 249 175 341 0 0 916 Saturation Flow Module: 1700 Final Sat.: 0 0 0 3400 0 1700 1700 3400 0 0 3020 380 Capacity Analysis Module: Vol/Sat: 0.00 0.00 0.00 0.03 0.00 0.15 0.10 0.10 0.00 0.00 0.30 0.30 Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #9 Canyon Rim Rd/Serrano Ave ******************************** 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 24 Level Of Service: XXXXXX ************************** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----|----|-----|------||-------| Control: Permitted Permitted Permitted Permitted Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green: Lanes: -----| Volume Module: Base Vol: 0 0 0 130 0 113 149 307 0 0 585 1.04 Initial Bse: 0 0 0 135 0 118 155 319 0 0 608 0 0 0 0 0 0 0 0 0 -3 Added Vol: 0 0 PasserByVol: 0 0 Initial Fut: 0 0 0 0 0 0 0 0 0 0 0 0 0 135 0 0 0 605 118 155 319 376 PHF Volume: 0 0 0 605 0 135 0 118 155 319 0 376 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 0 0 0 135 0 0 0 605 155 319 118 376 1.00 1.00 FinalVolume: 0 0 0 135 0 118 155 319 0 0 605 Saturation Flow Module: 1.00 Lanes: 0.00 0.00 0.00 1.07 xxxx 0.93 1.00 2.00 0.00 0.00 1.23 Final Sat.: 0 0 0 1819 0 1581 1700 3400 0 0 2096 1304 -----|----|-----|------| Capacity Analysis Module: Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #10 Cannon Street/Serrano Avenue ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5
Optimal Cycle: 64 Average Delay (sec/veh): Level Of Service: XXXXXX ******************************* Cannon Street Serrano Avenue Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Protected
Ovl Include Include Protected Include 0 0 0 Rights: $\bigcap \nabla Z \bigcap$ 0 0 0 0 0 0 0 Min. Green: 0 0 Y+R: 0 0 2 0 1 1 0 2 0 0 0 0 0 0 0 2 0 0 0 1 Volume Module: 0 594 320 46 1422 Base Vol: 0 0 0 0 1181 0 177 1.04 1.04 1.04 Initial Bse: 0 618 333 48 1479 0 0 0 0 1228 184 0 0 0 0 0 0 -5 Added Vol: -140 0 0 0 27 0 Ser. Hills2: Ω 0 0 11 0 0 0 Ω 0 0 0 Initial Fut: 0 645 319 48 1490 0 0 0 1223 1.00 1.00 User Adj: 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 48 1490 0 PHF Volume: 0 645 319 0 0 0 1223 184 0 0 0 0 0 0 0 0 0 0 Reduct Vol: ()Reduced Vol: 0 645 319 48 1490 0 0 0 1223 0 184 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 0 645 319 48 1490 0 0 0 0 1223 184 OvlAdjVol: 0 136 _____| Saturation Flow Module: 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 2.00 0.00 1.00 Final Sat.: 0 3400 1700 1700 3400 0 0 0 3400 0 1700 -----| Capacity Analysis Module: Vol/Sat: 0.00 0.19 0.19 0.03 0.44 0.00 0.00 0.00 0.00 0.36 0.00 0.11 OvlAdjV/S: 0.00 0.08 *** **** Crit Moves: ****

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #11 Cannon Street/ Taft Avenue ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5
Optimal Cycle: 180 Average Delay (sec/veh): XXXXXX Level Of Service: ******************************* Cannon Street Taft Avenue Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Permitted Include Include Ovl Permitted Rights: Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green: Y+R:-----||-----||-----||------| Volume Module: 44 707 0 0 2529 Base Vol: 159 136 1 160 2 \cap \cap 1.04 1.04 165 141 1 Initial Bse: 46 735 0 0 2630 166 2 0 0 -140 0 -5 0 0 0 Added Vol: 0 0 0 0 Ser. Hills2: Ω 27 0 0 11 0 0 0 0 \cap 0 0 Initial Fut: 46 748 0 0 2636 165 141 1 166 0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0 PHF Volume: 46 748 0 2636 165 141 1 166 2 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 0 Reduced Vol: 46 748 Ω 0 2636 165 141 166 2. 1 ()PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0 0 2636 165 141 1 FinalVolume: 46 748 166 OvlAdjVol: 121 _____| Saturation Flow Module: 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 2.00 0.00 1.00 1.88 0.12 0.99 0.01 1.00 Lanes: 1.00 0.00 0.00 0 1700 3199 201 1688 12 1700 Final Sat.: 1700 3400 1700 0 -----| Capacity Analysis Module: Vol/Sat: 0.03 0.22 0.00 0.00 0.82 0.82 0.08 0.08 0.10 0.00 0.00 0.00 OvlAdjV/S: 0.07 **** **** **** Crit Moves: ****

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #12 Cannon Street/E Santiago Canyon Road ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): 5 Average Delay (sec/veh): Loss Time (sec): XXXXXX 51 Level Of Service: Optimal Cycle: ******************************* Cannon Street E Santiago Canyon Road Street Name: East Bound North Bound South Bound East Bound West Bound L - T - R L - T - R Approach: Movement: Split Phase Split Phase Protected Protected Ovl Include Rights: Include Ignore 0 0 0 0 0 0 0 0 0 0 Min. Green: 0 0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: 2 0 1 1 0 Lanes: 1 0 1 1 0 2 1 1 0 1 Volume Module: Base Vol: 173 157 18 1505 462 702 270 826 78 16 479 327 Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 19 1565 480 730 281 859 Initial Bse: 180 163 81 17 498 340 0 -2 0 -3 0 -2 0 Added Vol: **-**4 0 0 0 -8 20 Ser. Hills2: Ω 0 0 11 0 Ω Ω 0 1 48 2.7 19 Initial Fut: 180 161 1573 480 728 277 879 81 18 546 359 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 0.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.00 180 PHF Volume: 161 19 1573 480 728 277 879 81 18 546 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 Reduced Vol: 180 161 19 1573 480 728 277 879 81 18 546 ()PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 0.00 277 879 19 1573 480 728 18 546 FinalVolume: 180 161 81 OvlAdjVol: 590 -----| Saturation Flow Module: Sat/Lane: 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.79 0.21 3.00 1.00 1.00 2.00 1.83 0.17 1.00 2.00 Lanes: 1.00 354 5100 1700 1700 3400 3113 Final Sat.: 1700 3046 287 1700 3400 1700 -----| Capacity Analysis Module: Vol/Sat: 0.11 0.05 0.05 0.31 0.28 0.43 0.08 0.28 0.28 0.01 0.16 0.00 OvlAdjV/S: 0.35 Crit Moves: **** ****

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #1 Nohl Ranch Rd/Stage Coach Rd ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5 Average Delay (sec/veh): Deptimal Cycle: 17 Level Of Service: XXXXXX ************************** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----|----|-----|------||-------| Control: Permitted Permitted Permitted Permitted Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #4 Kendra Dr/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5
Optimal Cycle: 22 Average Delay (sec/veh):
Level Of Service: XXXXXX ************************** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----| Control: Permitted Permitted Protected Protected Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 1 0 0 1 0 1 0 0 1 0 1 0 2 0 1 4.0 4.0 4.0 Y+R: Lanes: 1 0 2 0 1 -----| Volume Module: Base Vol: 10 0 11 36 2 64 100 1139 16 18 369 1.04 1.04 1.04 Initial Bse: 10 0 11 37 2 67 104 1185 17 19 384 59 0 0 0 0 0 -15 Added Vol: 0 0 0 -5 0 0 PasserByVol: 0 0 Initial Fut: 10 0 0 0 0 0 0 0 0 0 0 0 11 37 2 67 17 104 1180 19 369 59 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Volume: 10 0 Reduct Vol: 0 0 37 2 67 104 1180 17 19 369 59 11 0 0 0 0 0 0 0 0 0 0 37 2 Reduced Vol: 10 0 17 19 369 11 67 104 1180 59 PCE Adj: 1.00 MLF Adj: 1.00 11 FinalVolume: 10 0 37 2 67 104 1180 17 19 369 Saturation Flow Module: Sat/Lane: 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lanes: 1.00 0.00 1.00 1.00 0.03 0.97 1.00 2.00 1.00 1.00 2.00 1.00 Final Sat.: 1700 0 1700 1700 52 1648 1700 3400 1700 1700 3400 1700 Capacity Analysis Module: Vol/Sat: 0.01 0.00 0.01 0.02 0.04 0.04 0.06 0.35 0.01 0.01 0.11 0.03 Crit Moves: **** ***

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Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #5 Nohl Ranch Rd/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 21 Level Of Service: XXXXXX ************************* Serrano Ave Street Name: Nohl Ranch Rd Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - RProtected Protected Protected Protected Include Include Include 0 0 0 0 0 0 0 0 0 0 Rights: Include 0 0 0 Min. Green: Y+R: $\begin{smallmatrix} 0 & 0 & 0 & 0 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 \\ \end{smallmatrix}$ Volume Module: 0 0 0 Base Vol: 119 0 106 112 1071 0 0 338 117 1.04 110 116 1114 0 0 352 Initial Bse: 0 0 0 124 0 122 0 0 0 3 0 -8 -3 -30 0 Added Vol: -6 ()0 0 0 0 0 0 0 0 0 Ω Ω 0 PasserByVol: 127 0 Initial Fut: 0 0 0 102 113 1111 0 0 346 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 102 0 0 346 PHF Volume: 0 0 0 127 0 113 1111 122 0 0 0 0 0 0 0 0 0 0 Reduct Vol: ()Reduced Vol: 0 0 0 127 0 102 113 1111 0 0 346 122 1.00 1.00 0 0 346 FinalVolume: 0 0 0 127 0 102 113 1111 -----||-----||-----||-----| Saturation Flow Module: 1700 Final Sat.: 0 0 0 3400 0 1700 1700 3400 0 0 2514 886 Capacity Analysis Module: Vol/Sat: 0.00 0.00 0.00 0.04 0.00 0.06 0.07 0.33 0.00 0.00 0.14 0.14 Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #9 Canyon Rim Rd/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 21 Level Of Service: XXXXXX ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----|----|-----|------||-------| Control: Permitted Permitted Permitted Permitted Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green: Y+R: Lanes: -----| Volume Module: Base Vol: 0 0 0 168 0 64 84 1027 0 0 413 1.04 Initial Bse: 0 0 0 175 0 67 87 1068 0 0 430 105 0 0 0 0 0 0 -1 0 Added Vol: 0 0 PasserByVol: 0 0 Initial Fut: 0 0 0 0 0 0 0 0 0 0 430 87 1067 105 PHF Volume: 0 0 87 1067 0 430 0 175 0 67 0 105 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 175 0 67 0 Reduced Vol: 0 0 0 0 430 87 1067 105 1.00 1.00 FinalVolume: 0 0 0 175 0 67 87 1067 0 0 430 105 Saturation Flow Module: 1.00 Lanes: 0.00 0.00 0.00 1.45 0.00 0.55 1.00 2.00 0.00 0.00 1.61 0.39 Final Sat.: 0 0 0 2462 0 938 1700 3400 0 0 2732 668 -----|----|-----|------| Capacity Analysis Module: Vol/Sat: 0.00 0.00 0.00 0.07 0.00 0.07 0.05 0.31 0.00 0.00 0.16 0.16 Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #10 Cannon Street/Serrano Avenue ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): 5 Average Delay (sec/veh): Loss Time (sec): XXXXXX Optimal Cycle: 180 Level Of Service: ******************************* Cannon Street Serrano Avenue Street Name: Approach: North Bound South Bound East Bound Movement: L-T-R L-T-RWest Bound Protected Protected Protected
Ovl Include Include Protected Include 0 0 0 Rights: $\bigcap \nabla Z \bigcap$ 0 0 0 0 0 0 0 Min. Green: 0 0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R:0 0 2 0 1 1 0 2 0 0 0 0 0 0 0 -----||-----||-----||------| Volume Module: Base Vol: 0 1192 1473 127 758 0 376 63 0 0 Ω 1.04 1.04 1532 132 788 Initial Bse: 0 1240 0 0 0 0 391 0 0 -5 0 0 0 0 -8 Added Vol: 0 0 0 0 Ser. Hills2: 0 14 0 Ω 33 0 0 0 0 0 0 0 Initial Fut: 0 1254 1527 132 821 0 0 0 0 383 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 0 PHF Volume: 0 1254 1527 132 821 0 0 0 383 66 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 Reduced Vol: 0 1254 1527 132 821 0 0 Ω 383 0 66 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 0 1254 1527 132 821 0 0 0 0 383 OvlAdjVol: 1335 0 -----|----|-----|-----| Saturation Flow Module: 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 2.00 0.00 Lanes: 1.00 Final Sat.: 0 3400 1700 1700 3400 0 0 0 0 3400 0 1700 -----| Capacity Analysis Module: Vol/Sat: 0.00 0.37 0.90 0.08 0.24 0.00 0.00 0.00 0.00 0.11 0.00 0.04 OvlAdjV/S: 0.79 0.00 **** **** *** Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #11 Cannon Street/ Taft Avenue ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5
Optimal Cycle: 180 Average Delay (sec/veh): XXXXXX Level Of Service: ******************************* Cannon Street Taft Avenue Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Permitted Include Include Ovl Permitted Rights: Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green: Y+R:1 0 1 1 0 1 0 1 1 0 0 1 0 0 1 -----||-----||-----||------| Volume Module: 62 2273 1 1 987 Base Vol: 168 403 44 1 0 \cap Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 419 1 Initial Bse: 64 2364 1 1 1026 175 46 0 0 -5 0 0 0 0 0 Added Vol: -8 0 0 0 Ser. Hills2: 0 14 0 0 33 Ω 0 0 0 0 0 0 Initial Fut: 64 2373 1 1 1051 175 419 1 46 0 0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1 PHF Volume: 64 2373 1 1051 175 419 1 46 \cap 0 0 0 0 0 0 0 0 0 0 Reduct Vol: Reduced Vol: 64 2373 1 1 1051 175 419 46 () 1 ()PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 64 2373 1 1 1051 175 419 1 46 0 OvlAdjVol: 0 -----|----| ----||-----| Saturation Flow Module: 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.99 0.01 1.00 1.72 0.28 0.99 0.01 1.00 0.00 1.00 0.00 Lanes: 1 1700 2916 484 1696 4 1700 Final Sat.: 1700 3399 0 1700 -----|----|-----|-----| Capacity Analysis Module: Vol/Sat: 0.04 0.70 0.70 0.00 0.36 0.36 0.25 0.25 0.03 0.00 0.00 0.00 OvlAdjV/S: 0.00 **** **** **** *** Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #12 Cannon Street/E Santiago Canyon Road ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): 5 Loss Time (sec): Average Delay (sec/veh): XXXXXX Optimal Cycle: 55 Level Of Service: ******************************* Cannon Street E Santiago Canyon Road Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RSplit Phase Split Phase Protected Protected Include 0 0 0 Ovl Rights: Include Ignore 0 0 0 0 0 0 0 Min. Green: 0 0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: 2 0 1 1 0 1 0 2 0 1 Lanes: 1 0 1 1 0 2 1 1 0 1 Volume Module: 97 487 7 509 160 12 935 Base Vol: 322 693 424 94 1209 Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 Initial Bse: 101 506 7 529 166 335 721 441 98 12 972 0 0 0 -5 -2 0 0 Added Vol: -1 -3 0 0 -3 Ser. Hills2: Ω Ω 1 33 0 0 Ω 59 0 Ω 2.5 14 Initial Fut: 101 506 8 557 165 332 719 500 98 12 997 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 0.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 8 PHF Volume: 101 506 557 165 332 719 500 98 12 997 0 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: Reduced Vol: 101 506 8 557 165 332 719 500 98 12 997 ()1.00 1.00 1.00 1.00 1.00 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 0.00 8 557 165 FinalVolume: 101 506 332 719 500 98 12 997 OvlAdjVol: 0 -----| Saturation Flow Module: 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.97 0.03 3.00 1.00 1.00 2.00 1.67 0.33 1.00 2.00 Lanes: 1.00 55 5100 1700 1700 3400 2844 Final Sat.: 1700 3345 556 1700 3400 1700 -----| Capacity Analysis Module: Vol/Sat: 0.06 0.15 0.15 0.11 0.10 0.20 0.21 0.18 0.18 0.01 0.29 0.00 OvlAdjV/S: 0.00 **** *** Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #1 Nohl Ranch Rd/Stage Coach Rd ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5 Average Delay (sec/veh):
Optimal Cycle: 18 Level Of Service: XXXXXX ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----|----|-----|------||-------| Control: Permitted Permitted Permitted Permitted Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 Min. Green: Lanes: Volume Module: Base Vol: 4 489 0 0 263 59 206 0 3 0 0 Initial Bse: 4 489 0 0 263 59 206 0 3 0 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 0 PasserByVol: 0 0 Initial Fut: 4 489 0 0 0 0 0 0 0 0 0 263 59 206 0 3 0 0 ()PHF Volume: 4 489 59 0 0 0 0 263 206 0 3 0 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 206 0 Reduced Vol: 4 489 0 0 263 0 59 FinalVolume: 4 489 0 0 263 59 206 0 3 0 0 -----||-----||-----||-----| Saturation Flow Module: 1.00 Lanes: 1.00 2.00 0.00 0.00 1.63 0.37 1.00 0.00 1.00 0.00 0.00 0.00 Final Sat.: 1700 3400 0 0 2777 623 1700 0 1700 0 0 -----|-----|-----|-----| Capacity Analysis Module: Vol/Sat: 0.00 0.14 0.00 0.00 0.09 0.09 0.12 0.00 0.00 0.00 0.00 0.00 Crit Moves: **** ****

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #4 Kendra Dr/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 23 Level Of Service: XXXXXX ************************** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Control: Permitted Permitted Protected Protected Rights: Include Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Reduct Vol: 0 0 32 304 32 110 15 115 15 12 1072 35 0 0 0 0 0 0 0 0 0 0 32 Reduced Vol: 32 8 110 15 32 304 15 12 1072 115 3.5 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PCE Adj: 1.00 MLF Adj: FinalVolume: 32 8 32 110 15 115 32 304 15 12 1072 Saturation Flow Module: Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lanes: 1.00 0.20 0.80 1.00 0.12 0.88 1.00 2.00 1.00 1.00 2.00 1.00 Final Sat.: 1700 340 1360 1700 196 1504 1700 3400 1700 1700 3400 1700 -----|----|-----|------| Capacity Analysis Module: Vol/Sat: 0.02 0.02 0.02 0.06 0.08 0.08 0.02 0.09 0.01 0.01 0.32 Crit Moves: **** *** **************************

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #5 Nohl Ranch Rd/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 39 Level Of Service: XXXXXX ******************************* Serrano Ave Street Name: Nohl Ranch Rd Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - RProtected Protected Protected Protected Include Include Include Include Include Include Rights: 0 0 0 Min. Green: Y+R: $\begin{smallmatrix} 0 & 0 & 0 & 0 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 \\ \end{smallmatrix}$ Volume Module: 0 0 0 92 0 Base Vol: 226 184 286 0 0 925 108 Initial Bse: 0 0 0 92 0 226 184 286 0 0 925 0 0 0 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 PasserByVol: 0 0 0 0 0 0 0 Ω 0 0 Initial Fut: 0 0 0 92 0 226 0 925 184 286 0 275 0 1127 PHF Volume: 0 0 0 112 0 224 348 0 132 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 Reduced Vol: 0 0 0 0 112 0 275 224 348 0 1127 132 1.00 1.00 FinalVolume: 0 0 0 112 0 275 224 348 0 0 1127 -----||-----||-----||-----| Saturation Flow Module: 1700 Final Sat.: 0 0 0 3400 0 1700 1700 3400 0 0 3045 Capacity Analysis Module: Vol/Sat: 0.00 0.00 0.00 0.03 0.00 0.16 0.13 0.10 0.00 0.00 0.37 0.37 Crit Moves: ******************************

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #9 Canyon Rim Rd/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 25 Level Of Service: XXXXXX ************************** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Control: Permitted Permitted Permitted Permitted Include Include Rights: Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #10 Cannon Street/Serrano Avenue ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5
Optimal Cycle: 43 Average Delay (sec/veh): Level Of Service: XXXXXX ******************************* Cannon Street Serrano Avenue Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Protected
Ovl Include Include Protected Include 0 0 0 Rights: $\bigcap \nabla Z \bigcap$ 0 0 0 0 0 0 0 Min. Green: 0 0 Y+R:0 0 3 0 2 1 0 3 0 0 0 0 0 0 0 2 0 0 0 1 -----||-----||-----||------| Volume Module: Base Vol: 0 1321 315 46 1779 0 1189 0 0 0 177 1.00 1.00 1.00 Initial Bse: 0 1321 315 46 1779 0 0 0 0 1189 177 0 0 0 0 0 0 0 Added Vol: 0 0 0 0 0 0 0 0 0 Ω 0 0 0 0 \cap 0 0 PasserByVol: 0 Initial Fut: 0 1321 315 46 1779 0 0 0 1189 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 46 1779 0 PHF Volume: 0 1321 315 0 0 0 1189 177 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 Reduced Vol: 0 1321 315 46 1779 0 0 0 1189 177 0 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 0 1321 315 46 1779 0 0 0 0 1189 177 OvlAdjVol: 0 131 -----|----|-----|-----| Saturation Flow Module: 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.00 3.00 2.00 1.00 3.00 0.00 0.00 0.00 0.00 2.00 0.00 Lanes: 1.00 Final Sat.: 0 5100 3400 1700 5100 0 0 0 0 3400 0 1700 -----| Capacity Analysis Module: Vol/Sat: 0.00 0.26 0.09 0.03 0.35 0.00 0.00 0.00 0.00 0.35 0.00 0.10 OvlAdjV/S: 0.00 0.08 *** **** Crit Moves: ****

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #11 Cannon Street/ Taft Avenue ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5
Optimal Cycle: 44 Average Delay (sec/veh): Level Of Service: XXXXXX ******************************* Cannon Street Taft Avenue Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Permitted Include Include Ovl Permitted Rights: Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green: Y+R: 1 0 3 0 0 1 0 2 1 0 0 1 0 0 1 1 0 0 0 -----||-----||-----||------| Volume Module: 0 0 2894 Base Vol: 44 1429 159 136 1 160 2 \cap Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 44 1429 0 0 2894 159 136 1 160 0 0 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 0 0 0 \cap 0 0 Ω 0 \cap 0 PasserByVol: 0 Initial Fut: 44 1429 0 2894 159 136 1 160 1.00 1.00 User Adj: 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0 PHF Volume: 44 1429 0 2894 159 136 1 160 2 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 Reduced Vol: 44 1429 0 0 2894 159 136 160 2. 1 0 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 44 1429 0 0 2894 159 136 1 160 OvlAdjVol: 116 _____| Saturation Flow Module: 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 3.00 0.00 1.00 2.84 0.16 0.99 0.01 1.00 1.00 0.00 0.00 Lanes: 0 1700 4834 266 1688 12 1700 Final Sat.: 1700 5100 1700 0 -----| Capacity Analysis Module: Vol/Sat: 0.03 0.28 0.00 0.00 0.60 0.60 0.08 0.08 0.09 0.00 0.00 0.00 OvlAdjV/S: 0.07 **** **** Crit Moves: ****

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #12 Cannon Street/E Santiago Canyon Road ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5
Optimal Cycle: 51 Average Delay (sec/veh): Level Of Service: XXXXXX ******************************* Cannon Street E Santiago Canyon Road Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RSplit Phase Split Phase Protected Include Ignore Include Protected 0 Ignore Include 0 0 0 0 0 Rights: Ignore 0 0 0 0 0 0 Min. Green: Y+R: 2 0 1 1 0 3 0 3 0 1 2 0 3 0 1 2 0 3 0 1 Volume Module: 49 1919 537 Base Vol: 138 169 576 156 726 67 147 1321 1151 1.00 1.00 Initial Bse: 138 169 49 1919 537 576 156 726 67 147 1321 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 Ω 0 0 0 Ω Ω Ω Ω 0 0 0 0 PasserByVol: 49 1919 537 Initial Fut: 138 169 576 156 726 67 147 1321 1.00 1.00 1.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 User Adj: 0.00 1.00 1.00 1.00 0.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 0.00 49 PHF Volume: 138 169 1919 537 0 156 726 67 147 1321 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 Reduced Vol: 138 169 49 1919 537 0 156 726 67 147 1321 0 1.00 1.00 PCE Adj: 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 0.00 MLF Adj: 49 1919 537 0 156 726 FinalVolume: 138 169 67 -----| |---------||-----Saturation Flow Module: Sat/Lane: 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1.00 1.00 1.00 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lanes: 2.00 1.55 0.45 3.00 3.00 1.00 2.00 3.00 1.00 2.00 3.00 1.00 764 5100 5100 1700 3400 5100 1700 3400 5100 Final Sat.: 3400 2636 1700 -----||-----||-----||------| Capacity Analysis Module: Vol/Sat: 0.04 0.06 0.06 0.38 0.11 0.00 0.05 0.14 0.04 0.04 0.26 0.00 Crit Moves: **** **** ******************************

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #1 Nohl Ranch Rd/Stage Coach Rd ******************************* 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5 Average Delay (sec/veh):
Optimal Cycle: 16 Level Of Service: XXXXXX ************************** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Control: Permitted Permitted Permitted Permitted Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #4 Kendra Dr/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5
Optimal Cycle: 22 Average Delay (sec/veh):
Level Of Service: XXXXXX ************************** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----| Control: Permitted Permitted Protected Protected Include Rights: Include Include Include 0 0 0 0 0 0 0 0 0 0 0 Min. Green:

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Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #5 Nohl Ranch Rd/Serrano Ave ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh): Optimal Cycle: 21 Level Of Service: XXXXXX ************************** Serrano Ave Street Name: Nohl Ranch Rd Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - RProtected Protected Protected Protected Include Include Include Include Include Rights: Include 0 0 0 Min. Green: Y+R: $\begin{smallmatrix} 0 & 0 & 0 & 0 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 & 2 & 0 & 0 & 0 & 1 & 1 & 0 \\ \end{smallmatrix}$ Volume Module: 0 0 0 Base Vol: 142 0 66 81 1152 0 0 389 105 Initial Bse: 0 0 0 142 0 66 81 1152 0 0 389 0 0 0 0 0 0 0 0 0 142 0 66 0 0 0 0 0 0 Added Vol: 0 0 PasserByVol: 0 0 0 0 0 0 Ω 0 Initial Fut: 0 0 81 1152 0 0 389 1.00 66 81 1152 0 0 389 PHF Volume: 0 0 0 142 0 105 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 Reduced Vol: 66 0 0 0 0 142 0 81 1152 0 389 105 1.00 1.00 FinalVolume: 0 0 0 142 0 66 81 1152 0 0 389 -----||-----||-----||-----| Saturation Flow Module: 1700 Final Sat.: 0 0 0 3400 0 1700 1700 3400 0 0 2677 Capacity Analysis Module: Vol/Sat: 0.00 0.00 0.00 0.04 0.00 0.04 0.05 0.34 0.00 0.00 0.15 0.15 Crit Moves: *** ******************************

Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************** Intersection #9 Canyon Rim Rd/Serrano Ave ******************************** 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5 Average Delay (sec/veh):
Optimal Cycle: 23 Level Of Service: XXXXXX ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Control: Permitted Permitted Permitted Permitted Include Include Include Rights: Include 0 0 0 0 0 0 0 0 0 0 Min. Green: Lanes: -----| Volume Module: 0 322 0 74 87 1085 0 0 497 Base Vol: 0 0 1.00 Initial Bse: 0 0 0 322 0 74 87 1085 0 0 497 122 0 0 0 0 0 0 0 0 0 0 0 0 0 0 322 0 74 87 1085 0 0 Added Vol: 0 0 PasserByVol: 0 0 Initial Fut: 0 0 0 0 0 0 0 0 497 122 0 497 PHF Volume: 0 0 322 0 74 87 1085 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 0 0 0 322 0 74 87 1085 0 0 0 0 0 0 497 122 1.00 1.00 FinalVolume: 0 0 0 322 0 74 87 1085 0 0 497 -----| Saturation Flow Module: Lanes: 0.00 0.00 0.00 1.63 0.00 0.37 1.00 2.00 0.00 0.00 1.61 0.39 Final Sat.: 0 0 0 2765 0 635 1700 3400 0 0 2730 670 -----|----|-----|------| Capacity Analysis Module: Vol/Sat: 0.00 0.00 0.00 0.12 0.00 0.12 0.05 0.32 0.00 0.00 0.18 0.18

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0.00

OvlAdjV/S:

Crit Moves:

GP PM plus project Wed May 22, 2019 17:27:46 Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #10 Cannon Street/Serrano Avenue ******************************** 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5
Optimal Cycle: 26 Average Delay (sec/veh): Level Of Service: XXXXXX ******************************* Cannon Street Serrano Avenue Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Protected
Ovl Include Include Protected Include 0 0 0 Rights: $\bigcap \nabla Z \bigcap$ 0 0 0 0 0 0 0 Min. Green: 0 0 Y+R: 0 0 3 0 2 1 0 3 0 0 0 0 0 0 0 2 0 0 0 1 -----||-----||-----||------| Volume Module: Base Vol: 0 1534 1459 127 1901 0 355 63 0 0 Ω Initial Bse: 0 1534 1459 127 1901 0 0 0 0 355 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 0 0 0 0 Ω Ω 0 0 0 0 0 0 0 PasserByVol: 0 Initial Fut: 0 1534 1459 127 1901 0 0 0 355 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0 PHF Volume: 0 1534 1459 127 1901 0 0 0 355 63 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 Reduced Vol: 0 1534 1459 127 1901 0 0 Ω 355 Ω 6.3 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 0 1534 1459 127 1901 0 0 0 0 355 OvlAdjVol: 1104 0 -----|----|-----|-----| Saturation Flow Module: 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lanes: 0.00 3.00 2.00 1.00 3.00 0.00 0.00 0.00 0.00 2.00 0.00 1.00 Final Sat.: 0 5100 3400 1700 5100 0 0 0 3400 0 1700 -----| Capacity Analysis Module: Vol/Sat: 0.00 0.30 0.43 0.07 0.37 0.00 0.00 0.00 0.00 0.10 0.00 0.04

0.32

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #11 Cannon Street/ Taft Avenue ******************************** 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 5
Optimal Cycle: 52 Average Delay (sec/veh): Level Of Service: XXXXXX ******************************* Cannon Street Taft Avenue Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Permitted Include Include Ovl Permitted Rights: 0 Include 0 0 0 0 0 0 0 0 0 0 Min. Green: Y+R: 1 0 2 1 0 1 0 2 1 0 0 1 0 0 1 -----||-----||-----||------| Volume Module: 62 2601 1 1 2109 Base Vol: 168 403 44 1 0 \cap Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 62 2601 1 1 2109 168 403 1 44 0 0 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 0 0 0 0 0 0 Ω 0 0 0 PasserByVol: 1 Initial Fut: 62 2601 1 2109 168 403 1 44 0 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 PHF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1 403 PHF Volume: 62 2601 1 2109 168 1 44 \cap 0 0 0 0 0 0 0 0 0 0 Reduct Vol: Reduced Vol: 62 2601 1 1 2109 168 403 44 Ω 1 0 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 62 2601 1 1 2109 168 403 1 44 0 1 OvlAdjVol: 0 -----|----| ----||-----| Saturation Flow Module: 1700 1700 1700 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 2.99 0.01 1.00 2.78 0.22 0.99 0.01 1.00 0.00 1.00 0.00 Lanes: 2 1700 4724 376 1696 4 1700 Final Sat.: 1700 5098 0 1700 -----|----|-----|-----| Capacity Analysis Module: Vol/Sat: 0.04 0.51 0.51 0.00 0.45 0.45 0.24 0.24 0.03 0.00 0.00 0.00 OvlAdjV/S: 0.00 **** **** **** **** Crit Moves:

Level Of Service Computation Report ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative) ************************* Intersection #12 Cannon Street/E Santiago Canyon Road ******************************** 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 5
Optimal Cycle: 55 Average Delay (sec/veh): Level Of Service: XXXXXX ******************************* Cannon Street E Santiago Canyon Road Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R Split Phase Split Phase Protected Protected Include Ignore Include Ignore 0 0 0 0 0 0 0 0 0 Rights: 0 0 Min. Green: Y+R: $\begin{smallmatrix}2&0&1&1&0&&3&0&3&0&1&&2&0&3&0&1&&2&0&3&0&1\\\end{smallmatrix}$ Volume Module: 145 467 56 1518 223 Base Vol: 372 496 993 143 33 847 1752 372 496 993 Initial Bse: 145 467 56 1518 223 143 33 847 1752 0 0 0 0 0 0 0 0 0 0 Added Vol: 0 0 0 Ω Ω 0 0 0 Ω 0 Ω 0 0 0 PasserByVol: 56 Initial Fut: 145 467 1518 223 372 496 993 143 33 847 1.00 1.00 1.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 User Adj: 0.00 PHF Adj: 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 56 0 PHF Volume: 145 467 1518 223 496 993 143 33 847 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 Reduced Vol: 145 467 56 1518 223 0 496 993 143 33 847 0 PCE Adj: 1.00 1.00 1.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 1.00 0.00 1.00 1.00 1.00 0.00 MLF Adj: FinalVolume: 145 467 56 1518 223 0 496 993 143 33 847 -----| | ---------||----Saturation Flow Module: Sat/Lane: 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1700 1.00 1.00 1.00 Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lanes: 2.00 1.79 0.21 3.00 3.00 1.00 2.00 3.00 1.00 2.00 3.00 1.00 364 5100 5100 1700 3400 5100 1700 3400 5100 Final Sat.: 3400 3036 1700 Capacity Analysis Module: Vol/Sat: 0.04 0.15 0.15 0.30 0.04 0.00 0.15 0.19 0.08 0.01 0.17 0.00 Crit Moves: **** **** ******************************

APPENDIX C

HCM LEVEL OF SERVICE WORKSHEETS

Intersection						
Int Delay, s/veh	1					
		MES	NET	NES	0.51	057
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		ħβ		ሻ	^
Traffic Vol, veh/h	35	15	246	8	3	273
Future Vol, veh/h	35	15	246	8	3	273
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage,	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	38	16	267	9	3	297
	/linor1		//ajor1		Major2	
Conflicting Flow All	427	138	0	0	276	0
Stage 1	272	-	-	-	-	-
Stage 2	155	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	_	_	_	-
Follow-up Hdwy	3.52	3.32	_	-	2.22	-
Pot Cap-1 Maneuver	556	885	-	_	1284	_
Stage 1	749	-	_	_	_	_
Stage 2	857	_	_	_	_	_
Platoon blocked, %	001		_	_		_
Mov Cap-1 Maneuver	555	885			1284	_
Mov Cap-1 Maneuver	555	- 005	_		1204	
	748	-	-	-		-
Stage 1		-	-	-	-	-
Stage 2	857	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	11.3		0		0.1	
HCM LOS	В					
		NET	NES	A/DL 4	051	057
Minor Lane/Major Mvm	t	NBT	NBKV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-		1284	-
HCM Lane V/C Ratio		-	-		0.003	-
HCM Control Delay (s)		-	-		7.8	-
HCM Lane LOS		-	-	В	Α	-
HCM 95th %tile Q(veh)		-	-	0.3	0	-

N-157

Intersection						
Int Delay, s/veh	0.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
	WDL	WDK	<u>ND1</u>	NDI	SDL	↑
Lane Configurations	^			^	00	
Traffic Vol, veh/h	9	21	254	9	28	308
Future Vol, veh/h	9	21	254	9	28	308
Conflicting Peds, #/hr	0	0	0	0	0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	10	23	276	10	30	335
	Minor1		Major1		Major2	
Conflicting Flow All	475	143	0	0	286	0
Stage 1	281	-	-	-	-	-
Stage 2	194	-	-	-	-	-
Critical Hdwy	6.29	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	6.04	-	_	-	-	-
Follow-up Hdwy	3.67	3.32	-	_	2.22	-
Pot Cap-1 Maneuver	540	879	_	_	1273	_
Stage 1	715	-	_	_	-	_
Stage 2	781	_	_	_	_	_
Platoon blocked, %	701	_	_	_	_	_
-	524	879		-	1273	
Mov Cap-1 Maneuver			-	-		-
Mov Cap-2 Maneuver	524	-	-	-	-	-
Stage 1	694	-	-	-	-	-
Stage 2	781	-	-	-	-	-
Approach	WB		NB		SB	
			0		0.7	
HCM Control Delay, s	9.2		U		0.7	
HCM LOS	Α					
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	_	879	1273	-
HCM Lane V/C Ratio		_		0.026		_
HCM Control Delay (s)		_	_	9.2	7.9	_
HCM Lane LOS		_	_	Α.Σ	Α.5	_
HCM 95th %tile Q(veh)		-	_	0.1	0.1	-
HOW SOUT WITH Q(Ven)		-	-	U. I	U. I	-

Intersection						
Int Delay, s/veh	0.3					
		EDT	WDT	WDD	CDI	CDD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻ	† †	† ‡	_	_	7
Traffic Vol, veh/h	9	319	862	5	5	16
Future Vol, veh/h	9	319	862	5	5	16
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	10	347	937	5	5	17
NA - ' /NA'		_	4	_	I' C	
	lajor1		Major2		Minor2	4
Conflicting Flow All	942	0	-	0	1134	471
Stage 1	-	-	-	-	940	-
Stage 2	-	-	-	-	194	-
Critical Hdwy	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	724	-	-	-	196	539
Stage 1	-	-	-	-	340	-
Stage 2	-	-	-	-	820	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	724	_	_	-	193	539
Mov Cap-2 Maneuver	-	_	_	_	282	-
Stage 1	_	_	_	_	335	_
Stage 2	_	_	_	_	820	_
Jugo 2					520	
Approach	EB		WB		SB	
HCM Control Delay, s	0.3		0		11.9	
HCM LOS					В	
NA' 1 (NA - ' NA C		EBL	EBT	WBT	WBR	SBI n1
N/IIDOR I and/N/Iaior N/IV/mt			LDI	VVDI	WDR	
Minor Lane/Major Mvmt		701		_	-	539
Capacity (veh/h)		724	-			0.020
Capacity (veh/h) HCM Lane V/C Ratio		0.014	-	-		0.032
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		0.014 10	-	-	-	11.9
Capacity (veh/h) HCM Lane V/C Ratio		0.014				

Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	† 1>		ሻ	†	¥	, LOIK
Traffic Vol, veh/h	315	4	5	850	12	6
Future Vol, veh/h	315	4	5	850	12	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	_	-	0	-	0	-
Veh in Median Storage		_	-	0	0	_
Grade, %	0	<u>-</u>	_	0	0	<u>-</u>
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	342	4	5	924	13	7
IVIVIIIL FIOW	342	4	ິນ	924	13	1
Major/Minor I	Major1	N	Major2	ı	Minor1	
Conflicting Flow All	0	0	346	0	816	173
Stage 1	-	-	-	-	344	-
Stage 2	-	-	-	-	472	-
Critical Hdwy	-	_	4.14	_	6.84	6.94
Critical Hdwy Stg 1	_	_	-	_	5.84	-
Critical Hdwy Stg 2	_	_	_	_	5.84	_
Follow-up Hdwy	_	_	2.22	_	3.52	3.32
Pot Cap-1 Maneuver	_	_	1210	_	315	840
Stage 1	_	_	-	_	689	-
Stage 2	_	_	_	_	594	_
Platoon blocked, %	_	_		_	J34	
Mov Cap-1 Maneuver	_		1210	_	314	840
Mov Cap-1 Maneuver	_	_	1210	_	431	-
		-	-			
Stage 1	-	-	-	-	686	-
Stage 2	-	-	-	-	594	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		12.3	
HCM LOS					В	
					U	
Minor Lane/Major Mvm	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		515	-		1210	-
HCM Lane V/C Ratio		0.038	-	-	0.004	-
HCM Control Delay (s)		12.3	-	-	8	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh))	0.1	-	-	0	-

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻ	^	† 1>		Y	
Traffic Vol, veh/h	9	313	845	7	0	7
Future Vol, veh/h	9	313	845	7	0	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	_		-	None
Storage Length	0	-	_	-	0	-
Veh in Median Storage,		0	0	_	0	_
Grade, %	, <i>''</i>	0	0	_	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	10	340	918	8	0	8
IVIVIII(IIOVV	10	J + 0	310	U	U	U
Major/Minor N	Major1	N	//ajor2	N	/linor2	
Conflicting Flow All	926	0	-	0	1112	463
Stage 1	-	-	-	-	922	-
Stage 2	-	-	-	-	190	-
Critical Hdwy	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	_	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	734	-	-	-	203	546
Stage 1	-	-	_	-	348	-
Stage 2	_	-	-	_	823	-
Platoon blocked, %		_	_	_		
Mov Cap-1 Maneuver	734	_	_	_	200	546
Mov Cap-2 Maneuver	-	_	_	_	200	-
Stage 1	_	_	_	_	343	_
Stage 2	_	_	_	_	823	_
Olage 2					020	
Approach	EB		WB		SB	
HCM Control Delay, s	0.3		0		11.7	
HCM LOS					В	
Minor Lane/Major Mvmt	t	EBL	EBT	WBT	WBR :	SRI n1
Capacity (veh/h)		734		AADT	- 1000	546
HCM Lane V/C Ratio		0.013	-	-		0.014
		10	-	-	-	11.7
HCM Control Delay (s) HCM Lane LOS			-	-		
HCM 95th %tile Q(veh)		A 0	-	-	-	B 0
HOIVI 95(II %(IIIE Q(VEN)		U	-	-	-	U

Intersection						
Int Delay, s/veh	0.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
	WDL	WDK	<u>ND1</u>	NDI	SDL T	<u> </u>
Lane Configurations		10		40		
Traffic Vol, veh/h	9	10	256	12	11	205
Future Vol, veh/h	9	10	256	12	11	205
Conflicting Peds, #/hr	0	0	_ 0	0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	10	11	278	13	12	223
	. •		•		•=	
		_				
Major/Minor I	Minor1	N	Major1	ľ	Major2	
Conflicting Flow All	421	146	0	0	291	0
Stage 1	285	-	-	-	-	-
Stage 2	136	-	-	-	-	-
Critical Hdwy	6.84	6.94	_	-	4.14	-
Critical Hdwy Stg 1	5.84	_	_	_	_	_
Critical Hdwy Stg 2	5.84	_	_	-	_	_
Follow-up Hdwy	3.52	3.32	_	_	2.22	_
Pot Cap-1 Maneuver	561	875	_	_	1268	_
Stage 1	738	-	_	_	1200	_
	876	_		_	-	_
Stage 2	0/0	-	-	-	-	
Platoon blocked, %	550	075	-	-	4000	-
Mov Cap-1 Maneuver	556	875	-	-	1268	-
Mov Cap-2 Maneuver	556	-	-	-	-	-
Stage 1	731	-	-	-	-	-
Stage 2	876	-	-	-	-	-
Annroach	WB		NB		SB	
Approach						
HCM Control Delay, s	10.4		0		0.4	
HCM LOS	В					
Minor Lane/Major Mvm	t	NBT	NRRV	VBLn1	SBL	SBT
		-	-	688	1268	-
Capacity (veh/h)			-		0.009	
HCM Control Polov (a)		-	-			-
HCM Control Delay (s)		-	-	10.4	7.9	-
HCM Lane LOS		-	-	В	A	-
HCM 95th %tile Q(veh)		-	-	0.1	0	-

Intersection						
Int Delay, s/veh	0.9					
		W/PD	NDT	NIDD	CDI	SBT
Movement Configurations	WBL	WBR	NBT ↑ ↑	NBR	SBL	↑ ↑↑
Lane Configurations	0			0	00	
Traffic Vol, veh/h	8	19	268	9	28	214
Future Vol, veh/h	8	19	268	9	28	214
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	9	21	291	10	30	233
Major/Minor N	/linor1	N	/lajor1	N	Major2	
Conflicting Flow All	449	151	0	0	301	0
Stage 1	296	-	-	_	-	-
Stage 2	153	<u>-</u>	_	_	_	_
Critical Hdwy	6.29	6.94	_	-	4.14	_
Critical Hdwy Stg 1	5.84	0.34	_	_	4.14	
Critical Hdwy Stg 2	6.04	_	-	_	_	_
Follow-up Hdwy	3.67	3.32	_	_	2.22	_
Pot Cap-1 Maneuver	558	868	_		1257	_
Stage 1	703	-	_	_	1231	_
Stage 2	820	_	_		-	_
Platoon blocked, %	020	-	-	-	-	-
-	543	868	-	-	1257	-
Mov Cap-1 Maneuver			-	-	1237	-
Mov Cap-2 Maneuver	543	-	-	-	-	-
Stage 1	684	-	-	-	-	-
Stage 2	820	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9.2		0		0.9	
HCM LOS	Α		•			
I IOIVI LOO						
TIOW EOS						
		NET	NDE	MDL 4	051	ODT
Minor Lane/Major Mvmt	t	NBT	NBRV	VBLn1	SBL	SBT
Minor Lane/Major Mvmt		NBT -	-	868	1257	SBT -
Minor Lane/Major Mvml Capacity (veh/h) HCM Lane V/C Ratio	<u> </u>	NBT - -	-	868 0.024	1257 0.024	SBT - -
Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	t	-	-	868 0.024 9.2	1257 0.024 7.9	-
Minor Lane/Major Mvml Capacity (veh/h) HCM Lane V/C Ratio	t .	-	-	868 0.024	1257 0.024	-

Intersection						
Int Delay, s/veh	0.2					
	EDI	EDT	\\/DT	\\/DD	CDI	CDD
Movement	EBL	EBT	WBT	WBR	SBL	SBR *
Lane Configurations	ሻ		† ‡	-	-	
Traffic Vol, veh/h	9	1192	453	5	5	14
Future Vol, veh/h	9	1192	453	5	5	14
Conflicting Peds, #/hr	_ 0	_ 0	_ 0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	10	1296	492	5	5	15
Major/Minor N	/lajor1		Major2		Minor2	
						0.40
Conflicting Flow All	497	0	-		1163	249
Stage 1	-	-	-	-	495	-
Stage 2	-	-	-	-	668	-
Critical Hdwy	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	1063	-	-	-	188	751
Stage 1	-	-	-	-	578	-
Stage 2	-	-	-	-	471	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1063	-	-	-	186	751
Mov Cap-2 Maneuver	_	-	_	-	317	-
Stage 1	-	_	-	_	573	-
Stage 2	_	_	_	_	471	_
Approach	EB		WB		SB	
HCM Control Delay, s	0.1		0		9.9	
HCM LOS					Α	
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR S	SBI n1
		1063	LDI	VVDI	WDICC	751
Canacity (yoh/h)		0.009	-	-	-	0.02
Capacity (veh/h)			-	-	-	
HCM Lane V/C Ratio						0.0
HCM Lane V/C Ratio HCM Control Delay (s)		8.4	-	-	-	9.9
HCM Lane V/C Ratio				-	-	9.9 A 0.1

Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑ 1>		ሻ	^	Y	
Traffic Vol, veh/h	1182	10	0	443	10	3
Future Vol, veh/h	1182	10	0	443	10	3
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-	None	-	None
Storage Length	-	-	0	-	0	-
Veh in Median Storag	e.# 0	-	-	0	0	-
Grade, %	0	_	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1285	11	0	482	11	3
		• •	•		• •	
Major/Minor	Major1		Major2		/linor1	
Conflicting Flow All	0	0	1296	0	1532	648
Stage 1	-	-	-	-	1291	-
Stage 2	-	-	-	-	241	-
Critical Hdwy	-	-	4.14	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	-	-	2.22	-	3.52	3.32
Pot Cap-1 Maneuver	-	-	531	-	107	413
Stage 1	-	-	-	-	222	-
Stage 2	-	-	-	-	776	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	531	-	107	413
Mov Cap-2 Maneuver		-	-	-	187	-
Stage 1	-	-	-	-	222	-
Stage 2	-	-	-	-	776	-
Δ	- FD		1A/D		ND	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		23	
HCM LOS					С	
Minor Lane/Major Mvr	nt l	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		214	-	-	531	-
HCM Lane V/C Ratio		0.066	-	-	-	-
HCM Control Delay (s	s)	23	-	-	0	-
HCM Lane LOS		С	-	-	A	-
HCM 95th %tile Q(veh	n)	0.2	-	-	0	-

0.2	2					
EDI	г	CDT	WDT	WDD	CDI	CDD
				WRK		SBR
						3
						3
0		0				0
Free	e Fr	ree	Free	Free	Stop	Stop
-	- No	lone	-	None	-	None
0	J	-	-	-	0	-
		0	0	-	0	-
-				_		_
			-			92
						2
						3
24	+ 12	201	402	14	3	3
/laior1	1	N	/laior2	N	/linor2	
						248
						-
			-			-
4.14	ļ	-	-	-		6.94
-	-	-	-	-		-
-		-	-	-		-
2.22	2	-	-	-	3.52	3.32
1064	1	-	-	-	186	752
-	-	-	_		582	_
_				-		_
	_	_	_	_		_
	-		-	-	465	
1064		-	-	-	465	-
1064	4	-	- - -	- - -	465 182	- 752
-	4 -		- - - -	- - -	465 182 182	- 752 -
-	4 - -	- - -	- - - -	- - -	465 182 182 569	- 752 - -
-	4 - -	-	- - - -	- - -	465 182 182	- 752 -
-	4 - -	- - -	- - - -	- - -	465 182 182 569	- 752 - -
- - -	4 - - -	- - -	- - - - -	- - -	182 182 569 465	- 752 - -
- - - EB	4 - - -	- - -	- - - - - WB	- - -	182 182 569 465	- 752 - -
- - -	4 - - -	- - -	- - - - -	- - -	182 182 569 465 SB 17.6	- 752 - -
- - - EB	4 - - -	- - -	- - - - - WB	- - -	182 182 569 465	- 752 - -
- - - EB	4 - - -	- - -	- - - - - WB	- - -	182 182 569 465 SB 17.6	- 752 - -
EB 0.2	4 - - - 3 2	-	- - - - - WB	-	182 182 569 465 SB 17.6 C	- 752 - - -
- - - EB	4 - - - 3 2	- - - - -	- - - - - WB	- - -	465 182 182 569 465 SB 17.6 C	752 - - - -
EB 0.2	4 - - - 3 2 = 10	- - - - - - - 064	- - - - - WB 0	- - - - - - WBT	465 182 182 569 465 SB 17.6 C	752 - - - - SBLn1 293
EB 0.2	4 - - - 3 2 10 0.0	EBL 064	- - - - - WB 0	- - - - - - WBT	465 182 182 569 465 SB 17.6 C	752 - - - - - SBLn1 293 0.022
EB 0.2	4 - - - 3 2 10 0.0	EBL 064 .022 8.5	- - - - - - WB 0	- - - - - - - - - - - - - - - - - - -	465 182 182 569 465 SB 17.6 C	752 - - - - - - - - - - - - - - - - - - -
EB 0.2	44	EBL 064	- - - - - WB 0	- - - - - - WBT	465 182 182 569 465 SB 17.6 C	752 - - - - - SBLn1 293 0.022
	222 222 (C) Free (C) # 92 22 4.14 4.14 2.22 1064	EBL 22 1 22 1 0 Free I - N 0 # 92 2 2 4 1 Major1 496 4.14 - 2.22 1064	EBL EBT 22 1160 22 1160 0 0 Free Free - None 0 - # - 0 92 92 2 2 24 1261 Major1 N 496 0 4.14 2.22 - 1064 -	EBL EBT WBT 1 160 443 22 1160 443 0 0 0 Free Free Free - None 0 4 - 0 0 92 92 92 2 2 2 24 1261 482 Major1 Major2 496 0 4.14 2.22 - 1064	EBL EBT WBT WBR 22 1160 443 13 22 1160 443 13 0 0 0 0 0 Free Free Free Free - None - None 0 0 0 0 0 92 92 92 92 2 2 2 2 24 1261 482 14 Major1 Major2 M 496 0 - 0	EBL EBT WBT WBR SBL *** *** *** 22 1160 443 13 3 22 1160 443 13 3 0 0 0 0 0 Free Free Free Stop - None - 0 - None - 0 - 0 0 - 0 ** 0 0 - 0 92 92 92 92 92 2 2 2 2 2 24 1261 482 14 3 Major1 Major1 Major2 Minor2 496 0

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Intersection						
Int Delay, s/veh	1.1					
	WDI	WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	₩		† ‡		ሻ	† †
Traffic Vol, veh/h	36	16	256	8	3	264
Future Vol, veh/h	36	16	256	8	3	264
Conflicting Peds, #/hr	0	0	_ 0	0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	39	17	278	9	3	287
NA ' /NA'						
	/linor1		/lajor1		/lajor2	
Conflicting Flow All	433	144	0	0	287	0
Stage 1	283	-	-	-	-	-
Stage 2	150	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	_	-	-	-	_
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	551	877	-	-	1272	_
Stage 1	740	-	-	_	-	-
Stage 2	862	_	_	_	_	_
Platoon blocked, %	UUL		<u>-</u>	<u>-</u>		<u>-</u>
Mov Cap-1 Maneuver	550	877			1272	_
	550	- 011		-	1212	
Mov Cap-2 Maneuver			-			-
Stage 1	739	-	-	-	-	-
Stage 2	862	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	11.4		0		0.1	
HCM LOS	11.4 B		U		0.1	
I IOIVI LOG	D					
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	621	1272	-
HCM Lane V/C Ratio		-	_	0.091		-
HCM Control Delay (s)		-	-	11.4	7.8	-
HCM Lane LOS		_	_	В	A	_
HCM 95th %tile Q(veh)		_	_	0.3	0	_
Jili ootii 70tiio Q(VOII)				0.0	- 0	

Intersection						
Int Delay, s/veh	0.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	TIDE	77	↑ ↑	HOIN	ODL	† ††
Traffic Vol, veh/h	9	22	264	6	29	320
Future Vol, veh/h	9	22	264	6	29	320
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	-	None	-	None
Storage Length	-	0	-	None	_	NOHE
	.# 0		0	-	-	0
Veh in Median Storage	, # 0	-	0	-		0
Grade, %	92	92	92		- 02	92
Peak Hour Factor				92	92	
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	10	24	287	7	32	348
Major/Minor N	/linor1	N	Major1		Major2	
Conflicting Flow All	494	147	0	0	294	0
Stage 1	291	171	-	_	-	-
Stage 2	203	<u>-</u>	_	_	_	_
Critical Hdwy	6.29	6.94	_	_	4.14	_
Critical Hdwy Stg 1	5.84	0.34	_	_		_
Critical Hdwy Stg 2	6.04	_	-	_	-	<u>-</u>
	3.67	3.32	_	_	2.22	-
Follow-up Hdwy	527	873		<u>-</u>	1264	
Pot Cap-1 Maneuver	707		-	-		-
Stage 1		-	-	-	-	-
Stage 2	772	-	-	-	-	-
Platoon blocked, %	F44	070	-	-	1004	-
Mov Cap-1 Maneuver	511	873	-	-	1264	-
Mov Cap-2 Maneuver	511	-	-	-	-	-
Stage 1	685	-	-	-	-	-
Stage 2	772	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9.2		0		0.7	
HCM LOS	Α		U		0.1	
TIOM LOO						
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	873	1264	-
HCM Lane V/C Ratio		-	-	0.027		-
HCM Control Delay (s)		-	-	9.2	7.9	-
HCM Lane LOS		-	-	Α	Α	-
HCM 95th %tile Q(veh)		-	-	0.1	0.1	-
HCM 95th %tile Q(ven)		-	-	0.1	0.1	-

Intersection						
Int Delay, s/veh	0.3					
		FOT	MOT	14/55	051	055
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻ	† †	† ‡	_	_	7
Traffic Vol, veh/h	9	332	896	5	5	17
Future Vol, veh/h	9	332	896	5	5	17
Conflicting Peds, #/hr	_ 0	_ 0	_ 0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage,	,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	10	361	974	5	5	18
Major/Minor N	/lajor1	N	Major2	N	/linor2	
Conflicting Flow All	979	0	- -	0	1178	490
Stage 1	313	-	_	-	977	430
Stage 2		_	_	_	201	_
Critical Hdwy	4.14	-	-	-	6.84	6.94
•	4.14	-	-	-	5.84	0.94
Critical Hdwy Stg 1	-	-	-	-		-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	701	-	-	-	184	524
Stage 1	-	-	-	-	325	-
Stage 2	-	-	-	-	813	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	701	-	-	-	181	524
Mov Cap-2 Maneuver	-	-	-	-	269	-
Stage 1	-	-	-	-	320	-
Stage 2	-	-	-	-	813	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.3		0		12.1	
HCM LOS	0.0		U		В	
TICIVI LOS					D	
Minor Lane/Major Mvm	t	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)		701	-	-	-	524
HCM Lane V/C Ratio		0.014	-	-	-	0.035
HCM Control Delay (s)		10.2	-	-	-	12.1
HCM Lane LOS		В	-	-	-	В
HCM 95th %tile Q(veh)		0	-	-	-	0.1

Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	† ‡		*	^	Y	
Traffic Vol, veh/h	328	4	5	884	12	6
Future Vol, veh/h	328	4	5	884	12	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	0	-	0	-
Veh in Median Storage	e, # 0	-	_	0	0	-
Grade, %	0	-	_	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	357	4	5	961	13	7
WWITCHIOW	001	-	U	301	10	•
Major/Minor	Major1	N	Major2	N	Minor1	
Conflicting Flow All	0	0	361	0	850	181
Stage 1	-	-	-	-	359	-
Stage 2	-	-	-	-	491	-
Critical Hdwy	-	-	4.14	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	-	-	2.22	-	3.52	3.32
Pot Cap-1 Maneuver	-	-	1194	-	300	831
Stage 1	-	-	-	-	677	-
Stage 2	-	_	-	-	581	-
Platoon blocked, %	_	-		_		
Mov Cap-1 Maneuver	-	-	1194	-	299	831
Mov Cap-2 Maneuver	_	_	-	_	418	-
Stage 1	-	_	_	_	674	_
Stage 2	_	_	_	_	581	_
Olaye Z		_			JU 1	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		12.5	
HCM LOS					В	
Minor Long/Major M.		UDL 4	CDT	EDD	WDI	WDT
Minor Lane/Major Mvm	it f	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		501	-		1194	-
HCM Lane V/C Ratio		0.039	-		0.005	-
HCM Control Delay (s)		12.5	-	-	8	-
HCM Lane LOS HCM 95th %tile Q(veh		В	-	-	A	-
	1	0.1	_	_	0	-

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻ	^	† 1>		Y	
Traffic Vol, veh/h	9	326	879	7	0	7
Future Vol, veh/h	9	326	879	7	0	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	_	None	_		-	None
Storage Length	0	-	_	-	0	-
Veh in Median Storage,		0	0	_	0	-
Grade, %	_	0	0	_	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	10	354	955	8	0	8
WWWIICTIOW	10	004	300	U	U	U
Major/Minor N	/lajor1	Λ	//ajor2	N	Minor2	
Conflicting Flow All	963	0	-	0	1156	482
Stage 1	-	-	-	-	959	-
Stage 2	-	-	-	-	197	-
Critical Hdwy	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	711	-	-	-	190	530
Stage 1	_	-	_	-	333	-
Stage 2	-	-	_	_	817	_
Platoon blocked, %		_	_	_	•	
Mov Cap-1 Maneuver	711	_	_	_	187	530
Mov Cap-2 Maneuver	-	_	_	_	187	-
Stage 1	_	_	_	_	328	_
Stage 2	_	_	_	_	817	_
Stage 2					017	
Approach	EB		WB		SB	
HCM Control Delay, s	0.3		0		11.9	
HCM LOS					В	
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR :	SRI n1
				VVDI		
Capacity (veh/h)		711	-	-	-	530
HCM Courted Polocy (a)		0.014	-	-		0.014
HCM Long LOS		10.1	-	-	-	11.9
HCM C5th 0(tile O(tab)		В	-	-	-	В
HCM 95th %tile Q(veh)		0	-	-	-	0

Intersection						
Int Delay, s/veh	4					
	WDI	WED	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		† 1>	10	7	† †
Traffic Vol, veh/h	9	10	266	12	213	11
Future Vol, veh/h	9	10	266	12	213	11
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	10	11	289	13	232	12
manici ion	10	- 11	200	10	202	12
Major/Minor I	Minor1	N	/lajor1	N	Major2	
Conflicting Flow All	766	151	0	0	302	0
Stage 1	296	-	-	-	-	-
Stage 2	470	_	_	_	_	-
Critical Hdwy	6.84	6.94	_	_	4.14	_
Critical Hdwy Stg 1	5.84	-	_	_	- 1.17	_
Critical Hdwy Stg 2	5.84	_		<u>-</u>	-	<u>-</u>
	3.52	3.32	-	-	2.22	
Follow-up Hdwy			-	-		-
Pot Cap-1 Maneuver	339	868	-	-	1256	-
Stage 1	729	-	-	-	-	-
Stage 2	595	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	276	868	-	-	1256	-
Mov Cap-2 Maneuver	276	-	-	-	-	-
Stage 1	594	-	-	-	-	-
Stage 2	595	_	_	_	_	_
Olugo 2	000					
Approach	WB		NB		SB	
HCM Control Delay, s	13.8		0		8.1	
HCM LOS	В					
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	431	1256	-
HCM Lane V/C Ratio		-	_	0.048		-
HCM Control Delay (s)		-	-	13.8	8.5	-
HCM Lane LOS		_	_	В	A	_
HCM 95th %tile Q(veh)		_		0.2	0.7	_
Holvi Jour /oule Q(Ven)		_	_	0.2	0.7	

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Intersection						
Int Delay, s/veh	0.9					
					0.71	
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	↑ ↑			† ††
Traffic Vol, veh/h	8	20	279	6	29	223
Future Vol, veh/h	8	20	279	6	29	223
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	9	22	303	7	32	242
Major/Minor N	Minor1	N	Major1	ı	Major2	
Conflicting Flow All	468	155	0	0	310	0
Stage 1	307	-	-	U	310	-
Stage 2	161	_	_	_	_	_
Critical Hdwy	6.29	6.94	-	_	4.14	
•	5.84	0.94	-	-	4.14	_
Critical Hdwy Stg 1	6.04		-	-		
Critical Hdwy Stg 2		3.32	-	-	2.22	-
Follow-up Hdwy Pot Cap-1 Maneuver	3.67 545	863	-	-	1247	_
	694		-	-	1241	-
Stage 1	812	-	-	-	-	-
Stage 2	012	-	-	-	-	-
Platoon blocked, %	F00	000	-	-	1017	-
Mov Cap-1 Maneuver	529	863	-	-	1247	-
Mov Cap-2 Maneuver	529	-	-	-	-	-
Stage 1	673	-	-	-	-	-
Stage 2	812	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9.3		0		0.9	
HCM LOS	A				0.0	
	, ,					
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	000	1247	-
HCM Lane V/C Ratio		-	-		0.025	-
HCM Control Delay (s)		-	-	0.0	8	-
HCM Lane LOS		-	-	Α	Α	-
HCM 95th %tile Q(veh)		-	-	0.1	0.1	-

Intersection						
Int Delay, s/veh	0.2					
Movement	EDI	EDT	WPT	W/DD	CDI	CDD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	"	† †	† ‡		_	7
Traffic Vol, veh/h	9	1240	471	5	5	15
Future Vol, veh/h	9	1240	471	5	5	15
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage		0	0	-	0	-
Grade, %	, <i>''</i>	0	0	_	0	_
Peak Hour Factor	92	92	92	92	92	92
	2	2	2	2	2	2
Heavy Vehicles, %			512	5		
Mvmt Flow	10	1348	512	5	5	16
Major/Minor I	Major1	N	//ajor2	N	/linor2	
Conflicting Flow All	517	0	-	0	1209	259
					515	
Stage 1	-	-	-	-		-
Stage 2	-	-	-	-	694	-
Critical Hdwy	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	1045	-	-	-	175	740
Stage 1	-	-	-	-	565	-
Stage 2	_	_	_	_	457	_
Platoon blocked, %		_	_	_	101	
Mov Cap-1 Maneuver	1045	_	_	_	173	740
					304	740
Mov Cap-2 Maneuver	-	-	-	-		
Stage 1	-	-	-	-	559	-
Stage 2	-	-	-	-	457	-
Approach	EB		WB		SB	
			0		10	
HCM Control Delay, s	0.1		U			
HCM LOS					В	
Minor Lane/Major Mvm	t	EBL	EBT	WBT	WBR S	SBI n1
Capacity (veh/h)		1045	-	1101	-	740
				-		0.022
HCM Cantrol Dalay (a)		0.009	-	-		
HCM Control Delay (s)		8.5	-	-	-	10
HCM Lane LOS		Α	-	-	-	В
HCM 95th %tile Q(veh)		0	-	-	-	0.1
						0.

Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑ ↑	LUIX	VVDL Š	<u>₩</u>	NDL Y	NOI
	1229	10	0	461	10	3
·	1229	10	0	461	10	3
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	Stop -	None
Storage Length	_	NOITE	0	-	0	NOITE
Veh in Median Storage,		_	-	0	0	
Grade, %	# 0	-	_	0	0	_
Peak Hour Factor	92	92	92	92	92	92
		2	2			
Heavy Vehicles, %	2			2	2	2
Mvmt Flow	1336	11	0	501	11	3
Major/Minor M	ajor1	<u> </u>	Major2	N	Minor1	
Conflicting Flow All	0	0	1347	0	1593	674
Stage 1	_	-	_	-	1342	
Stage 2	_	_	_	-	251	-
Critical Hdwy	_	_	4.14	_	6.84	6.94
Critical Hdwy Stg 1	_	_	-	_	5.84	-
Critical Hdwy Stg 2	_	_	_	_	5.84	_
Follow-up Hdwy		_	2.22	<u>-</u>	3.52	3.32
Pot Cap-1 Maneuver	_	_	507	_	98	397
Stage 1	_	_	301	_	208	J91 -
Stage 2	_		-		768	-
		-	-		100	-
Platoon blocked, %	-		E07	-	00	207
Mov Cap-1 Maneuver	-	-	507	-	98	397
Mov Cap-2 Maneuver	-	-	-	-	176	-
Stage 1	-	-	-	-	208	-
Stage 2	-	-	-	-	768	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		24.2	
HCM LOS	U		U		C	
HOW LOO						
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		202	-	-	507	-
HCM Lane V/C Ratio		0.07	-	-	-	-
HCM Control Delay (s)		24.2	-	-	0	-
HCM Lane LOS		С	-	-	Α	-
HCM 95th %tile Q(veh)		0.2	-	-	0	-

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
	EBL T	<u></u> ★↑	WB1 ↑ ↑	WBK	SBL	SBK
Lane Configurations				1.1		2
Traffic Vol, veh/h	23	1206	461	14	3	3
Future Vol, veh/h	23	1206	461	14	3	3
Conflicting Peds, #/hr	0	0	0	0	0	0
3	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage,		0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	25	1311	501	15	3	3
Major/Minor M	ajor1	N	Major2	N	Minor2	
Conflicting Flow All	516	0	-	0	1215	258
Stage 1	-	-	_	-	509	230
	_	-	_	-	706	_
Stage 2	111		-			6.94
Critical Hdwy	4.14	-	-	-	6.84	
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
	1046	-	-	-	174	741
Stage 1	-	-	-	-	569	-
Stage 2	-	-	-	-	450	-
Platoon blocked, %		-	-	-		
	1046	-	-	-	170	741
Mov Cap-2 Maneuver	-	-	-	-	170	-
Stage 1	-	-	-	-	555	-
Stage 2	-	-	-	-	450	-
Annroach	EB		WB		SB	
Approach						
HCM Control Delay, s	0.2		0		18.3	
HCM LOS					С	
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR S	SBLn1
Capacity (veh/h)		1046	_	_	_	-
HCM Lane V/C Ratio		0.024	_	_	-	0.024
		8.5	_	_	_	
HCM Control Delay (s) HCM Lane LOS		Α	-	-	_	C
HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh)		A 0.1	-	-	-	0.1

Intersection						
Int Delay, s/veh	0.9					
	WBL	WBR	NBT	NBR	SBL	SBT
	WDL	WDK	IND ↑ }	NDK	SBL Š	<u> </u>
Lane Configurations		15		0		
Traffic Vol, veh/h	35	15	287	8	3	313
Future Vol, veh/h	35	15	287	8	3	313
Conflicting Peds, #/hr	0	0	0	0	0	0
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	38	16	312	9	3	340
Major/Minor M	inor1	1	/lajor1	N	Major2	
Conflicting Flow All	493	161	0	0	321	0
Stage 1	317	-	-	-	-	-
Stage 2	176	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	505	855	-	-	1236	-
Stage 1	711	-	-	-	-	-
Stage 2	837	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	504	855	-	-	1236	-
Mov Cap-2 Maneuver	504	-	-	-	-	-
Stage 1	710	-	-	-	_	-
Stage 2	837	-	-	_	_	_
- 1-1-13 -						
Approach	WB		NB		SB	
HCM Control Delay, s	11.9		0		0.1	
HCM LOS	В					
		NBT	NRR\	VBLn1	SBL	SBT
Minor Lane/Maior Mymt		וטוו	NDIN			ODT
Minor Lane/Major Mvmt				2/2	1236	-
Capacity (veh/h)		-				
Capacity (veh/h) HCM Lane V/C Ratio		-	-	0.095	0.003	-
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		-	-	0.095 11.9	0.003 7.9	-
Capacity (veh/h) HCM Lane V/C Ratio			-	0.095	0.003	

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Intersection						
Int Delay, s/veh	0.7					
		WED	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	† 1>			† ††
Traffic Vol, veh/h	9	21	295	9	28	348
Future Vol, veh/h	9	21	295	9	28	348
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	10	23	321	10	30	378
Major/Minor	line-1		lais=1		Maisro	
	/linor1		//ajor1		Major2	
Conflicting Flow All	537	166	0	0	331	0
Stage 1	326	-	-	-	-	-
Stage 2	211	-	-	-	-	-
Critical Hdwy	6.29	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	6.04	-	-	-	-	-
Follow-up Hdwy	3.67	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	498	849	-	-	1225	-
Stage 1	679	-	-	-	-	-
Stage 2	765	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	483	849	-	-	1225	-
Mov Cap-2 Maneuver	483	-	-	-	-	-
Stage 1	658	-	-	-	-	-
Stage 2	765	-	_	_	_	_
	. 00					
	1675					
Approach	WB		NB		SB	
HCM Control Delay, s	9.4		0		0.6	
HCM LOS	Α					
Minor Lane/Major Mvm	1	NBT	NRRV	VBLn1	SBL	SBT
		INDI				ושט
Capacity (veh/h)		-	-	0.0	1225	-
U('\\\ ana \//\\ Dati-		-		0.027	0.025	-
HCM Control Doloy (a)						
HCM Control Delay (s)		-	-	9.4	8	-
		- -	- -	9.4 A 0.1	A 0.1	- -

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Intersection						
Int Delay, s/veh	0.3					
			14/5-	14/5-	05:	055
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ች	† †	4 1	_	_	7
Traffic Vol, veh/h	9	348	1079	5	5	16
Future Vol, veh/h	9	348	1079	5	5	16
Conflicting Peds, #/hr	0	_ 0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	10	378	1173	5	5	17
Major/Minor I	Major1	N	Major2	N	/linor2	
Conflicting Flow All	1178	0	-	0	1385	589
Stage 1	-	-	_	U	1176	509
Stage 2	_	-	-	-	209	-
	4.14	-	-	_	6.84	6.94
Critical Hdwy		-	-	-		0.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	589	-	-	-	134	452
Stage 1	-	-	-	-	255	-
Stage 2	-	-	-	-	806	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	589	-	-	-	132	452
Mov Cap-2 Maneuver	-	-	-	-	213	-
Stage 1	-	-	-	-	251	-
Stage 2	-	-	-	-	806	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.3		0		13.3	
HCM LOS	0.3		U		13.3 B	
TICIVI LOS					D	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR S	SBLn1
Capacity (veh/h)		589	-	-	-	452
HCM Lane V/C Ratio		0.017	-	-	-	0.038
HCM Control Delay (s)		11.2	-	-	-	13.3
HCM Lane LOS		В	-	-	-	В
HCM 95th %tile Q(veh))	0.1	-	-	-	0.1

0.2					
0.2					
ERT	FBR	WRI	WRT	NRI	NBR
	בטול				אטא
	1				6
					6
					0
					Stop
					None
					INUITE
					_
					-
					92
					2
3/4	4	5	1160	13	7
Major1	N	Major2	ľ	Minor1	
0	0		0	966	189
-	-	-	-		-
_	-	-	-		-
-	-	4.14	-		6.94
_	_	_	_		-
_	_	_	_		_
_	_	2 22			3.32
_	_		_		821
	_	-			-
	_	_			_
	_	_		317	
		1177		251	821
	-	11//	-		021
	-	-	_		-
	-	-	-		-
_	-	-	-	517	-
EB		WB		NB	
0		0		13.2	
	VIDL 4	CDT	EDD	MDI	MOT
π Γ					WBT
		-			-
		-			-
		-			-
	В	-	-	A 0	-
)	0.1		_		-
	0 - - - - - - - - - - - - - - - - - - -	**************************************	1	1	1

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LDL	<u>†</u>	<u>₩</u>	אטא	SBL W	אומט
Traffic Vol. veh/h	9	342	1062	7	0	7
Future Vol, veh/h	9	342	1062	7	0	7
<u> </u>	0	0	0	0	0	0
Conflicting Peds, #/hr	Free	Free	Free	Free		
Sign Control RT Channelized		None		None	Stop	Stop
	-		-		-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	10	372	1154	8	0	8
Major/Minor M	lajor1	N	//ajor2	N	/linor2	
	1162	0	-		1364	581
Stage 1	- 1102	-	-	-	1158	501
ŭ						
Stage 2	-	-	-	-	206	-
,	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	597	-	-	-	139	457
Stage 1	-	-	-	-	261	-
Stage 2	-	-	-	-	808	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	597	-	-	-	137	457
Mov Cap-2 Maneuver	_	-	_	-	137	-
Stage 1	_	_	_	_	257	_
Stage 2	_	_	_	_	808	_
olago 2					000	
Approach	EB		WB		SB	
HCM Control Delay, s	0.3		0		13	
HCM LOS					В	
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR :	SRI n1
			LDI	VVDT	יאמיי	
Capacity (veh/h)		597 0.016	-	-	-	457 0.017
			-	-		13
HCM Carter Dalay (a)					-	1.5
HCM Control Delay (s)		11.1	-	-		
		11.1 B 0.1	-	-	_	B 0.1

Intersection						
Int Delay, s/veh	0.6					
		14/5-			0	05-
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		† 1>		ሻ	† †
Traffic Vol, veh/h	9	10	231	12	11	200
Future Vol, veh/h	9	10	231	12	11	200
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	10	11	251	13	12	217
Major/Minor N	linar1	N	Major1		/aior?	
	/linor1		Major1		Major2	
Conflicting Flow All	391	132	0	0	264	0
Stage 1	258	-	-	-	-	-
Stage 2	133	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	585	893	-	-	1297	-
Stage 1	761	-	-	-	-	-
Stage 2	879	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	580	893	-	_	1297	-
Mov Cap-2 Maneuver	580	-	-	-	-	-
Stage 1	754	-	-	-	-	-
Stage 2	879	-	-	-	-	-
ŭ						
A name a ala	MD		NID		O.D.	
Approach	WB		NB		SB	
HCM Control Delay, s	10.2		0		0.4	
HCM LOS	В					
Minor Lane/Major Mvmt		NBT	NBRV	WBLn1	SBL	SBT
Capacity (veh/h)			-		1297	
HCM Lane V/C Ratio		_			0.009	_
HCM Control Delay (s)		_	_		7.8	_
HCM Lane LOS		_	_	В	Α	_
HCM 95th %tile Q(veh)		_	_	0.4	0	_
				J .,		

Intersection						
Int Delay, s/veh	0.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
	VVDL	WDK 7	1\D1 ↑ \$	NDIX	ODL	↑
Lane Configurations	0			^	00	
Traffic Vol, veh/h	8	19	243	9	28	209
Future Vol, veh/h	8	19	243	9	28	209
Conflicting Peds, #/hr	0	0	0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	9	21	264	10	30	227
		_				
Major/Minor I	Minor1	N	//ajor1	ľ	Major2	
Conflicting Flow All	420	137	0	0	274	0
Stage 1	269	-	-	-	-	-
Stage 2	151	-	-	-	-	-
Critical Hdwy	6.29	6.94	_	-	4.14	-
Critical Hdwy Stg 1	5.84	-	_	_	-	_
Critical Hdwy Stg 2	6.04	_	_	_	_	_
Follow-up Hdwy	3.67	3.32	_	_	2.22	_
Pot Cap-1 Maneuver	579	886	_	_	1286	_
•	725			_	1200	
Stage 1		-	-	-	-	
Stage 2	822	-	-	-	-	-
Platoon blocked, %			-	-	1000	-
Mov Cap-1 Maneuver	563	886	-	-	1286	-
Mov Cap-2 Maneuver	563	-	-	-	-	-
Stage 1	705	-	-	-	-	-
Stage 2	822	-	-	-	-	-
A mayo o o b	WD		ND		CD	
Approach	WB		NB		SB	
HCM Control Delay, s	9.2		0		0.9	
HCM LOS	Α					
Minor Lane/Major Mvm	ŧ	NBT	NRR\	VBLn1	SBL	SBT
		-	-	886	1286	-
Capacity (veh/h)		-				
HCM Lane V/C Ratio		-	-	0.023		-
HCM Control Delay (s)		-	-	9.2	7.9	-
HCM Lane LOS		-	-	Α	Α	-
HCM 95th %tile Q(veh)		-	-	0.1	0.1	-

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LDL Š	<u> </u>	<u>₩</u>	אטא	ODL	JDK ř
Traffic Vol, veh/h	9	1301	510	5	5	14
Future Vol, veh/h	9	1301	510	5	5	14
<u> </u>	0	0	0	0	0	0
Conflicting Peds, #/hr		Free		Free		
Sign Control RT Channelized	Free	None	Free	None	Stop	Stop
	-		-	None	-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	10	1414	554	5	5	15
Major/Minor N	1ajor1	N	//ajor2	N	/linor2	
Conflicting Flow All	559	0	-		1284	280
Stage 1	-	-	_	-	557	200
•					727	
Stage 2	-	-	-	-		-
Critical Hdwy	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	1008	-	-	-	157	717
Stage 1	-	-	-	-	537	-
Stage 2	-	-	-	-	439	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1008	-	-	-	155	717
Mov Cap-2 Maneuver	-	-	-	-	286	-
Stage 1	-	-	-	-	532	-
Stage 2	-	-	-	-	439	-
,						
Annragah	ΓD		WD		CD	
Approach	EB		WB		SB	
HCM Control Delay, s	0.1		0		10.1	
HCM LOS					В	
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR S	SBLn1
Capacity (veh/h)		1008	_	_	_	717
HCM Lane V/C Ratio		0.01	_	_	_	0.021
		8.6	_	-	-	10.1
HCM Control Delay (s)						
HCM Control Delay (s)			_	_	_	R
HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh)		A 0	-	-	-	B 0.1

Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	†		ሻ	^	Y	
Traffic Vol, veh/h	1291	10	0	500	10	3
Future Vol. veh/h	1291	10	0	500	10	3
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-	None	-	None
Storage Length	_	-	0	-	0	-
Veh in Median Storag	e.# 0	-	-	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	1403	11	0	543	11	3
WIVIII(I IOW	1700	11	U	070	- 11	J
Major/Minor	Major1		Major2		/linor1	
Conflicting Flow All	0	0	1414	0	1681	707
Stage 1	-	-	-	-	1409	-
Stage 2	-	-	-	-	272	-
Critical Hdwy	-	-	4.14	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	-	-	2.22	-	3.52	3.32
Pot Cap-1 Maneuver	_	-	478	-	85	378
Stage 1	-	-	-	-	192	-
Stage 2	-	-	-	-	749	-
Platoon blocked, %	_	_		_		
Mov Cap-1 Maneuver	_	-	478	_	85	378
Mov Cap-2 Maneuver		_	-	_	162	-
Stage 1	_	_	_	_	192	_
Stage 2	_	_	_	_	749	_
Olago 2					7 10	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		25.8	
HCM LOS					D	
Minor Lane/Major Mvr	nt I	NBLn1	EBT	EBR	WBL	WBT
	nt I		LDI	LDN		
Capacity (veh/h)		187	-	-	478	-
HCM Cantrol Daloy (0.076	-	-	-	-
HCM Long LOS)	25.8	-	-	0	-
HCM Lane LOS	-\	D	-	-	A	-
HCM 95th %tile Q(veh	1)	0.2	-	-	0	-

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻ	^	† 1>		¥	
Traffic Vol, veh/h	22	1269	500	13	3	3
Future Vol, veh/h	22	1269	500	13	3	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	_		-		-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage		0	0	_	0	_
Grade, %	-,	0	0	_	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	24	1379	543	14	3	3
		1010	0.0			
		_				
	Major1		Major2		/linor2	
Conflicting Flow All	557	0	-	0	1288	279
Stage 1	-	-	-	-	550	-
Stage 2	-	-	-	-	738	-
Critical Hdwy	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	1010	-	-	-	156	718
Stage 1	-	-	-	-	542	-
Stage 2	-	-	-	-	434	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1010	-	-	-	152	718
Mov Cap-2 Maneuver	-	-	-	-	152	-
Stage 1	-	-	-	-	529	-
Stage 2	_	_	-	_	434	_
5g5 _						
	E D		14/5		0.5	
Approach	EB		WB		SB	
HCM Control Delay, s	0.1		0		19.7	
HCM LOS					С	
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1010	-	_	_	251
HCM Lane V/C Ratio		0.024	-	_	-	0.026
HCM Control Delay (s))	8.7	-	-	_	19.7
HCM Lane LOS		A	-	-	-	С
HCM 95th %tile Q(veh	1)	0.1	-	-	-	0.1
	,					

Intersection						
Int Delay, s/veh	1.1					
		14/55	Not	NES	051	057
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	₩		↑ ↑		ሻ	† †
Traffic Vol, veh/h	35	15	228	8	3	246
Future Vol, veh/h	35	15	228	8	3	246
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	38	16	248	9	3	267
Major/Minor I	Minor1	N	Major1	N	//ajor2	
Conflicting Flow All	393	129	0	0	257	0
Stage 1	253	123	-	-	201	-
Stage 2	140	_	_	_		
Critical Hdwy	6.84	6.94	-		4.14	
Critical Hdwy Stg 1	5.84	0.34	_	_	4.14	
Critical Hdwy Stg 2	5.84		-	-	_	
Follow-up Hdwy	3.52	3.32	_	_	2.22	-
Pot Cap-1 Maneuver	584	897	-	-	1305	-
	766	- 091	-	-	1303	-
Stage 1	872		-	-	-	_
Stage 2	0/2	-	-	-	-	-
Platoon blocked, %	E00	007	-	-	4205	-
Mov Cap-1 Maneuver	583	897	-	-	1305	-
Mov Cap-2 Maneuver	583	-	-	-	-	-
Stage 1	764	-	-	-	-	-
Stage 2	872	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	11		0		0.1	
HCM LOS	В				0.1	
TIOM EGG						
Minor Lane/Major Mvm	ıt	NBT	NBRV	WBLn1	SBL	SBT
Capacity (veh/h)		-	-	٠.	1305	-
HCM Lane V/C Ratio		-	-		0.002	-
HCM Control Delay (s)		-	-		7.8	-
HCM Lane LOS		-	-	В	Α	-
HCM 95th %tile Q(veh)		-	-	0.3	0	-

-						
Intersection						
Int Delay, s/veh	0					
	WPI	WPD	NDT	NDD	CDI	CDT
	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	† ‡			^
Traffic Vol, veh/h	0	3	255	2	0	327
Future Vol, veh/h	0	3	255	2	0	327
Conflicting Peds, #/hr	0	0	0	0	0	0
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	_	_	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	0	3	277	2	0	355
IVIVIIIL FIOW	U	3	211	2	U	300
Major/Minor M	linor1	N	Major1	N	/lajor2	
Conflicting Flow All	_	140	0	0	-	_
Stage 1	_	-	-	-		_
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	882	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	_	882	_	_	_	_
Mov Cap-2 Maneuver	_	-	_	_	_	_
Stage 1	_	_	_	_	_	_
Stage 2	-	-	-	-	-	-
Approach			ND		SB	
	WB		NB			
HCM Control Delay, s	9.1		0		0	
HCM Control Delay, s	9.1					
HCM Control Delay, s	9.1 A	NBT	0	VBLn1		
HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt	9.1 A	NBT -	0		0	
HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h)	9.1 A	NBT -	0 NBRV	882	0 SBT	
HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	9.1 A	NBT - -	0 NBRV	882 0.004	SBT -	
HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	9.1 A	- - -	NBRV - -	882 0.004 9.1	0 SBT - -	
HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	9.1 A	NBT - - -	0 NBRV	882 0.004	SBT -	

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Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻ	^	↑ ↑	WEIT	ODL	7
Traffic Vol, veh/h	3	319	862	2	5	14
Future Vol, veh/h	3	319	862	2	5	14
Conflicting Peds, #/hr	0	0	002	0	0	0
	Free	Free	Free	Free		
					Stop	Stop
RT Channelized	-	110110	-	None	-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	3	347	937	2	5	15
Major/Minor M	ajor1	N	Major2	N	Minor2	
Conflicting Flow All	939	0	-		1118	470
Stage 1	303	-	_	-	938	-
ŭ					180	
Stage 2	-	-	-	-		-
,	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	726	-	-	-	201	540
Stage 1	-	-	-	-	341	-
Stage 2	-	-	-	-	833	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	726	-	-	-	200	540
Mov Cap-2 Maneuver	-	-	-	-	287	-
Stage 1	-	_	-	_	340	_
Stage 2	_	_	_	_	833	_
olago 2					000	
Approach	EB		WB		SB	
HCM Control Delay, s	0.1		0		11.9	
HCM LOS					В	
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR :	2DI 51
			LDI	VVDI	WDK.	
Capacity (veh/h)		726	-	-	-	540
		0.004	-	-		0.028
HCM Cantral Dalay (a)					_	11.9
HCM Control Delay (s)		10	-	-		
		10 A 0	-	-	_	B 0.1

Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	†		ሻ	^	Y	
Traffic Vol, veh/h	315	4	5	847	12	6
Future Vol, veh/h	315	4	5	847	12	6
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	_	-	0	-	0	-
Veh in Median Storag		_	-	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	342	4	5	921	13	7
WWITH FIOW	342	4	5	921	13	1
Major/Minor	Major1	N	Major2	ľ	Minor1	
Conflicting Flow All	0	0	346	0	815	173
Stage 1	-	_	_	-	344	_
Stage 2	_	_	_	_	471	_
Critical Hdwy	-	_	4.14	_	6.84	6.94
Critical Hdwy Stg 1	_	_	_	_	5.84	-
Critical Hdwy Stg 2	_	_	_	_	5.84	_
Follow-up Hdwy	_	_	2.22	_	3.52	3.32
Pot Cap-1 Maneuver	_	_	1210	_	315	840
Stage 1	_	_	1210	_	689	-
Stage 2	_	_	_	_	594	_
Platoon blocked, %	_		_	_	JJ4	
Mov Cap-1 Maneuver			1210	_	314	840
		-	1210	-	431	040
Mov Cap-2 Maneuver		-	-	-		-
Stage 1	-	-	-	-	686	-
Stage 2	-	-	-	-	594	-
Approach	EB		WB		NB	
HCM Control Delay, s			0		12.3	
HCM LOS	0		U		12.3 B	
HOW LOO					U	
Minor Lane/Major Mvr	mt I	NBLn1	EBT	EBR		WBT
Capacity (veh/h)		515	-		1210	-
HCM Lane V/C Ratio		0.038	-	-	0.004	-
HCM Control Delay (s	s)	12.3	-	-	8	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(vel	h)	0.1	-	-	0	-

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LDL	<u>†</u>	<u>₩</u>	אטא	SBL W	אומט
Traffic Vol, veh/h	9	313	842	7	0	7
Future Vol, veh/h	9	313	842	7	0	7
<u> </u>	0	0	042	0	0	0
Conflicting Peds, #/hr	Free	Free	Free	Free		
Sign Control RT Channelized				None	Stop	Stop
	-		-		-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	10	340	915	8	0	8
Major/Minor N	1ajor1	N	Major2	N	/linor2	
Conflicting Flow All	923	0	-		1109	462
Stage 1	-	-	_	-	919	-
Stage 2	<u>-</u>	_	_	<u>-</u>	190	<u>-</u>
Critical Hdwy	4.14	_	_	_	6.84	6.94
Critical Hdwy Stg 1	4.14	_	_	_	5.84	0.34
		-	_		5.84	
Critical Hdwy Stg 2	-			-		2 22
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	736	-	-	-	204	547
Stage 1	-	-	-	-	349	-
Stage 2	-	-	-	-	823	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	736	-	-	-	201	547
Mov Cap-2 Maneuver	-	-	-	-	201	-
Stage 1	-	-	-	-	344	-
Stage 2	-	-	-	-	823	-
Annroach	EB		WB		SB	
Approach						
HCM Control Delay, s	0.3		0		11.7	
HCM LOS					В	
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		736	_	_	_	547
HCM Lane V/C Ratio		0.013	-	_	-	0.014
HCM Control Delay (s)		10	-	_	-	11.7
HCM Lane LOS		A	_	_	_	В
						0
HCM 95th %tile Q(veh)		0	-	_	_	U

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Intersection						
Int Delay, s/veh	0.6					
		WIDD	NDT	NDD	ODL	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	40	† ‡	40	ሻ	† †
Traffic Vol, veh/h	9	10	239	12	11	180
Future Vol, veh/h	9	10	239	12	11	180
Conflicting Peds, #/hr	0	0	_ 0	0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	10	11	260	13	12	196
Maiow/Minow	Air a nd		1-:1		Maia#0	
	/linor1		//ajor1		Major2	
Conflicting Flow All	389	137	0	0	273	0
Stage 1	267	-	-	-	-	-
Stage 2	122	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	587	886	-	-	1287	-
Stage 1	754	-	-	-	-	-
Stage 2	890	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	582	886	-	-	1287	-
Mov Cap-2 Maneuver	582	-	-	_	-	_
Stage 1	747	-	-	-	_	_
Stage 2	890	_	_	_	_	_
2.0.33 -						
Approach	WB		NB		SB	
HCM Control Delay, s	10.2		0		0.5	
HCM LOS	В					
Minor Lane/Major Mvmt		NBT	NDDV	VBLn1	SBL	SBT
		INDI				ODT
Capacity (veh/h)		-	-		1287	-
HCM Lana V//C Dati-		-			0.009	-
HCM Cantral Dalay (a)						
HCM Control Delay (s)		-	-		7.8	-
		- -	-	10.2 B	7.8 A 0	- -

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WDL	₩ 1	↑	NDIX	ODL	† ††
	٥			6	٥	
Traffic Vol, veh/h	0	2	268	6	0	217
Future Vol, veh/h	0	2	268	6	0	217
Conflicting Peds, #/hr	0	0	_ 0	0	0	_ 0
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	2	291	7	0	236
WWW.CT IOW		_	201	•	U	200
Major/Minor M	inor1	N	//ajor1	N	/lajor2	
Conflicting Flow All	-	149	0	0	-	-
Stage 1	_	-	_	_	_	_
Stage 2	_	_	_	_	_	_
Critical Hdwy	_	6.94	_	_	_	_
Critical Hdwy Stg 1	<u>-</u>	-	_		<u>-</u>	_
				-		
Critical Hdwy Stg 2	-	2.20	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	
Pot Cap-1 Maneuver	0	871	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	871	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	_	-	_	-	_	-
Stage 2	_	_	_	-	_	_
Olago Z						
Approach	WB		NB		SB	
HCM Control Delay, s	9.1		0		0	
HCM LOS	Α					
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		-	-	871	-	
HCM Lane V/C Ratio		-	-	0.002	-	
HCM Control Delay (s)		-	-	9.1	-	
HCM Lane LOS		_	_	Α	_	
HCM 95th %tile Q(veh)		_	_	0	_	
TOW COULT JULIO CO (VOII)				- 0		

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	T)	↑ ↑	↑ ↑	אוטוע	ODL	7
Traffic Vol, veh/h	10	1192	453	5	3	8
Future Vol, veh/h	10	1192	453	5	3	8
Conflicting Peds, #/hr	0	0	400	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized						
	-		-	None	-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage,	,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	11	1296	492	5	3	9
Major/Minor N	Major1	N	Major2	N	/linor2	
Conflicting Flow All	497	0	<u> </u>		1165	249
	497	-		-	495	249
Stage 1			-	-	670	
Stage 2	-	-	-			-
Critical Hdwy	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	1063	-	-	-	187	751
Stage 1	-	-	-	-	578	-
Stage 2	-	-	-	-	470	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1063	-	-	-	185	751
Mov Cap-2 Maneuver	_	_	-	_	315	-
Stage 1	_	_	_	_	572	_
Stage 2	_	_	_	_	470	_
Olago Z					770	
Approach	EB		WB		SB	
HCM Control Delay, s	0.1		0		9.9	
HCM LOS					Α	
		EDI	EST	14/5-	MES	0DL 4
Minor Lane/Major Mvmt	t	EBL	EBT	WBT	WBR :	
Capacity (veh/h)		1063	-	-	-	751
HCM Lane V/C Ratio		0.01	-	-	-	0.012
HCM Control Delay (s)		8.4	-	-	-	9.9
HCM Lane LOS		Α	-	-	-	Α
HCM 95th %tile Q(veh)		0	-	-	-	0

Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑ 1>	רטוג	YVDL	<u>₩</u>	NDL Y	וטוז
Traffic Vol, veh/h	1180	10	0	443	10	3
Future Vol, veh/h	1180	10	0	443	10	3
<u>'</u>	0	0	0	443	0	0
Conflicting Peds, #/hr Sign Control	Free	Free		Free		
RT Channelized			Free	None	Stop	Stop
	-		-		-	None
Storage Length	<u>-</u>	-	0	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1283	11	0	482	11	3
Major/Minor N	/lajor1	N	Major2	N	/linor1	
Conflicting Flow All	0	0	1294		1530	647
Stage 1	-	U	1294	-	1289	- 047
Stage 2	-	-	-	_	241	-
Critical Hdwy		-	4.14	_	6.84	6.94
		-		-	5.84	0.94
Critical Hdwy Stg 1	-	-	-			
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	-	-	2.22	-	3.52	3.32
Pot Cap-1 Maneuver	-	-	531	-	108	414
Stage 1	-	-	-	-	222	-
Stage 2	-	-	-	-	776	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	531	-	108	414
Mov Cap-2 Maneuver	-	-	-	-	188	-
Stage 1	-	-	-	-	222	-
Stage 2	-	-	-	-	776	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		22.9	
HCM LOS	U		U		22.9 C	
HOW LOS					C	
Minor Lane/Major Mvmt	t 1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		215	-	-	531	-
HCM Lane V/C Ratio		0.066	-	-	_	-
HCM Control Delay (s)		22.9	-	-	0	-
HCM Lane LOS		C	-	-	A	-
HCM 95th %tile Q(veh)		0.2	_	-	0	_

Intersection						
Int Delay, s/veh	0.2					
		CPT.	MOT	WED	ODL	ODD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻ	† †	ŧβ		Y	
Traffic Vol, veh/h	22	1158	443	13	3	3
Future Vol, veh/h	22	1158	443	13	3	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage,		0	0	-	0	-
Grade, %	_	0	0	_	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	24	1259	482	14	3	3
IVIVIIIL FIOW	24	1259	402	14	ა	J
Major/Minor M	1ajor1	N	Major2	N	/linor2	
Conflicting Flow All	496	0	-	0	1167	248
Stage 1	-	-	_	-	489	-
Stage 2	_	_			678	
			-	-		-
Critical Hdwy	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	1064	-	-	-	187	752
Stage 1	-	-	-	-	582	-
Stage 2	-	-	-	-	466	-
Platoon blocked, %		-	-	-		
	1064	_	_	_	183	752
Mov Cap-2 Maneuver	-	_	_	_	183	-
Stage 1	_	_	_	_	569	_
					466	
Stage 2	-	-	-	-	400	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.2		0		17.5	
HCM LOS	0.2		U		C	
I IOW LOS					U	
				MOT	M/DD (SBLn1
Minor Lane/Major Mvmt		EBL	EBT	WBT	MDL	
			EBT -	WB1	WDK (
Capacity (veh/h)		1064	-	-	-	294
Capacity (veh/h) HCM Lane V/C Ratio		1064 0.022		-	-	294 0.022
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		1064 0.022 8.5	- - -	- - -	- - -	294 0.022 17.5
Capacity (veh/h) HCM Lane V/C Ratio		1064 0.022	-	-	-	294 0.022

Intersection						
Int Delay, s/veh	1.1					
		14/5-			0-1	0==
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		ŧβ		ሻ	† †
Traffic Vol, veh/h	36	16	238	8	3	257
Future Vol, veh/h	36	16	238	8	3	257
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	39	17	259	9	3	279
WWW.CT IOW	00		200	J	•	210
Major/Minor	Minor1		//ajor1	N	Major2	
Conflicting Flow All	410	134	0	0	268	0
Stage 1	264	-	-	-	-	-
Stage 2	146	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	_	4.14	_
Critical Hdwy Stg 1	5.84	-	_	_	_	_
Critical Hdwy Stg 2	5.84	_	_	-	_	_
Follow-up Hdwy	3.52	3.32	_	_	2.22	_
Pot Cap-1 Maneuver	570	890	_	_	1293	_
Stage 1	756	-	_	_	1233	_
	866	_	-	_	-	
Stage 2	000	-	-	-	-	
Platoon blocked, %	F00	000	-	-	4000	-
Mov Cap-1 Maneuver	569	890	-	-	1293	-
Mov Cap-2 Maneuver	569	-	-	-	-	-
Stage 1	754	-	-	-	-	-
Stage 2	866	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	11.2		0		0.1	
HCM LOS	11.2 B		U		0.1	
I IOIVI LOS	Б					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		_	_	640	1293	_
HCM Lane V/C Ratio		_	_	0.088		-
HCM Control Delay (s)		_	_	11.2	7.8	-
HCM Lane LOS		-	_	В	Α.	_
HCM 95th %tile Q(veh	1	-	-	0.3	0	
HOIVI 95(II %(IIIE Q(VEN)	-	-	0.3	U	-

Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	TTDL	71017	↑ ↑	אטוז	ODL	†
Traffic Vol, veh/h	0	4	265	2	0	339
Future Vol, veh/h	0	4	265	2	0	339
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	_	0	_	-	_	-
Veh in Median Storage,	# 0	-	0	_	_	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	0	4	288	2	0	368
WWW. LOW	U	7	200		U	000
	/linor1		//ajor1	N	/lajor2	
Conflicting Flow All	-	145	0	0	-	-
Stage 1	-	-	-	-	_	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	_	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	876	_	-	0	-
Stage 1	0	-	-	_	0	_
Stage 2	0	_	_	_	0	_
Platoon blocked, %	J		_	_		_
Mov Cap-1 Maneuver	_	876	_	_	_	_
Mov Cap-1 Maneuver	_	- 070	_	_	_	
Stage 1	_	<u>-</u>	-	_	_	-
			-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9.1		0		0	
HCM LOS	Α				•	
	, ,					
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		-	-	876	-	
HCM Lane V/C Ratio		-	-	0.005	-	
HCM Control Delay (s)		-	-	9.1	-	
HCM Lane LOS		-	-	Α	-	
HCM 95th %tile Q(veh)		-	-	0	-	

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻ	^	† 1>			7
Traffic Vol, veh/h	3	332	896	2	5	15
Future Vol, veh/h	3	332	896	2	5	15
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	_	None	-		-	None
Storage Length	0	-	-	-	_	0
Veh in Median Storage,		0	0	-	0	_
Grade, %	-	0	0	_	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	3	361	974	2	5	16
IVIVIII(I IOW	J	301	314		J	10
Major/Minor Major/Minor	ajor1	N	Major2	N	Minor2	
Conflicting Flow All	976	0	-	0	1162	488
Stage 1	-	-	-	-	975	-
Stage 2	-	-	-	-	187	-
	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	703	-	_	-	188	526
Stage 1	_	-	-	-	326	-
Stage 2	-	-	_	-	826	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	703	-	_	_	187	526
Mov Cap-2 Maneuver	-	_	_	_	274	-
Stage 1	_	_	_	_	325	_
Stage 2	_	_	_	_	826	_
Olago Z					020	
Approach	EB		WB		SB	
HCM Control Delay, s	0.1		0		12.1	
HCM LOS					В	
		EBL	EBT	WBT	WBR :	CDI n1
Minor Lang/Major Mumt				VVDI	WDK -	
Minor Lane/Major Mvmt		702				526
Capacity (veh/h)		703	-	-		
Capacity (veh/h) HCM Lane V/C Ratio		0.005	-	-	-	0.031
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		0.005 10.1	-	-	-	0.031 12.1
Capacity (veh/h) HCM Lane V/C Ratio		0.005	-		-	0.031

Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑ ↑	רטו	VVDL آ	↑ ↑	W M	וטוז
Traffic Vol, veh/h	328	4	5	881	12	6
Future Vol, veh/h	328	4	5	881	12	6
Conflicting Peds, #/hr	0	0	0	001	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	riee -	None	Stop -	None
Storage Length	-	None -	0	-	0	None
Veh in Median Storage			-	0	0	
	s, # 0 0			0	0	
Grade, %		-	-			-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	357	4	5	958	13	7
Major/Minor	Major1	N	Major2	N	/linor1	
Conflicting Flow All	0	0	361	0	848	181
Stage 1	-	-	-	-	359	-
Stage 2	<u>-</u>	_	_	<u>-</u>	489	<u>-</u>
Critical Hdwy	_		4.14	_	6.84	6.94
Critical Hdwy Stg 1	_		7.17		5.84	0.54
Critical Hdwy Stg 2		-	-	-	5.84	
, ,	-	-	2.22	-	3.52	3.32
Follow-up Hdwy	-	-	1194	-		831
Pot Cap-1 Maneuver	-	-	1194	-	300	031
Stage 1	-	-	-	-	677	-
Stage 2	-	-	-	-	582	-
Platoon blocked, %	-	-	1101	-	000	004
Mov Cap-1 Maneuver	-	-	1194	-	299	831
Mov Cap-2 Maneuver	-	-	-	-	419	-
Stage 1	-	-	-	-	674	-
Stage 2	-	-	-	-	582	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		12.5	
HCM LOS					В	
Minor Lane/Major Mvm	nt 1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		502	-		1194	-
HCM Lane V/C Ratio		0.039	_		0.005	_
HCM Control Delay (s)		12.5	_	_	_	_
HCM Lane LOS		12.0 B	_	_	A	_
HCM 95th %tile Q(veh)	0.1	_	_	0	_
		J. 1				

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻ	† †	↑ ↑	TIDIC	₩.	ODIN
Traffic Vol, veh/h	9	326	876	7	0	7
Future Vol, veh/h	9	326	876	7	0	7
Conflicting Peds, #/hr	0	0	0/0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	
Storage Length	0	-	_	-	0	-
Veh in Median Storage		0	0	_	0	_
Grade, %		0	0	<u>-</u>	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	10	354	952	8	0	8
MINITE FIOW	10	334	902	0	U	0
Major/Minor	Major1	N	Major2	N	Minor2	
Conflicting Flow All	960	0	-	0	1153	480
Stage 1	-	-	-	-	956	-
Stage 2	-	-	-	-	197	-
Critical Hdwy	4.14	-	_	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	_	_	-	5.84	-
Follow-up Hdwy	2.22	_	_	_	3.52	3.32
Pot Cap-1 Maneuver	712	_	-	_	191	532
Stage 1	- 12	_	_	_	334	-
Stage 2	_	_	_	_	817	_
Platoon blocked, %		_		_	017	
Mov Cap-1 Maneuver	712			_	188	532
Mov Cap-1 Maneuver		_	_	_	188	- 552
Stage 1		-	-		329	
	-	-		-		-
Stage 2	-	-	-	-	817	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.3		0		11.9	
HCM LOS					В	
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WBR :	
Capacity (veh/h)		712	-	-	-	
HCM Lane V/C Ratio		0.014	-	-		0.014
HCM Control Delay (s)	10.1	-	-	-	11.9
HCM Lane LOS		В	-	-	-	В
HCM 95th %tile Q(veh	1)	0	-	-	-	0

Movement	Intersection						
Movement WBL WBR NBT NBR SBL SBT Lane Configurations Traffic Vol, veh/h Future Vol, veh/h 9 10 249 12 11 188 Conflicting Peds, #/hr 0		0.6					
Lane Configurations	<u> </u>		MDD	NDT	NDD	ODI	OPT
Traffic Vol, veh/h 9 10 249 12 11 188 Future Vol, veh/h 9 10 249 12 11 188 Future Vol, veh/h 9 10 249 12 11 188 Conflicting Peds, #/hr 0 0 0 0 0 0 0 Sign Control Stop Stop Free Free Free Free Free Free RT Channelized - None - None - None Storage Length 0 100 Veh in Median Storage, # 0 - 0 (0) Grade, % 0 - 0 (0) Peak Hour Factor 92 92 92 92 92 92 92 92 92 92 92 92 92			WBK		NRK		
Future Vol, veh/h Conflicting Peds, #/hr O Conflicting Flow All Conflicting Flow All Conflicting Flow All Conflicting Howy Conflicting How			40		40		
Conflicting Peds, #/hr O O O O O O O O O							
Sign Control Stop Stop Free None Control Control Control Control Page P							
RT Channelized - None - None - None Storage Length 0 100							_ 0
Storage Length 0				Free		Free	
Veh in Median Storage, # 0 - 0 - - 0 Grade, % 0 - 0 - - 0 Peak Hour Factor 92 92 92 92 92 92 Heavy Vehicles, % 2 <td></td> <td></td> <td></td> <td></td> <td>None</td> <td></td> <td>None</td>					None		None
Grade, % 0 - 0 - - 0 Peak Hour Factor 92			-		-	100	-
Peak Hour Factor 92			-		-	-	0
Heavy Vehicles, % 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2							0
Mount Flow 10 11 271 13 12 204 Major/Minor Minor1 Major1 Major2 Conflicting Flow All 404 142 0 0 284 0 Stage 1 278 - - - - - Stage 2 126 - - - - - Critical Hdwy 5tage 1 5.84 - - - - - Critical Hdwy Stg 2 5.84 - <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td>92</td></t<>							92
Major/Minor Minor1 Major1 Major2 Conflicting Flow All 404 142 0 0 284 0 Stage 1 278 - - - - - Stage 2 126 - - - - - Critical Hdwy 6.84 6.94 - 4.14 - <t< td=""><td>Heavy Vehicles, %</td><td>2</td><td>2</td><td>2</td><td>2</td><td></td><td>2</td></t<>	Heavy Vehicles, %	2	2	2	2		2
Conflicting Flow All 404 142 0 0 284 0 Stage 1 278 - - - - Stage 2 126 - - - - Critical Hdwy 6.84 6.94 - - 4.14 Critical Hdwy Stg 1 5.84 - - - - Critical Hdwy Stg 2 5.84 - - - - Follow-up Hdwy 3.52 3.32 - - 2.22 Pot Cap-1 Maneuver 575 880 - - 1275 Stage 1 744 - - - - Stage 2 886 - - - - Mov Cap-1 Maneuver 570 880 - - 1275 Mov Cap-2 Maneuver 570 - - - - Stage 2 886 - - - - Approach WB NB SB HCM Control Delay, s 10.3 0 0.4	Mvmt Flow	10	11	271	13	12	204
Conflicting Flow All 404 142 0 0 284 0 Stage 1 278 - - - - Stage 2 126 - - - - Critical Hdwy 6.84 6.94 - - 4.14 Critical Hdwy Stg 1 5.84 - - - - Critical Hdwy Stg 2 5.84 - - - - Follow-up Hdwy 3.52 3.32 - - 2.22 Pot Cap-1 Maneuver 575 880 - - 1275 Stage 1 744 - - - - Stage 2 886 - - - - Mov Cap-1 Maneuver 570 880 - - 1275 Mov Cap-2 Maneuver 570 - - - - Stage 2 886 - - - - Approach WB NB SB HCM Control Delay, s 10.3 0 0.4							
Conflicting Flow All 404 142 0 0 284 0 Stage 1 278 - - - - Stage 2 126 - - - - Critical Hdwy 6.84 6.94 - - 4.14 Critical Hdwy Stg 1 5.84 - - - - Critical Hdwy Stg 2 5.84 - - - - Follow-up Hdwy 3.52 3.32 - - 2.22 Pot Cap-1 Maneuver 575 880 - - 1275 Stage 1 744 - - - - Stage 2 886 - - - - Mov Cap-1 Maneuver 570 880 - - 1275 Mov Cap-2 Maneuver 570 - - - - Stage 2 886 - - - - Approach WB NB SB HCM Control Delay, s 10.3 0 0.4	NA - ' /NA' NA	l'		M - 1 4		4 0	
Stage 1 278 -							
Stage 2 126 -			142	0	0	284	0
Critical Hdwy 6.84 6.94 - - 4.14 Critical Hdwy Stg 1 5.84 - - - - Critical Hdwy Stg 2 5.84 - - - - Follow-up Hdwy 3.52 3.32 - 2.22 Pot Cap-1 Maneuver 575 880 - 1275 Stage 1 744 - - - Stage 2 886 - - - - Platoon blocked, % -	•		-	-	-	-	-
Critical Hdwy Stg 1 5.84 - - - - Critical Hdwy Stg 2 5.84 - - - - Follow-up Hdwy 3.52 3.32 - - 2.22 Pol Cap-1 Maneuver 575 880 - - 1275 Stage 1 744 - - - - Stage 2 886 - - - - - Mov Cap-1 Maneuver 570 880 - - 1275 Mov Cap-2 Maneuver 570 -				-	-		-
Critical Hdwy Stg 2 5.84 - - - - Follow-up Hdwy 3.52 3.32 - 2.22 Pot Cap-1 Maneuver 575 880 - - 1275 Stage 1 744 - - - - Stage 2 886 - - - - Platoon blocked, % - - - - - Mov Cap-1 Maneuver 570 880 - - 1275 Mov Cap-2 Maneuver 570 - - - - Stage 1 737 - - - - Stage 2 886 - - - - Approach WB NB SB HCM Control Delay, s 10.3 0 0.4 HCM LOS B Minor Lane/Major Mvmt NBT NBRWBLn1 SBL SBT NBCWBLn1 SBL SBT NBCWBLn1 SBL SBT NBCWBLn2 SBT NBCWBLn2 SBT NBCWBLn2 SBT NBCWBLn3 NBCWBLN2 SBT NBCWBLN2 S	Critical Hdwy		6.94	-	-	4.14	-
Follow-up Hdwy 3.52 3.32 - 2.22 Pot Cap-1 Maneuver 575 880 - 1275 Stage 1 744	Critical Hdwy Stg 1	5.84	-	-	-	-	-
Pot Cap-1 Maneuver 575 880 - - 1275 Stage 1 744 - - - - Stage 2 886 - - - - Platoon blocked, % - - - - - - Mov Cap-1 Maneuver 570 880 - - 1275 Mov Cap-2 Maneuver 570 - - - - Stage 1 737 - - - - Stage 2 886 - - - - Approach WB NB SB HCM Control Delay, s 10.3 0 0.4 HCM LOS B Minor Lane/Major Mvmt NBT NBRWBLn1 SB SB Capacity (veh/h) - - 700 1275 HCM Lane V/C Ratio - - 0.03 0.009 HCM Control Delay (s) - 10.3 7.9	Critical Hdwy Stg 2	5.84	-	-	-		-
Stage 1 744 - - - Stage 2 886 - - - Platoon blocked, % - - - - Mov Cap-1 Maneuver 570 880 - - 1275 Mov Cap-2 Maneuver 570 - - - - Stage 1 737 - - - - - Stage 2 886 - - - - - - Approach WB NB SB HCM Control Delay, s 10.3 0 0.4 - HCM LOS B Minor Lane/Major Mvmt NBT NBRWBLn1 SB SB Capacity (veh/h) - - 700 1275 HCM Lane V/C Ratio - - 0.03 0.009 HCM Control Delay (s) - 10.3 7.9	Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Stage 2 886 - - - - Platoon blocked, % Mov Cap-1 Maneuver 570 880 - - 1275 Mov Cap-2 Maneuver 570 - - - - Stage 1 737 - - - - Stage 2 886 - - - - Approach WB NB SB HCM Control Delay, s 10.3 0 0.4 HCM LOS B Minor Lane/Major Mvmt NBT NBRWBLn1 SB Capacity (veh/h) - - 700 1275 HCM Lane V/C Ratio - - 0.03 0.009 HCM Control Delay (s) - 10.3 7.9	Pot Cap-1 Maneuver	575	880	-	-	1275	-
Platoon blocked, % - - Mov Cap-1 Maneuver 570 880 - - 1275 Mov Cap-2 Maneuver 570 -	Stage 1	744	-	-	-	-	-
Platoon blocked, % - - Mov Cap-1 Maneuver 570 880 - - 1275 Mov Cap-2 Maneuver 570 -		886	_	-	-	-	-
Mov Cap-1 Maneuver 570 880 - - 1275 Mov Cap-2 Maneuver 570 - <td></td> <td></td> <td></td> <td>-</td> <td>_</td> <td></td> <td>-</td>				-	_		-
Mov Cap-2 Maneuver 570 -		570	880	_	-	1275	_
Stage 1 737 -				_	_		_
Stage 2 886 - - - - Approach WB NB SB HCM Control Delay, s 10.3 0 0.4 HCM LOS B Minor Lane/Major Mvmt NBT NBRWBLn1 SBL SBT Capacity (veh/h) - - 700 1275 HCM Lane V/C Ratio - - 0.03 0.009 HCM Control Delay (s) - - 10.3 7.9	•			_	_		_
Approach WB NB SB HCM Control Delay, s 10.3 0 0.4 HCM LOS B Minor Lane/Major Mvmt NBT NBRWBLn1 SBL SB1 Capacity (veh/h) - - 700 1275 HCM Lane V/C Ratio - - 0.03 0.009 HCM Control Delay (s) - 10.3 7.9					_		
HCM Control Delay, s 10.3 0 0.4	οιαγ ο Δ	000	_	_	_	_	_
HCM Control Delay, s 10.3 0 0.4							
Minor Lane/Major Mvmt NBT NBRWBLn1 SBL SBT Capacity (veh/h) - - 700 1275 HCM Lane V/C Ratio - - 0.03 0.009 HCM Control Delay (s) - 10.3 7.9	Approach	WB		NB		SB	
Minor Lane/Major Mvmt NBT NBRWBLn1 SBL SBT Capacity (veh/h) - - 700 1275 HCM Lane V/C Ratio - - 0.03 0.009 HCM Control Delay (s) - - 10.3 7.9	HCM Control Delay, s	10.3		0		0.4	
Minor Lane/Major Mvmt NBT NBRWBLn1 SBL SBT Capacity (veh/h) - - 700 1275 HCM Lane V/C Ratio - - 0.03 0.009 HCM Control Delay (s) - - 10.3 7.9		В					
Capacity (veh/h) - - 700 1275 HCM Lane V/C Ratio - - 0.03 0.009 HCM Control Delay (s) - 10.3 7.9							
Capacity (veh/h) - - 700 1275 HCM Lane V/C Ratio - - 0.03 0.009 HCM Control Delay (s) - 10.3 7.9	Minar Lana/Major Mymt		NDT	NDDI	MDI n1	CDI	CDT
HCM Lane V/C Ratio 0.03 0.009 HCM Control Delay (s) 10.3 7.9			INDI	INDIN			SDI
HCM Control Delay (s) 10.3 7.9			-	-			-
							-
							-
	HCM Lane LOS		-	-			-
HCM 95th %tile Q(veh) 0.1 0	HCM 95th %tile Q(veh)		-	-	0.1	0	-

Intersection						
Int Delay, s/veh	0.1					
		WED	Not	NDD	051	057
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	4 1			† ††
Traffic Vol, veh/h	0	3	279	6	0	226
Future Vol, veh/h	0	3	279	6	0	226
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	3	303	7	0	246
Major/Minor N	1inor1	N	Major1	N	/lajor2	
Conflicting Flow All	-	155	0	0	//ajuiz -	_
Stage 1		100				
Stage 1 Stage 2	-	-	-	-	-	-
Critical Hdwy		6.94		-		-
	-		-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	863	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	863	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB		NB		SB	
	9.2		0		0	
HCM Control Delay, s HCM LOS			U		U	
HCIVI LOS	Α					
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		-	-	863	-	
HCM Lane V/C Ratio		-	-	0.004	-	
HCM Control Delay (s)		-	-	9.2	-	
		_	_	A	_	
HUM Lane LUS						
HCM Lane LOS HCM 95th %tile Q(veh)		_	_	0	_	

Intersection						
Int Delay, s/veh	0.1					
		CDT	MOT	MDD	ODI	000
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ነ	† †	↑ ↑	-	0	7
Traffic Vol, veh/h	10	1240	471	5	3	9
Future Vol, veh/h	10	1240	471	5	3	9
Conflicting Peds, #/hr	0	_ 0	_ 0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage,		0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	11	1348	512	5	3	10
Major/Minor M	1ajor1	N	Major2	N	Minor2	
Conflicting Flow All	517	0		0	1211	259
Stage 1	-	-	_	-	515	-
Stage 2	_	_	_	_	696	_
Critical Hdwy	4.14	_	_	_	6.84	6.94
Critical Hdwy Stg 1	_	_	_	_	5.84	-
Critical Hdwy Stg 2	_	_	_	_	5.84	_
Follow-up Hdwy	2.22	_	_	_	3.52	3.32
Pot Cap-1 Maneuver	1045	_	_	_	175	740
Stage 1	-	_	_	_	565	-
Stage 2	-	-	-	_	456	_
Platoon blocked, %		_	_	_		
Mov Cap-1 Maneuver	1045	_	_	_	173	740
Mov Cap-2 Maneuver	-	_	_	_	304	-
Stage 1	_	_	_	_	559	_
Stage 2	_	_	_	_	456	_
Olago Z					100	
Approach	EB		WB		SB	
HCM Control Delay, s	0.1		0		9.9	
HCM LOS					Α	
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1045		-	-	
HCM Lane V/C Ratio		0.01	_	_		0.013
HCM Control Delay (s)		8.5	_	_	_	9.9
HCM Lane LOS		Α	_	<u>-</u>	_	Α
HCM 95th %tile Q(veh)		0	_		_	0

Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑ Ъ	בטוע	ሻ	†	₩.	HOIL
	1227	10	0	461	10	3
,	1227	10	0	461	10	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	_	-	0	-	0	-
Veh in Median Storage,		_	-	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
	1334	11	0	501	11	3
INIVIIIL FIOW	1334	1.1	U	501	- 11	3
Major/Minor M	1ajor1	N	Major2	N	/linor1	
Conflicting Flow All	0	0	1345	0	1591	673
Stage 1	-	-	-	-	1340	-
Stage 2	_	-	-	-	251	-
Critical Hdwy	_	-	4.14	_	6.84	6.94
Critical Hdwy Stg 1	-	-	_	_	5.84	_
Critical Hdwy Stg 2	-	_	-	_	5.84	-
Follow-up Hdwy	_	_	2.22	_	3.52	3.32
Pot Cap-1 Maneuver	-	_	508	_	98	398
Stage 1	_	_	-	_	209	-
Stage 2	_	_	_	_	768	_
Platoon blocked, %	_	_		_	100	
Mov Cap-1 Maneuver	_	_	508	_	98	398
Mov Cap-2 Maneuver	_	_	-	_	177	-
Stage 1	_	_	_	_	209	_
Stage 2	_	_	_	_	768	_
Stage 2		_	-	_	700	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		24.1	
HCM LOS					С	
Minor Long/Major Mount		UDL 4	EDT	EDD	WDI	WDT
Minor Lane/Major Mvmt	·	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		203	-	-	508	-
HI : 1/1 ODO 1//(' L)Otio		0.07	-	-	-	-
HCM Lane V/C Ratio				-	0	-
HCM Control Delay (s)		24.1	-			
		C 0.2	-	- -	A 0	-

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻ	^	∱ ∱		Y	
Traffic Vol, veh/h	23	1204	461	14	3	3
Future Vol, veh/h	23	1204	461	14	3	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	_	None	-	None
Storage Length	0	-	_	-	0	-
Veh in Median Storage		0	0	-	0	-
Grade, %	, <i>"</i>	0	0	_	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	25	1309	501	15	3	3
IVIVIIIL FIOW	25	1309	501	10	J	J
Major/Minor N	Major1	N	Major2	N	Minor2	
Conflicting Flow All	516	0	_	0	1214	258
Stage 1	_	_	-	_	509	_
Stage 2	_	_	_	_	705	_
Critical Hdwy	4.14	_	_	_	6.84	6.94
Critical Hdwy Stg 1		_	_	<u>-</u>	5.84	0.54
Critical Hdwy Stg 2		_		_	5.84	
	2.22	-			3.52	3.32
Follow-up Hdwy		-	-	-		
Pot Cap-1 Maneuver	1046	-	-	-	174	741
Stage 1	-	-	-	-	569	-
Stage 2	-	-	-	-	451	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1046	-	-	-	170	741
Mov Cap-2 Maneuver	-	-	-	-	170	-
Stage 1	-	-	-	-	555	-
Stage 2	-	-	-	-	451	-
Approach	EB		WB		SB	
	0.2		0		18.3	
HCM Control Delay, s	0.2		U			
HCM LOS					С	
Minor Lane/Major Mvm	t	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1046	_	-	-	277
HCM Lane V/C Ratio		0.024	-	-	_	0.024
HCM Control Delay (s)		8.5	_	_	_	18.3
HCM Lane LOS		A	_	_	-	C
HCM 95th %tile Q(veh)		0.1	_			0.1
		U.	_	_	_	U. I

Intersection						
Int Delay, s/veh	1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
	WDL	WDK	1\D1 ↑ \$	INDIX	SDL	<u> </u>
Lane Configurations		45		0		
Traffic Vol, veh/h	35	15	284	8	3	300
Future Vol, veh/h	35	15	284	8	3	300
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	38	16	309	9	3	326
WWITETIOW	00	10	000	3	U	020
Major/Minor I	Minor1	N	//ajor1	N	Major2	
Conflicting Flow All	483	159	0	0	318	0
Stage 1	314	-	-	-	-	-
Stage 2	169	_	_	_	_	_
Critical Hdwy	6.84	6.94	_	_	4.14	_
Critical Hdwy Stg 1	5.84	-	_	_	-	_
Critical Hdwy Stg 2	5.84	_	_		_	_
				_	2.22	
Follow-up Hdwy	3.52	3.32	-	-		-
Pot Cap-1 Maneuver	513	858	-	-	1239	-
Stage 1	714	-	-	-	-	-
Stage 2	843	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	512	858	-	-	1239	-
Mov Cap-2 Maneuver	512	-	-	-	-	-
Stage 1	713	-	-	-	-	-
Stage 2	843	_	-	_	_	_
Clago L	0.10					
Approach	WB		NB		SB	
HCM Control Delay, s	11.8		0		0.1	
HCM LOS	В					
NAT: I /NA NA		NET	NDD	MDL 4	051	OPT
Minor Lane/Major Mvm	t	NBT	NBK	VBLn1	SBL	SBT
Capacity (veh/h)		-	-		1239	-
HCM Lane V/C Ratio		-		0.093	0.003	-
HCM Control Delay (s)		-	-	11.8	7.9	-
HCM Lane LOS		-	-	В	Α	-
HCM 95th %tile Q(veh)		-	-	0.3	0	-

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	∱ Ъ			↑ ↑↑
Traffic Vol, veh/h	0	3	296	7	0	349
Future Vol, veh/h	0	3	296	7	0	349
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	3	322	8	0	379
Major/Minor M	linor1	N	/lajor1	N	/lajor2	
Conflicting Flow All	-	165	0	0	- -	
						-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	
Pot Cap-1 Maneuver	0	850	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	850	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB		NB		SB	
	9.3		0		0	
HCM Control Delay, s HCM LOS	9.3 A		U		U	
I IOW LOS	Α.					
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		-	-	850	-	
HCM Lane V/C Ratio		-	-	0.004	-	
HCM Control Delay (s)		-	-		-	
HCM Lane LOS		-	-	Α	-	
HCM 95th %tile Q(veh)		-	-	0	-	

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
	EBL Ĭ	<u></u> ★↑	WB1 ↑ ↑	WDK	ODL	SBK 7
Lane Configurations Traffic Vol, veh/h		77 348	T ₩ 1079	1	E	13
•	8			4	5	
Future Vol, veh/h	8	348	1079	4	5	13
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage		0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	9	378	1173	4	5	14
Major/Minor N	Major1	N	Major2	N	Minor2	
Conflicting Flow All	1177	0		0	1382	589
Stage 1	-	_	_	-	1175	-
Stage 2	_	_	_	_	207	_
Critical Hdwy	4.14	_	_	_	6.84	6.94
Critical Hdwy Stg 1	T. IT	_	_	<u>-</u>	5.84	- 0.54
Critical Hdwy Stg 2	_	_		_	5.84	_
Follow-up Hdwy	2.22	_	_	_	3.52	3.32
Pot Cap-1 Maneuver	589		-	_	135	452
•	509	_	_	_	256	402
Stage 1	-	-	-		807	
Stage 2	-	-	-	-	007	-
Platoon blocked, %	F00	-	-	-	400	450
Mov Cap-1 Maneuver	589	-	-	-	133	452
Mov Cap-2 Maneuver	-	-	-	-	214	-
Stage 1	-	-	-	-	252	-
Stage 2	-	-	-	-	807	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.3		0		13.2	
HCM LOS	0.0				В	
TOW LOO					J	
Minor Lane/Major Mvm	t	EBL	EBT	WBT	WBR :	
Capacity (veh/h)		589	-	-	-	
HCM Lane V/C Ratio		0.015	-	-		0.031
HCM Control Delay (s)		11.2	-	-	-	13.2
HCM Lane LOS		В	-	-	-	В
HCM 95th %tile Q(veh)		0	-	-	-	0.1

N-209

Intersection						
Int Delay, s/veh	0.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	† ‡		ሻ	↑ ↑	¥	
Traffic Vol, veh/h	347	4	5	1066	12	6
Future Vol, veh/h	347	4	5	1066	12	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-	None	-	None
Storage Length	_	-	0	-	0	-
Veh in Median Storage	e,# 0	_	-	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	377	4	5	1159	13	7
IVIVIIIL FIOW	311	4	ິນ	1109	13	ı
Major/Minor	Major1	N	Major2	ļ	Minor1	
Conflicting Flow All	0	0	381	0	969	191
Stage 1	-	-	-	-	379	-
Stage 2	-	-	-	-	590	-
Critical Hdwy	-	_	4.14	-	6.84	6.94
Critical Hdwy Stg 1	-	_	-	_	5.84	-
Critical Hdwy Stg 2	-	_	-	_	5.84	_
Follow-up Hdwy	_	_	2.22	_	3.52	3.32
Pot Cap-1 Maneuver	_	_	1174	_	251	818
Stage 1	_	_	-	_	662	-
Stage 2	_	_	_	_	517	_
Platoon blocked, %	<u>-</u>	_		_	517	
Mov Cap-1 Maneuver		_	1174	_	250	818
Mov Cap-1 Maneuver		_	- 11/4	_	374	010
		-			659	_
Stage 1	-	-	-	-		
Stage 2	-	-	-	-	517	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		13.2	
HCM LOS					В	
Minor Lane/Major Mvr	nt i	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		457	-	-	1174	-
HCM Lane V/C Ratio		0.043	-	-	0.005	-
HCM Control Delay (s)	13.2	-	-	8.1	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh	1)	0.1	-	-	0	-
·						

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
	EBL T	<u></u> ★↑	WB1 ↑ ↑	WDK	SBL	אמט
Lane Configurations				7		7
Traffic Vol. veh/h	9	345	1061	7	0	7
Future Vol, veh/h	9	345 0	1061	7	0	7
Conflicting Peds, #/hr	0		0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage,		0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	10	375	1153	8	0	8
Major/Minor N	/lajor1	N	Major2	N	Minor2	
Conflicting Flow All	1161	0	-	0	1365	581
Stage 1	-	-	_	-	1157	-
Stage 2	_	_	_	<u>-</u>	208	<u>-</u>
Critical Hdwy	4.14		_	_	6.84	6.94
Critical Hdwy Stg 1	4.14	_	_	_	5.84	0.34
Critical Hdwy Stg 2	_		-		5.84	<u>-</u>
	2.22	-	-	-	3.52	3.32
Follow-up Hdwy	597		-	-		457
Pot Cap-1 Maneuver		-	-	-	139	
Stage 1	-	-	-	-	261	-
Stage 2	-	-	-	-	807	-
Platoon blocked, %		-	-	-	4.0	4
Mov Cap-1 Maneuver	597	-	-	-	137	457
Mov Cap-2 Maneuver	-	-	-	-	137	-
Stage 1	-	-	-	-	257	-
Stage 2	-	-	-	-	807	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.3		0		13	
HCM LOS	0.5		U		B	
HCIVI LOS					D	
Minor Lane/Major Mvmt		EBL	EBT	WBT	WBR :	SBL _{n1}
Capacity (veh/h)		597	-	-	-	457
HCM Lane V/C Ratio		0.016	-	-	-	0.017
HCM Control Delay (s)		11.1	-	-	-	13
HCM Lane LOS		В	-	-	-	В
HCM 95th %tile Q(veh)		0.1	-	-	-	0.1
, ,						

Intersection						
Int Delay, s/veh	0.7					
		14/5-			0	05-
	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		ħβ		ሻ	† †
Traffic Vol, veh/h	9	10	198	12	11	160
Future Vol, veh/h	9	10	198	12	11	160
Conflicting Peds, #/hr	0	0	0	0	0	0
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	10	11	215	13	12	174
		_				
	inor1		/lajor1		Major2	
Conflicting Flow All	333	114	0	0	228	0
Stage 1	222	-	-	-	-	-
Stage 2	111	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	636	917	-	-	1337	-
Stage 1	794	-	-	-	-	-
Stage 2	901	-	-	-	-	-
Platoon blocked, %			_	-		_
Mov Cap-1 Maneuver	630	917	_	_	1337	_
Mov Cap-2 Maneuver	630	-	_	-	-	_
Stage 1	787	_	_	_	_	_
Stage 2	901	_			_	
Glaye Z	JU 1	_	-	-	_	_
Approach	WB		NB		SB	
HCM Control Delay, s	9.9		0		0.5	
HCM LOS	Α					
Minor Long/Maior M.		NDT	NDD	MDI 4	CDI	CDT
Minor Lane/Major Mvmt		NBT	NRKA	VBLn1	SBL	SBT
0 " (1 ")			-	754	1337	-
Capacity (veh/h)				0.00-		
HCM Lane V/C Ratio		-	-		0.009	-
HCM Lane V/C Ratio HCM Control Delay (s)		- -	-	9.9	7.7	-
HCM Lane V/C Ratio		- - -				

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
	WDL	₩DIX	↑	NDIX	ODL	<u>↑</u>
Lane Configurations	^			4	^	
Traffic Vol, veh/h	0	2	243	1	0	212
Future Vol, veh/h	0	2	243	1	0	212
Conflicting Peds, #/hr	0	0	0	0	0	0
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	2	264	1	0	230
IVIVIIIL FIOW	U	2	204		U	230
Major/Minor Mi	inor1	N	Major1	N	/lajor2	
Conflicting Flow All	_	133	0	0		_
Stage 1	_	-	-	-	_	_
Stage 2	_		_	_	<u>-</u>	_
		6.04				
Critical Hdwy	-	6.94	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	-	-	-	-
Pot Cap-1 Maneuver	0	892	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			_	_		_
Mov Cap-1 Maneuver	_	892	_	_	_	_
Mov Cap-1 Maneuver						
	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9		0		0	
HCM LOS	A		U		U	
HOW LOS	A					
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		_	-		_	
HCM Lane V/C Ratio		_		0.002	_	
HCM Control Delay (s)				9	_	
HCM Lane LOS			_	A	_	
		-	-	0	-	
HCM 95th %tile Q(veh)						

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻ	^	† 1>			7
Traffic Vol, veh/h	4	1301	510	2	3	8
Future Vol, veh/h	4	1301	510	2	3	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	0	_	-	-	_	0
Veh in Median Storage		0	0	_	0	_
Grade, %	-	0	0	_	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	4	1414	554	2	3	9
WWW.CTIOW	_	1717	004		U	3
Major/Minor N	Major1	Λ	//ajor2	N	Minor2	
Conflicting Flow All	556	0	-	0	1270	278
Stage 1	-	-	-	-	555	-
Stage 2	-	-	-	-	715	-
Critical Hdwy	4.14	-	-	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	-	-	-	5.84	-
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	1011	-	-	-	160	719
Stage 1	-	-	-	-	539	-
Stage 2	-	-	-	-	446	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1011	-	-	_	159	719
Mov Cap-2 Maneuver	-	_	_	_	293	-
Stage 1	_	_	_	_	537	_
Stage 2	_	_	_	_	446	_
Olago Z					110	
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		10.1	
HCM LOS					В	
Minor Lane/Major Mvm	ıt	EBL	EBT	WBT	WBR :	SRI n1
		1011		AADT	- 1000	719
Capacity (veh/h) HCM Lane V/C Ratio		0.004		-		0.012
HCM Control Delay (s)		8.6	-	-	-	10.1
HCM Lane LOS			-	-	-	
HCM 95th %tile Q(veh)	\	A 0	-	-		B 0
HOW SOUL WILLE M(Ven)	1	U	-	-	-	U

Intersection						
Int Delay, s/veh	0.2				_	
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	†		ሻ	^	Y	
Traffic Vol, veh/h	1286	10	0	497	10	3
Future Vol, veh/h	1286	10	0	497	10	3
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-	None	-	None
Storage Length	_	-	0	-	0	-
Veh in Median Storag	e.# 0	_	-	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	1398	11	0	540	11	3
IVIVIII(I IOW	1000	- 11	U	J+0	- 11	3
Major/Minor	Major1	N	Major2	N	/linor1	
Conflicting Flow All	0	0	1409	0	1674	705
Stage 1	-	_	-	-	1404	-
Stage 2	-	-	-	-	270	-
Critical Hdwy	-	_	4.14	-	6.84	6.94
Critical Hdwy Stg 1	-	-	-	-	5.84	-
Critical Hdwy Stg 2	-	_	-	_	5.84	-
Follow-up Hdwy	_	_	2.22	_	3.52	3.32
Pot Cap-1 Maneuver	_	_	480	_	86	379
Stage 1	_	_	-	_	193	-
Stage 2	_	_	_	_	751	_
Platoon blocked, %	_	_		_	701	
Mov Cap-1 Maneuver		_	480	_	86	379
Mov Cap-1 Maneuver		<u>-</u>	-	<u>-</u>	163	- 010
Stage 1	_	_	_	_	193	_
•	_	-	_	-	751	_
Stage 2	-	-	_	-	731	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		25.7	
HCM LOS					D	
Minor Long/Major M.	~4 ·	MDL 4	EDT	EDD	WDI	WDT
Minor Lane/Major Mvr	nt i	VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		188	-	-	480	-
HCM Lane V/C Ratio	,	0.075	-	-	-	-
HCM Control Delay (s)	25.7	-	-	0	-
HCM Lane LOS	,	D	-	-	A	-
HCM 95th %tile Q(veh	1)	0.2	-	-	0	-

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	T T	<u> </u>	<u>₩</u>	אטא	SBL W	אומט
Traffic Vol, veh/h	22	1264	497	13	3	3
Future Vol, veh/h	22	1264	497	13	3	3
	0	1204	497	0	0	0
Conflicting Peds, #/hr						
Sign Control RT Channelized	Free	Free None	Free	Free None	Stop	Stop
	- 0	None -	-		-	None
Storage Length			-	-	0	-
Veh in Median Storage	,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	24	1374	540	14	3	3
Major/Minor N	Major1	N	Major2	N	Minor2	
Conflicting Flow All	554	0	-		1282	277
Stage 1	-	-	_	-	547	-
Stage 2	_	_	_	_	735	_
Critical Hdwy	4.14		_		6.84	6.94
Critical Hdwy Stg 1		-	_	_	5.84	0.34
	-	-	-		5.84	
Critical Hdwy Stg 2	-	-	-	-		2 22
Follow-up Hdwy	2.22	-	-	-	3.52	3.32
Pot Cap-1 Maneuver	1012	-	-	-	157	720
Stage 1	-	-	-	-	544	-
Stage 2	-	-	-	-	435	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1012	-	-	-	153	720
Mov Cap-2 Maneuver	-	-	-	-	153	-
Stage 1	-	-	-	-	531	-
Stage 2	-	-	-	-	435	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.1		0		19.7	
HCM LOS	0.1		U		C	
TICIVI LOG					U	
Minor Lane/Major Mvm	t	EBL	EBT	WBT	WBR S	SBLn1
Capacity (veh/h)		1012	-	-	-	252
HCM Lane V/C Ratio		0.024	-	-	-	0.026
HCM Control Delay (s)		8.6	-	-	-	19.7
HCM Lane LOS		Α	-	-	-	С
HCM 95th %tile Q(veh)		0.1	-	-	-	0.1